



Crown Castle
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065

July 16, 2019

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

**RE: Notice of Exempt Modification for T-Mobile:
822765 - T-Mobile Site ID: CT11025B
10 Sylvia Street, Branford, CT 06405
Latitude: 41° 17' 38.16" / Longitude: -72° 47' 8.54"**

Dear Ms. Bachman:

T-Mobile currently maintains nine (9) antennas at the 122-foot mount on the existing 125-foot Monopole Tower, located at 10 Sylvia Street, Branford, CT. The tower is owned by Crown Castle and the property is owned by 322 East Main Street LLC. T-Mobile now intends to replace six (6) existing antennas with three (3) new 1900/2100 MHz antennas and three (3) new 600/700 MHz antennas. T-Mobile is also proposing tower mount modifications as shown on the enclosed mount analysis.

Planned Modifications:

Tower:

Remove:

(6) 1 5/8" Coax

Remove and Replace:

(3) LNX 6515DS-A1M Antenna (**REMOVE**) - (3) RFS-APXVAARR24_43-U-NA20 Antenna 600/700 MHz (**REPLACE**)

(3) AIR21 KRC118023-1_B2P_B4A Antenna (**REMOVE**) – (3) AIR32_B66A_B2A Antenna 1900/2100 MHz (**REPLACE**)

(3) RRUS11 B12 (**REMOVE**) – (3) Radio 4449 B71/B12 (**REPLACE**)

Install New:

(3) 1 5/8" Hybrid Fiber Line

Existing to Remain:

(6) 1 5/8" Coax

(1) Fiber line

- (3) AIR21 KRC118023-1_B2P_B4A Antenna 1900/2100 MHz
- (3) TMA

Ground:

- Upgrade to existing ground cabinet. (Internally)
- Upgrade existing breakers.

The facility was approved by the Town of Branford Planning and Zoning Commission in Application Number 98-9.3 on November 10, 1998. The approval was given without conditions.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to James B. Cosgrove, First Selectman for the Town of Branford, Harry Smith, Town Planner, Crown Castle as the tower owner, and 322 East Main Street LLC, the property owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Anne Marie Zsamba.

Sincerely,

Anne Marie Zsamba
Real Estate Specialist
3 Corporate Park Drive, Suite 101
Clifton Park, NY 12065
(201) 236-9224

Melanie A. Bachman

Page 3

AnneMarie.Zsamba@crowncastle.com

Attachments

cc:

James B. Cosgrove, First Selectman
Town of Branford
Town Hall – Selectman’s Office
1019 Main Street
Branford, CT 06405
203.488.8394

Harry Smith, Town Planner
Town of Branford
Town Hall – Planning Department
1019 Main Street
Branford, CT 06405
203.488.1255

322 East Main Street, Property Owner
375 Fairfield Avenue
Stamford, CT 06902
203.967.8367

Crown Castle, Tower Owner

ORIGIN ID:GFLA (518) 373-3523
ANNIE MARIE ZSAMBA
CROWN CASTLE
3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

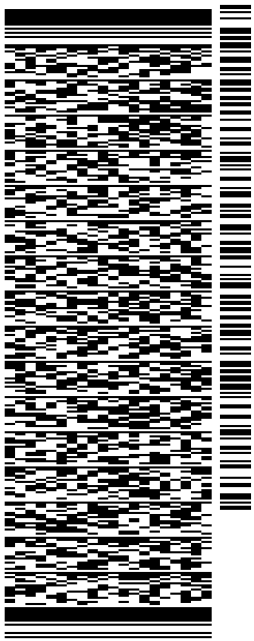
SHIP DATE: 12JUL19
ACTWGT: 4.00 LB
CAD: 104924194IN/ET4100

BILL SENDER

TO **MELANIE BACHMAN**
CONNECTICUT SITING COUNCIL
10 FRANKLIN SQUARE

NEW BRITAIN CT 06051

(860) 827-2951 REF: 1765 6880
INV/ DEPT:
PO:



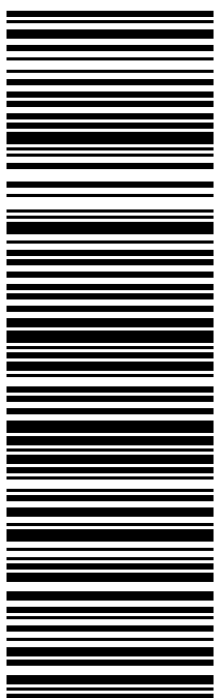
J191019010701uv

565.J2/A6F9/23AD

TRK# 7757 1283 4228
0201

MON - 15 JUL 10:30A
PRIORITY OVERNIGHT

SEBDLA
06051
CT-US BDL



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Warning: Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.

Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com. FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our ServiceGuide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

ORIGIN ID:GFLA (518) 373-3523
ANNE MARIE ZSAMBRA
CROWN CASTLE
3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

SHIP DATE: 12JUL19
ACTWGT: 2.00 LB
CAD: 104924194INNET4100

BILL SENDER

TO **JAMES B. COSGROVE, FIRST SELECTMAN**

TOWN OF BRANFORD

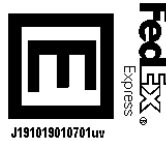
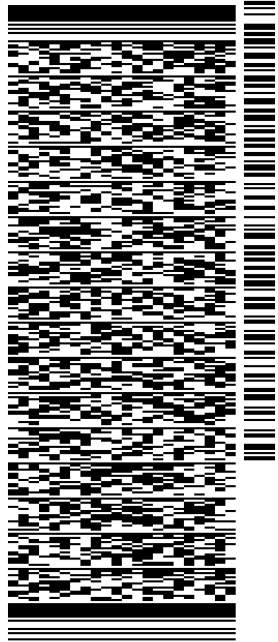
1019 MAIN STREET

TOWN HALL - SELECTMAN'S OFFICE

BRANFORD CT 06405

REF: 1765 6690
(203) 488-8394
INV:
PO: DEPT:

565J2/A6F9/23AD



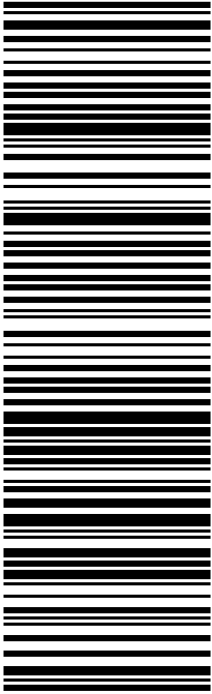
J191019010701uv

TRK# 7757 1297 2335
0201

MON - 15 JUL 10:30A
PRIORITY OVERNIGHT

SE RSPA

06405
CT-US BDL



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SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

SHIP DATE: 12JUL19
ACTWGT: 2.00 LB
CAD: 104924194INNET4100

BILL SENDER

TO HARRY SMITH, TOWN PLANNER

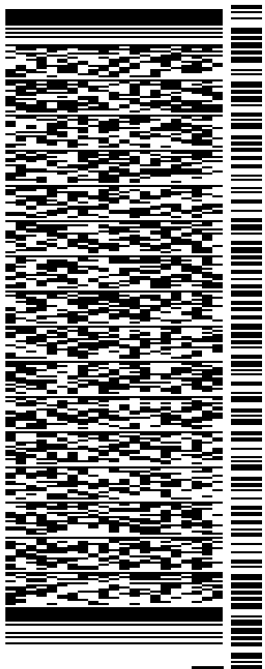
TOWN OF BRANFORD

TOWN HALL - PLANNING DEPT

1019 MAIN STREET

BRANFORD CT 06405

(203) 488-1255 REF: 1734.7890
INV: DEPT:
PO:



J191019010701uv

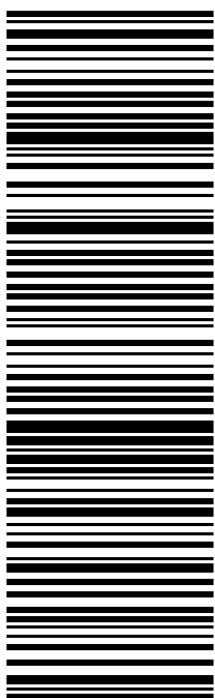
565.J2/A6F9/23AD

TRK# 7757 1303 1532
0201

MON - 15 JUL 10:30A
PRIORITY OVERNIGHT

SE RSPA

06405
CT-US BDL



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ANNE MARIE ZSAMBA
CROWN CASTLE
3 CORPORATE PARK DRIVE
SUITE 101
CLIFTON PARK, NY 12065
UNITED STATES US

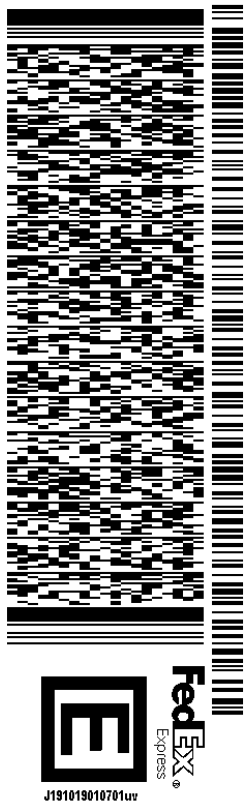
SHIP DATE: 12JUL19
ACTWGT: 2.00 LB
CAD: 104924194INNET4100
BILL SENDER

TO **322 EAST MAIN STREET LLC**

375 FAIRFIELD AVENUE, BLD 1

STAMFORD CT 06902

(203) 967-8367 REF: 1734.7890
INV/ PO: DEPT:



TRK# 7757 1308 4319
0201
MON - 15 JUL 10:30A
PRIORITY OVERNIGHT

ST JSDA
06902
CT-US JFK


565.J2/A6F9/23AD

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Exhibit A

Original Facility Approval

VOL. 662 PAGE 502
 PLANNING AND ZONING COMMISSION
 TOWN OF BRANFORD TOWN HALL DRIVE P.O. BOX 150
 Branford, Connecticut 06405 488-1255

NOTICE OF DECISION

November 10, 1998

J. Brendan Sharkey, Esq. For Omnipoint Communications, Inc.
 25 VanZant Street #18E
 East Norwalk, Connecticut 06855

SUBJECT: Special Exception APPLICATION # 98-9.3

LOCATION: 10 Sylvia Street

OWNERS OF RECORD: TKJ SYLVIA ASSOCIATES, LLC

Dear Sir:

At a meeting of the Branford Planning & Zoning Commission held on Thursday,
November 5, 1998, the Commission voted to:

Approve your above subject application with the conditions noted below.

Very truly yours,

Shirley Rasmussen
 Shirley Rasmussen
 Town Planner

NOTE: This Special Exception shall become effective only after it is filed on the Land Records
 in the office of the Town Clerk.

1. Omnipoint must construct tower so that it can easily be extended to provide spaces for two
 (2) other carriers for co-location purposes.

NOTE: Special Exception shall become null and void in the event the applicant fails to obtain a
 building permit within one (1) year of date of approval.
 (Per Section 31.7 of the Branford Zoning Regulations)

RECEIVED FOR RECORD Nov 19 1998
 at 3:49 p.m. AND RECORDED BY
 GEORGETTE A. LASKE
 BRANFORD TOWN CLERK

Exhibit B

Property Card



Property Information

Owner	322 EAST MAIN STREET LLC
Address	10 SYLVIA ST
Mailing Address	375 FAIRFIELD AVE STAMFORD , CT 06902
Land Use	- COMM WHS MDL96
Land Class	C

Census Tract	
Neighborhood	400
Zoning	BL
Acreage	0.95
Utilities	Public Water,Septic
Lot Setting/ Desc	/ Level

Photo

PARCEL VALUATIONS (Assessed value = 70% of Appraised Value)

	Appraised	Assessed
Buildings	87900	61500
Outbuildings	16900	11800
Improvements	108000	75500
Extras	3200	2200
Land	467500	327300
Total	575500	402800
Previous		

Construction Details

Year Built	1960
Stories	1
Building Style	Warehouse
Building Use	Ind/Comm
Building Condition	03
Total Rooms	
Bedrooms	
Full Bathrooms	
Half Bathrooms	
Bath Style	
Kitchen Style	
Roof Style	Gable/Hip
Roof Cover	Asphalt

EXTERIOR WALLS:

Primary	MASONRY
Secondary	Concr/Cinder

INTERIOR WALLS:

Primary	Minim/Masonry
Secondary	

FLOORS:

Primary	Concr-Finished
Secondary	Linoleum

HEATING/AC:

Heating Type	Hot Air-no Duc
Heating Fuel	Gas
AC Type	None

BUILDING AREA:

Effective Building Area	
Gross Building Area	4820
Total Living Area	2620

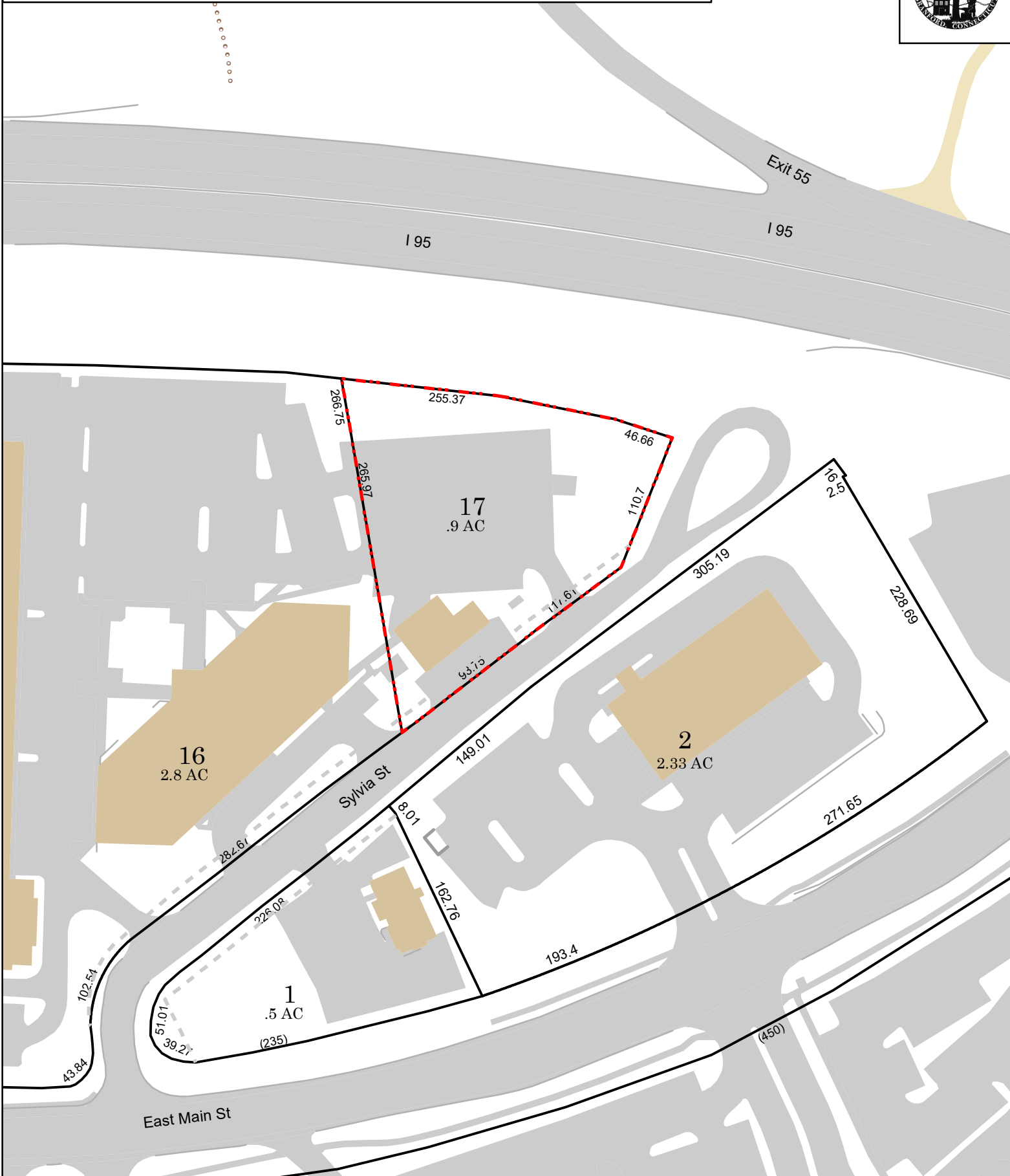
SALES HISTORY:

Sale Date	5/2/2013
Sale Price	468000
Book/ Page	1132/0054

Town of Branford, Connecticut - Assessment Parcel Map

Parcel: G05-F05-004-00017

Address: 10 SYLVIA ST



Approximate Scale: 1 inch : 100 feet

Grand List Date October 2018

Disclaimer:

This map is for informational purposes only. All information is subject to verification by any user. The Town of Branford and its mapping contractors assume no legal responsibility for the information contained herein.

Exhibit C

Construction Drawings

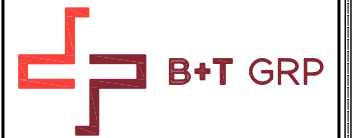


T-MOBILE SITE NAME:
BRANFORD/I-95/X55/DTN1

T-MOBILE SITE NUMBER:
CT11025B

CROWN BU: 822765 / APP#: 479795
67D92DB CONFIGURATION

10 SYLVIA ST
 BRANFORD, CT 06405
 EXISTING 125'-0" MONOPOLE



PROJECT SUMMARY

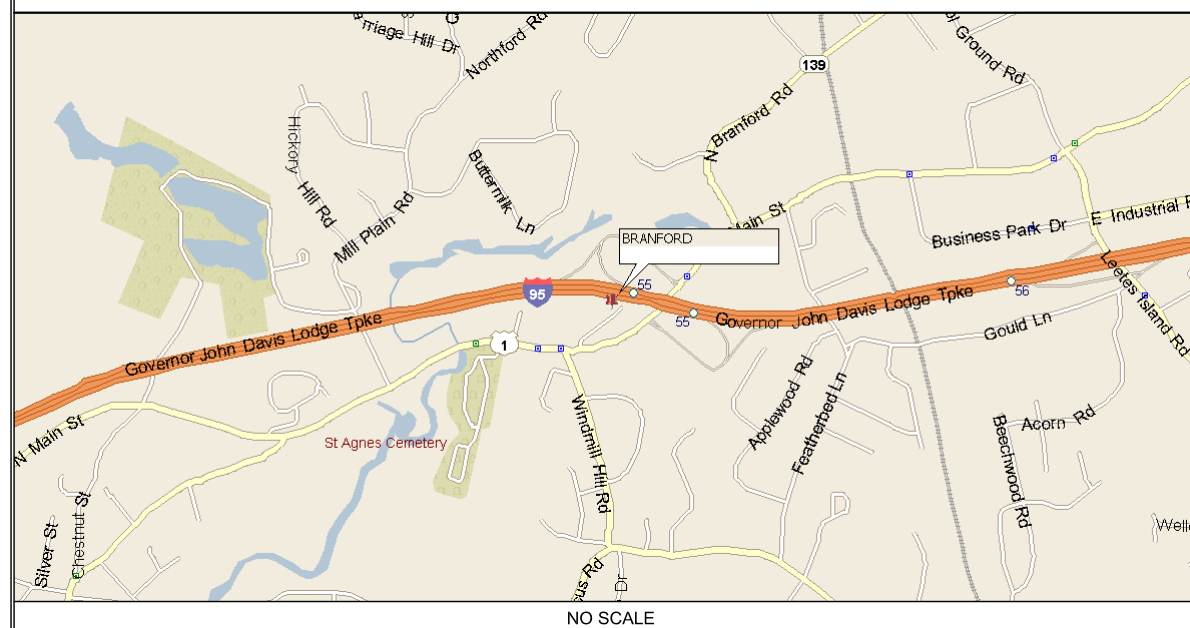
SITE TYPE: EXISTING EQUIPMENT UPGRADE
 SITE ADDRESS: 10 SYLVIA ST
 BRANFORD, CT 06405
 JURISDICTION: NEW HAVEN COUNTY

NAD83
 LATITUDE: 41.293911° N
 LONGITUDE: 72.785729° W
 TOWER OWNER: CROWN CASTLE
 3200 HORIZON DRIVE, SUITE 150
 KING OF PRUSSIA, PA 19406
 JASON SMITH
 (610) 635-3225

CUSTOMER/APPLICANT: T-MOBILE
 4 SYLVAN WAY
 PARSIPPANY, NJ 07054
 (973) 397-4800

OCCUPANCY TYPE: UNMANNED
 A.D.A. COMPLIANCE: FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION

LOCATION MAP



DRAWING INDEX

SHEET #	SHEET DESCRIPTION	REV. #
T-1	TITLE SHEET	1
A-1	OVERALL SITE PLAN	1
A-2	ANTENNA/CABLE SCHEDULE AND AZIMUTH PLANS	1
A-3	TOWER ELEVATION	1
A-4	ANTENNA AND RRU DETAILS	1
E-1	PANEL SCHEDULE AND ONE-LINE DIAGRAM	1

CT11025B
 BU #: 822765
 BRANFORD/I-95/X55/DTN1
 10 SYLVIA ST
 BRANFORD, CT 06405
 EXISTING 125'-0" MONOPOLE

PROJECT NO: 101126.005.01
 CHECKED BY: RMC

ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION
0	6/5/19	GEH	CONSTRUCTION
1	7/2/19	JJD	CONSTRUCTION

CONTACT INFORMATION

A&E FIRM: B+T GROUP
 1717 S. BOULDER, STE. 300
 TULSA, OK 74119
 CONTACT: MIKE OAKES
 PHONE: (918) 587-4630

ELECTRIC PROVIDER: N/A
 TELCO PROVIDER: N/A

DRIVING DIRECTIONS

DEPART BRADLEY INTERNATIONAL AIRPORT ON TERMINAL RD. ROAD NAME CHANGES TO BRADLEY FIELD CONNECTOR. ROAD NAME CHANGES TO CT-20 [BRADLEY FIELD CONNECTOR]. TAKE RAMP (RIGHT) ONTO I-91 [RICHARD P HORAN MEMORIAL HWY]. TAKE RAMP (LEFT) ONTO I-95 [GOVERNOR JOHN DAVIS LODGE TPKE]. AT EXIT 55, TURN RIGHT ONTO RAMP. KEEP RIGHT TO STAY ON RAMP. TURN LEFT ONTO US-1 [E MAIN ST]. TURN RIGHT ONTO SYLVIA ST. ARRIVE AT BRANFORD.

PROJECT DESCRIPTION

- THE PROPOSED PROJECT INCLUDES:
- REMOVE (1) EXISTING DUS31.
 - REMOVE (1) EXISTING XMU.
 - REMOVE (6) EXISTING ANTENNAS AT 122'-0".
 - REMOVE (3) EXISTING RRUS AT 122'-0".
 - REMOVE (6) EXISTING COAX.
 - INSTALL (2) NEW BB 6630S.
 - INSTALL (6) NEW ANTENNAS AT 122'-0".
 - INSTALL (3) NEW RRUS AT 122'-0".
 - INSTALL (3) NEW 1 1/4" HYBRID CABLE FOR NEW ANTENNAS.
 - MODIFY EXISTING MOUNTS PER MOUNT ANALYSIS BY B+T GROUP DATED 5/9/19.
 - INSTALL (1) NEW ELECTRICAL PANEL BOARD & UPGRADE CONDUITS AND WIRES PER NEC REQUIREMENTS.
 - UPGRADE EXISTING 6131 BREAKER FROM 100A TO 125A.

DO NOT SCALE DRAWINGS

ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR 11X17. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

A/E DOCUMENT REVIEW STATUS

TITLE	SIGNATURE	DATE
T-MOBILE PROP:		
T-MOBILE R.F. MGR.:		
T-MOBILE NetOps:		
T-MOBILE CONST. MGR.:		
INTERCONNECT:		
T-MOBILE SITE DEV. MGR.:		
PROPERTY OWNER:		
PLANNING:		

THE FOLLOWING PARTIES HEREBY APPROVE AND ACCEPT THESE DOCUMENTS AND AUTHORIZE THE CONTRACTOR TO PROCEED WITH THE CONSTRUCTION DESCRIBED HEREIN. ALL DOCUMENTS ARE SUBJECT TO REVIEW BY THE LOCAL BUILDING DEPARTMENT AND MAY IMPOSE CHANGES OR MODIFICATIONS.



CALL CONNECTICUT ONE CALL
 (800) 922-4455
 CALL 3 WORKING DAYS
 BEFORE YOU DIG!



CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

CODE TYPE	CODE
BUILDING/DWELLING	IBC 2015
STRUCTURAL	IBC 2015
MECHANICAL	IMC 2015
ELECTRICAL	NEC 2017

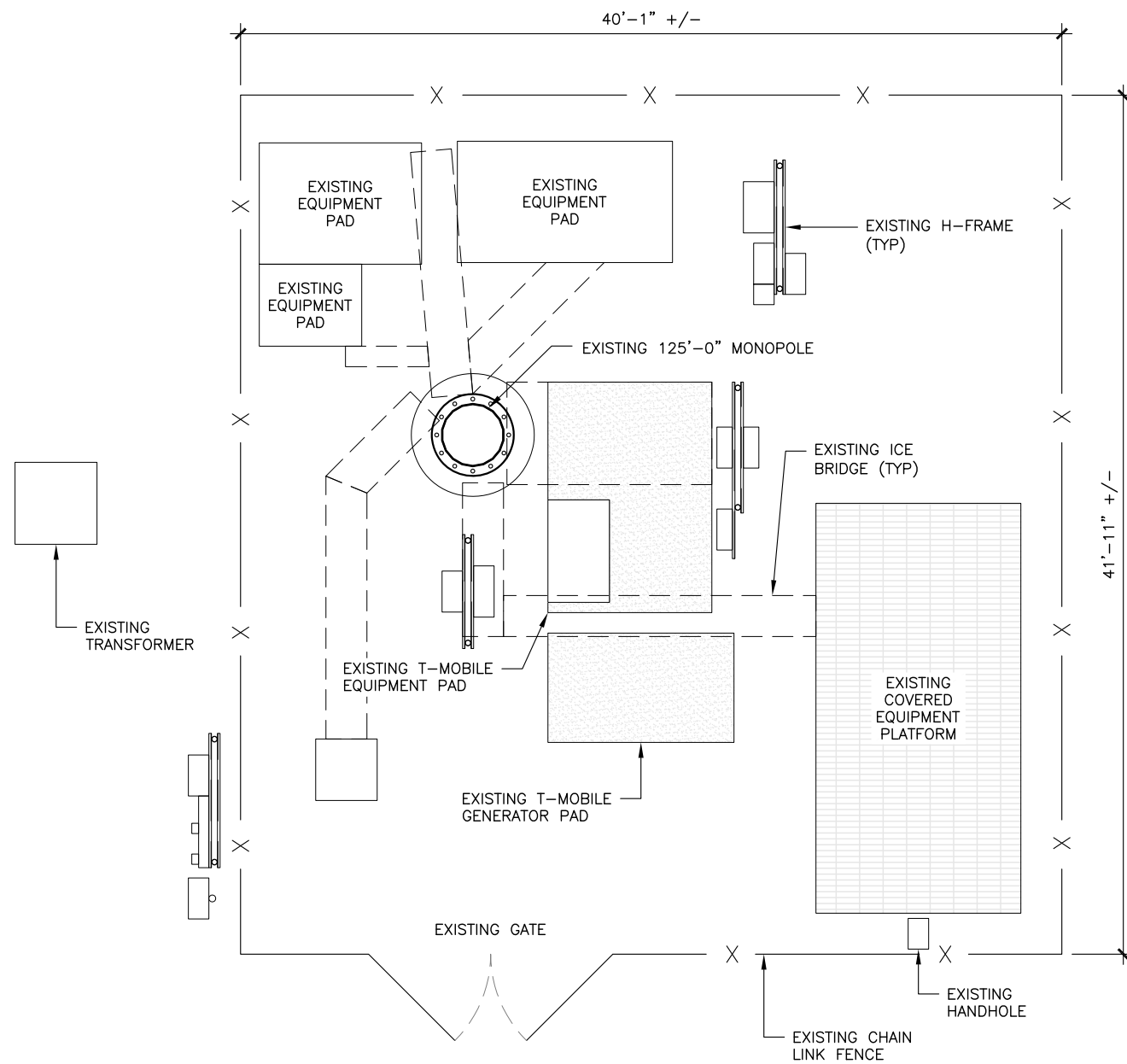
B&T ENGINEERING, INC.
 PEC.0001564
 Expires 2/10/20



IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: **T-1** REVISION: **1**

101126_822765_Branford.dwg - Sheet:A-1 - User: ghoyes - Jul 02, 2019 - 9:41am



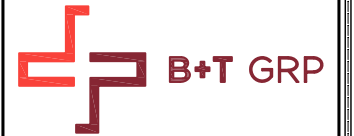
1 OVERALL SITE PLAN
 SCALE: 0' 1' 4' 8' 20'



GENERAL NOTES:

1. SUBJECT PROPERTY IS KNOWN AS BLOCK TBD LOT TBD AS SHOWN ON THE BRANFORD TOWNSHIP TAX MAP AND IS SITUATED AT 10 SYLVIA ST, BRANFORD, CT 06405.
2. APPLICANT: T-MOBILE
 A DELAWARE LIMITED LIABILITY COMPANY
 4 SYLVAN WAY
 PARSIPPANY, NEW JERSEY 07054
 (973) 397-4800

 TOWER OWNER: CROWN CASTLE INTERNATIONAL
- THE APPLICANT IS TO UPDATE THEIR NETWORK BY INSTALLING SIX (6) NEW PANEL ANTENNAS, THREE (3) RRUS, AND THREE (3) ADDITIONAL CABLES MOUNTED ON AN EXISTING MONOPOLE.
3. THIS FACILITY SHALL BE VISITED ON THE AVERAGE OF ONCE A MONTH FOR MAINTENANCE AND SHALL BE MONITORED FROM A REMOTE FACILITY.
4. THE EXISTING SITE IS LOCATED AT LATITUDE OF 41.293911' N± AND LONGITUDE OF 72.785729' W±. THE HORIZONTAL DATUM ARE IN TERMS OF NORTH AMERICAN DATUM OF 1983 (NAD 83).
5. THIS SET OF PLANS HAS BEEN PREPARED FOR THE PURPOSES OF MUNICIPAL AND AGENCY REVIEW AND APPROVAL. THIS SET OF PLANS SHALL NOT BE UTILIZED AS CONSTRUCTION DOCUMENTS UNTIL ALL CONDITIONS OF APPROVAL HAVE BEEN SATISFIED AND EACH OF THE DRAWINGS HAVE BEEN REVISED TO INDICATED "ISSUED FOR CONSTRUCTION"
6. ALL MATERIALS, WORKMANSHIP, AND CONSTRUCTION FOR THE SITE IMPROVEMENTS SHOWN HEREON SHALL BE IN ACCORDANCE WITH:
 - 6.A. CURRENT PREVAILING MUNICIPAL AND/OR COUNTY SPECIFICATIONS, STANDARDS, AND REQUIREMENTS.
 - 6.B. CURRENT PREVAILING UTILITY COMPANY AUTHORITY SPECIFICATIONS, STANDARDS AND REQUIREMENTS.
7. THE CONTRACTOR SHALL NOTIFY B+T GROUP, P.A. IMMEDIATELY IF ANY FIELD-CONDITIONS ENCOUNTERED DIFFER FROM THOSE REPRESENTED HEREON, AND/OR IF SUCH CONDITIONS WOULD OR COULD RENDER THE DESIGNS SHOWN HEREON INAPPROPRIATE AND/OR INEFFECTIVE.
8. THE CONTRACTOR IS RESPONSIBLE TO PROTECT, REPAIR AND/OR REPLACE ANY DAMAGED STRUCTURES, UTILITIES OR LANDSCAPED AREA WHICH MAY BE DISTURBED DURING THE CONSTRUCTION OF THIS FACILITY.
9. THE CONSTRUCTION CONTRACTOR IS SOLELY RESPONSIBLE FOR DETERMINING ALL CONSTRUCTION MEANS AND METHODS. THE CONSTRUCTION CONTRACTOR IS ALSO RESPONSIBLE FOR ALL JOB SITE SAFETY.
10. SITE INFORMATION SHOWN TAKEN FROM CROWN CASTLE SITE PLANS AND FROM CROWN CASTLE INSPECTION PHOTOS.
11. NO GUARANTEE IS MADE NOR SHOULD BE ASSUMED AS TO THE COMPLETENESS OR ACCURACY OF THE HORIZONTAL OR VERTICAL LOCATIONS. ALL PARTIES UTILIZING THIS INFORMATION SHALL FIELD VERIFY THE ACCURACY AND COMPLETENESS OF THE INFORMATION SHOWN PRIOR TO CONSTRUCTION ACTIVITIES.
12. ALL IMPROVEMENTS SHALL BE SUBJECT TO INSPECTION AND APPROVAL BY THE TOWNSHIP ENGINEER WHO WILL BE GIVEN PROPER NOTIFICATION PRIOR TO THE START OF ANY CONSTRUCTION.



CT11025B
 BU #: 822765
 BRANFORD/1-95/X55/DTN1
 10 SYLVIA ST
 BRANFORD, CT 06405
 EXISTING 125'-0" MONOPOLE

PROJECT NO: 101126.005.01
 CHECKED BY: RMC

ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION
0	6/5/19	GEH	CONSTRUCTION
1	7/2/19	JJD	CONSTRUCTION

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 Expires 2/10/20



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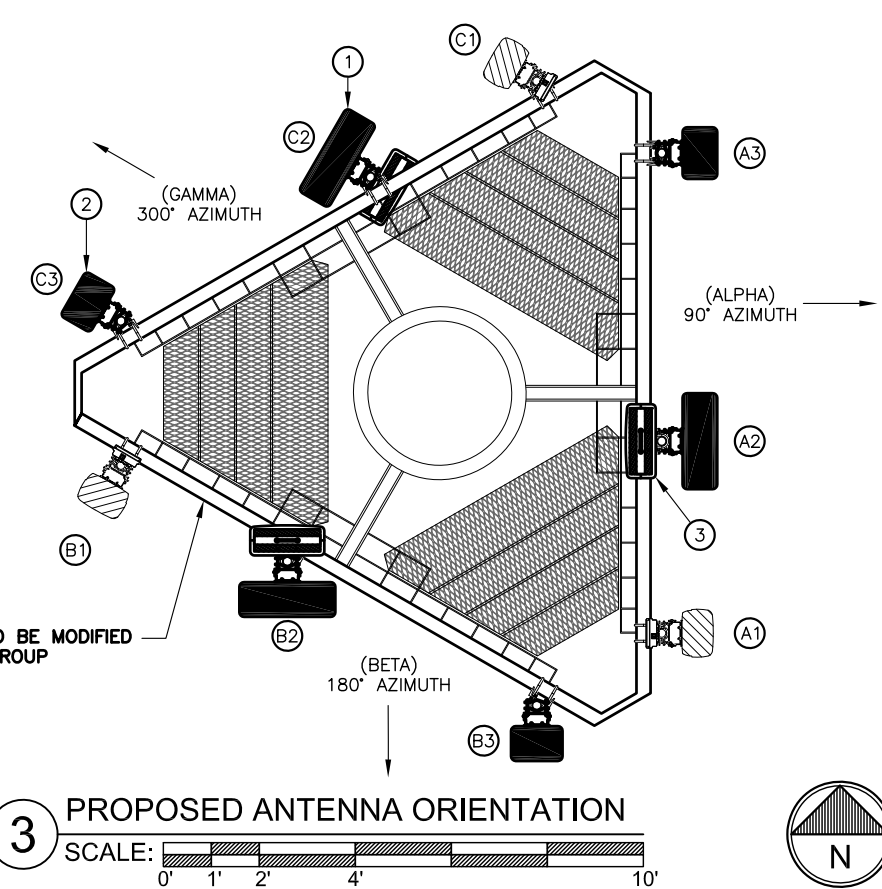
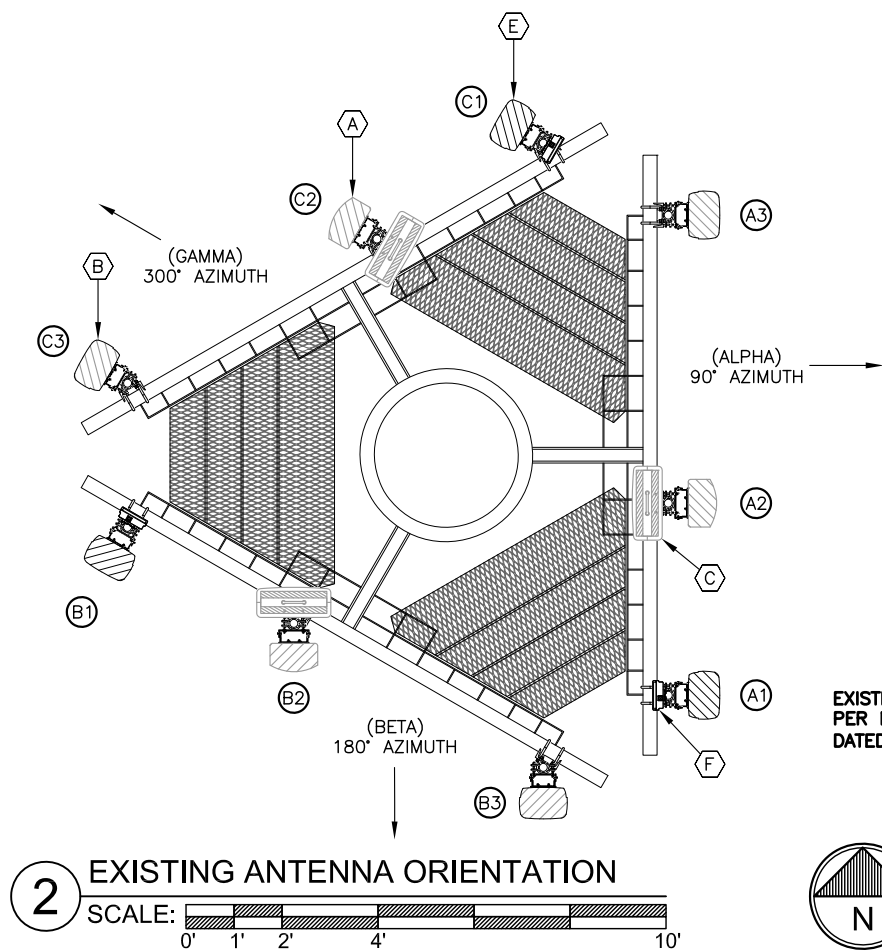
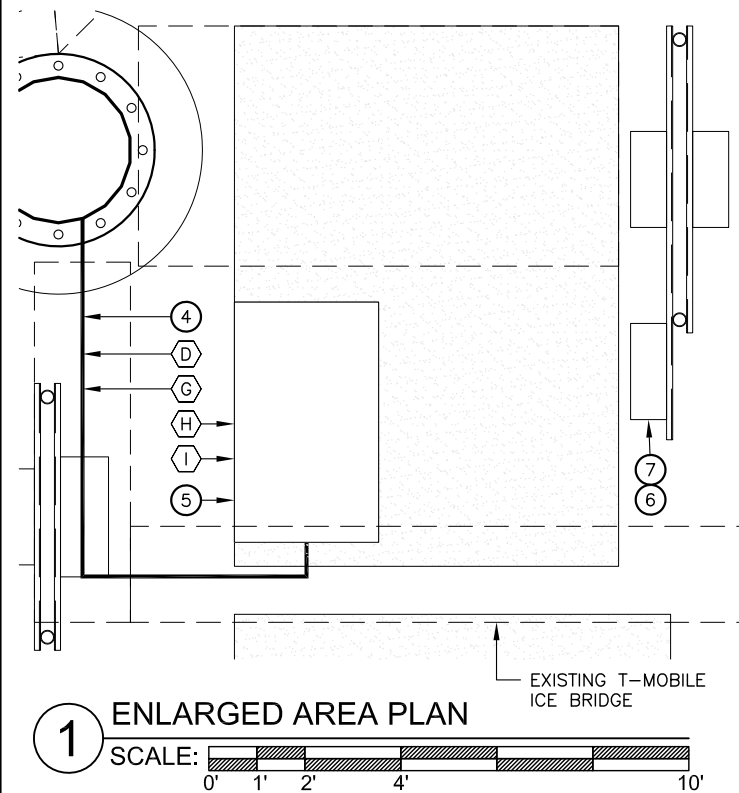


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SHEET NUMBER: **A-2** REVISION: **1**

ANTENNA AND CABLE SCHEDULE											
SECTOR	POSITION	EXISTING ANTENNAS	PROPOSED ANTENNA CONFIGURATION		E-TILT	M-TILT	ANTENNA CENTERLINE	TMA/RRU	CABLES	JUMPER TYPE	CABLE LENGTH
90° - ALPHA	A3	ERICSSON AIR32 KRD901146-1_B66A_B2A	LTE	B71+ B12	2°/2°	-	122'-0"	-	-	(2) FIBER	-
	A2	RFS APXVAARR24_43-U-NA20	LTE	B71+ B12	2°	0°		0/1	(1) 1 5/8" HYBRID FIBER TRUNK	(4) COAX (1) FIBER	172'-0"
	A1	ERICSSON AIR21 KRC118023-1_B2A_B4P	GSM UMTS	-	2°/2°	-		1/0	(2) 1 5/8" COAX	(2) COAX (2) FIBER	172'-0"
180° - BETA	B3	ERICSSON AIR32 KRD901146-1_B66A_B2A	LTE	B71+ B12	2°/2°	-	122'-0"	-	-	(2) FIBER	-
	B2	RFS APXVAARR24_43-U-NA20	LTE	B71+ B12	2°	0°		0/1	(1) 1 5/8" HYBRID FIBER TRUNK	(4) COAX (1) FIBER	172'-0"
	B1	ERICSSON AIR21 KRC118023-1_B2A_B4P	GSM UMTS	-	2°/2°	-		1/0	(2) 1 5/8" COAX	(2) COAX (2) FIBER	172'-0"
300° - GAMMA	G3	ERICSSON AIR32 KRD901146-1_B66A_B2A	LTE	B71+ B12	2°/2°	-	122'-0"	-	(1) 1 5/8" HYBRID FIBER TRUNK	(2) FIBER	-
	G2	RFS APXVAARR24_43-U-NA20	LTE	B71+ B12	2°	0°		0/1	(1) 1 5/8" HYBRID FIBER TRUNK	(4) COAX (1) FIBER	172'-0"
	G1	ERICSSON AIR21 KRC118023-1_B2A_B4P	GSM UMTS	-	2°/2°	-		1/0	(2) 1 5/8" COAX	(2) COAX (2) FIBER	172'-0"


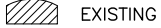

LEGEND	
EXISTING/DEMOLITION NOTES	INSTALLATION NOTES
(A) EXISTING ANDREW LNX-6515DS-A1M ANTENNA TO BE REMOVED (TOTAL OF 3)	(1) INSTALL RFS APXVAARR24_43-U-NA20 (8 FT) ANTENNAS ON EXISTING MOUNT. (TYP. OF 1 PER SECTOR, TOTAL OF 3)
(B) EXISTING ERICSSON AIR21 KRC118023-1_B2P_B4A ANTENNA TO BE REMOVED (TOTAL OF 3)	(2) INSTALL ERICSSON AIR32 KRD901146-1_B66A_B2A ANTENNAS ON EXISTING MOUNT (TYP. OF 1 PER SECTOR, TOTAL OF 3)
(C) EXISTING RRUS11 B12 TO BE REMOVED (TOTAL OF 3)	(3) INSTALL RADIO 4449 B71/B12 (TYP. OF 1 PER SECTOR, TOTAL OF 3)
(D) EXISTING 1-5/8" COAX TO BE REMOVED (TOTAL OF 6)	(4) INSTALL (3) 1 5/8" HYBRID FIBER TRUNK FROM EQUIPMENT TO ANTENNAS FOLLOWING EXISTING ROUTING
(E) EXISTING ERICSSON AIR21 KRC118023-1_B2A_B4P ANTENNA TO REMAIN (TOTAL OF 3)	(5) INSTALL (2) NEW BB6630S
(F) EXISTING ERICSSON KRY 112 TMA TO REMAIN (TOTAL OF 3)	(6) UPGRADE MAIN BREAKER FROM 100A TO 225A
(G) EXISTING 1-5/8" COAX TO REMAIN (TOTAL OF 6)	(7) UPGRADE 6131 BREAKER FROM 100A TO 125A
(H) EXISTING XMU TO BE REMOVED (TOTAL OF 1)	
(I) EXISTING DUS31 TO BE REMOVED (TOTAL OF 1)	

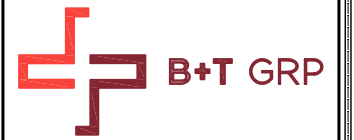
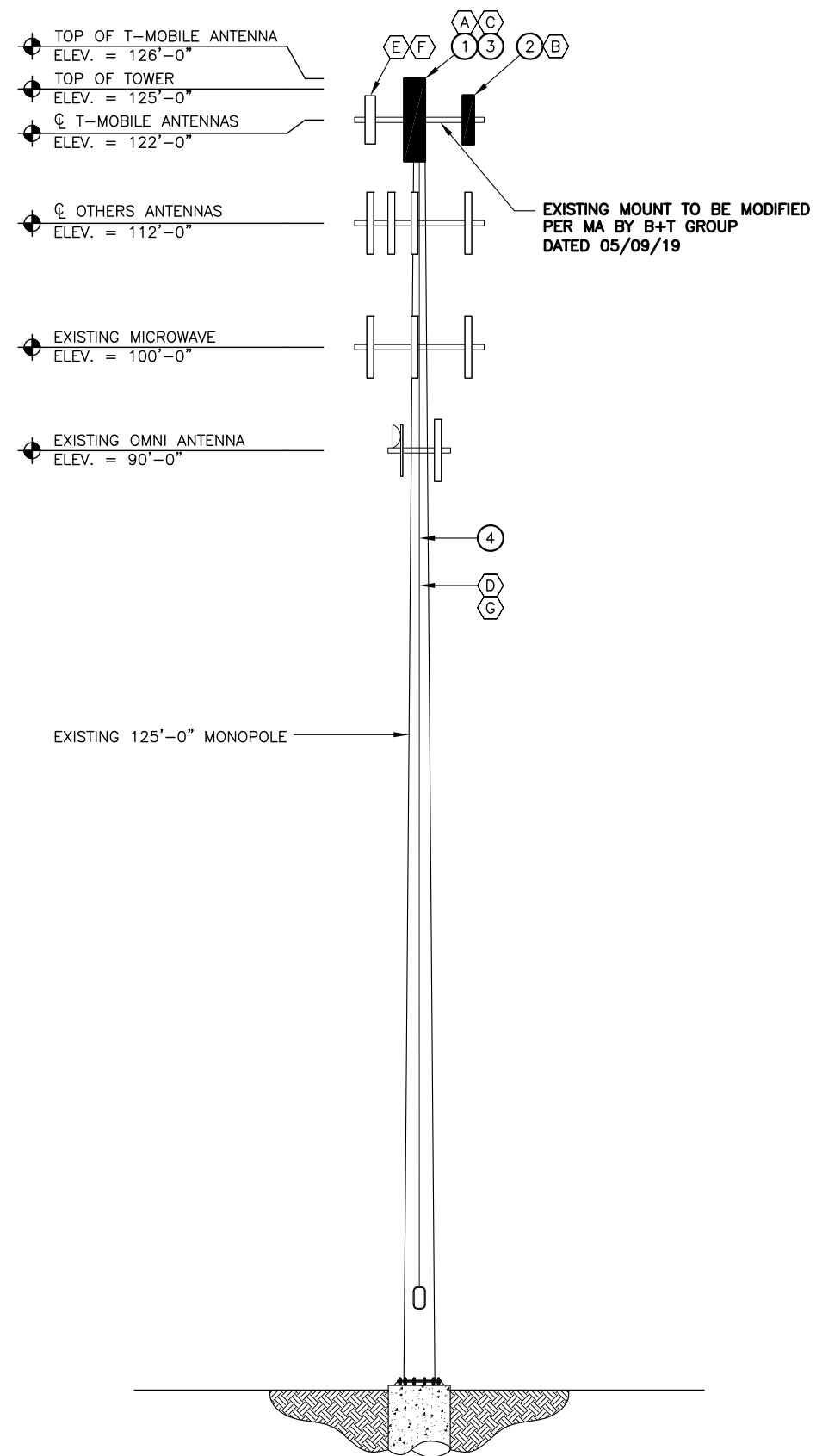


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LEGEND	
EXISTING/DEMOLITION NOTES	INSTALLATION NOTES
(A) EXISTING ANDREW LNX-6515DS-A1M ANTENNA TO BE REMOVED (TOTAL OF 3)	(1) INSTALL RFS APXVAARR24_43-U-NA20 (8 FT) ANTENNAS ON EXISTING MOUNT. (TYP. OF 1 PER SECTOR, TOTAL OF 3)
(B) EXISTING ERICSSON AIR21 KRC118023-1_B2P_B4A ANTENNA TO BE REMOVED (TOTAL OF 3)	(2) INSTALL ERICSSON AIR32 KRD901146-1_B66A_B2A ANTENNAS ON EXISTING MOUNT (TYP. OF 1 PER SECTOR, TOTAL OF 3)
(C) EXISTING RRUS11 B12 TO BE REMOVED (TOTAL OF 3)	(3) INSTALL RADIO 4449 B71/B12 (TYP. OF 1 PER SECTOR, TOTAL OF 3)
(D) EXISTING 1-5/8" COAX TO BE REMOVED (TOTAL OF 6)	(4) INSTALL (3) 1 5/8" HYBRID FIBER TRUNK FROM EQUIPMENT TO ANTENNAS FOLLOWING EXISTING ROUTING
(E) EXISTING ERICSSON AIR21 KRC118023-1_B2A_B4P ANTENNA TO REMAIN (TOTAL OF 3)	
(F) EXISTING ERICSSON KRY 112 TMA TO REMAIN (TOTAL OF 3)	
(G) EXISTING 1-5/8" COAX TO REMAIN (TOTAL OF 6)	

EXISTING TOWER IS SUFFICIENT PER STRUCTURAL ANALYSIS BY B+T GROUP DATED 5/17/19.

- LEGEND:**
-  NEW
 -  EXISTING
 -  FUTURE



CT11025B
 BU #: 822765
 BRANFORD/1-95/X55/DTN1
 10 SYLVIA ST
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PROJECT NO: 101126.005.01
 CHECKED BY: RMC

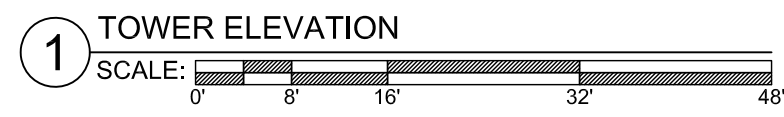
ISSUED FOR:

REV	DATE	DRWN	DESCRIPTION
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1	7/2/19	JJD	CONSTRUCTION

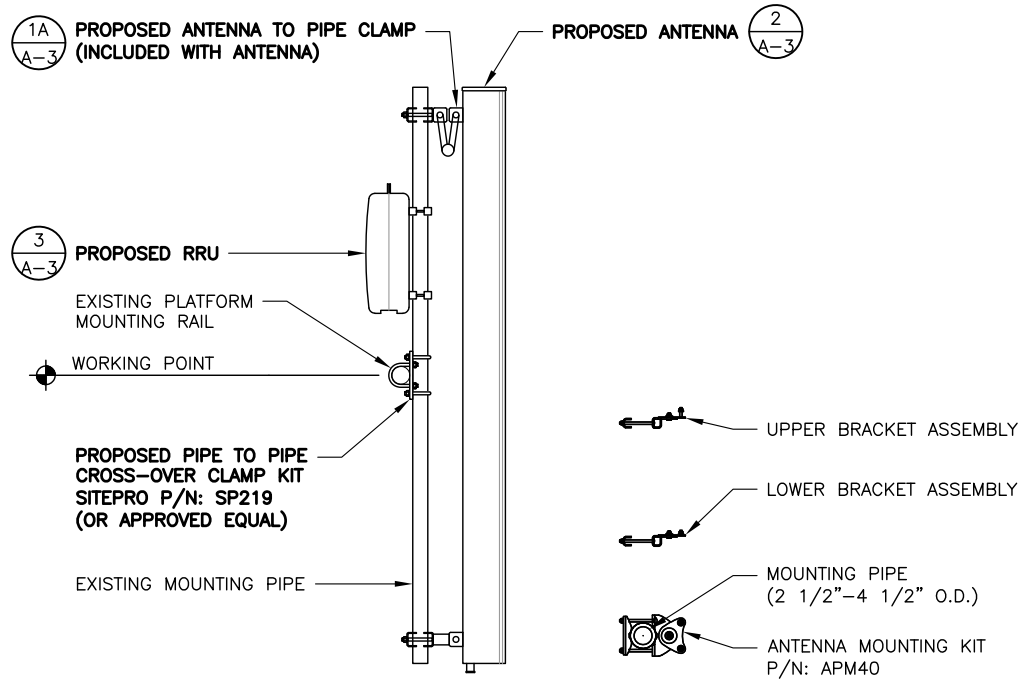
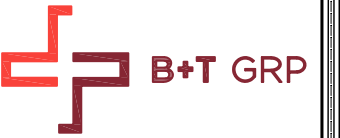
B&T ENGINEERING, INC.
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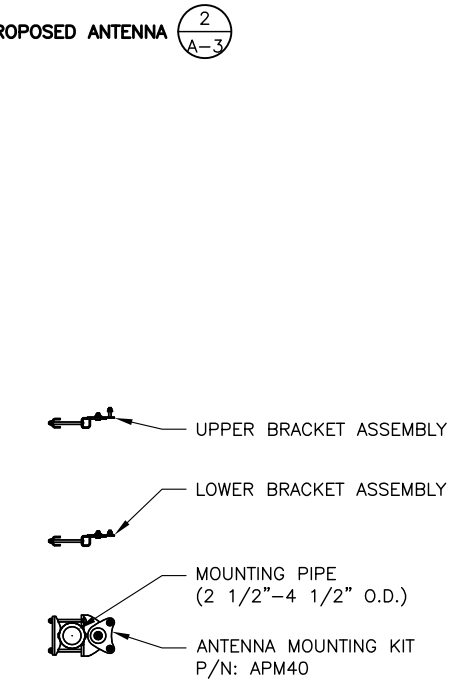
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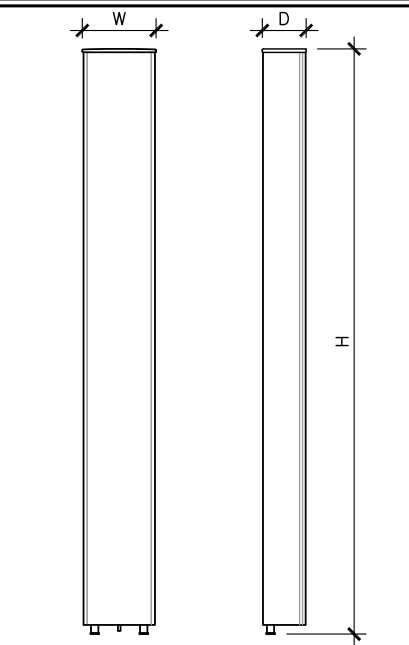
SHEET NUMBER: **A-3** REVISION: **1**



1 PROPOSED ANTENNA & RRU MOUNTING DETAIL
SCALE: N.T.S.

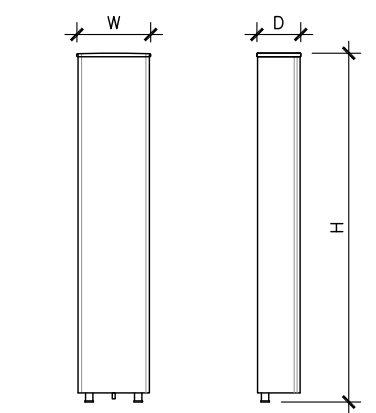


1A ANTENNA MOUNTING BRACKET
SCALE: N.T.S.



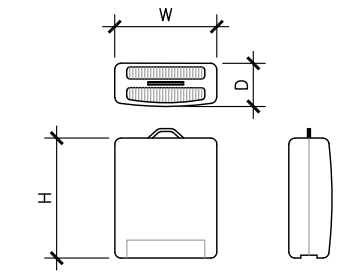
ANTENNA SPECS	
MANUFACTURER	RFS
MODEL #	APXVAARR24_43-U-NA20
WIDTH	24.0"
DEPTH	8.7"
HEIGHT	95.9"
WEIGHT	128.0 LBS

2 L700/L600 ANTENNA DETAIL
SCALE: N.T.S.



ANTENNA SPECS	
MANUFACTURER	ERICSSON
MODEL #	AIR32 KRD901146-1_B66A_B2A
WIDTH	12.9"
DEPTH	8.7"
HEIGHT	56.6"
WEIGHT	132.2 LBS

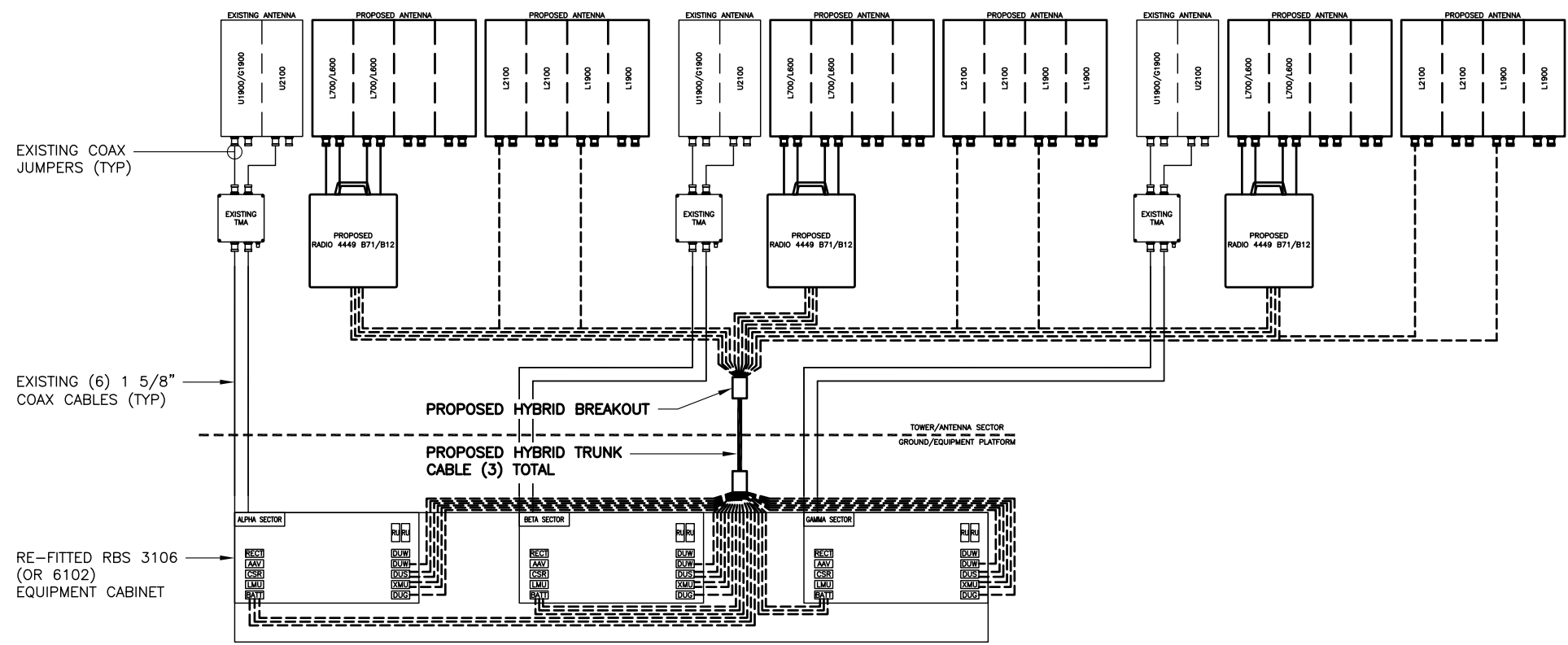
2A L1900/L2100 ANTENNA DETAIL
SCALE: N.T.S.



RRU SPECIFICATIONS	
MANUFACTURER	ERICSSON
MODEL #	4449
WIDTH	13.2"
DEPTH	10.4"
HEIGHT	14.9"
WEIGHT	74 LBS

3 REMOTE RADIO UNIT (RRU)
SCALE: N.T.S.

- NOTES:**
1. TAG ALL EXISTING AND PROPOSED CABLES/JUMPERS PER T-MOBILE SPECIFICATIONS.
 2. SEE RF SCHEDULE FOR CABLE AND JUMPER LENGTHS.
 3. REFER TO ANTENNA ORIENTATION ON SHEET C-3 FOR EXACT ANTENNA POSITIONING.



4 ANTENNA & CABLING SCHEMATIC
SCALE: N.T.S.

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B&T ENGINEERING, INC.
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CT11025B
 BU #: 822765
 BRANFORD/I-95/X55/DTN1
 10 SYLVIA ST
 BRANFORD, CT 06405
 EXISTING 125'-0" MONOPOLE

FINAL PANEL SCHEDULE									
LOAD	POLES	AMPS	BUS			AMPS	POLES	LOAD	
			L1	L2	L3				
UNKNOWN	1	20A	1		2	50A	2	UNKNOWN	
6131	2	125A	3		4				
LED FLOOD	1	20A	5		6				
			7		8				

RATED VOLTAGE: 120/240 3 PHASE, 4 WIRE
 BRANCH POLES: 12 24 30 42 APPROVED MF'RS
 RATED AMPS: 100 225 400
 CABINET: SURFACE FLUSH NEMA 1 3R 4X
 MAIN LUGS ONLY MAIN 225 AMPS BREAKER FUSED SWITCH HINGED DOOR KEYED DOOR LATCH
 FUSED CIRCUIT BREAKER BRANCH DEVICES TO BE GFCI BREAKERS FULL NEUTRAL BUS GROUND BAR
 ALL BREAKERS MUST BE RATED TO INTERRUPT A SHORT CIRCUIT ISC OF 10,000 AMPS SYMMETRICAL

EXISTING 100A BREAKER PANEL TO BE REPLACED W/ NEW 225A BREAKER PANEL. SQUARE D P/N: Q0342MQ225RB (OR APPROVED EQUAL)
 REPLACE EXISTING BREAKER IN POSITION 3 & 5 WITH A NEW 2P 125A BREAKER
 REPLACE EXISTING WIRES FOR EXISTING 6131 CABINET WITH (3) 2/0 AWG THWN (COPPER) AND (1) #6G AWG. MINIMUM CONDUIT SIZE TO BE 2"
 FINAL PANEL DESIGN AND CALCULATIONS FOR WIRE SIZE WERE BASED OFF OF EXISTING DOCUMENTS AND PHOTOS

1 FINAL T-MOBILE PANEL DETAIL
 SCALE: N.T.S.

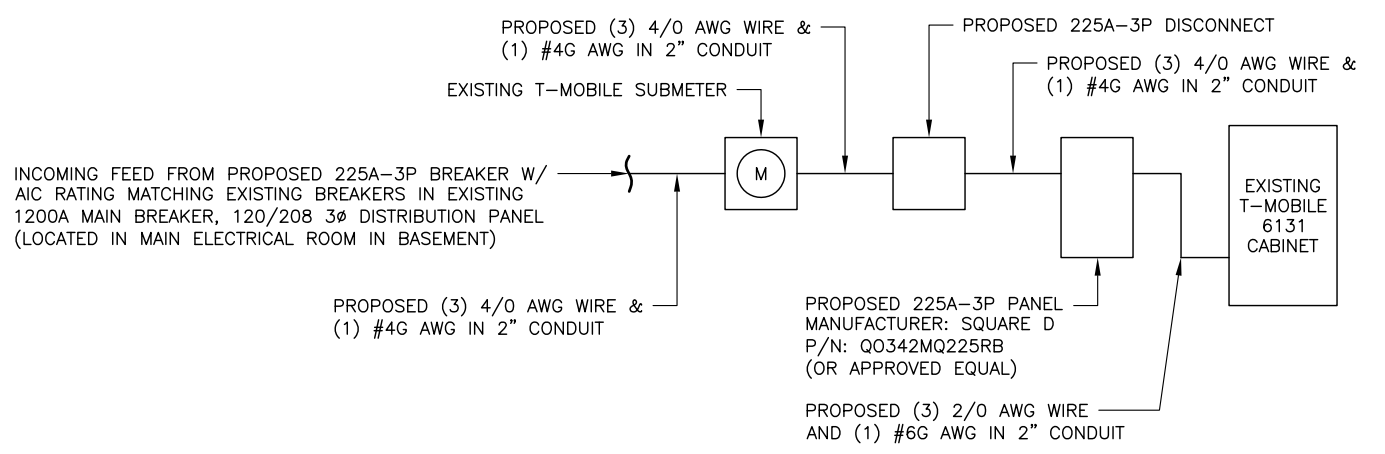
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2 ONE-LINE DIAGRAM
 SCALE: N.T.S.

SHEET NUMBER: E-1
 REVISION: 1

Exhibit D

Structural Analysis Report



Date: **May 17, 2019**

Amanda D Brown
Crown Castle
3530 Toringdon Way
Charlotte, NC 28277

B+T Group
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630

Subject: **Structural Analysis Report**

Carrier Designation: **T-Mobile Co-Locate**
Carrier Site Number: CT11025B
Carrier Site Name: Brandford/I-95/X55/Dtn1

Crown Castle Designation: **Crown Castle BU Number:** 822765
Crown Castle Site Name: Branford/ I-95/ X55/ Dtn1
Crown Castle JDE Job Number: 559227
Crown Castle Work Order Number: 1734970
Crown Castle Order Number: 479795 Rev. 0

Engineering Firm Designation: **B+T Group Project Number:** 101126.004.01

Site Data: **10 Sylvania St., Branford, New Haven County, CT**
Latitude 41° 17' 38.16", Longitude -72° 47' 8.54"
125 Foot - Monopole Tower

Dear Amanda D Brown,

B+T Group is pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above mentioned tower.

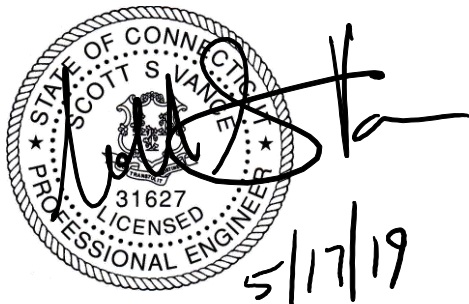
The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration	Sufficient Capacity
	96.0%

This analysis utilizes an ultimate 3-second gust wind speed of 130 mph as required by the 2018 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: John Landon

Respectfully submitted by: B+T Engineering, Inc.
COA: PEC.0001564 Expires: 10-02-2020



Scott S. Vance, P.E.

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tnxTower Output

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7) APPENDIX C

Additional Calculations

1) INTRODUCTION

This tower is a 125 ft. Monopole tower designed by Pirod Manufactures Inc. in January of 1999. The tower was originally designed for a wind speed of 85 mph per TIA/EIA-222-F. This tower has been modified by B+T Group in October of 2015 and those modifications are incorporated in this analysis.

2) ANALYSIS CRITERIA

TIA-222 Revision:	TIA-222-H
Risk Category:	II
Wind Speed:	130 mph
Exposure Category:	B
Topographic Factor:	1
Ice Thickness:	1.5 in
Wind Speed with Ice:	50 mph
Service Wind Speed:	60 mph

Table 1 - Proposed Equipment Configuration

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
122.0	122.0	3	Ericsson	AIR 32 B2A/B66AA	10	1-5/8
		3	Ericsson	ERICSSON AIR 21 B2A B4P		
		3	Ericsson	KRY 112 144/1		
		3	Ericsson	RADIO 4449 B12/B71		
		3	SitePro1	AHCP		
		3	SitePro1	SCX2-K		
		3	Rfs Celwave	APXVAARR24_43-U-NA20		
		3	--	17' x 2.375" Horizontal Pipe		
		1	--	Platform Mount [LP 405-1]		

Table 2 - Other Considered Equipment

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
112.0	113.0	3	Alcatel Lucent	B13 RRH 4X30	2	1-5/8
		3	Alcatel Lucent	B4 RRH2X60-4R		
		6	Andrew	HBXX-6517DS-A2M		
		6	Andrew	LNX-6514DS-A1M		
		2	Raycap	RXXDC-3315-PF-48		
	1	--	Platform Mount [LP 303-1]			
100.0	100.0	2	Andrew	SBNHH-1D65A	12 2 6	1-1/4 3/8 3/4
		1	Andrew	SBNHH-1D65A		
		1	Cci Antennas	HPA-65R-BUU-H6		
		3	Ericsson	RRUS 32		
		3	Ericsson	RRUS 4449 B5/B12		
		3	Ericsson	RRUS 8843 B2/B66A		
		4	Kathrein	80010964		
		2	Kathrein	80010965		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
		2	Powerwave Tech.	7770.00		
		6	Powerwave Tech.	LGP21401		
		1	Raycap	DC6-48-60-18-8F		
		2	Raycap	DC6-48-60-18-8F		
		1	--	T-Arm Mount [TA 602-3]		
90.0	90.0	2	Dragonwave	A-ANT-18G-2-C	4 2	1-1/4 1/2
		3	Alcatel Lucent	PCS 1900MHZ 4X45W-65MHZ		
		6	Alcatel Lucent	RRH2X50-800		
		3	Alcatel Lucent	TD-RRH8X20-25		
		3	Commscope	NNVV-65B-R4		
		3	Dragonwave	AIRPAIR ODU		
		3	Rfs Celwave	APXVTM14-ALU-I20		
		1	Site Pro 1	HRK12 Handrail Kit		
		1	Site Pro 1	RMQP Platform Mount		

3) ANALYSIS PROCEDURE

Table 3 - Documents Provided

Document	Remarks	Reference	Source
Online Order Information	T-Mobile Co-locate, Rev. 0	479795	CCI Sites
Tower Manufacturer Drawing	PiROD, Inc. Drawing No: 204417-B	3552248	CCI Sites
Mount Analysis	B+T Group, Project No: 101126.003.01	Date: 05/09/2019	On File
Tower Modification Drawing	B+T Group, Date: 10/27/2015	5952282	CCI Sites
Post Modification Inspection	TEP, Date: 04/11/2016	6215120	CCI Sites
Foundation Drawing	PiROD, Inc. Drawing No: 204417-B	3910040	CCI Sites
Geotech Report	F&P Associates, FPA No: 98A191ER1	3552247	CCI Sites
Antenna Configuration	Crown CAD Package	Date: 05/01/2019	CCI Sites

3.1) Analysis Method

tnxTower (version 8.0.5.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

3.2) Assumptions

- 1) The tower and structures were built and have been maintained in accordance with the manufacturer's specification.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 3) Mount areas and weights are assumed based on photographs provided.

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the tower.

4) ANALYSIS RESULTS

Table 4 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	125 - 120	Pole	TP24x24x0.375	1	-4.738	1104.673	2.1	Pass
L2	120 - 115	Pole	TP24x24x0.375	2	-5.378	1104.673	6.3	Pass
L3	115 - 110	Pole	TP24x24x0.375	3	-8.760	1104.673	13.1	Pass
L4	110 - 105	Pole	TP24x24x0.375	4	-9.427	1104.673	21.3	Pass
L5	105 - 100	Pole	TP24x24x0.375	5	-10.104	1104.673	29.8	Pass
L6	100 - 95	Pole	TP30x30x0.375	6	-13.929	1376.613	27.9	Pass
L7	95 - 90	Pole	TP30x30x0.375	7	-14.839	1376.613	35.9	Pass
L8	90 - 85	Pole	TP30x30x0.375	8	-19.075	1376.613	46.2	Pass
L9	85 - 80	Pole	TP30x30x0.375	9	-20.051	1376.613	56.3	Pass
L10	80 - 75	Pole	TP36x36x0.375	10	-21.155	1564.605	47.4	Pass
L11	75 - 73.5	Pole	TP36x36x0.375	11	-21.484	1564.605	49.6	Pass
L12	73.5 - 73.25	Pole + Reinf.	TP36x36x0.5625	12	-21.564	2485.518	39.4	Pass
L13	73.25 - 68.25	Pole + Reinf.	TP36x36x0.5625	13	-22.998	2485.518	45.4	Pass
L14	68.25 - 63.25	Pole + Reinf.	TP36x36x0.5625	14	-24.460	2485.518	51.6	Pass
L15	63.25 - 60	Pole + Reinf.	TP36x36x0.5625	15	-25.414	2485.518	55.7	Pass
L16	60 - 59.75	Pole + Reinf.	TP42x42x0.525	16	-25.499	2698.153	38.0	Pass
L17	59.75 - 54.75	Pole + Reinf.	TP42x42x0.525	17	-27.098	2698.153	42.5	Pass
L18	54.75 - 49.75	Pole + Reinf.	TP42x42x0.525	18	-28.708	2698.153	47.0	Pass
L19	49.75 - 44.75	Pole + Reinf.	TP42x42x0.525	19	-30.325	2698.153	51.8	Pass
L20	44.75 - 40	Pole + Reinf.	TP42x42x0.525	20	-31.869	2698.153	56.4	Pass
L21	40 - 39.75	Pole + Reinf.	TP48x48x0.5563	21	-31.969	3191.685	41.7	Pass
L22	39.75 - 34.75	Pole + Reinf.	TP48x48x0.5563	22	-33.851	3191.685	45.4	Pass
L23	34.75 - 29.75	Pole + Reinf.	TP48x48x0.5563	23	-35.742	3191.685	49.2	Pass
L24	29.75 - 24.75	Pole + Reinf.	TP48x48x0.5563	24	-37.640	3191.685	53.1	Pass
L25	24.75 - 20	Pole + Reinf.	TP48x48x0.5563	25	-39.448	3191.685	56.9	Pass
L26	20 - 19.75	Pole + Reinf.	TP54x54x0.5875	26	-39.564	3722.491	43.6	Pass
L27	19.75 - 14.75	Pole + Reinf.	TP54x54x0.5875	27	-41.749	3722.491	46.7	Pass
L28	14.75 - 9.75	Pole + Reinf.	TP54x54x0.5875	28	-43.942	3722.491	49.9	Pass
L29	9.75 - 4.75	Pole + Reinf.	TP54x54x0.5875	29	-46.142	3722.491	53.2	Pass
L30	4.75 - 0	Pole + Reinf.	TP54x54x0.5875	30	-48.236	3722.491	56.4	Pass
							Summary	
						Pole (L9)	56.9	Pass
						Rating =	56.9	Pass

Table 5 - Tower Component Stresses vs. Capacity – LC7

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Flange Connection	100	50.9	Pass
1	Bridge Stiffeners	80	54.5	Pass
1	Flange Connection	80	52.2	Pass
1	Bridge Stiffeners	60	65.3	Pass
1	Flange Connection	60	61.1	Pass
1	Bridge Stiffeners	40	61.3	Pass
1	Flange Connection	40	64.8	Pass
1	Bridge Stiffeners	20	60.7	Pass
1	Flange Connection	20	64.7	Pass
1	Anchor Rods Brackets	Base	33.3	Pass
1	Anchor Rods	Base	52.2	Pass
1	Base plate	Base	96.0	Pass
1	Base Foundation (Structure)	Base	69.5	Pass
1	Base Foundation (Soil Interaction)	Base	62.9	Pass

Structure Rating (max from all components) =	96.0%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H Section 15.5

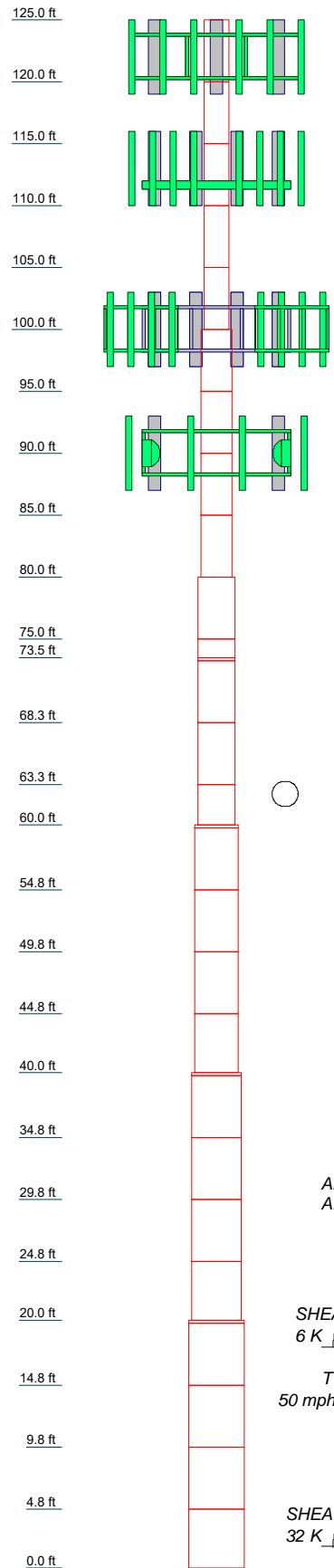
4.1) Recommendations

The tower and its foundations have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

APPENDIX A

TNXTOWER OUTPUT

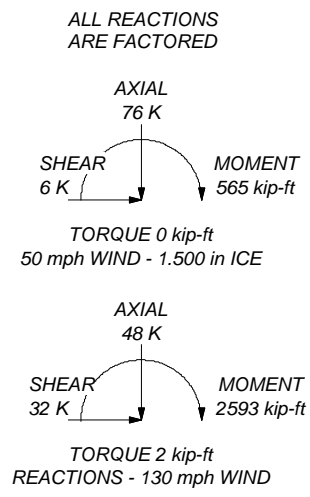
Section	Size	Length (ft)	Grade	Weight (K)
1				5.000
2				5.000
3				5.000
4				5.000
5				5.000
6				5.000
7				5.000
8				5.000
9				5.000
10				5.000
11				
12				
13				5.000
14				5.000
15				5.000
16				5.000
17				5.000
18				5.000
19				5.000
20				5.000
21				5.000
22				5.000
23				5.000
24				5.000
25				5.000
26				5.000
27				5.000
28				5.000
29				5.000
30				5.000
				4.750



MATERIAL STRENGTH					
GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-42	42 ksi	63 ksi			

TOWER DESIGN NOTES

1. Tower designed for Exposure B to the TIA-222-H Standard.
2. Tower designed for a 130 mph basic wind in accordance with the TIA-222-H Standard.
3. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 60 mph wind.
5. Tower Risk Category II.
6. Topographic Category 1 with Crest Height of 0.000 ft
7. TIA-222-H Annex S
8. TOWER RATING: 56.9%



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	Project:	Client: Crown Castle	Drawn by: Shashank.S.Rao
	Code: TIA-222-H	Date: 05/16/19	App'd:
	Path:		Scale: NTS
			Dwg No. E-1

Vx

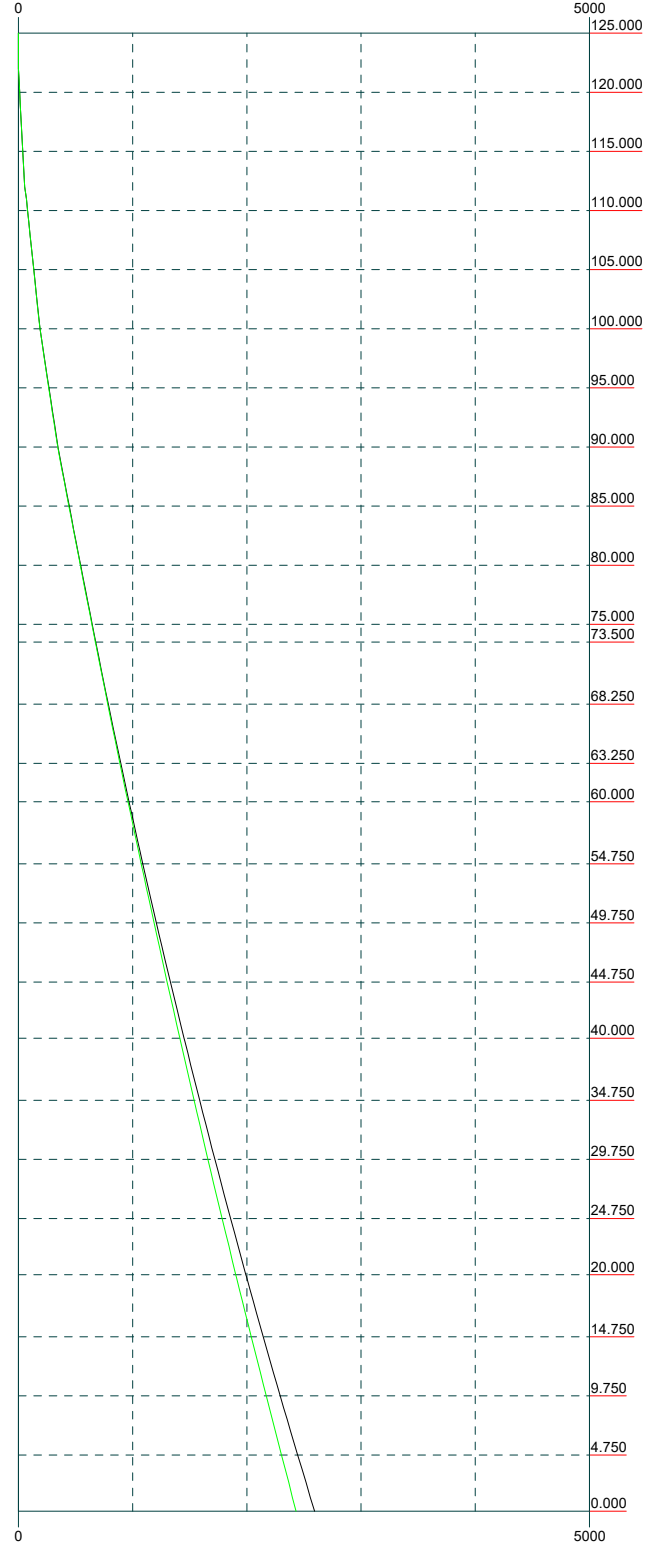
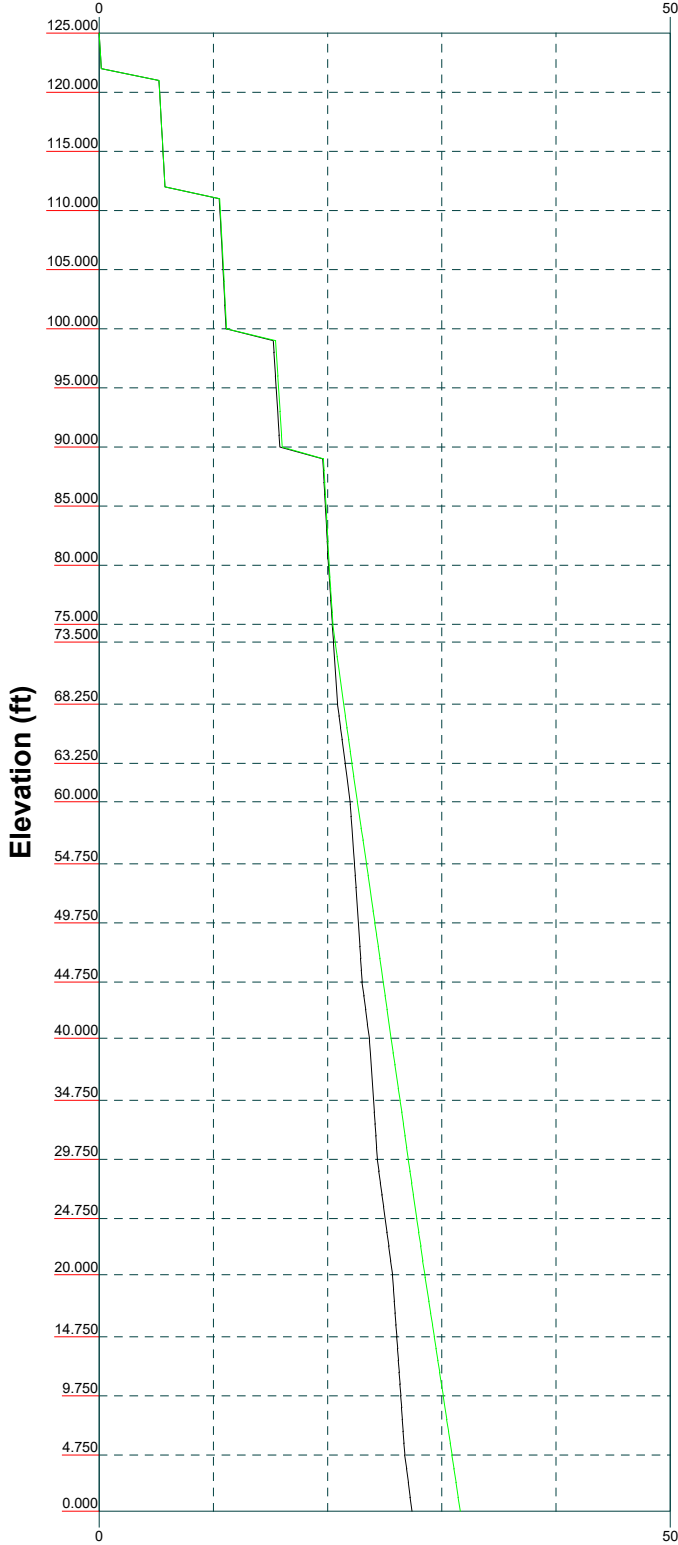
Vz


Mx

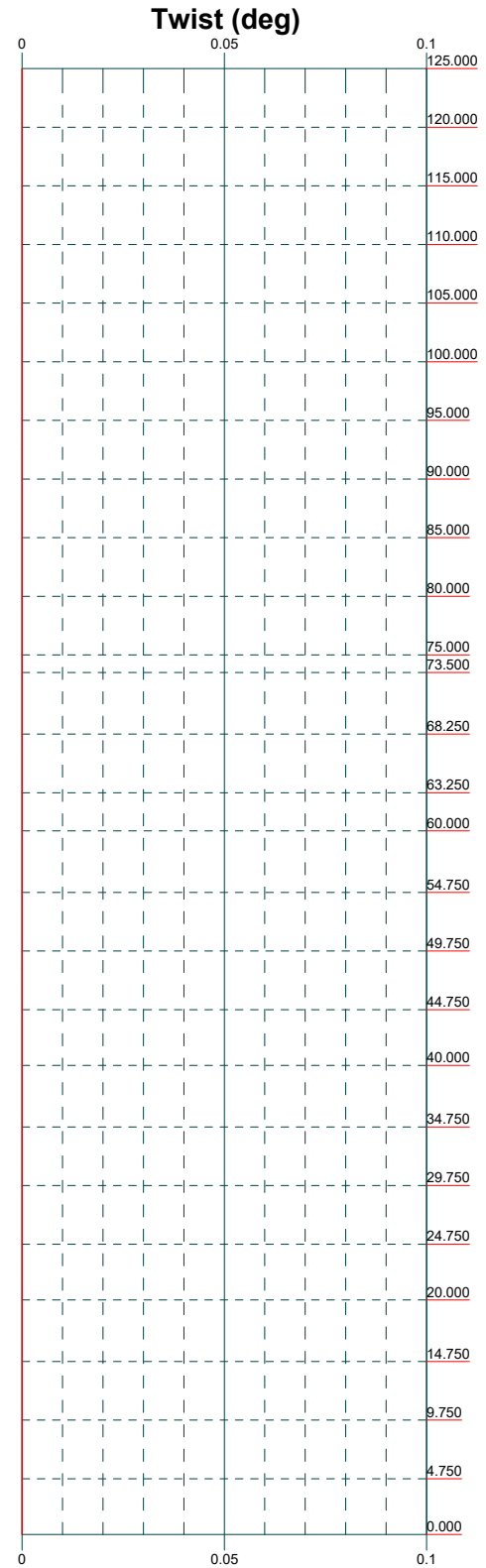
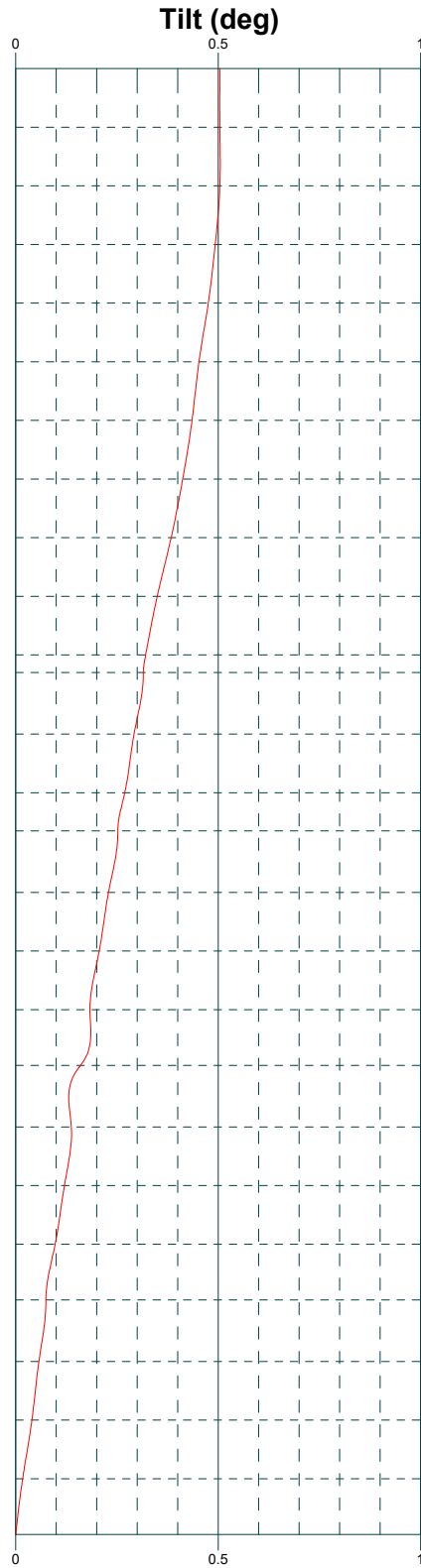
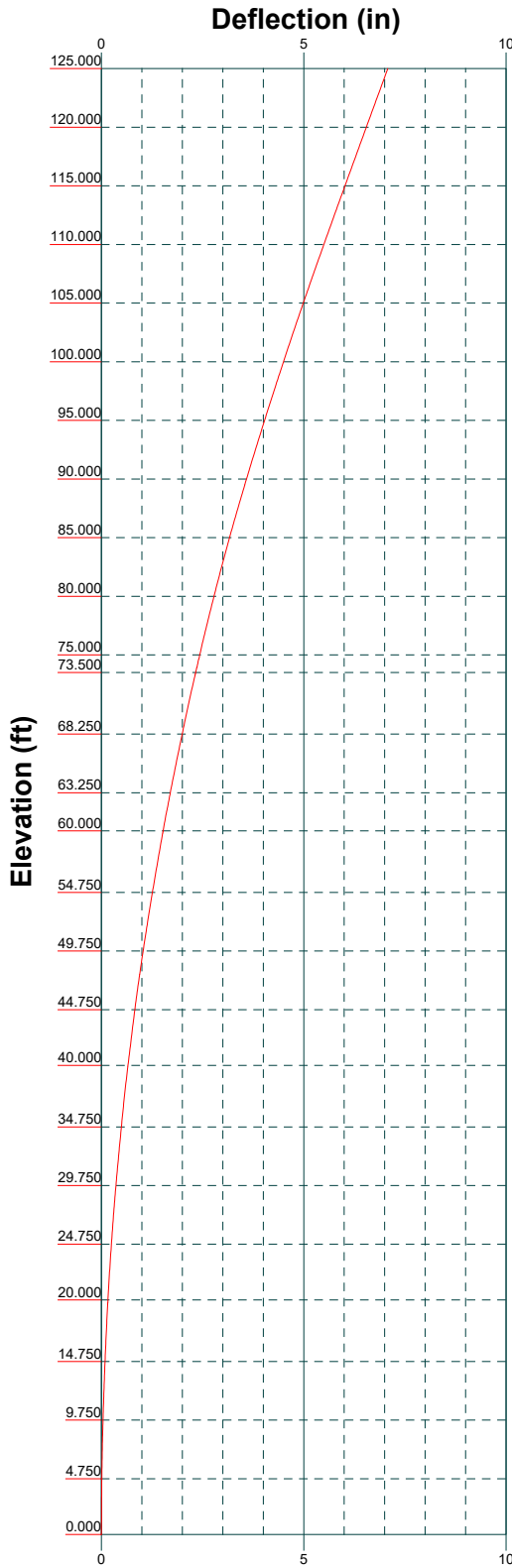
Mz

Global Mast Shear (K)

Global Mast Moment (kip-ft)



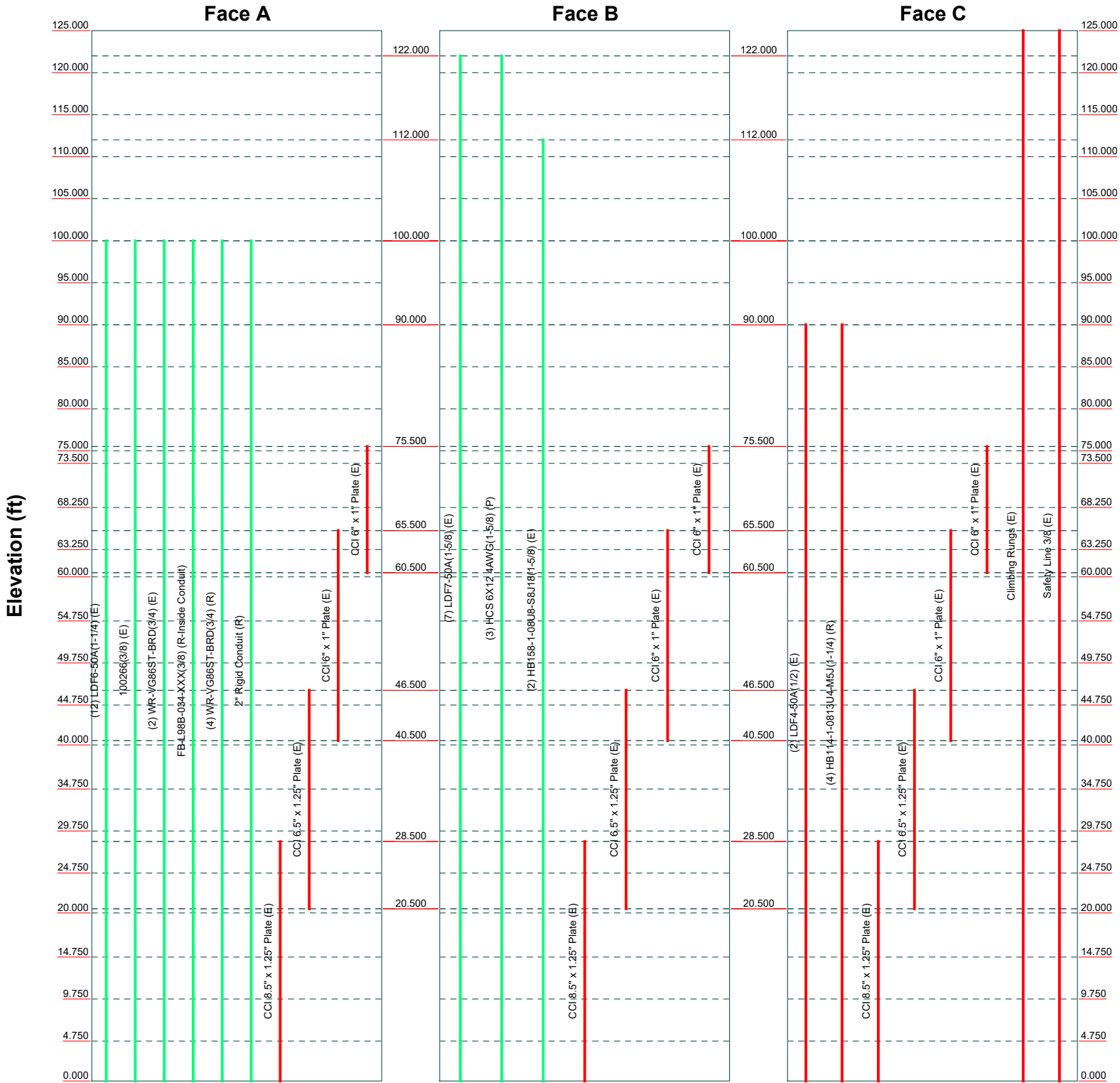
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	<p>Project:</p>	<p>Client: Crown Castle</p>	<p>Drawn by: Shashank.S.Rao</p>	<p>App'd:</p>
	<p>Code: TIA-222-H</p>	<p>Date: 05/16/19</p>	<p>Scale: NTS</p>	
	<p>Path:</p>	<p>Dwg No. E-4</p>		<p>Path:</p>



Feed Line Distribution Chart

0' - 125'

— Round
 — Flat
 — App In Face
 — App Out Face
 — Truss Leg



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Job: 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 82276)		
Project:		
Client: Crown Castle	Drawn by: Shashank.S.Rao	App'd:
Code: TIA-222-H	Date: 05/16/19	Scale: NTS
Path:	Dwg No. E-7	

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)	Page 1 of 34
	Project	Date 19:32:46 05/16/19
	Client Crown Castle	Designed by Shashank.S.Rao

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower base elevation above sea level: 56.000 ft.

Basic wind speed of 130 mph.

Risk Category II.

Exposure Category B.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.000 ft.

Nominal ice thickness of 1.500 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 60 mph.

TIA-222-H Annex S.

TOWER RATING: 56.9%.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.05.

Tower analysis based on target reliabilities in accordance with Annex S.

Load Modification Factors used: $K_{es}(F_w) = 0.95$, $K_{es}(t_i) = 0.85$.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate √ Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks √ Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Ignore KL/ry For 60 Deg. Angle Legs 	<ul style="list-style-type: none"> Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation √ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-H Bracing Resist. Exemption Use TIA-222-H Tension Splice Exemption <div style="background-color: #e0e0e0; text-align: center; padding: 2px;">Poles</div> <ul style="list-style-type: none"> √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets Pole Without Linear Attachments Pole With Shroud Or No Appurtenances Outside and Inside Corner Radii Are Known
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	<p>Project</p>	<p>Date 19:32:46 05/16/19</p>
	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Pole Section Geometry

Section	Elevation ft	Section Length ft	Pole Size	Pole Grade	Socket Length ft
L1	125.000-120.000	5.000	P24x0.375	A53-B-42 (42 ksi)	
L2	120.000-115.000	5.000	P24x0.375	A53-B-42 (42 ksi)	
L3	115.000-110.000	5.000	P24x0.375	A53-B-42 (42 ksi)	
L4	110.000-105.000	5.000	P24x0.375	A53-B-42 (42 ksi)	
L5	105.000-100.000	5.000	P24x0.375	A53-B-42 (42 ksi)	
L6	100.000-95.000	5.000	P30x0.375	A53-B-42 (42 ksi)	
L7	95.000-90.000	5.000	P30x0.375	A53-B-42 (42 ksi)	
L8	90.000-85.000	5.000	P30x0.375	A53-B-42 (42 ksi)	
L9	85.000-80.000	5.000	P30x0.375	A53-B-42 (42 ksi)	
L10	80.000-75.000	5.000	P36x0.375	A53-B-42 (42 ksi)	
L11	75.000-73.500	1.500	P36x0.375	A53-B-42 (42 ksi)	
L12	73.500-73.250	0.250	P36x0.5625	A53-B-42 (42 ksi)	
L13	73.250-68.250	5.000	P36x0.5625	A53-B-42 (42 ksi)	
L14	68.250-63.250	5.000	P36x0.5625	A53-B-42 (42 ksi)	
L15	63.250-60.000	3.250	P36x0.5625	A53-B-42 (42 ksi)	
L16	60.000-59.750	0.250	P42x0.525	A53-B-42 (42 ksi)	
L17	59.750-54.750	5.000	P42x0.525	A53-B-42 (42 ksi)	
L18	54.750-49.750	5.000	P42x0.525	A53-B-42 (42 ksi)	
L19	49.750-44.750	5.000	P42x0.525	A53-B-42 (42 ksi)	
L20	44.750-40.000	4.750	P42x0.525	A53-B-42 (42 ksi)	
L21	40.000-39.750	0.250	P48x0.55625	A53-B-42 (42 ksi)	
L22	39.750-34.750	5.000	P48x0.55625	A53-B-42 (42 ksi)	
L23	34.750-29.750	5.000	P48x0.55625	A53-B-42 (42 ksi)	
L24	29.750-24.750	5.000	P48x0.55625	A53-B-42 (42 ksi)	
L25	24.750-20.000	4.750	P48x0.55625	A53-B-42 (42 ksi)	
L26	20.000-19.750	0.250	P54x0.5875	A53-B-42 (42 ksi)	
L27	19.750-14.750	5.000	P54x0.5875	A53-B-42 (42 ksi)	
L28	14.750-9.750	5.000	P54x0.5875	A53-B-42 (42 ksi)	
L29	9.750-4.750	5.000	P54x0.5875	A53-B-42 (42 ksi)	

<p style="text-align: center;">tnxTower</p> <p style="text-align: center;">B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)	Page 4 of 34
	Project	Date 19:32:46 05/16/19
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Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft ²	in					in	in	in
L23				1	1	0.970732			
34.750-29.750									
L24				1	1	0.970732			
29.750-24.750									
L25				1	1	0.970732			
24.750-20.000									
L26				1	1	0.96417			
20.000-19.750									
L27				1	1	0.96417			
19.750-14.750									
L28				1	1	0.96417			
14.750-9.750									
L29				1	1	0.96417			
9.750-4.750									
L30				1	1	0.96417			
4.750-0.000									

Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Sector	Exclude From Torque Calculation	Component Type	Placement	Total Number	Number Per Row	Start/End Position	Width or Diameter	Perimeter	Weight
				ft				in	in	klf
LDF4-50A(1/2)(E)	C	No	Surface Ar (CaAa)	90.000 - 0.000	2	1	0.410 0.425	0.630		0.000
HB114-1-0813U4-M5J(1-1/4)(R)	C	No	Surface Ar (CaAa)	90.000 - 0.000	4	4	0.250 0.400	1.540		0.001
&&										
CCI 8.5" x 1.25" Plate (E)	A	No	Surface Af (CaAa)	28.500 - 0.000	1	1	0.000 0.000	8.500	19.500	0.000
CCI 8.5" x 1.25" Plate (E)	B	No	Surface Af (CaAa)	28.500 - 0.000	1	1	0.000 0.000	8.500	19.500	0.000
CCI 8.5" x 1.25" Plate (E)	C	No	Surface Af (CaAa)	28.500 - 0.000	1	1	0.000 0.000	8.500	19.500	0.000
&&										
CCI 6.5" x 1.25" Plate (E)	A	No	Surface Af (CaAa)	46.500 - 20.500	1	1	0.000 0.000	6.500	15.500	0.000
CCI 6.5" x 1.25" Plate (E)	B	No	Surface Af (CaAa)	46.500 - 20.500	1	1	0.000 0.000	6.500	15.500	0.000
CCI 6.5" x 1.25" Plate (E)	C	No	Surface Af (CaAa)	46.500 - 20.500	1	1	0.000 0.000	6.500	15.500	0.000
&&										
CCI 6" x 1" Plate (E)	A	No	Surface Af (CaAa)	65.500 - 40.500	1	1	0.000 0.000	6.000	14.000	0.000
CCI 6" x 1" Plate (E)	B	No	Surface Af (CaAa)	65.500 - 40.500	1	1	0.000 0.000	6.000	14.000	0.000
CCI 6" x 1" Plate (E)	C	No	Surface Af (CaAa)	65.500 - 40.500	1	1	0.000 0.000	6.000	14.000	0.000
&&										
CCI 6" x 1" Plate (E)	A	No	Surface Af (CaAa)	75.500 - 60.500	1	1	0.000 0.000	6.000	14.000	0.000
CCI 6" x 1" Plate (E)	B	No	Surface Af (CaAa)	75.500 - 60.500	1	1	0.000 0.000	6.000	14.000	0.000
CCI 6" x 1" Plate (E)	C	No	Surface Af (CaAa)	75.500 - 60.500	1	1	0.000 0.000	6.000	14.000	0.000

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	<p>Project</p>	<p>Date 19:32:46 05/16/19</p>
	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Description	Sector	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	Number Per Row	Start/End Position	Width or Diameter in	Perimeter in	Weight klf
(E) * & & *			(CaAa)	60.500			0.000			
Climbing Rungs (E)	C	No	Surface Ar (CaAa)	125.000 - 0.000	1	1	0.000 0.100	0.000		0.001
Safety Line 3/8 (E) * & & *	C	No	Surface Ar (CaAa)	125.000 - 0.000	1	1	0.010 0.030	0.375		0.000
Bridge Stiffener (78"x4.5"x1.0")-80' (E-BS)	A	No	Surface Af (CaAa)	80.000 - 80.000	1	1	0.000 0.000	4.500	11.000	0.000
Bridge Stiffener (78"x4.5"x1.0")-80' (E-BS)	B	No	Surface Af (CaAa)	80.000 - 80.000	1	1	0.000 0.000	4.500	11.000	0.000
Bridge Stiffener (78"x4.5"x1.0")-80' (E-BS) * * * *	C	No	Surface Af (CaAa)	80.000 - 80.000	1	1	0.000 0.000	4.500	11.000	0.000

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C _{AA} ft ² /ft	Weight klf
LDF7-50A(1-5/8) (E)	B	No	No	Inside Pole	122.000 - 0.000	7	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.001 0.001 0.001 0.001
HCS 6X12 4AWG(1-5/8) (P) * & & *	B	No	No	Inside Pole	122.000 - 0.000	3	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.002 0.002 0.002 0.002
HB158-1-08U8-S8J 18(1-5/8) (E) * & & *	B	No	No	Inside Pole	112.000 - 0.000	2	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.001 0.001 0.001 0.001
LDF6-50A(1-1/4) (E)	A	No	No	Inside Pole	100.000 - 0.000	12	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.001 0.001 0.001 0.001
100266(3/8) (E)	A	No	No	Inside Pole	100.000 - 0.000	1	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000
WR-VG86ST-BRD(3/4) (E)	A	No	No	Inside Pole	100.000 - 0.000	2	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.001 0.001 0.001 0.001
FB-L98B-034-XXX(3/8) (R-Inside Conduit)	A	No	No	Inside Pole	100.000 - 0.000	1	No Ice 1/2" Ice 1" Ice 2" Ice	0.000 0.000 0.000 0.000	0.000 0.000 0.000 0.000

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	<p>Project</p>	<p>Date 19:32:46 05/16/19</p>
	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		CAAA ft ² /ft	Weight klf
WR-VG86ST-BRD(3/4)(R)	A	No	No	Inside Pole	100.000 - 0.000	4	No Ice	0.000	0.001
							1/2" Ice	0.000	0.001
							1" Ice	0.000	0.001
							2" Ice	0.000	0.001
2" Rigid Conduit (R)	A	No	No	Inside Pole	100.000 - 0.000	1	No Ice	0.000	0.003
							1/2" Ice	0.000	0.003
							1" Ice	0.000	0.003
							2" Ice	0.000	0.003
* & & *									
*									

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	CAAA In Face ft ²	CAAA Out Face ft ²	Weight K
L1	125.000-120.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.026
		C	0.000	0.000	0.188	0.000	0.004
L2	120.000-115.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.065
		C	0.000	0.000	0.188	0.000	0.004
L3	115.000-110.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.070
		C	0.000	0.000	0.188	0.000	0.004
L4	110.000-105.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.078
		C	0.000	0.000	0.188	0.000	0.004
L5	105.000-100.000	A	0.000	0.000	0.000	0.000	0.000
		B	0.000	0.000	0.000	0.000	0.078
		C	0.000	0.000	0.188	0.000	0.004
L6	100.000-95.000	A	0.000	0.000	0.000	0.000	0.068
		B	0.000	0.000	0.000	0.000	0.078
		C	0.000	0.000	0.188	0.000	0.004
L7	95.000-90.000	A	0.000	0.000	0.000	0.000	0.068
		B	0.000	0.000	0.000	0.000	0.078
		C	0.000	0.000	0.188	0.000	0.004
L8	90.000-85.000	A	0.000	0.000	0.000	0.000	0.068
		B	0.000	0.000	0.000	0.000	0.078
		C	0.000	0.000	3.583	0.000	0.029
L9	85.000-80.000	A	0.000	0.000	0.000	0.000	0.068
		B	0.000	0.000	0.000	0.000	0.078
		C	0.000	0.000	3.583	0.000	0.029
L10	80.000-75.000	A	0.000	0.000	0.500	0.000	0.068
		B	0.000	0.000	0.500	0.000	0.078
		C	0.000	0.000	4.082	0.000	0.029
L11	75.000-73.500	A	0.000	0.000	1.500	0.000	0.020
		B	0.000	0.000	1.500	0.000	0.023
		C	0.000	0.000	2.575	0.000	0.009
L12	73.500-73.250	A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.250	0.000	0.004
		C	0.000	0.000	0.429	0.000	0.001
L13	73.250-68.250	A	0.000	0.000	5.000	0.000	0.068
		B	0.000	0.000	5.000	0.000	0.078

<p>tnxTower</p> <p>B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p>Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)</p>	<p>Page 7 of 34</p>
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	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Tower Section	Tower Elevation ft	Face	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L14	68.250-63.250	C	0.000	0.000	8.582	0.000	0.029
		A	0.000	0.000	7.250	0.000	0.068
		B	0.000	0.000	7.250	0.000	0.078
L15	63.250-60.000	C	0.000	0.000	10.833	0.000	0.029
		A	0.000	0.000	6.000	0.000	0.044
		B	0.000	0.000	6.000	0.000	0.051
		C	0.000	0.000	8.329	0.000	0.019
L16	60.000-59.750	A	0.000	0.000	0.250	0.000	0.003
		B	0.000	0.000	0.250	0.000	0.004
		C	0.000	0.000	0.429	0.000	0.001
L17	59.750-54.750	A	0.000	0.000	5.000	0.000	0.068
		B	0.000	0.000	5.000	0.000	0.078
		C	0.000	0.000	8.582	0.000	0.029
L18	54.750-49.750	A	0.000	0.000	5.000	0.000	0.068
		B	0.000	0.000	5.000	0.000	0.078
		C	0.000	0.000	8.582	0.000	0.029
L19	49.750-44.750	A	0.000	0.000	6.896	0.000	0.068
		B	0.000	0.000	6.896	0.000	0.078
		C	0.000	0.000	10.478	0.000	0.029
L20	44.750-40.000	A	0.000	0.000	9.396	0.000	0.065
		B	0.000	0.000	9.396	0.000	0.074
		C	0.000	0.000	12.799	0.000	0.028
L21	40.000-39.750	A	0.000	0.000	0.271	0.000	0.003
		B	0.000	0.000	0.271	0.000	0.004
		C	0.000	0.000	0.450	0.000	0.001
L22	39.750-34.750	A	0.000	0.000	5.417	0.000	0.068
		B	0.000	0.000	5.417	0.000	0.078
		C	0.000	0.000	8.999	0.000	0.029
L23	34.750-29.750	A	0.000	0.000	5.417	0.000	0.068
		B	0.000	0.000	5.417	0.000	0.078
		C	0.000	0.000	8.999	0.000	0.029
L24	29.750-24.750	A	0.000	0.000	10.729	0.000	0.068
		B	0.000	0.000	10.729	0.000	0.078
		C	0.000	0.000	14.312	0.000	0.029
L25	24.750-20.000	A	0.000	0.000	11.333	0.000	0.065
		B	0.000	0.000	11.333	0.000	0.074
		C	0.000	0.000	14.737	0.000	0.028
L26	20.000-19.750	A	0.000	0.000	0.354	0.000	0.003
		B	0.000	0.000	0.354	0.000	0.004
		C	0.000	0.000	0.533	0.000	0.001
L27	19.750-14.750	A	0.000	0.000	7.083	0.000	0.068
		B	0.000	0.000	7.083	0.000	0.078
		C	0.000	0.000	10.666	0.000	0.029
L28	14.750-9.750	A	0.000	0.000	7.083	0.000	0.068
		B	0.000	0.000	7.083	0.000	0.078
		C	0.000	0.000	10.666	0.000	0.029
L29	9.750-4.750	A	0.000	0.000	7.083	0.000	0.068
		B	0.000	0.000	7.083	0.000	0.078
		C	0.000	0.000	10.666	0.000	0.029
L30	4.750-0.000	A	0.000	0.000	6.729	0.000	0.065
		B	0.000	0.000	6.729	0.000	0.074
		C	0.000	0.000	10.133	0.000	0.028

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
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	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L1	125.000-120.000	A	1.454	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.026
		C		0.000	0.000	3.095	0.000	0.033
L2	120.000-115.000	A	1.448	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.065
		C		0.000	0.000	3.083	0.000	0.033
L3	115.000-110.000	A	1.441	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.070
		C		0.000	0.000	3.070	0.000	0.032
L4	110.000-105.000	A	1.435	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.078
		C		0.000	0.000	3.057	0.000	0.032
L5	105.000-100.000	A	1.428	0.000	0.000	0.000	0.000	0.000
		B		0.000	0.000	0.000	0.000	0.078
		C		0.000	0.000	3.044	0.000	0.032
L6	100.000-95.000	A	1.421	0.000	0.000	0.000	0.000	0.068
		B		0.000	0.000	0.000	0.000	0.078
		C		0.000	0.000	3.029	0.000	0.032
L7	95.000-90.000	A	1.413	0.000	0.000	0.000	0.000	0.068
		B		0.000	0.000	0.000	0.000	0.078
		C		0.000	0.000	3.014	0.000	0.031
L8	90.000-85.000	A	1.406	0.000	0.000	0.000	0.000	0.068
		B		0.000	0.000	0.000	0.000	0.078
		C		0.000	0.000	10.326	0.000	0.145
L9	85.000-80.000	A	1.397	0.000	0.000	0.000	0.000	0.068
		B		0.000	0.000	0.000	0.000	0.078
		C		0.000	0.000	10.291	0.000	0.144
L10	80.000-75.000	A	1.389	0.000	0.000	0.610	0.000	0.073
		B		0.000	0.000	0.610	0.000	0.083
		C		0.000	0.000	10.864	0.000	0.148
L11	75.000-73.500	A	1.383	0.000	0.000	1.830	0.000	0.036
		B		0.000	0.000	1.830	0.000	0.039
		C		0.000	0.000	4.899	0.000	0.058
L12	73.500-73.250	A	1.381	0.000	0.000	0.305	0.000	0.006
		B		0.000	0.000	0.305	0.000	0.006
		C		0.000	0.000	0.816	0.000	0.010
L13	73.250-68.250	A	1.376	0.000	0.000	6.097	0.000	0.120
		B		0.000	0.000	6.097	0.000	0.129
		C		0.000	0.000	16.297	0.000	0.193
L14	68.250-63.250	A	1.366	0.000	0.000	8.956	0.000	0.142
		B		0.000	0.000	8.956	0.000	0.152
		C		0.000	0.000	19.114	0.000	0.214
L15	63.250-60.000	A	1.357	0.000	0.000	7.480	0.000	0.105
		B		0.000	0.000	7.480	0.000	0.111
		C		0.000	0.000	14.058	0.000	0.151
L16	60.000-59.750	A	1.353	0.000	0.000	0.318	0.000	0.006
		B		0.000	0.000	0.318	0.000	0.006
		C		0.000	0.000	0.823	0.000	0.009
L17	59.750-54.750	A	1.347	0.000	0.000	6.347	0.000	0.118
		B		0.000	0.000	6.347	0.000	0.128
		C		0.000	0.000	16.425	0.000	0.188
L18	54.750-49.750	A	1.335	0.000	0.000	6.335	0.000	0.118
		B		0.000	0.000	6.335	0.000	0.127
		C		0.000	0.000	16.361	0.000	0.186
L19	49.750-44.750	A	1.322	0.000	0.000	8.680	0.000	0.136
		B		0.000	0.000	8.680	0.000	0.145
		C		0.000	0.000	18.649	0.000	0.202
L20	44.750-40.000	A	1.307	0.000	0.000	11.749	0.000	0.155
		B		0.000	0.000	11.749	0.000	0.164
		C		0.000	0.000	21.162	0.000	0.217
L21	40.000-39.750	A	1.299	0.000	0.000	0.336	0.000	0.006

<p style="text-align: center;">tnxTower</p> <p style="text-align: center;">B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	<p>Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)</p>	<p>Page 9 of 34</p>
	<p>Project</p>	<p>Date 19:32:46 05/16/19</p>
	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
		B		0.000	0.000	0.336	0.000	0.006
		C		0.000	0.000	0.830	0.000	0.009
L22	39.750-34.750	A	1.291	0.000	0.000	6.707	0.000	0.119
		B		0.000	0.000	6.707	0.000	0.129
		C		0.000	0.000	16.545	0.000	0.183
L23	34.750-29.750	A	1.272	0.000	0.000	6.689	0.000	0.119
		B		0.000	0.000	6.689	0.000	0.128
		C		0.000	0.000	16.448	0.000	0.179
L24	29.750-24.750	A	1.251	0.000	0.000	12.918	0.000	0.162
		B		0.000	0.000	12.918	0.000	0.171
		C		0.000	0.000	22.587	0.000	0.220
L25	24.750-20.000	A	1.226	0.000	0.000	13.541	0.000	0.160
		B		0.000	0.000	13.541	0.000	0.169
		C		0.000	0.000	22.627	0.000	0.213
L26	20.000-19.750	A	1.212	0.000	0.000	0.415	0.000	0.006
		B		0.000	0.000	0.415	0.000	0.007
		C		0.000	0.000	0.890	0.000	0.009
L27	19.750-14.750	A	1.195	0.000	0.000	8.278	0.000	0.124
		B		0.000	0.000	8.278	0.000	0.134
		C		0.000	0.000	17.709	0.000	0.176
L28	14.750-9.750	A	1.155	0.000	0.000	8.238	0.000	0.122
		B		0.000	0.000	8.238	0.000	0.131
		C		0.000	0.000	17.498	0.000	0.170
L29	9.750-4.750	A	1.096	0.000	0.000	8.179	0.000	0.119
		B		0.000	0.000	8.179	0.000	0.128
		C		0.000	0.000	17.188	0.000	0.160
L30	4.750-0.000	A	0.980	0.000	0.000	7.660	0.000	0.107
		B		0.000	0.000	7.660	0.000	0.116
		C		0.000	0.000	15.752	0.000	0.135

Feed Line Center of Pressure

Section	Elevation ft	CP _X in	CP _Z in	CP _X Ice in	CP _Z Ice in
L1	125.000-120.000	-0.015	0.369	-0.161	2.256
L2	120.000-115.000	-0.015	0.369	-0.160	2.250
L3	115.000-110.000	-0.015	0.369	-0.160	2.243
L4	110.000-105.000	-0.015	0.369	-0.159	2.236
L5	105.000-100.000	-0.015	0.369	-0.159	2.229
L6	100.000-95.000	-0.016	0.370	-0.167	2.345
L7	95.000-90.000	-0.016	0.370	-0.166	2.336
L8	90.000-85.000	-3.110	3.948	-2.699	4.452
L9	85.000-80.000	-3.110	3.948	-2.697	4.444
L10	80.000-75.000	-2.913	3.700	-2.755	4.546
L11	75.000-73.500	-1.487	1.889	-1.857	3.062
L12	73.500-73.250	-1.487	1.889	-1.856	3.061
L13	73.250-68.250	-1.487	1.889	-1.855	3.057
L14	68.250-63.250	-0.942	1.196	-1.575	2.594
L15	63.250-60.000	-0.823	1.044	-1.384	2.277
L16	60.000-59.750	-1.636	2.079	-1.993	3.282
L17	59.750-54.750	-1.636	2.079	-1.991	3.277
L18	54.750-49.750	-1.636	2.079	-1.987	3.266
L19	49.750-44.750	-1.369	1.739	-1.752	2.877
L20	44.750-40.000	-0.869	1.103	-1.466	2.404
L21	40.000-39.750	-1.701	2.161	-2.083	3.416
L22	39.750-34.750	-1.701	2.161	-2.079	3.408

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Section	Elevation	CP _x	CP _z	CP _x	CP _z
	ft	in	in	Ice in	Ice in
L23	34.750-29.750	-1.701	2.161	-2.072	3.390
L24	29.750-24.750	-0.898	1.141	-1.545	2.522
L25	24.750-20.000	-0.845	1.073	-1.454	2.368
L26	20.000-19.750	-1.584	2.013	-2.015	3.279
L27	19.750-14.750	-1.584	2.013	-2.008	3.261
L28	14.750-9.750	-1.584	2.013	-1.990	3.219
L29	9.750-4.750	-1.584	2.013	-1.964	3.156
L30	4.750-0.000	-1.584	2.013	-1.912	3.030

Note: For pole sections, center of pressure calculations do not consider feed line shielding.

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L1	36	Climbing Rungs	120.00 - 125.00	1.0000	1.0000
L1	37	Safety Line 3/8	120.00 - 125.00	1.0000	1.0000
L2	36	Climbing Rungs	115.00 - 120.00	1.0000	1.0000
L2	37	Safety Line 3/8	115.00 - 120.00	1.0000	1.0000
L3	36	Climbing Rungs	110.00 - 115.00	1.0000	1.0000
L3	37	Safety Line 3/8	110.00 - 115.00	1.0000	1.0000
L4	36	Climbing Rungs	105.00 - 110.00	1.0000	1.0000
L4	37	Safety Line 3/8	105.00 - 110.00	1.0000	1.0000
L5	36	Climbing Rungs	100.00 - 105.00	1.0000	1.0000
L5	37	Safety Line 3/8	100.00 - 105.00	1.0000	1.0000
L6	36	Climbing Rungs	95.00 - 100.00	1.0000	1.0000
L6	37	Safety Line 3/8	95.00 - 100.00	1.0000	1.0000
L7	36	Climbing Rungs	90.00 - 95.00	1.0000	1.0000
L7	37	Safety Line 3/8	90.00 - 95.00	1.0000	1.0000
L8	15	LDF4-50A(1/2)	85.00 - 90.00	1.0000	1.0000
L8	18	HB114-1-0813U4-M5J(1-1/4)	85.00 - 90.00	1.0000	1.0000
L8	36	Climbing Rungs	85.00 - 90.00	1.0000	1.0000
L8	37	Safety Line 3/8	85.00 - 90.00	1.0000	1.0000
L9	15	LDF4-50A(1/2)	80.00 - 85.00	1.0000	1.0000
L9	18	HB114-1-0813U4-M5J(1-1/4)	80.00 - 85.00	1.0000	1.0000
L9	36	Climbing Rungs	80.00 - 85.00	1.0000	1.0000
L9	37	Safety Line 3/8	80.00 - 85.00	1.0000	1.0000
L10	15	LDF4-50A(1/2)	75.00 - 80.00	1.0000	1.0000
L10	18	HB114-1-0813U4-M5J(1-1/4)	75.00 - 80.00	1.0000	1.0000
L10	32	CCI 6" x 1" Plate	75.00 - 75.50	1.0000	1.0000
L10	33	CCI 6" x 1" Plate	75.00 - 75.50	1.0000	1.0000

tnxTower

B+T Group
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Job
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Client
Crown Castle
Designed by
Shashank.S.Rao

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L10	34	CCI 6" x 1" Plate	75.00 - 75.50	1.0000	1.0000
L10	36	Climbing Rungs	75.00 - 80.00	1.0000	1.0000
L10	37	Safety Line 3/8	75.00 - 80.00	1.0000	1.0000
L11	15	LDF4-50A(1/2)	73.50 - 75.00	1.0000	1.0000
L11	18	HB114-1-0813U4-M5J(1-1/4)	73.50 - 75.00	1.0000	1.0000
L11	32	CCI 6" x 1" Plate	73.50 - 75.00	1.0000	1.0000
L11	33	CCI 6" x 1" Plate	73.50 - 75.00	1.0000	1.0000
L11	34	CCI 6" x 1" Plate	73.50 - 75.00	1.0000	1.0000
L11	36	Climbing Rungs	73.50 - 75.00	1.0000	1.0000
L11	37	Safety Line 3/8	73.50 - 75.00	1.0000	1.0000
L12	15	LDF4-50A(1/2)	73.25 - 73.50	1.0000	1.0000
L12	18	HB114-1-0813U4-M5J(1-1/4)	73.25 - 73.50	1.0000	1.0000
L12	32	CCI 6" x 1" Plate	73.25 - 73.50	1.0000	1.0000
L12	33	CCI 6" x 1" Plate	73.25 - 73.50	1.0000	1.0000
L12	34	CCI 6" x 1" Plate	73.25 - 73.50	1.0000	1.0000
L12	36	Climbing Rungs	73.25 - 73.50	1.0000	1.0000
L12	37	Safety Line 3/8	73.25 - 73.50	1.0000	1.0000
L13	15	LDF4-50A(1/2)	68.25 - 73.25	1.0000	1.0000
L13	18	HB114-1-0813U4-M5J(1-1/4)	68.25 - 73.25	1.0000	1.0000
L13	32	CCI 6" x 1" Plate	68.25 - 73.25	1.0000	1.0000
L13	33	CCI 6" x 1" Plate	68.25 - 73.25	1.0000	1.0000
L13	34	CCI 6" x 1" Plate	68.25 - 73.25	1.0000	1.0000
L13	36	Climbing Rungs	68.25 - 73.25	1.0000	1.0000
L13	37	Safety Line 3/8	68.25 - 73.25	1.0000	1.0000
L14	15	LDF4-50A(1/2)	63.25 - 68.25	1.0000	1.0000
L14	18	HB114-1-0813U4-M5J(1-1/4)	63.25 - 68.25	1.0000	1.0000
L14	28	CCI 6" x 1" Plate	63.25 - 65.50	1.0000	1.0000
L14	29	CCI 6" x 1" Plate	63.25 - 65.50	1.0000	1.0000
L14	30	CCI 6" x 1" Plate	63.25 - 65.50	1.0000	1.0000
L14	32	CCI 6" x 1" Plate	63.25 - 68.25	1.0000	1.0000
L14	33	CCI 6" x 1" Plate	63.25 - 68.25	1.0000	1.0000
L14	34	CCI 6" x 1" Plate	63.25 - 68.25	1.0000	1.0000
L14	36	Climbing Rungs	63.25 - 68.25	1.0000	1.0000
L14	37	Safety Line 3/8	63.25 - 68.25	1.0000	1.0000
L15	15	LDF4-50A(1/2)	60.00 - 63.25	1.0000	1.0000
L15	18	HB114-1-0813U4-M5J(1-1/4)	60.00 - 63.25	1.0000	1.0000
L15	28	CCI 6" x 1" Plate	60.00 - 63.25	1.0000	1.0000
L15	29	CCI 6" x 1" Plate	60.00 - 63.25	1.0000	1.0000
L15	30	CCI 6" x 1" Plate	60.00 - 63.25	1.0000	1.0000
L15	32	CCI 6" x 1" Plate	60.50 - 63.25	1.0000	1.0000
L15	33	CCI 6" x 1" Plate	60.50 - 63.25	1.0000	1.0000
L15	34	CCI 6" x 1" Plate	60.50 - 63.25	1.0000	1.0000
L15	36	Climbing Rungs	60.00 - 63.25	1.0000	1.0000
L15	37	Safety Line 3/8	60.00 - 63.25	1.0000	1.0000
L16	15	LDF4-50A(1/2)	59.75 - 60.00	1.0000	1.0000
L16	18	HB114-1-0813U4-M5J(1-1/4)	59.75 - 60.00	1.0000	1.0000
L16	28	CCI 6" x 1" Plate	59.75 - 60.00	1.0000	1.0000
L16	29	CCI 6" x 1" Plate	59.75 - 60.00	1.0000	1.0000
L16	30	CCI 6" x 1" Plate	59.75 - 60.00	1.0000	1.0000
L16	36	Climbing Rungs	59.75 - 60.00	1.0000	1.0000
L16	37	Safety Line 3/8	59.75 - 60.00	1.0000	1.0000
L17	15	LDF4-50A(1/2)	54.75 - 59.75	1.0000	1.0000
L17	18	HB114-1-0813U4-M5J(1-1/4)	54.75 - 59.75	1.0000	1.0000
L17	28	CCI 6" x 1" Plate	54.75 - 59.75	1.0000	1.0000
L17	29	CCI 6" x 1" Plate	54.75 - 59.75	1.0000	1.0000

tnxTower

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Client

Crown Castle

Designed by
Shashank.S.Rao

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L17	30	CCI 6" x 1" Plate	54.75 - 59.75	1.0000	1.0000
L17	36	Climbing Rungs	54.75 - 59.75	1.0000	1.0000
L17	37	Safety Line 3/8	54.75 - 59.75	1.0000	1.0000
L18	15	LDF4-50A(1/2)	49.75 - 54.75	1.0000	1.0000
L18	18	HB114-1-0813U4-M5J(1-1/4)	49.75 - 54.75	1.0000	1.0000
L18	28	CCI 6" x 1" Plate	49.75 - 54.75	1.0000	1.0000
L18	29	CCI 6" x 1" Plate	49.75 - 54.75	1.0000	1.0000
L18	30	CCI 6" x 1" Plate	49.75 - 54.75	1.0000	1.0000
L18	36	Climbing Rungs	49.75 - 54.75	1.0000	1.0000
L18	37	Safety Line 3/8	49.75 - 54.75	1.0000	1.0000
L19	15	LDF4-50A(1/2)	44.75 - 49.75	1.0000	1.0000
L19	18	HB114-1-0813U4-M5J(1-1/4)	44.75 - 49.75	1.0000	1.0000
L19	24	CCI 6.5" x 1.25" Plate	44.75 - 46.50	1.0000	1.0000
L19	25	CCI 6.5" x 1.25" Plate	44.75 - 46.50	1.0000	1.0000
L19	26	CCI 6.5" x 1.25" Plate	44.75 - 46.50	1.0000	1.0000
L19	28	CCI 6" x 1" Plate	44.75 - 49.75	1.0000	1.0000
L19	29	CCI 6" x 1" Plate	44.75 - 49.75	1.0000	1.0000
L19	30	CCI 6" x 1" Plate	44.75 - 49.75	1.0000	1.0000
L19	36	Climbing Rungs	44.75 - 49.75	1.0000	1.0000
L19	37	Safety Line 3/8	44.75 - 49.75	1.0000	1.0000
L20	15	LDF4-50A(1/2)	40.00 - 44.75	1.0000	1.0000
L20	18	HB114-1-0813U4-M5J(1-1/4)	40.00 - 44.75	1.0000	1.0000
L20	24	CCI 6.5" x 1.25" Plate	40.00 - 44.75	1.0000	1.0000
L20	25	CCI 6.5" x 1.25" Plate	40.00 - 44.75	1.0000	1.0000
L20	26	CCI 6.5" x 1.25" Plate	40.00 - 44.75	1.0000	1.0000
L20	28	CCI 6" x 1" Plate	40.50 - 44.75	1.0000	1.0000
L20	29	CCI 6" x 1" Plate	40.50 - 44.75	1.0000	1.0000
L20	30	CCI 6" x 1" Plate	40.50 - 44.75	1.0000	1.0000
L20	36	Climbing Rungs	40.00 - 44.75	1.0000	1.0000
L20	37	Safety Line 3/8	40.00 - 44.75	1.0000	1.0000
L21	15	LDF4-50A(1/2)	39.75 - 40.00	1.0000	1.0000
L21	18	HB114-1-0813U4-M5J(1-1/4)	39.75 - 40.00	1.0000	1.0000
L21	24	CCI 6.5" x 1.25" Plate	39.75 - 40.00	1.0000	1.0000
L21	25	CCI 6.5" x 1.25" Plate	39.75 - 40.00	1.0000	1.0000
L21	26	CCI 6.5" x 1.25" Plate	39.75 - 40.00	1.0000	1.0000
L21	36	Climbing Rungs	39.75 - 40.00	1.0000	1.0000
L21	37	Safety Line 3/8	39.75 - 40.00	1.0000	1.0000
L22	15	LDF4-50A(1/2)	34.75 - 39.75	1.0000	1.0000
L22	18	HB114-1-0813U4-M5J(1-1/4)	34.75 - 39.75	1.0000	1.0000
L22	24	CCI 6.5" x 1.25" Plate	34.75 - 39.75	1.0000	1.0000
L22	25	CCI 6.5" x 1.25" Plate	34.75 - 39.75	1.0000	1.0000
L22	26	CCI 6.5" x 1.25" Plate	34.75 - 39.75	1.0000	1.0000
L22	36	Climbing Rungs	34.75 - 39.75	1.0000	1.0000
L22	37	Safety Line 3/8	34.75 - 39.75	1.0000	1.0000
L23	15	LDF4-50A(1/2)	29.75 - 34.75	1.0000	1.0000
L23	18	HB114-1-0813U4-M5J(1-1/4)	29.75 - 34.75	1.0000	1.0000
L23	24	CCI 6.5" x 1.25" Plate	29.75 - 34.75	1.0000	1.0000
L23	25	CCI 6.5" x 1.25" Plate	29.75 - 34.75	1.0000	1.0000
L23	26	CCI 6.5" x 1.25" Plate	29.75 - 34.75	1.0000	1.0000
L23	36	Climbing Rungs	29.75 - 34.75	1.0000	1.0000
L23	37	Safety Line 3/8	29.75 - 34.75	1.0000	1.0000
L24	15	LDF4-50A(1/2)	24.75 - 29.75	1.0000	1.0000
L24	18	HB114-1-0813U4-M5J(1-1/4)	24.75 - 29.75	1.0000	1.0000
L24	20	CCI 8.5" x 1.25" Plate	24.75 - 28.50	1.0000	1.0000
L24	21	CCI 8.5" x 1.25" Plate	24.75 - 28.50	1.0000	1.0000

tnxTower

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Project

Date
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Client

Crown Castle

Designed by
 Shashank.S.Rao

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
L24	22	CCI 8.5" x 1.25" Plate	24.75 - 28.50	1.0000	1.0000
L24	24	CCI 6.5" x 1.25" Plate	24.75 - 29.75	1.0000	1.0000
L24	25	CCI 6.5" x 1.25" Plate	24.75 - 29.75	1.0000	1.0000
L24	26	CCI 6.5" x 1.25" Plate	24.75 - 29.75	1.0000	1.0000
L24	36	Climbing Rungs	24.75 - 29.75	1.0000	1.0000
L24	37	Safety Line 3/8	24.75 - 29.75	1.0000	1.0000
L25	15	LDF4-50A(1/2)	20.00 - 24.75	1.0000	1.0000
L25	18	HB114-1-0813U4-M5J(1-1/4)	20.00 - 24.75	1.0000	1.0000
L25	20	CCI 8.5" x 1.25" Plate	20.00 - 24.75	1.0000	1.0000
L25	21	CCI 8.5" x 1.25" Plate	20.00 - 24.75	1.0000	1.0000
L25	22	CCI 8.5" x 1.25" Plate	20.00 - 24.75	1.0000	1.0000
L25	24	CCI 6.5" x 1.25" Plate	20.50 - 24.75	1.0000	1.0000
L25	25	CCI 6.5" x 1.25" Plate	20.50 - 24.75	1.0000	1.0000
L25	26	CCI 6.5" x 1.25" Plate	20.50 - 24.75	1.0000	1.0000
L25	36	Climbing Rungs	20.00 - 24.75	1.0000	1.0000
L25	37	Safety Line 3/8	20.00 - 24.75	1.0000	1.0000
L26	15	LDF4-50A(1/2)	19.75 - 20.00	1.0000	1.0000
L26	18	HB114-1-0813U4-M5J(1-1/4)	19.75 - 20.00	1.0000	1.0000
L26	20	CCI 8.5" x 1.25" Plate	19.75 - 20.00	1.0000	1.0000
L26	21	CCI 8.5" x 1.25" Plate	19.75 - 20.00	1.0000	1.0000
L26	22	CCI 8.5" x 1.25" Plate	19.75 - 20.00	1.0000	1.0000
L26	36	Climbing Rungs	19.75 - 20.00	1.0000	1.0000
L26	37	Safety Line 3/8	19.75 - 20.00	1.0000	1.0000
L27	15	LDF4-50A(1/2)	14.75 - 19.75	1.0000	1.0000
L27	18	HB114-1-0813U4-M5J(1-1/4)	14.75 - 19.75	1.0000	1.0000
L27	20	CCI 8.5" x 1.25" Plate	14.75 - 19.75	1.0000	1.0000
L27	21	CCI 8.5" x 1.25" Plate	14.75 - 19.75	1.0000	1.0000
L27	22	CCI 8.5" x 1.25" Plate	14.75 - 19.75	1.0000	1.0000
L27	36	Climbing Rungs	14.75 - 19.75	1.0000	1.0000
L27	37	Safety Line 3/8	14.75 - 19.75	1.0000	1.0000
L28	15	LDF4-50A(1/2)	9.75 - 14.75	1.0000	1.0000
L28	18	HB114-1-0813U4-M5J(1-1/4)	9.75 - 14.75	1.0000	1.0000
L28	20	CCI 8.5" x 1.25" Plate	9.75 - 14.75	1.0000	1.0000
L28	21	CCI 8.5" x 1.25" Plate	9.75 - 14.75	1.0000	1.0000
L28	22	CCI 8.5" x 1.25" Plate	9.75 - 14.75	1.0000	1.0000
L28	36	Climbing Rungs	9.75 - 14.75	1.0000	1.0000
L28	37	Safety Line 3/8	9.75 - 14.75	1.0000	1.0000
L29	15	LDF4-50A(1/2)	4.75 - 9.75	1.0000	1.0000
L29	18	HB114-1-0813U4-M5J(1-1/4)	4.75 - 9.75	1.0000	1.0000
L29	20	CCI 8.5" x 1.25" Plate	4.75 - 9.75	1.0000	1.0000
L29	21	CCI 8.5" x 1.25" Plate	4.75 - 9.75	1.0000	1.0000
L29	22	CCI 8.5" x 1.25" Plate	4.75 - 9.75	1.0000	1.0000
L29	36	Climbing Rungs	4.75 - 9.75	1.0000	1.0000
L29	37	Safety Line 3/8	4.75 - 9.75	1.0000	1.0000
L30	15	LDF4-50A(1/2)	0.00 - 4.75	1.0000	1.0000
L30	18	HB114-1-0813U4-M5J(1-1/4)	0.00 - 4.75	1.0000	1.0000
L30	20	CCI 8.5" x 1.25" Plate	0.00 - 4.75	1.0000	1.0000
L30	21	CCI 8.5" x 1.25" Plate	0.00 - 4.75	1.0000	1.0000
L30	22	CCI 8.5" x 1.25" Plate	0.00 - 4.75	1.0000	1.0000
L30	36	Climbing Rungs	0.00 - 4.75	1.0000	1.0000
L30	37	Safety Line 3/8	0.00 - 4.75	1.0000	1.0000

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)	Page 14 of 34
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	Client Crown Castle	Designed by Shashank.S.Rao

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	CAAA		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft ²	ft ²	K	
ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	6.329	5.642	0.112
			0.000	0.000			1/2" Ice	6.775	6.426	0.169
			0.000	0.000			1" Ice	7.214	7.131	0.233
							2" Ice	8.117	8.591	0.383
ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	6.329	5.642	0.112
			0.000	0.000			1/2" Ice	6.775	6.426	0.169
			0.000	0.000			1" Ice	7.214	7.131	0.233
							2" Ice	8.117	8.591	0.383
ERICSSON AIR 21 B2A B4P w/ Mount Pipe (E)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	6.329	5.642	0.112
			0.000	0.000			1/2" Ice	6.775	6.426	0.169
			0.000	0.000			1" Ice	7.214	7.131	0.233
							2" Ice	8.117	8.591	0.383
KRY 112 144/1 (E)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	0.350	0.175	0.011
			0.000	0.000			1/2" Ice	0.426	0.234	0.014
			0.000	0.000			1" Ice	0.509	0.301	0.019
							2" Ice	0.698	0.456	0.032
KRY 112 144/1 (E)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	0.350	0.175	0.011
			0.000	0.000			1/2" Ice	0.426	0.234	0.014
			0.000	0.000			1" Ice	0.509	0.301	0.019
							2" Ice	0.698	0.456	0.032
KRY 112 144/1 (E)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	0.350	0.175	0.011
			0.000	0.000			1/2" Ice	0.426	0.234	0.014
			0.000	0.000			1" Ice	0.509	0.301	0.019
							2" Ice	0.698	0.456	0.032
AIR 32 B2A/B66AA w/ Mount Pipe (P)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	6.747	6.070	0.153
			0.000	0.000			1/2" Ice	7.202	6.867	0.214
			0.000	0.000			1" Ice	7.648	7.583	0.282
							2" Ice	8.565	9.063	0.441
AIR 32 B2A/B66AA w/ Mount Pipe (P)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	6.747	6.070	0.153
			0.000	0.000			1/2" Ice	7.202	6.867	0.214
			0.000	0.000			1" Ice	7.648	7.583	0.282
							2" Ice	8.565	9.063	0.441
AIR 32 B2A/B66AA w/ Mount Pipe (P)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	6.747	6.070	0.153
			0.000	0.000			1/2" Ice	7.202	6.867	0.214
			0.000	0.000			1" Ice	7.648	7.583	0.282
							2" Ice	8.565	9.063	0.441
APXVAARR24_43-U-NA20 w/ Mount Pipe (P)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	20.480	11.024	0.161
			0.000	0.000			1/2" Ice	21.231	12.550	0.297
			0.000	0.000			1" Ice	21.990	14.099	0.444
							2" Ice	23.444	16.451	0.775
APXVAARR24_43-U-NA20 w/ Mount Pipe (P)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	20.480	11.024	0.161
			0.000	0.000			1/2" Ice	21.231	12.550	0.297
			0.000	0.000			1" Ice	21.990	14.099	0.444
							2" Ice	23.444	16.451	0.775
APXVAARR24_43-U-NA20 w/ Mount Pipe (P)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	20.480	11.024	0.161
			0.000	0.000			1/2" Ice	21.231	12.550	0.297
			0.000	0.000			1" Ice	21.990	14.099	0.444
							2" Ice	23.444	16.451	0.775
RADIO 4449 B12/B71 (P)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	1.650	1.300	0.075
			0.000	0.000			1/2" Ice	1.810	1.445	0.092
			0.000	0.000			1" Ice	1.978	1.597	0.112
							2" Ice	2.336	1.924	0.161

tnxTower B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265	Job		101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)		Page		15 of 34	
	Project				Date		19:32:46 05/16/19	
	Client		Crown Castle		Designed by		Shashank.S.Rao	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	CAAA		Weight	
			Horz	Lateral			Front	Side		
			ft	ft	°	ft	ft ²	ft ²	K	
RADIO 4449 B12/B71 (P)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	1.650	1.300	0.075
			0.000	0.000			1/2" Ice	1.810	1.445	0.092
			0.000	0.000			1" Ice	1.978	1.597	0.112
							2" Ice	2.336	1.924	0.161
RADIO 4449 B12/B71 (P)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	1.650	1.300	0.075
			0.000	0.000			1/2" Ice	1.810	1.445	0.092
			0.000	0.000			1" Ice	1.978	1.597	0.112
							2" Ice	2.336	1.924	0.161
17' x 2.375" Horizontal Mount Pipe (P-Mount Mod)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	4.037	4.037	0.080
			0.000	0.000			1/2" Ice	5.766	5.766	0.110
			0.000	0.000			1" Ice	7.510	7.510	0.151
							2" Ice	11.050	11.050	0.266
17' x 2.375" Horizontal Mount Pipe (P-Mount Mod)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	4.037	4.037	0.080
			0.000	0.000			1/2" Ice	5.766	5.766	0.110
			0.000	0.000			1" Ice	7.510	7.510	0.151
							2" Ice	11.050	11.050	0.266
17' x 2.375" Horizontal Mount Pipe (P-Mount Mod)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	4.037	4.037	0.080
			0.000	0.000			1/2" Ice	5.766	5.766	0.110
			0.000	0.000			1" Ice	7.510	7.510	0.151
							2" Ice	11.050	11.050	0.266
L 2 1/2x2 1/2x1/4x15.62" (P-Mount Mod)	A	From Leg	4.000	0.000	0.000	122.000	No Ice	0.325	0.007	0.020
			0.000	0.000			1/2" Ice	0.430	0.025	0.022
			0.000	0.000			1" Ice	0.542	0.051	0.025
							2" Ice	0.787	0.126	0.037
L 2 1/2x2 1/2x1/4x15.62" (P-Mount Mod)	B	From Leg	4.000	0.000	0.000	122.000	No Ice	0.325	0.007	0.020
			0.000	0.000			1/2" Ice	0.430	0.025	0.022
			0.000	0.000			1" Ice	0.542	0.051	0.025
							2" Ice	0.787	0.126	0.037
L 2 1/2x2 1/2x1/4x15.62" (P-Mount Mod)	C	From Leg	4.000	0.000	0.000	122.000	No Ice	0.325	0.007	0.020
			0.000	0.000			1/2" Ice	0.430	0.025	0.022
			0.000	0.000			1" Ice	0.542	0.051	0.025
							2" Ice	0.787	0.126	0.037
Platform Mount [LP 405-1] (E)	C	None			0.000	122.000	No Ice	20.800	20.800	1.800
							1/2" Ice	28.100	28.100	2.066
							1" Ice	35.400	35.400	2.332
							2" Ice	50.000	50.000	2.864
* & & *							No Ice			
							1/2" Ice			
							1" Ice			
							2" Ice			
(2) HBXX-6517DS-A2M w/ Mount Pipe (E)	A	From Leg	4.000	0.000	0.000	112.000	No Ice	8.765	6.963	0.067
			0.000	0.000			1/2" Ice	9.342	8.182	0.137
			1.000	0.000			1" Ice	9.889	9.144	0.215
							2" Ice	10.994	11.022	0.398
(2) HBXX-6517DS-A2M w/ Mount Pipe (E)	B	From Leg	4.000	0.000	0.000	112.000	No Ice	8.765	6.963	0.067
			0.000	0.000			1/2" Ice	9.342	8.182	0.137
			1.000	0.000			1" Ice	9.889	9.144	0.215
							2" Ice	10.994	11.022	0.398
(2) HBXX-6517DS-A2M w/ Mount Pipe (E)	C	From Leg	4.000	0.000	0.000	112.000	No Ice	8.765	6.963	0.067
			0.000	0.000			1/2" Ice	9.342	8.182	0.137
			1.000	0.000			1" Ice	9.889	9.144	0.215
							2" Ice	10.994	11.022	0.398
(2) LNX-6514DS-A1M w/ Mount Pipe (E)	A	From Leg	4.000	0.000	0.000	112.000	No Ice	8.411	7.082	0.065
			0.000	0.000			1/2" Ice	8.975	8.273	0.134
			1.000	0.000			1" Ice	9.505	9.185	0.211
							2" Ice	10.585	11.023	0.393
(2) LNX-6514DS-A1M w/ Mount Pipe (E)	B	From Leg	4.000	0.000	0.000	112.000	No Ice	8.411	7.082	0.065
			0.000	0.000			1/2" Ice	8.975	8.273	0.134
			1.000	0.000			1" Ice	9.505	9.185	0.211
							2" Ice	10.585	11.023	0.393

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	CAAs		Weight	
			Horz	Vert			Front	Side		
			ft	ft	°	ft	ft ²	ft ²	K	
(2) LNX-6514DS-A1M w/ Mount Pipe (E)	C	From Leg	4.000	0.000	0.000	112.000	No Ice	8.411	7.082	0.065
			0.000				1/2" Ice	8.975	8.273	0.134
			1.000				1" Ice	9.505	9.185	0.211
							2" Ice	10.585	11.023	0.393
B4 RRH2X60-4R (E)	A	From Leg	4.000	0.000	0.000	112.000	No Ice	3.356	2.005	0.055
			0.000				1/2" Ice	3.613	2.238	0.078
			1.000				1" Ice	3.877	2.477	0.105
							2" Ice	4.425	2.976	0.170
B4 RRH2X60-4R (E)	B	From Leg	4.000	0.000	0.000	112.000	No Ice	3.356	2.005	0.055
			0.000				1/2" Ice	3.613	2.238	0.078
			1.000				1" Ice	3.877	2.477	0.105
							2" Ice	4.425	2.976	0.170
B4 RRH2X60-4R (E)	C	From Leg	4.000	0.000	0.000	112.000	No Ice	3.356	2.005	0.055
			0.000				1/2" Ice	3.613	2.238	0.078
			1.000				1" Ice	3.877	2.477	0.105
							2" Ice	4.425	2.976	0.170
B13 RRH 4X30 (E)	A	From Leg	4.000	0.000	0.000	112.000	No Ice	2.055	1.320	0.056
			0.000				1/2" Ice	2.241	1.475	0.073
			1.000				1" Ice	2.433	1.638	0.093
							2" Ice	2.841	1.997	0.142
B13 RRH 4X30 (E)	B	From Leg	4.000	0.000	0.000	112.000	No Ice	2.055	1.320	0.056
			0.000				1/2" Ice	2.241	1.475	0.073
			1.000				1" Ice	2.433	1.638	0.093
							2" Ice	2.841	1.997	0.142
B13 RRH 4X30 (E)	C	From Leg	4.000	0.000	0.000	112.000	No Ice	2.055	1.320	0.056
			0.000				1/2" Ice	2.241	1.475	0.073
			1.000				1" Ice	2.433	1.638	0.093
							2" Ice	2.841	1.997	0.142
RXXDC-3315-PF-48 (E)	A	From Leg	4.000	0.000	0.000	112.000	No Ice	3.708	2.192	0.032
			0.000				1/2" Ice	3.950	2.395	0.062
			1.000				1" Ice	4.200	2.606	0.097
							2" Ice	4.723	3.049	0.176
RXXDC-3315-PF-48 (E)	B	From Leg	4.000	0.000	0.000	112.000	No Ice	3.708	2.192	0.032
			0.000				1/2" Ice	3.950	2.395	0.062
			1.000				1" Ice	4.200	2.606	0.097
							2" Ice	4.723	3.049	0.176
Platform Mount [LP 303-1] (E)	C	None			0.000	112.000	No Ice	14.660	14.660	1.250
							1/2" Ice	18.870	18.870	1.481
							1" Ice	23.080	23.080	1.713
							2" Ice	31.500	31.500	2.175
* & & * 7770.00 w/ Mount Pipe (E)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	5.746	4.254	0.055
			0.000				1/2" Ice	6.179	5.014	0.103
			0.000				1" Ice	6.607	5.711	0.157
							2" Ice	7.488	7.155	0.287
7770.00 w/ Mount Pipe (E)	C	From Leg	4.000	0.000	0.000	100.000	No Ice	5.746	4.254	0.055
			0.000				1/2" Ice	6.179	5.014	0.103
			0.000				1" Ice	6.607	5.711	0.157
							2" Ice	7.488	7.155	0.287
HPA-65R-BUU-H6 w/ Mount Pipe (E)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	9.895	8.113	0.077
			0.000				1/2" Ice	10.470	9.304	0.158
			0.000				1" Ice	11.010	10.209	0.248
							2" Ice	12.112	12.014	0.456
SBNHH-1D65A w/ Mount Pipe (E)	B	From Leg	4.000	0.000	0.000	100.000	No Ice	3.040	2.450	0.054
			0.000				1/2" Ice	3.340	2.750	0.104
			0.000				1" Ice	3.650	3.050	0.162
							2" Ice	4.310	3.680	0.307

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	CAA Front	CAA Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft ²	ft ²	K	
SBNHH-1D65A w/ Mount Pipe (E)	C	From Leg	4.000	0.000	0.000	100.000	No Ice	3.040	2.450	0.054
			0.000				1/2" Ice	3.340	2.750	0.104
			0.000				1" Ice	3.650	3.050	0.162
							2" Ice	4.310	3.680	0.307
DC6-48-60-18-8F (E)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	1.212	1.212	0.033
			0.000				1/2" Ice	1.892	1.892	0.055
			0.000				1" Ice	2.105	2.105	0.080
							2" Ice	2.570	2.570	0.138
(2) LGP21401 (E)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	1.104	0.207	0.014
			0.000				1/2" Ice	1.239	0.274	0.021
			0.000				1" Ice	1.381	0.348	0.030
							2" Ice	1.688	0.521	0.055
(2) LGP21401 (E)	B	From Leg	4.000	0.000	0.000	100.000	No Ice	1.104	0.207	0.014
			0.000				1/2" Ice	1.239	0.274	0.021
			0.000				1" Ice	1.381	0.348	0.030
							2" Ice	1.688	0.521	0.055
(2) LGP21401 (E)	C	From Leg	4.000	0.000	0.000	100.000	No Ice	1.104	0.207	0.014
			0.000				1/2" Ice	1.239	0.274	0.021
			0.000				1" Ice	1.381	0.348	0.030
							2" Ice	1.688	0.521	0.055
(2) 80010965 w/ Mount Pipe (R)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	14.051	7.628	0.125
			0.000				1/2" Ice	14.688	8.903	0.222
			0.000				1" Ice	15.303	9.963	0.327
							2" Ice	16.530	11.925	0.569
(2) 80010964 w/ Mount Pipe (R)	B	From Leg	4.000	0.000	0.000	100.000	No Ice	10.235	5.509	0.105
			0.000				1/2" Ice	10.742	6.369	0.180
			0.000				1" Ice	11.237	7.119	0.262
							2" Ice	12.252	8.643	0.451
(2) 80010964 w/ Mount Pipe (R)	C	From Leg	4.000	0.000	0.000	100.000	No Ice	10.235	5.509	0.105
			0.000				1/2" Ice	10.742	6.369	0.180
			0.000				1" Ice	11.237	7.119	0.262
							2" Ice	12.252	8.643	0.451
SBNHH-1D65A w/ Mount Pipe (R)	B	From Leg	4.000	0.000	0.000	100.000	No Ice	3.040	2.450	0.054
			0.000				1/2" Ice	3.340	2.750	0.104
			0.000				1" Ice	3.650	3.050	0.162
							2" Ice	4.310	3.680	0.307
RRUS 8843 B2/B66A (R)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	1.639	1.353	0.072
			0.000				1/2" Ice	1.799	1.500	0.090
			0.000				1" Ice	1.966	1.655	0.110
							2" Ice	2.323	1.986	0.159
RRUS 8843 B2/B66A (R)	B	From Leg	4.000	0.000	0.000	100.000	No Ice	1.639	1.353	0.072
			0.000				1/2" Ice	1.799	1.500	0.090
			0.000				1" Ice	1.966	1.655	0.110
							2" Ice	2.323	1.986	0.159
RRUS 8843 B2/B66A (R)	C	From Leg	4.000	0.000	0.000	100.000	No Ice	1.639	1.353	0.072
			0.000				1/2" Ice	1.799	1.500	0.090
			0.000				1" Ice	1.966	1.655	0.110
							2" Ice	2.323	1.986	0.159
RRUS 4449 B5/B12 (R)	A	From Leg	4.000	0.000	0.000	100.000	No Ice	1.968	1.408	0.071
			0.000				1/2" Ice	2.144	1.564	0.090
			0.000				1" Ice	2.328	1.727	0.111
							2" Ice	2.718	2.075	0.163
RRUS 4449 B5/B12 (R)	B	From Leg	4.000	0.000	0.000	100.000	No Ice	1.968	1.408	0.071
			0.000				1/2" Ice	2.144	1.564	0.090
			0.000				1" Ice	2.328	1.727	0.111
							2" Ice	2.718	2.075	0.163
RRUS 4449 B5/B12 (R)	C	From Leg	4.000	0.000	0.000	100.000	No Ice	1.968	1.408	0.071

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CAA Front ft ²	CAA Side ft ²	Weight K
(R)			0.000 0.000			1/2" Ice 2.144 1" Ice 2.328 2" Ice 2.718	1.564 1.727 2.075	0.090 0.111 0.163
DC6-48-60-18-8F (R)	A	From Leg	4.000 0.000 0.000	0.000	100.000	No Ice 1.212 1/2" Ice 1.892 1" Ice 2.105 2" Ice 2.570	1.212 1.892 2.105 2.570	0.033 0.055 0.080 0.138
DC6-48-60-18-8F (R)	B	From Leg	4.000 0.000 0.000	0.000	100.000	No Ice 1.212 1/2" Ice 1.892 1" Ice 2.105 2" Ice 2.570	1.212 1.892 2.105 2.570	0.033 0.055 0.080 0.138
RRUS 32 (R)	A	From Leg	4.000 0.000 0.000	0.000	100.000	No Ice 2.857 1/2" Ice 3.083 1" Ice 3.316 2" Ice 3.805	1.777 1.968 2.166 2.583	0.055 0.077 0.103 0.165
RRUS 32 (R)	B	From Leg	4.000 0.000 0.000	0.000	100.000	No Ice 2.857 1/2" Ice 3.083 1" Ice 3.316 2" Ice 3.805	1.777 1.968 2.166 2.583	0.055 0.077 0.103 0.165
RRUS 32 (R)	C	From Leg	4.000 0.000 0.000	0.000	100.000	No Ice 2.857 1/2" Ice 3.083 1" Ice 3.316 2" Ice 3.805	1.777 1.968 2.166 2.583	0.055 0.077 0.103 0.165
T-Arm Mount [TA 602-3] (E)	C	None		0.000	100.000	No Ice 11.590 1/2" Ice 15.440 1" Ice 19.290 2" Ice 26.990	11.590 15.440 19.290 26.990	0.774 0.990 1.206 1.639
&&								
AIRPAIR ODU (E)	A	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1.007 1/2" Ice 1.141 1" Ice 1.282 2" Ice 1.587	0.456 0.561 0.673 0.919	0.012 0.019 0.028 0.053
AIRPAIR ODU (E)	B	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1.007 1/2" Ice 1.141 1" Ice 1.282 2" Ice 1.587	0.456 0.561 0.673 0.919	0.012 0.019 0.028 0.053
AIRPAIR ODU (E)	C	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1.007 1/2" Ice 1.141 1" Ice 1.282 2" Ice 1.587	0.456 0.561 0.673 0.919	0.012 0.019 0.028 0.053
NNVV-65B-R4 w/ Mount Pipe (R)	A	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 12.509 1/2" Ice 13.108 1" Ice 13.672 2" Ice 14.822	7.413 8.598 9.496 11.328	0.103 0.194 0.293 0.520
NNVV-65B-R4 w/ Mount Pipe (R)	B	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 12.509 1/2" Ice 13.108 1" Ice 13.672 2" Ice 14.822	7.413 8.598 9.496 11.328	0.103 0.194 0.293 0.520
NNVV-65B-R4 w/ Mount Pipe (R)	C	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 12.509 1/2" Ice 13.108 1" Ice 13.672 2" Ice 14.822	7.413 8.598 9.496 11.328	0.103 0.194 0.293 0.520
APXVTM14-ALU-I20 w/ Mount Pipe (R)	A	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 6.580 1/2" Ice 7.031 1" Ice 7.473 2" Ice 8.385	4.959 5.754 6.472 7.941	0.077 0.132 0.193 0.339
APXVTM14-ALU-I20 w/ Mount Pipe (R)	B	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 6.580 1/2" Ice 7.031 1" Ice 7.473 2" Ice 8.385	4.959 5.754 6.472 7.941	0.077 0.132 0.193 0.339

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	Client Crown Castle	Designed by Shashank.S.Rao

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	CAA Front ft ²	CAA Side ft ²	Weight K	
Mount Pipe (R)			0.000 0.000		1/2" Ice 1" Ice 2" Ice	7.031 7.473 8.385	5.754 6.472 7.941	0.132 0.193 0.339	
APXVTM14-ALU-I20 w/ Mount Pipe (R)	C	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	6.580 7.031 7.473 8.385	4.959 5.754 6.472 7.941	0.077 0.132 0.193 0.339
(3) RRH2X50-800 (R)	A	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.701 1.864 2.035 2.398	1.282 1.428 1.580 1.908	0.053 0.070 0.090 0.138
(3) RRH2X50-800 (R)	C	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.701 1.864 2.035 2.398	1.282 1.428 1.580 1.908	0.053 0.070 0.090 0.138
PCS 1900MHZ 4X45W-65MHZ (R)	A	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.322 2.527 2.739 3.185	2.238 2.441 2.651 3.093	0.060 0.083 0.110 0.173
(2) PCS 1900MHZ 4X45W-65MHZ (R)	B	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.322 2.527 2.739 3.185	2.238 2.441 2.651 3.093	0.060 0.083 0.110 0.173
(2) TD-RRH8X20-25 (R)	B	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.045 4.298 4.557 5.098	1.535 1.714 1.901 2.295	0.070 0.097 0.128 0.201
TD-RRH8X20-25 (R)	C	From Leg	4.000 0.000 0.000	0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.045 4.298 4.557 5.098	1.535 1.714 1.901 2.295	0.070 0.097 0.128 0.201
Miscellaneous [NA 507-1] (R)	C	None		0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	4.800 6.700 8.600 12.400	4.800 6.700 8.600 12.400	0.245 0.294 0.343 0.441
Platform Mount [LP 303-1] (R)	C	None		0.000	90.000	No Ice 1/2" Ice 1" Ice 2" Ice	14.660 18.870 23.080 31.500	14.660 18.870 23.080 31.500	1.250 1.481 1.713 2.175
~~*									

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight K	
Dragonwave A-ANT-18G-2-C (E)	B	Paraboloid w/Shroud (HP)	From Leg	4.000 0.000 0.000	90.000		90.000	2.175	No Ice 1/2" Ice 1" Ice	3.715 4.006 4.296	0.027 0.048 0.068

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Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert ft	Azimuth Adjustment °	3 dB Beam Width °	Elevation ft	Outside Diameter ft	Aperture Area ft ²	Weight K
Dragonwave A-ANT-18G-2-C (E)	C	Paraboloid w/Shroud (HP)	From Leg	4.000 0.000 0.000	30.000		90.000	2.175	2" Ice 4.876 No Ice 3.715 1/2" Ice 4.006 1" Ice 4.296 2" Ice 4.876	0.109 0.027 0.048 0.068 0.109
&&										

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service

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Comb. No.	Description
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	125 - 120	Pole	Max Tension	14	0.000	0.000	0.000
			Max. Compression	26	-9.085	-0.002	-0.033
			Max. Mx	8	-4.743	-10.731	-0.003
			Max. My	14	-4.738	-0.001	-10.744
			Max. Vy	8	5.292	-10.731	-0.003
			Max. Vx	14	5.296	-0.001	-10.744
			Max. Torque	8			0.000
L2	120 - 115	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-9.989	-0.006	-0.064
			Max. Mx	8	-5.382	-37.922	-0.003
			Max. My	14	-5.378	-0.003	-37.961
			Max. Vy	8	5.584	-37.922	-0.003
			Max. Vx	14	5.589	-0.003	-37.961
			Max. Torque	10			-0.000
L3	115 - 110	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-17.836	-0.612	0.251
			Max. Mx	8	-8.761	-80.072	0.017
			Max. My	2	-8.758	-0.090	80.079
			Max. Vy	8	10.573	-80.072	0.017
			Max. Vx	14	10.607	-0.227	-79.960
			Max. Torque	12			0.392
L4	110 - 105	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-18.753	-0.617	0.220
			Max. Mx	8	-9.430	-133.622	-0.095
			Max. My	2	-9.427	0.021	133.764
			Max. Vy	8	10.848	-133.622	-0.095
			Max. Vx	14	10.883	-0.345	-133.685
			Max. Torque	12			0.392
L5	105 - 100	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-19.669	-0.617	0.187
			Max. Mx	8	-10.107	-188.514	-0.207
			Max. My	2	-10.104	0.133	188.792
			Max. Vy	8	11.111	-188.514	-0.207
			Max. Vx	14	11.146	-0.463	-188.755
			Max. Torque	12			0.392
L6	100 - 95	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-28.160	-1.147	2.459
			Max. Mx	8	-13.938	-265.365	0.342
			Max. My	2	-13.929	0.006	267.145
			Max. Vy	8	15.503	-265.365	0.342
			Max. Vx	14	15.712	-0.686	-265.810
			Max. Torque	20			-2.360
L7	95 - 90	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-29.351	-1.147	2.418
			Max. Mx	8	-14.849	-343.660	0.294
			Max. My	2	-14.840	0.050	346.430
			Max. Vy	8	15.819	-343.660	0.294
			Max. Vx	14	16.028	-0.738	-345.156
			Max. Torque	20			-2.360

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L8	90 - 85	Pole	Max. Torque	20			-2.360
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-37.247	-1.631	1.342
			Max. Mx	8	-19.079	-442.143	0.309
			Max. My	2	-19.075	-0.170	444.650
			Max. Vy	8	19.812	-442.143	0.309
			Max. Vx	2	-19.868	-0.170	444.650
L9	85 - 80	Pole	Max. Torque	20			-2.360
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-38.553	-1.631	1.147
			Max. Mx	8	-20.055	-541.850	0.606
			Max. My	2	-20.051	-0.248	544.592
			Max. Vy	8	20.080	-541.850	0.606
			Max. Vx	2	-20.137	-0.248	544.592
L10	80 - 75	Pole	Max. Torque	24			-1.787
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-40.067	-1.631	0.916
			Max. Mx	8	-21.158	-643.069	0.894
			Max. My	2	-21.155	-0.326	646.037
			Max. Vy	8	20.418	-643.069	0.894
			Max. Vx	2	-20.474	-0.326	646.037
L11	75 - 73.5	Pole	Max. Torque	24			-1.787
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-40.562	-1.631	0.847
			Max. Mx	8	-21.488	-673.761	0.980
			Max. My	2	-21.484	-0.349	676.797
			Max. Vy	8	20.518	-673.761	0.980
			Max. Vx	14	20.665	-0.287	-676.193
L12	73.5 - 73.25	Pole	Max. Torque	24			-1.787
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-40.663	-1.631	0.836
			Max. Mx	8	-21.567	-678.891	0.995
			Max. My	2	-21.564	-0.354	681.939
			Max. Vy	8	20.531	-678.891	0.995
			Max. Vx	14	20.697	-0.278	-681.365
L13	73.25 - 68.25	Pole	Max. Torque	24			-1.787
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-42.678	-1.631	0.608
			Max. Mx	8	-23.028	-782.388	1.282
			Max. My	14	-22.998	-0.095	-786.682
			Max. Vy	8	20.874	-782.388	1.282
			Max. Vx	14	21.414	-0.095	-786.682
L14	68.25 - 63.25	Pole	Max. Torque	24			-1.787
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-44.757	-1.631	0.381
			Max. Mx	8	-24.490	-888.409	1.569
			Max. My	14	-24.460	0.087	-895.592
			Max. Vy	8	21.543	-888.409	1.569
			Max. Vx	14	22.136	0.087	-895.592
L15	63.25 - 60	Pole	Max. Torque	24			-1.787
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-46.144	-1.631	0.235
			Max. Mx	8	-25.444	-959.105	1.755
			Max. My	14	-25.414	0.206	-968.317
			Max. Vy	8	21.973	-959.105	1.755
			Max. Vx	14	22.608	0.206	-968.317
L16	60 - 59.75	Pole	Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-46.254	-1.631	0.222
			Max. Mx	8	-25.529	-964.599	1.770
			Max. My	14	-25.499	0.215	-973.976

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L17	59.75 - 54.75	Pole	Max. Vy	8	21.989	-964.599	1.770
			Max. Vx	14	22.644	0.215	-973.976
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-48.449	-1.631	-0.037
			Max. Mx	8	-27.135	-1075.444	2.047
			Max. My	14	-27.098	0.399	-1089.157
			Max. Vy	8	22.356	-1075.444	2.047
L18	54.75 - 49.75	Pole	Max. Vx	14	23.411	0.399	-1089.157
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-50.637	-1.631	-0.293
			Max. Mx	8	-28.750	-1188.066	2.325
			Max. My	14	-28.708	0.582	-1208.091
			Max. Vy	8	22.702	-1188.066	2.325
			Max. Vx	14	24.146	0.582	-1208.091
L19	49.75 - 44.75	Pole	Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-52.873	-1.631	-0.547
			Max. Mx	8	-30.370	-1302.357	2.601
			Max. My	14	-30.325	0.765	-1330.676
			Max. Vy	8	23.025	-1302.357	2.601
			Max. Vx	14	24.873	0.765	-1330.676
			Max. Torque	24			-1.786
L20	44.75 - 40	Pole	Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-55.072	-1.631	-0.785
			Max. Mx	8	-31.910	-1413.202	2.864
			Max. My	14	-31.869	0.940	-1450.491
			Max. Vy	8	23.660	-1413.202	2.864
			Max. Vx	14	25.565	0.940	-1450.491
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
L21	40 - 39.75	Pole	Max. Compression	26	-55.197	-1.631	-0.799
			Max. Mx	8	-32.010	-1419.117	2.877
			Max. My	14	-31.969	0.949	-1456.888
			Max. Vy	8	23.673	-1419.117	2.877
			Max. Vx	14	25.598	0.949	-1456.888
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-57.694	-1.631	-1.080
L22	39.75 - 34.75	Pole	Max. Mx	8	-33.893	-1538.351	3.144
			Max. My	14	-33.851	1.133	-1586.820
			Max. Vy	8	24.029	-1538.351	3.144
			Max. Vx	14	26.354	1.133	-1586.820
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-60.181	-1.631	-1.357
			Max. Mx	8	-35.783	-1659.291	3.411
L23	34.75 - 29.75	Pole	Max. My	14	-35.742	1.316	-1720.417
			Max. Vy	8	24.358	-1659.291	3.411
			Max. Vx	14	27.066	1.316	-1720.417
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-62.789	-1.631	-1.629
			Max. Mx	8	-37.675	-1782.765	3.676
			Max. My	14	-37.640	1.500	-1857.651
L24	29.75 - 24.75	Pole	Max. Vy	8	25.043	-1782.765	3.676
			Max. Vx	14	27.810	1.500	-1857.651
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-65.277	-1.631	-1.882
			Max. Mx	8	-37.675	-1782.765	3.676
			Max. My	14	-37.640	1.500	-1857.651
			Max. Vy	8	25.043	-1782.765	3.676
L25	24.75 - 20	Pole	Max. Vx	14	27.810	1.500	-1857.651
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-65.277	-1.631	-1.882

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Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L26	20 - 19.75	Pole	Max. Mx	8	-39.476	-1903.224	3.928
			Max. My	14	-39.448	1.675	-1991.467
			Max. Vy	8	25.691	-1903.224	3.928
			Max. Vx	14	28.520	1.675	-1991.467
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-65.418	-1.631	-1.897
			Max. Mx	8	-39.591	-1909.647	3.941
			Max. My	14	-39.564	1.684	-1998.604
			Max. Vy	8	25.703	-1909.647	3.941
L27	19.75 - 14.75	Pole	Max. Vx	14	28.553	1.684	-1998.604
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-68.230	-1.631	-2.188
			Max. Mx	8	-41.773	-2039.051	4.197
			Max. My	14	-41.749	1.867	-2143.418
			Max. Vy	8	26.066	-2039.051	4.197
			Max. Vx	14	29.348	1.867	-2143.418
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
L28	14.75 - 9.75	Pole	Max. Compression	26	-71.018	-1.631	-2.468
			Max. Mx	8	-43.961	-2170.228	4.452
			Max. My	14	-43.942	2.050	-2292.164
			Max. Vy	8	26.416	-2170.228	4.452
			Max. Vx	14	30.130	2.050	-2292.164
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-73.769	-1.631	-2.735
			Max. Mx	8	-46.152	-2303.128	4.706
			Max. My	14	-46.142	2.234	-2444.787
L29	9.75 - 4.75	Pole	Max. Vy	6	26.768	-2115.564	1223.975
			Max. Vx	14	30.900	2.234	-2444.787
			Max. Torque	24			-1.786
			Max Tension	1	0.000	0.000	0.000
			Max. Compression	26	-76.317	-1.631	-2.962
			Max. Mx	8	-48.237	-2430.928	4.947
			Max. My	14	-48.236	2.407	-2593.314
			Max. Vy	6	27.393	-2244.163	1298.285
			Max. Vx	14	31.622	2.407	-2593.314
			Max. Torque	24			-1.786
L30	4.75 - 0	Pole	Max. Compression	26	-76.317	-1.631	-2.962
			Max. Mx	8	-48.237	-2430.928	4.947
			Max. My	14	-48.236	2.407	-2593.314
			Max. Vy	6	27.393	-2244.163	1298.285
			Max. Vx	14	31.622	2.407	-2593.314
			Max. Torque	24			-1.786

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	26	76.317	-0.000	0.000
	Max. H _x	23	36.182	27.343	15.774
	Max. H _z	2	48.242	-0.015	27.114
	Max. M _x	2	2437.013	-0.015	27.114
	Max. M _z	8	2430.928	-27.060	0.067
	Max. Torsion	10	1.692	-23.438	-13.544
	Min. Vert	19	36.182	23.391	-13.534
	Min. H _x	6	48.242	-27.384	15.840
	Min. H _z	14	48.242	0.036	-31.611
	Min. M _x	14	-2593.314	0.036	-31.611
	Min. M _z	20	-2425.652	27.014	-0.031

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. Torsion	24	-1.786	13.484	23.480

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturing Moment, M _x kip-ft	Overturing Moment, M _z kip-ft	Torque kip-ft
Dead Only	40.202	0.000	0.000	0.642	-0.419	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	48.242	0.015	-27.114	-2437.013	-1.455	1.064
0.9 Dead+1.0 Wind 0 deg - No Ice	36.182	0.015	-27.114	-2424.317	-1.319	1.059
1.2 Dead+1.0 Wind 30 deg - No Ice	48.242	13.588	-23.453	-2107.554	-1220.678	0.505
0.9 Dead+1.0 Wind 30 deg - No Ice	36.182	13.588	-23.453	-2096.599	-1214.102	0.503
1.2 Dead+1.0 Wind 60 deg - No Ice	48.242	27.384	-15.840	-1298.285	-2244.163	-0.389
0.9 Dead+1.0 Wind 60 deg - No Ice	36.182	27.384	-15.840	-1291.829	-2232.550	-0.387
1.2 Dead+1.0 Wind 90 deg - No Ice	48.242	27.060	-0.067	-4.947	-2430.928	-1.020
0.9 Dead+1.0 Wind 90 deg - No Ice	36.182	27.060	-0.067	-5.116	-2417.953	-1.015
1.2 Dead+1.0 Wind 120 deg - No Ice	48.242	23.438	13.544	1218.860	-2105.780	-1.692
0.9 Dead+1.0 Wind 120 deg - No Ice	36.182	23.438	13.544	1212.223	-2094.521	-1.686
1.2 Dead+1.0 Wind 150 deg - No Ice	48.242	13.547	23.443	2108.652	-1217.663	-1.553
0.9 Dead+1.0 Wind 150 deg - No Ice	36.182	13.547	23.443	2097.311	-1211.096	-1.547
1.2 Dead+1.0 Wind 180 deg - No Ice	48.242	-0.036	31.611	2593.314	2.407	-0.913
0.9 Dead+1.0 Wind 180 deg - No Ice	36.182	-0.036	31.611	2579.844	2.531	-0.908
1.2 Dead+1.0 Wind 210 deg - No Ice	48.242	-13.533	23.431	2107.099	1214.630	-0.130
0.9 Dead+1.0 Wind 210 deg - No Ice	36.182	-13.533	23.431	2095.767	1208.346	-0.128
1.2 Dead+1.0 Wind 240 deg - No Ice	48.242	-23.391	13.534	1217.221	2100.031	0.653
0.9 Dead+1.0 Wind 240 deg - No Ice	36.182	-23.391	13.534	1210.596	2089.067	0.652
1.2 Dead+1.0 Wind 270 deg - No Ice	48.242	-27.014	0.031	3.161	2425.652	1.270
0.9 Dead+1.0 Wind 270 deg - No Ice	36.182	-27.014	0.031	2.959	2412.965	1.266
1.2 Dead+1.0 Wind 300 deg - No Ice	48.242	-27.343	-15.774	-1292.963	2239.760	1.624
0.9 Dead+1.0 Wind 300 deg - No Ice	36.182	-27.343	-15.774	-1286.528	2228.430	1.617
1.2 Dead+1.0 Wind 330 deg - No Ice	48.242	-13.484	-23.480	-2110.440	1210.848	1.786
0.9 Dead+1.0 Wind 330 deg - No Ice	36.182	-13.484	-23.480	-2099.470	1204.578	1.779
1.2 Dead+1.0 Ice+1.0 Temp	76.317	0.000	-0.000	2.962	-1.631	-0.000

<p style="text-align: center;">tnxTower</p> <p style="text-align: center;">B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)	Page 26 of 34
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Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	76.317	0.001	-6.227	-556.673	-1.781	0.205
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	76.317	3.127	-5.387	-481.101	-282.653	0.055
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	76.317	5.478	-3.158	-278.596	-490.092	-0.147
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	76.317	6.236	-0.011	2.074	-561.977	-0.280
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	76.317	5.401	3.112	282.788	-487.072	-0.396
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	76.317	3.122	5.387	487.132	-282.331	-0.340
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	76.317	-0.005	6.306	565.370	-1.339	-0.178
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	76.317	-3.117	5.383	486.714	278.249	0.013
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	76.317	-5.392	3.108	282.257	482.590	0.194
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	76.317	-6.227	0.004	3.294	557.715	0.325
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	76.317	-5.472	-3.148	-277.851	486.123	0.383
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	76.317	-3.111	-5.393	-481.764	277.787	0.382
Dead+Wind 0 deg - Service	40.202	0.003	-5.440	-486.828	-0.620	0.213
Dead+Wind 30 deg - Service	40.202	2.726	-4.705	-420.948	-244.422	0.101
Dead+Wind 60 deg - Service	40.202	5.494	-3.178	-259.152	-449.133	-0.078
Dead+Wind 90 deg - Service	40.202	5.429	-0.013	-0.502	-486.429	-0.205
Dead+Wind 120 deg - Service	40.202	4.702	2.717	244.216	-421.411	-0.339
Dead+Wind 150 deg - Service	40.202	2.718	4.703	422.144	-243.819	-0.311
Dead+Wind 180 deg - Service	40.202	-0.007	6.342	519.117	0.153	-0.183
Dead+Wind 210 deg - Service	40.202	-2.715	4.701	421.833	242.555	-0.026
Dead+Wind 240 deg - Service	40.202	-4.693	2.715	243.889	419.603	0.131
Dead+Wind 270 deg - Service	40.202	-5.420	0.006	1.120	484.716	0.255
Dead+Wind 300 deg - Service	40.202	-5.486	-3.165	-258.087	447.594	0.325
Dead+Wind 330 deg - Service	40.202	-2.705	-4.711	-421.525	241.798	0.358

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-40.202	0.000	0.000	40.202	0.000	0.000%
2	0.015	-48.242	-27.114	-0.015	48.242	27.114	0.000%
3	0.015	-36.182	-27.114	-0.015	36.182	27.114	0.000%
4	13.588	-48.242	-23.453	-13.588	48.242	23.453	0.000%
5	13.588	-36.182	-23.453	-13.588	36.182	23.453	0.000%
6	27.384	-48.242	-15.840	-27.384	48.242	15.840	0.000%
7	27.384	-36.182	-15.840	-27.384	36.182	15.840	0.000%
8	27.060	-48.242	-0.067	-27.060	48.242	0.067	0.000%
9	27.060	-36.182	-0.067	-27.060	36.182	0.067	0.000%
10	23.438	-48.242	13.544	-23.438	48.242	-13.544	0.000%
11	23.438	-36.182	13.544	-23.438	36.182	-13.544	0.000%
12	13.547	-48.242	23.443	-13.547	48.242	-23.443	0.000%
13	13.547	-36.182	23.443	-13.547	36.182	-23.443	0.000%
14	-0.036	-48.242	31.611	0.036	48.242	-31.611	0.000%
15	-0.036	-36.182	31.611	0.036	36.182	-31.611	0.000%
16	-13.533	-48.242	23.431	13.533	48.242	-23.431	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
17	-13.533	-36.182	23.431	13.533	36.182	-23.431	0.000%
18	-23.391	-48.242	13.534	23.391	48.242	-13.534	0.000%
19	-23.391	-36.182	13.534	23.391	36.182	-13.534	0.000%
20	-27.014	-48.242	0.031	27.014	48.242	-0.031	0.000%
21	-27.014	-36.182	0.031	27.014	36.182	-0.031	0.000%
22	-27.343	-48.242	-15.774	27.343	48.242	15.774	0.000%
23	-27.343	-36.182	-15.774	27.343	36.182	15.774	0.000%
24	-13.484	-48.242	-23.480	13.484	48.242	23.480	0.000%
25	-13.484	-36.182	-23.480	13.484	36.182	23.480	0.000%
26	0.000	-76.317	0.000	-0.000	76.317	0.000	0.000%
27	0.001	-76.317	-6.227	-0.001	76.317	6.227	0.000%
28	3.127	-76.317	-5.387	-3.127	76.317	5.387	0.000%
29	5.478	-76.317	-3.158	-5.478	76.317	3.158	0.000%
30	6.236	-76.317	-0.011	-6.236	76.317	0.011	0.000%
31	5.401	-76.317	3.112	-5.401	76.317	-3.112	0.000%
32	3.122	-76.317	5.387	-3.122	76.317	-5.387	0.000%
33	-0.005	-76.317	6.306	0.005	76.317	-6.306	0.000%
34	-3.117	-76.317	5.383	3.117	76.317	-5.383	0.000%
35	-5.392	-76.317	3.108	5.392	76.317	-3.108	0.000%
36	-6.227	-76.317	0.004	6.227	76.317	-0.004	0.000%
37	-5.472	-76.317	-3.148	5.472	76.317	3.148	0.000%
38	-3.111	-76.317	-5.393	3.111	76.317	5.393	0.000%
39	0.003	-40.202	-5.440	-0.003	40.202	5.440	0.000%
40	2.726	-40.202	-4.705	-2.726	40.202	4.705	0.000%
41	5.494	-40.202	-3.178	-5.494	40.202	3.178	0.000%
42	5.429	-40.202	-0.013	-5.429	40.202	0.013	0.000%
43	4.702	-40.202	2.717	-4.702	40.202	-2.717	0.000%
44	2.718	-40.202	4.703	-2.718	40.202	-4.703	0.000%
45	-0.007	-40.202	6.342	0.007	40.202	-6.342	0.000%
46	-2.715	-40.202	4.701	2.715	40.202	-4.701	0.000%
47	-4.693	-40.202	2.715	4.693	40.202	-2.715	0.000%
48	-5.420	-40.202	0.006	5.420	40.202	-0.006	0.000%
49	-5.486	-40.202	-3.165	5.486	40.202	3.165	0.000%
50	-2.705	-40.202	-4.711	2.705	40.202	4.711	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00091460
3	Yes	4	0.00000001	0.00056447
4	Yes	5	0.00000001	0.00023313
5	Yes	5	0.00000001	0.00011128
6	Yes	5	0.00000001	0.00024367
7	Yes	5	0.00000001	0.00011507
8	Yes	4	0.00000001	0.00092801
9	Yes	4	0.00000001	0.00057556
10	Yes	5	0.00000001	0.00021312
11	Yes	5	0.00000001	0.00010133
12	Yes	5	0.00000001	0.00024479
13	Yes	5	0.00000001	0.00011716
14	Yes	4	0.00000001	0.00080380
15	Yes	4	0.00000001	0.00048043
16	Yes	5	0.00000001	0.00022544
17	Yes	5	0.00000001	0.00010755

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18	Yes	5	0.0000001	0.00021978
19	Yes	5	0.0000001	0.00010475
20	Yes	5	0.0000001	0.00002888
21	Yes	4	0.0000001	0.00064875
22	Yes	5	0.0000001	0.00025574
23	Yes	5	0.0000001	0.00012121
24	Yes	5	0.0000001	0.00021152
25	Yes	5	0.0000001	0.00010059
26	Yes	4	0.0000001	0.00001851
27	Yes	5	0.0000001	0.00038706
28	Yes	5	0.0000001	0.00039747
29	Yes	5	0.0000001	0.00039949
30	Yes	5	0.0000001	0.00038964
31	Yes	5	0.0000001	0.00039824
32	Yes	5	0.0000001	0.00039718
33	Yes	5	0.0000001	0.00038691
34	Yes	5	0.0000001	0.00039324
35	Yes	5	0.0000001	0.00039238
36	Yes	5	0.0000001	0.00038361
37	Yes	5	0.0000001	0.00039431
38	Yes	5	0.0000001	0.00039428
39	Yes	4	0.0000001	0.00009304
40	Yes	4	0.0000001	0.00014719
41	Yes	4	0.0000001	0.00015350
42	Yes	4	0.0000001	0.00009274
43	Yes	4	0.0000001	0.00013996
44	Yes	4	0.0000001	0.00016056
45	Yes	4	0.0000001	0.00009348
46	Yes	4	0.0000001	0.00014093
47	Yes	4	0.0000001	0.00013823
48	Yes	4	0.0000001	0.00009484
49	Yes	4	0.0000001	0.00016806
50	Yes	4	0.0000001	0.00013934

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	125 - 120	7.076	41	0.506	0.001
L2	120 - 115	6.547	41	0.505	0.001
L3	115 - 110	6.019	41	0.502	0.001
L4	110 - 105	5.497	41	0.494	0.001
L5	105 - 100	4.988	41	0.478	0.001
L6	100 - 95	4.499	41	0.454	0.001
L7	95 - 90	4.032	41	0.437	0.001
L8	90 - 85	3.586	45	0.415	0.001
L9	85 - 80	3.167	45	0.385	0.001
L10	80 - 75	2.782	45	0.349	0.001
L11	75 - 73.5	2.430	45	0.323	0.001
L12	73.5 - 73.25	2.330	45	0.315	0.001
L13	73.25 - 68.25	2.313	45	0.314	0.001
L14	68.25 - 63.25	1.996	45	0.293	0.000
L15	63.25 - 60	1.702	45	0.268	0.000
L16	60 - 59.75	1.525	45	0.251	0.000
L17	59.75 - 54.75	1.512	45	0.250	0.000
L18	54.75 - 49.75	1.261	45	0.230	0.000
L19	49.75 - 44.75	1.031	45	0.208	0.000
L20	44.75 - 40	0.826	45	0.183	0.000
L21	40 - 39.75	0.657	45	0.158	0.000

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L22	39.75 - 34.75	0.648	45	0.157	0.000
L23	34.75 - 29.75	0.494	45	0.138	0.000
L24	29.75 - 24.75	0.359	45	0.118	0.000
L25	24.75 - 20	0.247	45	0.096	0.000
L26	20 - 19.75	0.162	45	0.074	0.000
L27	19.75 - 14.75	0.158	45	0.073	0.000
L28	14.75 - 9.75	0.090	45	0.056	0.000
L29	9.75 - 4.75	0.040	45	0.039	0.000
L30	4.75 - 0	0.010	45	0.019	0.000

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
122.000	ERICSSON AIR 21 B2A B4P w/ Mount Pipe	41	6.758	0.506	0.002	166399
112.000	(2) HBXX-6517DS-A2M w/ Mount Pipe	41	5.705	0.498	0.002	32015
100.000	7770.00 w/ Mount Pipe	41	4.499	0.454	0.001	13959
90.000	Dragonwave A-ANT-18G-2-C	45	3.586	0.415	0.001	11007

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	125 - 120	35.392	14	2.528	0.007
L2	120 - 115	32.745	14	2.527	0.007
L3	115 - 110	30.108	14	2.509	0.007
L4	110 - 105	27.500	14	2.468	0.007
L5	105 - 100	24.954	14	2.390	0.007
L6	100 - 95	22.510	14	2.272	0.007
L7	95 - 90	20.173	14	2.188	0.006
L8	90 - 85	17.940	14	2.074	0.005
L9	85 - 80	15.841	14	1.928	0.004
L10	80 - 75	13.916	14	1.744	0.003
L11	75 - 73.5	12.154	14	1.617	0.003
L12	73.5 - 73.25	11.653	14	1.575	0.003
L13	73.25 - 68.25	11.571	14	1.570	0.003
L14	68.25 - 63.25	9.981	14	1.464	0.002
L15	63.25 - 60	8.510	14	1.342	0.002
L16	60 - 59.75	7.626	14	1.255	0.002
L17	59.75 - 54.75	7.561	14	1.250	0.002
L18	54.75 - 49.75	6.303	14	1.150	0.002
L19	49.75 - 44.75	5.156	14	1.039	0.001
L20	44.75 - 40	4.131	14	0.916	0.001
L21	40 - 39.75	3.282	14	0.789	0.001
L22	39.75 - 34.75	3.241	14	0.784	0.001
L23	34.75 - 29.75	2.468	14	0.691	0.001
L24	29.75 - 24.75	1.796	14	0.591	0.001
L25	24.75 - 20	1.234	14	0.482	0.001

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Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L26	20 - 19.75	0.809	14	0.370	0.000
L27	19.75 - 14.75	0.790	14	0.366	0.000
L28	14.75 - 9.75	0.450	14	0.282	0.000
L29	9.75 - 4.75	0.201	14	0.193	0.000
L30	4.75 - 0	0.049	14	0.097	0.000

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
122.000	ERICSSON AIR 21 B2A B4P w/ Mount Pipe	14	33.804	2.528	0.008	33332
112.000	(2) HBXX-6517DS-A2M w/ Mount Pipe	14	28.538	2.488	0.008	6442
100.000	7770.00 w/ Mount Pipe	14	22.510	2.272	0.007	2808
90.000	Dragonwave A-ANT-18G-2-C	14	17.940	2.074	0.005	2213

Compression Checks

Pole Design Data

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u K	φP _n K	Ratio P _u / φP _n
L1	125 - 120 (1)	P24x0.375	5.000	0.000	0.0	27.833	-4.738	1052.070	0.005
L2	120 - 115 (2)	P24x0.375	5.000	0.000	0.0	27.833	-5.378	1052.070	0.005
L3	115 - 110 (3)	P24x0.375	5.000	0.000	0.0	27.833	-8.760	1052.070	0.008
L4	110 - 105 (4)	P24x0.375	5.000	0.000	0.0	27.833	-9.427	1052.070	0.009
L5	105 - 100 (5)	P24x0.375	5.000	0.000	0.0	27.833	-10.104	1052.070	0.010
L6	100 - 95 (6)	P30x0.375	5.000	0.000	0.0	34.901	-13.929	1311.060	0.011
L7	95 - 90 (7)	P30x0.375	5.000	0.000	0.0	34.901	-14.839	1311.060	0.011
L8	90 - 85 (8)	P30x0.375	5.000	0.000	0.0	34.901	-19.075	1311.060	0.015
L9	85 - 80 (9)	P30x0.375	5.000	0.000	0.0	34.901	-20.051	1311.060	0.015
L10	80 - 75 (10)	P36x0.375	5.000	0.000	0.0	41.970	-21.155	1490.100	0.014
L11	75 - 73.5 (11)	P36x0.375	1.500	0.000	0.0	41.970	-21.484	1490.100	0.014
L12	73.5 - 73.25 (12)	P36x0.5625	0.250	0.000	0.0	62.623	-21.564	2367.160	0.009
L13	73.25 - 68.25 (13)	P36x0.5625	5.000	0.000	0.0	62.623	-22.998	2367.160	0.010
L14	68.25 - 63.25 (14)	P36x0.5625	5.000	0.000	0.0	62.623	-24.460	2367.160	0.010
L15	63.25 - 60 (15)	P36x0.5625	3.250	0.000	0.0	62.623	-25.414	2367.160	0.011
L16	60 - 59.75 (16)	P42x0.525	0.250	0.000	0.0	68.406	-25.499	2569.670	0.010
L17	59.75 - 54.75 (17)	P42x0.525	5.000	0.000	0.0	68.406	-27.098	2569.670	0.011
L18	54.75 - 49.75 (18)	P42x0.525	5.000	0.000	0.0	68.406	-28.708	2569.670	0.011
L19	49.75 - 44.75 (19)	P42x0.525	5.000	0.000	0.0	68.406	-30.325	2569.670	0.012

tnxTower

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Project

Date
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Client

Crown Castle

Designed by
 Shashank.S.Rao

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	A in^2	P_u K	ϕP_n K	Ratio $\frac{P_u}{\phi P_n}$
L20	44.75 - 40 (20)	P42x0.525	4.750	0.000	0.0	68.406	-31.869	2569.670	0.012
L21	40 - 39.75 (21)	P48x0.55625	0.250	0.000	0.0	82.909	-31.969	3039.700	0.011
L22	39.75 - 34.75 (22)	P48x0.55625	5.000	0.000	0.0	82.909	-33.851	3039.700	0.011
L23	34.75 - 29.75 (23)	P48x0.55625	5.000	0.000	0.0	82.909	-35.742	3039.700	0.012
L24	29.75 - 24.75 (24)	P48x0.55625	5.000	0.000	0.0	82.909	-37.640	3039.700	0.012
L25	24.75 - 20 (25)	P48x0.55625	4.750	0.000	0.0	82.909	-39.448	3039.700	0.013
L26	20 - 19.75 (26)	P54x0.5875	0.250	0.000	0.0	98.583	-39.564	3545.230	0.011
L27	19.75 - 14.75 (27)	P54x0.5875	5.000	0.000	0.0	98.583	-41.749	3545.230	0.012
L28	14.75 - 9.75 (28)	P54x0.5875	5.000	0.000	0.0	98.583	-43.942	3545.230	0.012
L29	9.75 - 4.75 (29)	P54x0.5875	5.000	0.000	0.0	98.583	-46.142	3545.230	0.013
L30	4.75 - 0 (30)	P54x0.5875	4.750	0.000	0.0	98.583	-48.236	3545.230	0.014

Pole Bending Design Data

Section No.	Elevation ft	Size	M_{ux} kip-ft	ϕM_{ux} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M_{uy} kip-ft	ϕM_{uy} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
L1	125 - 120 (1)	P24x0.375	10.744	623.717	0.017	0.000	623.717	0.000
L2	120 - 115 (2)	P24x0.375	37.961	623.717	0.061	0.000	623.717	0.000
L3	115 - 110 (3)	P24x0.375	80.092	623.717	0.128	0.000	623.717	0.000
L4	110 - 105 (4)	P24x0.375	133.802	623.717	0.215	0.000	623.717	0.000
L5	105 - 100 (5)	P24x0.375	188.902	623.717	0.303	0.000	623.717	0.000
L6	100 - 95 (6)	P30x0.375	267.145	947.858	0.282	0.000	947.858	0.000
L7	95 - 90 (7)	P30x0.375	346.430	947.858	0.365	0.000	947.858	0.000
L8	90 - 85 (8)	P30x0.375	444.650	947.858	0.469	0.000	947.858	0.000
L9	85 - 80 (9)	P30x0.375	544.592	947.858	0.575	0.000	947.858	0.000
L10	80 - 75 (10)	P36x0.375	646.038	1338.808	0.483	0.000	1338.808	0.000
L11	75 - 73.5 (11)	P36x0.375	676.798	1338.808	0.506	0.000	1338.808	0.000
L12	73.5 - 73.25 (12)	P36x0.5625	681.939	2105.042	0.324	0.000	2105.042	0.000
L13	73.25 - 68.25 (13)	P36x0.5625	786.683	2105.042	0.374	0.000	2105.042	0.000
L14	68.25 - 63.25 (14)	P36x0.5625	895.592	2105.042	0.425	0.000	2105.042	0.000
L15	63.25 - 60 (15)	P36x0.5625	968.317	2105.042	0.460	0.000	2105.042	0.000
L16	60 - 59.75 (16)	P42x0.525	973.975	2600.925	0.374	0.000	2600.925	0.000
L17	59.75 - 54.75 (17)	P42x0.525	1089.158	2600.925	0.419	0.000	2600.925	0.000
L18	54.75 - 49.75 (18)	P42x0.525	1208.092	2600.925	0.464	0.000	2600.925	0.000
L19	49.75 - 44.75 (19)	P42x0.525	1330.675	2600.925	0.512	0.000	2600.925	0.000
L20	44.75 - 40 (20)	P42x0.525	1450.492	2600.925	0.558	0.000	2600.925	0.000
L21	40 - 39.75 (21)	P48x0.55625	1456.892	3569.342	0.408	0.000	3569.342	0.000
L22	39.75 - 34.75 (22)	P48x0.55625	1586.817	3569.342	0.445	0.000	3569.342	0.000
L23	34.75 - 29.75 (23)	P48x0.55625	1720.417	3569.342	0.482	0.000	3569.342	0.000
L24	29.75 - 24.75 (24)	P48x0.55625	1857.650	3569.342	0.520	0.000	3569.342	0.000
L25	24.75 - 20 (25)	P48x0.55625	1991.467	3569.342	0.558	0.000	3569.342	0.000

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	<p>Project</p>	<p>Date 19:32:46 05/16/19</p>
	<p>Client Crown Castle</p>	<p>Designed by Shashank.S.Rao</p>

Section No.	Elevation ft	Size	M_{ux} kip-ft	ϕM_{ux} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{ux}}$	M_{uy} kip-ft	ϕM_{uy} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{uy}}$
L26	20 - 19.75 (26)	P54x0.5875	1998.608	4739.867	0.422	0.000	4739.867	0.000
L27	19.75 - 14.75 (27)	P54x0.5875	2143.417	4739.867	0.452	0.000	4739.867	0.000
L28	14.75 - 9.75 (28)	P54x0.5875	2292.167	4739.867	0.484	0.000	4739.867	0.000
L29	9.75 - 4.75 (29)	P54x0.5875	2444.792	4739.867	0.516	0.000	4739.867	0.000
L30	4.75 - 0 (30)	P54x0.5875	2593.317	4739.867	0.547	0.000	4739.867	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V_u K	ϕV_n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T_u kip-ft	ϕT_n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	125 - 120 (1)	P24x0.375	5.296	315.621	0.017	0.000	655.568	0.000
L2	120 - 115 (2)	P24x0.375	5.589	315.621	0.018	0.000	655.568	0.000
L3	115 - 110 (3)	P24x0.375	10.600	315.621	0.034	0.339	655.568	0.001
L4	110 - 105 (4)	P24x0.375	10.889	315.621	0.035	0.392	655.568	0.001
L5	105 - 100 (5)	P24x0.375	11.152	315.621	0.035	0.392	655.568	0.001
L6	100 - 95 (6)	P30x0.375	15.703	395.779	0.040	0.258	994.725	0.000
L7	95 - 90 (7)	P30x0.375	16.018	395.779	0.040	0.258	994.725	0.000
L8	90 - 85 (8)	P30x0.375	19.868	395.779	0.050	1.065	994.725	0.001
L9	85 - 80 (9)	P30x0.375	20.137	395.779	0.051	1.065	994.725	0.001
L10	80 - 75 (10)	P36x0.375	20.474	454.187	0.045	1.065	1094.275	0.001
L11	75 - 73.5 (11)	P36x0.375	20.575	454.187	0.045	1.065	1094.275	0.001
L12	73.5 - 73.25 (12)	P36x0.5625	20.587	710.147	0.029	1.065	2212.542	0.000
L13	73.25 - 68.25 (13)	P36x0.5625	21.414	710.147	0.030	0.913	2212.542	0.000
L14	68.25 - 63.25 (14)	P36x0.5625	22.136	710.147	0.031	0.913	2212.542	0.000
L15	63.25 - 60 (15)	P36x0.5625	22.608	710.147	0.032	0.913	2212.542	0.000
L16	60 - 59.75 (16)	P42x0.525	22.644	775.727	0.029	0.913	2800.233	0.000
L17	59.75 - 54.75 (17)	P42x0.525	23.411	775.727	0.030	0.913	2800.233	0.000
L18	54.75 - 49.75 (18)	P42x0.525	24.147	775.727	0.031	0.913	2800.233	0.000
L19	49.75 - 44.75 (19)	P42x0.525	24.873	775.727	0.032	0.913	2800.233	0.000
L20	44.75 - 40 (20)	P42x0.525	25.565	775.727	0.033	0.913	2800.233	0.000
L21	40 - 39.75 (21)	P48x0.55625	25.598	940.182	0.027	0.913	3775.583	0.000
L22	39.75 - 34.75 (22)	P48x0.55625	26.354	940.182	0.028	0.913	3775.583	0.000
L23	34.75 - 29.75 (23)	P48x0.55625	27.066	940.182	0.029	0.913	3775.583	0.000
L24	29.75 - 24.75 (24)	P48x0.55625	27.811	940.182	0.030	0.913	3775.583	0.000
L25	24.75 - 20 (25)	P48x0.55625	28.520	940.182	0.030	0.913	3775.583	0.000
L26	20 - 19.75 (26)	P54x0.5875	28.553	1117.930	0.026	0.913	4954.017	0.000
L27	19.75 - 14.75 (27)	P54x0.5875	29.348	1117.930	0.026	0.913	4954.017	0.000
L28	14.75 - 9.75 (28)	P54x0.5875	30.130	1117.930	0.027	0.913	4954.017	0.000
L29	9.75 - 4.75 (29)	P54x0.5875	30.900	1117.930	0.028	0.913	4954.017	0.000
L30	4.75 - 0 (30)	P54x0.5875	31.622	1117.930	0.028	0.913	4954.017	0.000

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Pole Interaction Design Data

Section No.	Elevation ft	Ratio P_u ϕP_n	Ratio M_{ux} ϕM_{nx}	Ratio M_{uy} ϕM_{ny}	Ratio V_u ϕV_n	Ratio T_u ϕT_n	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	125 - 120 (1)	0.005	0.017	0.000	0.017	0.000	0.022	1.050	4.8.2 ✓
L2	120 - 115 (2)	0.005	0.061	0.000	0.018	0.000	0.066	1.050	4.8.2 ✓
L3	115 - 110 (3)	0.008	0.128	0.000	0.034	0.001	0.138	1.050	4.8.2 ✓
L4	110 - 105 (4)	0.009	0.215	0.000	0.035	0.001	0.225	1.050	4.8.2 ✓
L5	105 - 100 (5)	0.010	0.303	0.000	0.035	0.001	0.314	1.050	4.8.2 ✓
L6	100 - 95 (6)	0.011	0.282	0.000	0.040	0.000	0.294	1.050	4.8.2 ✓
L7	95 - 90 (7)	0.011	0.365	0.000	0.040	0.000	0.378	1.050	4.8.2 ✓
L8	90 - 85 (8)	0.015	0.469	0.000	0.050	0.001	0.486	1.050	4.8.2 ✓
L9	85 - 80 (9)	0.015	0.575	0.000	0.051	0.001	0.593	1.050	4.8.2 ✓
L10	80 - 75 (10)	0.014	0.483	0.000	0.045	0.001	0.499	1.050	4.8.2 ✓
L11	75 - 73.5 (11)	0.014	0.506	0.000	0.045	0.001	0.522	1.050	4.8.2 ✓
L12	73.5 - 73.25 (12)	0.009	0.324	0.000	0.029	0.000	0.334	1.050	4.8.2 ✓
L13	73.25 - 68.25 (13)	0.010	0.374	0.000	0.030	0.000	0.384	1.050	4.8.2 ✓
L14	68.25 - 63.25 (14)	0.010	0.425	0.000	0.031	0.000	0.437	1.050	4.8.2 ✓
L15	63.25 - 60 (15)	0.011	0.460	0.000	0.032	0.000	0.472	1.050	4.8.2 ✓
L16	60 - 59.75 (16)	0.010	0.374	0.000	0.029	0.000	0.385	1.050	4.8.2 ✓
L17	59.75 - 54.75 (17)	0.011	0.419	0.000	0.030	0.000	0.430	1.050	4.8.2 ✓
L18	54.75 - 49.75 (18)	0.011	0.464	0.000	0.031	0.000	0.477	1.050	4.8.2 ✓
L19	49.75 - 44.75 (19)	0.012	0.512	0.000	0.032	0.000	0.524	1.050	4.8.2 ✓
L20	44.75 - 40 (20)	0.012	0.558	0.000	0.033	0.000	0.571	1.050	4.8.2 ✓
L21	40 - 39.75 (21)	0.011	0.408	0.000	0.027	0.000	0.419	1.050	4.8.2 ✓
L22	39.75 - 34.75 (22)	0.011	0.445	0.000	0.028	0.000	0.457	1.050	4.8.2 ✓
L23	34.75 - 29.75 (23)	0.012	0.482	0.000	0.029	0.000	0.495	1.050	4.8.2 ✓
L24	29.75 - 24.75 (24)	0.012	0.520	0.000	0.030	0.000	0.534	1.050	4.8.2 ✓
L25	24.75 - 20 (25)	0.013	0.558	0.000	0.030	0.000	0.572	1.050	4.8.2 ✓

<p style="text-align: center;">tnxTower</p> <p style="text-align: center;">B+T Group 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 295-0265</p>	Job 101126.004.01 - Branford I-95 X55 Dtn1, CT (BU# 822765)	Page 34 of 34
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Section No.	Elevation ft	Ratio	Ratio	Ratio	Ratio	Ratio	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
		P_u	M_{ux}	M_{uy}	V_u	T_u			
L26	20 - 19.75 (26)	0.011	0.422	0.000	0.026	0.000	0.433	1.050	4.8.2 ✓
L27	19.75 - 14.75 (27)	0.012	0.452	0.000	0.026	0.000	0.465	1.050	4.8.2 ✓
L28	14.75 - 9.75 (28)	0.012	0.484	0.000	0.027	0.000	0.497	1.050	4.8.2 ✓
L29	9.75 - 4.75 (29)	0.013	0.516	0.000	0.028	0.000	0.530	1.050	4.8.2 ✓
L30	4.75 - 0 (30)	0.014	0.547	0.000	0.028	0.000	0.562	1.050	4.8.2 ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail	
L1	125 - 120	Pole	P24x0.375	1	-4.738	1104.673	**	**	
L2	120 - 115	Pole	P24x0.375	2	-5.378	1104.673	**	**	
L3	115 - 110	Pole	P24x0.375	3	-8.760	1104.673	**	**	
L4	110 - 105	Pole	P24x0.375	4	-9.427	1104.673	**	**	
L5	105 - 100	Pole	P24x0.375	5	-10.104	1104.673	**	**	
L6	100 - 95	Pole	P30x0.375	6	-13.929	1376.613	**	**	
L7	95 - 90	Pole	P30x0.375	7	-14.839	1376.613	**	**	
L8	90 - 85	Pole	P30x0.375	8	-19.075	1376.613	**	**	
L9	85 - 80	Pole	P30x0.375	9	-20.051	1376.613	**	**	
L10	80 - 75	Pole	P36x0.375	10	-21.155	1564.605	**	**	
L11	75 - 73.5	Pole	P36x0.375	11	-21.484	1564.605	**	**	
L12	73.5 - 73.25	Pole	P36x0.5625	12	-21.564	2485.518	**	**	
L13	73.25 - 68.25	Pole	P36x0.5625	13	-22.998	2485.518	**	**	
L14	68.25 - 63.25	Pole	P36x0.5625	14	-24.460	2485.518	**	**	
L15	63.25 - 60	Pole	P36x0.5625	15	-25.414	2485.518	**	**	
L16	60 - 59.75	Pole	P42x0.525	16	-25.499	2698.153	**	**	
L17	59.75 - 54.75	Pole	P42x0.525	17	-27.098	2698.153	**	**	
L18	54.75 - 49.75	Pole	P42x0.525	18	-28.708	2698.153	**	**	
L19	49.75 - 44.75	Pole	P42x0.525	19	-30.325	2698.153	**	**	
L20	44.75 - 40	Pole	P42x0.525	20	-31.869	2698.153	**	**	
L21	40 - 39.75	Pole	P48x0.55625	21	-31.969	3191.685	**	**	
L22	39.75 - 34.75	Pole	P48x0.55625	22	-33.851	3191.685	**	**	
L23	34.75 - 29.75	Pole	P48x0.55625	23	-35.742	3191.685	**	**	
L24	29.75 - 24.75	Pole	P48x0.55625	24	-37.640	3191.685	**	**	
L25	24.75 - 20	Pole	P48x0.55625	25	-39.448	3191.685	**	**	
L26	20 - 19.75	Pole	P54x0.5875	26	-39.564	3722.491	**	**	
L27	19.75 - 14.75	Pole	P54x0.5875	27	-41.749	3722.491	**	**	
L28	14.75 - 9.75	Pole	P54x0.5875	28	-43.942	3722.491	**	**	
L29	9.75 - 4.75	Pole	P54x0.5875	29	-46.142	3722.491	**	**	
L30	4.75 - 0	Pole	P54x0.5875	30	-48.236	3722.491	**	**	
							Summary		
							Pole (L9)	**	**
							RATING =	**	**

** Check Additional Calculations

APPENDIX B
BASE LEVEL DRAWING

((OTHER CONSIDERED EQUIPMENT)—IN CONDUIT—474807)
(1) 3/8" TO 100 FT LEVEL
(OTHER CONSIDERED EQUIPMENT)
(1) 3/8" TO 100 FT LEVEL
(6) 3/4" TO 100 FT LEVEL
(12) 1-1/4" TO 100 FT LEVEL

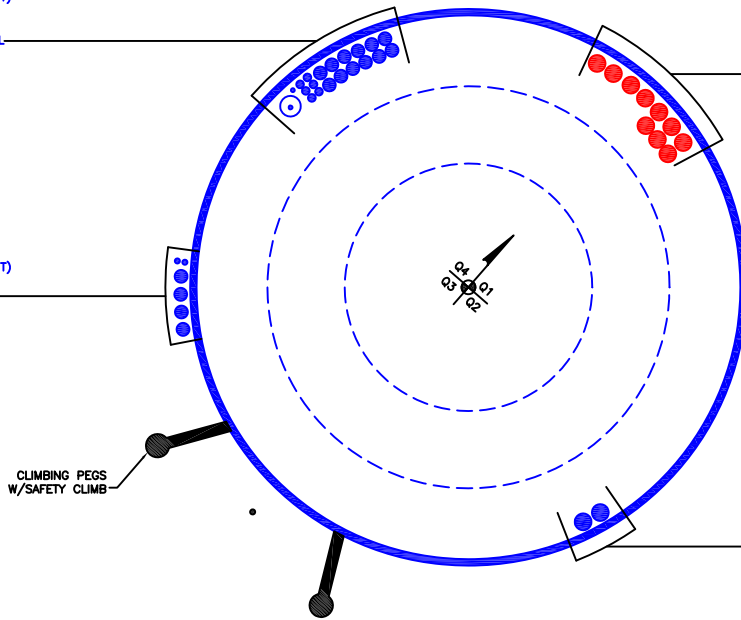
(PROPOSED EQUIPMENT CONFIGURATION)
(10) 1-5/8" TO 122 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)
(2) 1/2" TO 90 FT LEVEL
(4) 1-1/4" TO 90 FT LEVEL

CLIMBING PEGS
W/SAFETY CLIMB

(OTHER CONSIDERED EQUIPMENT)
(2) 1-5/8" TO 112 FT LEVEL

BUSINESS UNIT: 822765



APPENDIX C
ADDITIONAL CALCULATIONS

Site BU: 822765

Work Order: 1734970



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Pole Geometry

	Pole Height Above Base (ft)	Section Length (ft)	Lap Splice Length (ft)	Number of Sides	Top Diameter (in)	Bottom Diameter (in)	Wall Thickness (in)	Bend Radius (in)	Pole Material
1	125	25		0	24	24	0.375		A53-B-42
2	100	20		0	30.00	30	0.375		A53-B-42
3	80	20		0	36.00	36	0.375		A53-B-42
4	60	20		0	42.00	42	0.375		A53-B-42
5	40	20		0	48.00	48	0.375		A53-B-42
6	20	20		0	54.00	54	0.375		A53-B-42

Reinforcement Configuration

	Bottom Effective Elevation (ft)	Top Effective Elevation (ft)	Type	Model	Number																					
						1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18			
1	0	20	plate	CCI-CFP-085125	3	0					120							240								
2	20	40	plate	CCI-CFP-065125	3	0					120							240								
3	40	60	plate	CCI-CFP-060100	3	0					120							240								
4	60	73.5	plate	CCI-CFP-060100-15	3	0					120							240								
5																										
6																										
7																										
8																										
9																										
10																										

Reinforcement Details

	B (in)	H (in)	Gross Area (in ²)	Pole Face to Centroid (in)	Bottom Termination Length (in)	Top Termination Length (in)	L _u (in)	Net Area (in ²)	Bolt Hole Size (in)	Reinforcement Material
1	8.5	1.25	10.625	0.625	45.000	90.000	24.000	9.063	1.1875	A572-65
2	6.5	1.25	8.125	0.625	90.000	66.000	16.000	6.563	1.1875	A572-65
3	6	1	6	0.5	66.000	48.000	16.000	4.750	1.1875	A572-65
4	6	1	6	0.5	48.000	24.000	24.000	4.750	1.1875	A572-65

TNX Geometry Input

Increment (ft): 5

	Section Height (ft)	Section Length (ft)	Lap Splice Length (ft)	Number of Sides	Top Diameter (in)	Bottom Diameter (in)	Wall Thickness (in)	Tapered Pole Grade	Weight Multiplier
1	125 - 120	5		0	24.000	24.000	0.375	A53-B-42	1.000
2	120 - 115	5		0	24.000	24.000	0.375	A53-B-42	1.000
3	115 - 110	5		0	24.000	24.000	0.375	A53-B-42	1.000
4	110 - 105	5		0	24.000	24.000	0.375	A53-B-42	1.000
5	105 - 100	5	0	0	24.000	24.000	0.375	A53-B-42	1.000
6	100 - 95	5		0	30.000	30.000	0.375	A53-B-42	1.000
7	95 - 90	5		0	30.000	30.000	0.375	A53-B-42	1.000
8	90 - 85	5		0	30.000	30.000	0.375	A53-B-42	1.000
9	85 - 80	5	0	0	30.000	30.000	0.375	A53-B-42	1.000
10	80 - 75	5		0	36.000	36.000	0.375	A53-B-42	1.000
11	75 - 73.5	1.5		0	36.000	36.000	0.375	A53-B-42	1.000
12	73.5 - 73.25	0.25		0	36.000	36.000	0.5625	A53-B-42	0.958
13	73.25 - 68.25	5		0	36.000	36.000	0.5625	A53-B-42	0.958
14	68.25 - 63.25	5		0	36.000	36.000	0.5625	A53-B-42	0.958
15	63.25 - 60	3.25	0	0	36.000	36.000	0.5625	A53-B-42	0.958
16	60 - 59.75	0.25		0	42.000	42.000	0.525	A53-B-42	0.980
17	59.75 - 54.75	5		0	42.000	42.000	0.525	A53-B-42	0.980
18	54.75 - 49.75	5		0	42.000	42.000	0.525	A53-B-42	0.980
19	49.75 - 44.75	5		0	42.000	42.000	0.525	A53-B-42	0.980
20	44.75 - 40	4.75	0	0	42.000	42.000	0.525	A53-B-42	0.980
21	40 - 39.75	0.25		0	48.000	48.000	0.55625	A53-B-42	0.971
22	39.75 - 34.75	5		0	48.000	48.000	0.55625	A53-B-42	0.971
23	34.75 - 29.75	5		0	48.000	48.000	0.55625	A53-B-42	0.971
24	29.75 - 24.75	5		0	48.000	48.000	0.55625	A53-B-42	0.971
25	24.75 - 20	4.75	0	0	48.000	48.000	0.55625	A53-B-42	0.971
26	20 - 19.75	0.25		0	54.000	54.000	0.5875	A53-B-42	0.964
27	19.75 - 14.75	5		0	54.000	54.000	0.5875	A53-B-42	0.964
28	14.75 - 9.75	5		0	54.000	54.000	0.5875	A53-B-42	0.964
29	9.75 - 4.75	5		0	54.000	54.000	0.5875	A53-B-42	0.964
30	4.75 - 0	4.75		0	54.000	54.000	0.5875	A53-B-42	0.964

TNX Section Forces

Increment (ft):		TNX Output				
	5	Section Height (ft)		P _u (K)	M _{ux} (kip-ft)	V _u (K)
1		125 - 120	4.74	10.74	5.30	
2		120 - 115	5.38	37.96	5.59	
3		115 - 110	8.75	80.11	10.57	
4		110 - 105	9.43	133.80	10.89	
5		105 - 100	10.10	188.90	11.15	
6		100 - 95	13.93	267.14	15.70	
7		95 - 90	14.84	346.43	16.02	
8		90 - 85	19.08	444.65	19.87	
9		85 - 80	20.05	544.59	20.14	
10		80 - 75	21.15	646.04	20.47	
11		75 - 73.5	21.48	676.80	20.57	
12		73.5 - 73.25	21.56	681.94	20.59	
13		73.25 - 68.25	23.00	786.68	21.41	
14		68.25 - 63.25	24.46	895.59	22.14	
15		63.25 - 60	25.41	968.32	22.61	
16		60 - 59.75	25.50	973.98	22.64	
17		59.75 - 54.75	27.10	1089.16	23.41	
18		54.75 - 49.75	28.71	1208.09	24.15	
19		49.75 - 44.75	30.33	1330.68	24.87	
20		44.75 - 40	31.87	1450.49	25.56	
21		40 - 39.75	31.97	1456.89	25.60	
22		39.75 - 34.75	33.85	1586.82	26.35	
23		34.75 - 29.75	35.74	1720.42	27.07	
24		29.75 - 24.75	37.64	1857.65	27.81	
25		24.75 - 20	39.45	1991.47	28.52	
26		20 - 19.75	39.56	1998.60	28.55	
27		19.75 - 14.75	41.75	2143.42	29.35	
28		14.75 - 9.75	43.94	2292.17	30.13	
29		9.75 - 4.75	46.14	2444.79	30.90	
30		4.75 - 0	48.24	2593.31	31.62	

Analysis Results

Elevation (ft)	Component Type	Size	Critical Element	% Capacity	Pass / Fail
125 - 120	Pole	TP24x24x0.375	Pole	2.1%	Pass
120 - 115	Pole	TP24x24x0.375	Pole	6.3%	Pass
115 - 110	Pole	TP24x24x0.375	Pole	13.1%	Pass
110 - 105	Pole	TP24x24x0.375	Pole	21.3%	Pass
105 - 100	Pole	TP24x24x0.375	Pole	29.8%	Pass
100 - 95	Pole	TP30x30x0.375	Pole	27.9%	Pass
95 - 90	Pole	TP30x30x0.375	Pole	35.9%	Pass
90 - 85	Pole	TP30x30x0.375	Pole	46.2%	Pass
85 - 80	Pole	TP30x30x0.375	Pole	56.3%	Pass
80 - 75	Pole	TP36x36x0.375	Pole	47.4%	Pass
75 - 73.5	Pole	TP36x36x0.375	Pole	49.6%	Pass
73.5 - 73.25	Pole + Reinf.	TP36x36x0.5625	Reinf. 4 Compression	39.4%	Pass
73.25 - 68.25	Pole + Reinf.	TP36x36x0.5625	Reinf. 4 Compression	45.4%	Pass
68.25 - 63.25	Pole + Reinf.	TP36x36x0.5625	Reinf. 4 Compression	51.6%	Pass
63.25 - 60	Pole + Reinf.	TP36x36x0.5625	Reinf. 4 Compression	55.7%	Pass
60 - 59.75	Pole + Reinf.	TP42x42x0.525	Pole	38.0%	Pass
59.75 - 54.75	Pole + Reinf.	TP42x42x0.525	Pole	42.5%	Pass
54.75 - 49.75	Pole + Reinf.	TP42x42x0.525	Pole	47.0%	Pass
49.75 - 44.75	Pole + Reinf.	TP42x42x0.525	Pole	51.8%	Pass
44.75 - 40	Pole + Reinf.	TP42x42x0.525	Pole	56.4%	Pass
40 - 39.75	Pole + Reinf.	TP48x48x0.5563	Pole	41.7%	Pass
39.75 - 34.75	Pole + Reinf.	TP48x48x0.5563	Pole	45.4%	Pass
34.75 - 29.75	Pole + Reinf.	TP48x48x0.5563	Pole	49.2%	Pass
29.75 - 24.75	Pole + Reinf.	TP48x48x0.5563	Pole	53.1%	Pass
24.75 - 20	Pole + Reinf.	TP48x48x0.5563	Pole	56.9%	Pass
20 - 19.75	Pole + Reinf.	TP54x54x0.5875	Pole	43.6%	Pass
19.75 - 14.75	Pole + Reinf.	TP54x54x0.5875	Pole	46.7%	Pass
14.75 - 9.75	Pole + Reinf.	TP54x54x0.5875	Pole	49.9%	Pass
9.75 - 4.75	Pole + Reinf.	TP54x54x0.5875	Pole	53.2%	Pass
4.75 - 0	Pole + Reinf.	TP54x54x0.5875	Pole	56.4%	Pass
				Summary	
			Pole	56.9%	Pass
			Reinforcement	55.7%	Pass
			Overall	56.9%	Pass

Additional Calculations

Section Elevation (ft)	Moment of Inertia (in ⁴)			Area (in ²)			% Capacity*				
	Pole	Reinf.	Total	Pole	Reinf.	Total	Pole	R1	R2	R3	R4
125 - 120	1942	n/a	1942	27.83	n/a	27.83	2.1%				
120 - 115	1942	n/a	1942	27.83	n/a	27.83	6.3%				
115 - 110	1942	n/a	1942	27.83	n/a	27.83	13.1%				
110 - 105	1942	n/a	1942	27.83	n/a	27.83	21.3%				
105 - 100	1942	n/a	1942	27.83	n/a	27.83	29.8%				
100 - 95	3829	n/a	3829	34.90	n/a	34.90	27.9%				
95 - 90	3829	n/a	3829	34.90	n/a	34.90	35.9%				
90 - 85	3829	n/a	3829	34.90	n/a	34.90	46.2%				
85 - 80	3829	n/a	3829	34.90	n/a	34.90	56.3%				
80 - 75	6659	n/a	6659	41.97	n/a	41.97	47.4%				
75 - 73.5	6659	n/a	6659	41.97	n/a	41.97	49.6%				
73.5 - 73.25	6659	3108	9767	41.97	18.00	59.97	34.0%				39.4%
73.25 - 68.25	6659	3108	9767	41.97	18.00	59.97	39.2%				45.4%
68.25 - 63.25	6659	3108	9767	41.97	18.00	59.97	44.5%				51.6%
63.25 - 60	6659	3108	9767	41.97	18.00	59.97	48.1%				55.7%
60 - 59.75	10622	4188	14810	49.04	18.00	67.04	38.0%			35.0%	
59.75 - 54.75	10622	4188	14810	49.04	18.00	67.04	42.5%			39.1%	
54.75 - 49.75	10622	4188	14810	49.04	18.00	67.04	47.0%			43.3%	
49.75 - 44.75	10622	4188	14810	49.04	18.00	67.04	51.8%			47.6%	
44.75 - 40	10622	4188	14810	49.04	18.00	67.04	56.4%			51.9%	
40 - 39.75	15908	7435	23343	56.11	24.38	80.48	41.7%		37.2%		
39.75 - 34.75	15908	7435	23343	56.11	24.38	80.48	45.4%		40.5%		
34.75 - 29.75	15908	7435	23343	56.11	24.38	80.48	49.2%		43.9%		
29.75 - 24.75	15908	7435	23343	56.11	24.38	80.48	53.1%		47.4%		
24.75 - 20	15908	7435	23343	56.11	24.38	80.48	56.9%		50.8%		
20 - 19.75	22710	12261	34970	63.18	31.88	95.05	43.6%	41.5%			
19.75 - 14.75	22710	12261	34970	63.18	31.88	95.05	46.7%	44.5%			
14.75 - 9.75	22710	12261	34970	63.18	31.88	95.05	49.9%	47.5%			
9.75 - 4.75	22710	12261	34970	63.18	31.88	95.05	53.2%	50.7%			
4.75 - 0	22710	12261	34970	63.18	31.88	95.05	56.4%	53.7%			

Note: Section capacity checked in 5 degree increments.
Rating per TIA-222-H Section 15.5.

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Bolted Bridge Stiffeners Reinforcement Check

Assumptions / Notes:

1. For design purposes, it is assumed that the proposed bridge stiffeners are to take the full load.
2. The plastification of the pole is not considered.
3. All shear and axial loads are taken by the flange bolts.

1. PARAMETERS

Elevation **80** ft Apply TIA-222-H Section 15.5?

Rev. H
Yes

1.1 tnxTower Reactions

Moment (M)	544.59 k-ft
Axial Load (P)	20.05 kip
Shear Load (V)	20.13 kips

1.2 Shaft Properties at the Flange

Upper Shaft Diameter (Dshaft1)	30 in
Upper Shaft Thichkness (t1)	0.375 in
Lower Shaft Diameter (Dshaft2)	36 in
Lower Shaft Thickness (t2)	0.375 in
Shaft Grade:	
Fyshaft	42 ksi
Fushaft	63 ksi

1.3 Existing Bridge Stiffeners Properties

(Verify existing bolted connection for reduced moment.)

Number of Proposed Bridge Stiffeners (Nexist)	3
Thickness of Proposed Bridge Stiffeners (texist)	1 in
Width of Proposed Bridge Stiffeners (wexist)	4.5 in
Existing Bridge Stiffener Grade:	
Fyex	65 ksi
Fuex	80 ksi
Diameter to the centroid of Existing Bridge Stiffeners (BCexist)	37 in
Gross Area of One Existing Bridge Stiffener (Ag_exist)	4.5 in ²
Radius of Gyration about x-axis (rx)	0.289 in
Moment of Inertia of Existing Bridge Stiffeners (Iexist)	2310 in ⁴

1.4 Flange Bolt Properties

Number of Flange Bolts (Nbolts)	24
Diameter of Flange Bolts	1 in
Bolt Circle of Flange Bolts (BCbolts)	33 in
Gross Area of One Flange Bolt (Ag_bolts)	0.785 in ²
Moment of Inertia of Flange Bolts (Ibolts)	2566 in ⁴

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1.5 Division of Forces

Total Gross Area (Ag_total)	32.35 in ²
Total Moment of Inertia (Itotal)	4876 in ⁴

1.6 Reactions to Existing Bridge Stiffeners

Moment Reaction to Existing Bridge Stiffeners (Mexist)	258.0 kip-ft
Axial Reaction to Existing Bridge Stiffeners (Pexist)	0.0 kips
Shear Reaction to Flange Bolts (Vexist)	0.0 kips

1.7 Reactions to Flange Bolts

(It is assumed that all shear and axial loads are taken by the flange bolts)

Moment Reaction to Flange Bolts (Mbolts)	286.6 kip-ft	} Check Flange Connection with these Reactions
Axial Reaction to Flange Bolts (Pbolts)	20.1 kips	
Shear Reaction to Flange Bolts (Vbolts)	20.1 kips	

2. Existing Bridge Stiffener

2.1 Maximum Axial Forces in Single Existing

Outer Radius of Bolt Circle (C)	18.5 in
Critical Compression Bending Stress (Pcomp)	111.6 kips
Critical Tension Bending Stress (Ptens)	111.6 kips

2.2 Available Compression Strength

[AISC 15th Edition E3-1]

Resistance Factor (ϕ_c)	0.9	
Unbraced Length (Lu)	16 in	
Effective Length Factor (K)	1	
Strength of Bridge Stiffener:		
Fy	65 ksi	
Fu	80 ksi	
Effective Length of Member (Lc)	16.00 in	[AISC 15th Edition E3-2]
Elastic Buckling Stress (Fe)	93.2 ksi	[AISC 15th Edition, Eq. E3-4]
Limit	99.5	
Determination of Critical Stress (Fcr)	48.5 ksi	
[AISC 15th Edition, Eqs. E3-2 and E3-3]		

Allowable Compressive Strength (ϕP_n)	196.6 kips
[AISC 15th Ed., Eqs. J4-6 and E3-1]	

Check Compressive Strength (Checkcomp)

54.1% **Pass**

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2.3 Available Tension Strength

Gross Section Yield

Available Tension Yield Strength (ϕP_t)
[AISC 15th Ed., Ch.D2] 263.3 kips

Net Section Fracture

Bolt Hole Diameter (BH) 1.188 in
 Thickness (T) 1.00 in
 Net Area (A_{net}) 3.250 in²
 Net Area Limitation (A_e) 3.250 in²
 Available Fractile Strength (ϕP_{tr}) 195.0 kips

Tension Check

Controlling Mode of Failure (Check mode) (ϕP_{nt})	Fracture Controls 195.0 kips
Controlling Tension Mode Check (Checktension)	54.5% Pass

SUMMARY

tnxTower Reactions

M 544.6 kip-ft
 P 20.1 kip
 V 20.1 kip

Flange Bolts

Diameter of Flange Bolts (Dbolts) 1.00 in
 Bolt Circle of Flange Bolts (BCbolts) 33.0 in
 Loads to Flange Bolts

Mbolts	286.6 kip-ft	} See Flange tool for Flange Bolt and Plate Capacities
P	20.1 kip	
V	20.1 kip	

Existing Jump Plates

Moment to Existing Bridge Stiffeners (M_{exist})	258.0 kip-ft
Number of Existing Bridge Stiffeners (N_{exist})	3
Thickness (t_{exist})	1.00 in
Width (w_{exist})	4.50 in
Controlling Capacity of Existing Bridge Stiffeners ($Capacity_{max}$)	54.5% Pass

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Bolted Bridge Stiffeners Reinforcement Check

Assumptions / Notes:

1. For design purposes, it is assumed that the proposed bridge stiffeners are to take the full load.
2. The plastification of the pole is not considered.
3. All shear and axial loads are taken by the flange bolts.

1. PARAMETERS

Elevation 60 ft Apply TIA-222-H Section 15.5?

Rev. H
Yes

1.1 tnxTower Reactions

Moment (M)	968.31	k-ft
Axial Load (P)	25.41	kip
Shear Load (V)	22.6	kips

1.2 Shaft Properties at the Flange

Upper Shaft Diameter (Dshaft1)	36	in
Upper Shaft Thichkness (t1)	0.375	in
Lower Shaft Diameter (Dshaft2)	42	in
Lower Shaft Thickness (t2)	0.375	in
Shaft Grade:		
Fyshaft	42	ksi
Fushaft	63	ksi

1.3 Existing Bridge Stiffeners Properties

(Verify existing bolted connection for reduced moment.)

Number of Proposed Bridge Stiffeners (Nexist)	3	
Thickness of Proposed Bridge Stiffeners (texist)	1	in
Width of Proposed Bridge Stiffeners (wexist)	6	in
Existing Bridge Stiffener Grade:		
Fyex	65	ksi
Fuex	80	ksi
Diameter to the centroid of Existing Bridge Stiffeners (BCexist)	43	in
Gross Area of One Existing Bridge Stiffener (Ag_exist)	6	in ²
Radius of Gyration about x-axis (rx)	0.289	in
Moment of Inertia of Existing Bridge Stiffeners (Iexist)	4160	in ⁴

1.4 Flange Bolt Properties

Number of Flange Bolts (Nbolts)	28	
Diameter of Flange Bolts	1	in
Bolt Circle of Flange Bolts (BCbolts)	39	in
Gross Area of One Flange Bolt (Ag_bolts)	0.785	in ²
Moment of Inertia of Flange Bolts (Ibolts)	4181	in ⁴

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1.5 Division of Forces

Total Gross Area (Ag_total)	39.99 in ²
Total Moment of Inertia (Itotal)	8341 in ⁴

1.6 Reactions to Existing Bridge Stiffeners

Moment Reaction to Existing Bridge Stiffeners (Mexist)	482.9 kip-ft
Axial Reaction to Existing Bridge Stiffeners (Pexist)	0.0 kips
Shear Reaction to Flange Bolts (Vexist)	0.0 kips

1.7 Reactions to Flange Bolts

(It is assumed that all shear and axial loads are taken by the flange bolts)

Moment Reaction to Flange Bolts (Mbolts)	485.4 kip-ft	} Check Flange Connection with these Reactions
Axial Reaction to Flange Bolts (Pbolts)	25.4 kips	
Shear Reaction to Flange Bolts (Vbolts)	22.6 kips	

2. Existing Bridge Stiffener

2.1 Maximum Axial Forces in Single Existing

Outer Radius of Bolt Circle (C)	21.5 in
Critical Compression Bending Stress (Pcomp)	179.7 kips
Critical Tension Bending Stress (Ptens)	179.7 kips

2.2 Available Compression Strength

[AISC 15th Edition E3-1]

Resistance Factor (ϕ_c)	0.9	
Unbraced Length (Lu)	16 in	
Effective Length Factor (K)	1	
Strength of Bridge Stiffener:		
Fy	65 ksi	
Fu	80 ksi	
Effective Length of Member (Lc)	16.00 in	[AISC 15th Edition E3-2]
Elastic Buckling Stress (Fe)	93.2 ksi	[AISC 15th Edition, Eq. E3-4]
Limit	99.5	
Determination of Critical Stress (Fcr)	48.5 ksi	
[AISC 15th Edition, Eqs. E3-2 and E3-3]		

Allowable Compressive Strength (ϕP_n)	262.1 kips
[AISC 15th Ed., Eqs. J4-6 and E3-1]	

Check Compressive Strength (Checkcomp)	65.3%	Pass
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2.3 Available Tension Strength

Gross Section Yield

Available Tension Yield Strength (ϕP_t)
[AISC 15th Ed., Ch.D2] 351.0 kips

Net Section Fracture

Bolt Hole Diameter (BH) 1.188 in
 Thickness (T) 1.00 in
 Net Area (A_{net}) 4.750 in²
 Net Area Limitation (A_e) 4.750 in²
 Available Fractile Strength (ϕP_{tr}) 285.0 kips

Tension Check

Controlling Mode of Failure (Check mode) (ϕP_{nt})	Fracture Controls 285.0 kips
Controlling Tension Mode Check (Checktension)	60.1% Pass

SUMMARY

tnxTower Reactions

M 968.3 kip-ft
 P 25.4 kip
 V 22.6 kip

Flange Bolts

Diameter of Flange Bolts (Dbolts) 1.00 in
 Bolt Circle of Flange Bolts (BCbolts) 39.0 in
 Loads to Flange Bolts

Mbolts	485.4 kip-ft	}	See Flange tool for Flange Bolt and Plate Capacities
P	25.4 kip		
V	22.6 kip		

Existing Jump Plates

Moment to Existing Bridge Stiffeners (M _{exist})	482.9 kip-ft
Number of Existing Bridge Stiffeners (N _{exist})	3
Thickness (t _{exist})	1.00 in
Width (w _{exist})	6.00 in
Controlling Capacity of Existing Bridge Stiffeners (Capacity _{max})	65.3% Pass

PROJECT	101126_004_01 - Branford/ I-95/ Intiti		
SUBJECT	Bridge Stiffener - 40 ft		
DATE	05-16-19	PAGE	1 OF 3



V1.0.1

Bolted Bridge Stiffeners Reinforcement Check

Assumptions / Notes:

1. For design purposes, it is assumed that the proposed bridge stiffeners are to take the full load.
2. The plastification of the pole is not considered.
3. All shear and axial loads are taken by the flange bolts.

1. PARAMETERS

Elevation **40** ft

Apply TIA-222-H Section 15.5?

Rev. H
Yes

1.1 tnxTower Reactions

Moment (M)	1450.49 k-ft
Axial Load (P)	31.86 kip
Shear Load (V)	25.56 kips

1.2 Shaft Properties at the Flange

Upper Shaft Diameter (Dshaft1)	42 in
Upper Shaft Thichkness (t1)	0.375 in
Lower Shaft Diameter (Dshaft2)	48 in
Lower Shaft Thickness (t2)	0.375 in
Shaft Grade:	
Fyshaft	42 ksi
Fushaft	63 ksi

1.3 Existing Bridge Stiffeners Properties

(Verify existing bolted connection for reduced moment.)

Number of Proposed Bridge Stiffeners (Nexist)	3
Thickness of Proposed Bridge Stiffeners (texist)	1.25 in
Width of Proposed Bridge Stiffeners (wexist)	6.5 in
Existing Bridge Stiffener Grade:	
Fyex	65 ksi
Fuex	80 ksi
Diameter to the centroid of Existing Bridge Stiffeners (BCexist)	49 in
Gross Area of One Existing Bridge Stiffener (Ag_exist)	8.125 in ²
Radius of Gyration about x-axis (rx)	0.361 in
Moment of Inertia of Existing Bridge Stiffeners (Iexist)	7316 in ⁴

1.4 Flange Bolt Properties

Number of Flange Bolts (Nbolts)	32
Diameter of Flange Bolts	1 in
Bolt Circle of Flange Bolts (BCbolts)	45 in
Gross Area of One Flange Bolt (Ag_bolts)	0.785 in ²
Moment of Inertia of Flange Bolts (Ibolts)	6362 in ⁴

PROJECT	101126_004_01 - Branford/ I-95/ Intiti		
SUBJECT	Bridge Stiffener - 40 ft		
DATE	05-16-19	PAGE 2	OF 3



V1.0.1

1.5 Division of Forces

Total Gross Area (Ag_total)	49.51 in ²
Total Moment of Inertia (Itotal)	13677 in ⁴

1.6 Reactions to Existing Bridge Stiffeners

Moment Reaction to Existing Bridge Stiffeners (Mexist)	775.8 kip-ft
Axial Reaction to Existing Bridge Stiffeners (Pexist)	0.0 kips
Shear Reaction to Flange Bolts (Vexist)	0.0 kips

1.7 Reactions to Flange Bolts

(It is assumed that all shear and axial loads are taken by the flange bolts)

Moment Reaction to Flange Bolts (Mbolts)	674.7 kip-ft	} Check Flange Connection with these Reactions
Axial Reaction to Flange Bolts (Pbolts)	31.9 kips	
Shear Reaction to Flange Bolts (Vbolts)	25.6 kips	

2. Existing Bridge Stiffener

2.1 Maximum Axial Forces in Single Existing

Outer Radius of Bolt Circle (C)	24.5 in
Critical Compression Bending Stress (Pcomp)	253.3 kips
Critical Tension Bending Stress (Ptens)	253.3 kips

2.2 Available Compression Strength

[AISC 15th Edition E3-1]

Resistance Factor (ϕ_c)	0.9	
Unbraced Length (Lu)	16 in	
Effective Length Factor (K)	1	
Strength of Bridge Stiffener:		
Fy	65 ksi	
Fu	80 ksi	
Effective Length of Member (Lc)	16.00 in	[AISC 15th Edition E3-2]
Elastic Buckling Stress (Fe)	145.6 ksi	[AISC 15th Edition, Eq. E3-4]
Limit	99.5	
Determination of Critical Stress (Fcr)	53.9 ksi	
[AISC 15th Edition, Eqs. E3-2 and E3-3]		

Allowable Compressive Strength (ϕP_n)	394.3 kips
[AISC 15th Ed., Eqs. J4-6 and E3-1]	

Check Compressive Strength (Checkcomp)	61.2%	Pass
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PROJECT	101126_004_01 - Branford/ I-95/ Intiti		
SUBJECT	Bridge Stiffener - 40 ft		
DATE	05-16-19	PAGE	3 OF 3



V1.0.1

2.3 Available Tension Strength

Gross Section Yield

Available Tension Yield Strength (ϕP_t)
[AISC 15th Ed., Ch.D2] 475.3 kips

Net Section Fracture

Bolt Hole Diameter (BH) 1.188 in
 Thickness (T) 1.25 in
 Net Area (A_{net}) 6.563 in²
 Net Area Limitation (A_e) 6.563 in²
 Available Fractile Strength (ϕP_{tr}) 393.8 kips

Tension Check

Controlling Mode of Failure (Check mode) (ϕP_{nt})	Fracture Controls 393.8 kips
Controlling Tension Mode Check (Checktension)	61.3% Pass

SUMMARY

tnxTower Reactions

M 1450.5 kip-ft
 P 31.9 kip
 V 25.6 kip

Flange Bolts

Diameter of Flange Bolts (Dbolts) 1.00 in
 Bolt Circle of Flange Bolts (BCbolts) 45.0 in
 Loads to Flange Bolts

Mbolts	674.7 kip-ft	} See Flange tool for Flange Bolt and Plate Capacities
P	31.9 kip	
V	25.6 kip	

Existing Jump Plates

Moment to Existing Bridge Stiffeners (M_{exist})	775.8 kip-ft
Number of Existing Bridge Stiffeners (N_{exist})	3
Thickness (t_{exist})	1.25 in
Width (w_{exist})	6.50 in
Controlling Capacity of Existing Bridge Stiffeners ($Capacity_{max}$)	61.3% Pass

PROJECT	101126_004_01 - Branford/ I-95/ Intiti		
SUBJECT	Bridge Stiffener - 20 ft		
DATE	05-16-19	PAGE	1 OF 3



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 Tulsa, OK 74119
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V1.0.1

Bolted Bridge Stiffeners Reinforcement Check

Assumptions / Notes:

1. For design purposes, it is assumed that the proposed bridge stiffeners are to take the full load.
2. The plastification of the pole is not considered.
3. All shear and axial loads are taken by the flange bolts.

1. PARAMETERS

Elevation **20** ft

Apply TIA-222-H Section 15.5?

Rev. H

Yes

1.1 tnxTower Reactions

Moment (M)	1991.46 k-ft
Axial Load (P)	39.44 kip
Shear Load (V)	28.51 kips

1.2 Shaft Properties at the Flange

Upper Shaft Diameter (Dshaft1)	48 in
Upper Shaft Thichkness (t1)	0.375 in
Lower Shaft Diameter (Dshaft2)	54 in
Lower Shaft Thickness (t2)	0.375 in
Shaft Grade:	
Fyshaft	42 ksi
Fushaft	63 ksi

1.3 Existing Bridge Stiffeners Properties

(Verify existing bolted connection for reduced moment.)

Number of Proposed Bridge Stiffeners (Nexist)	3
Thickness of Proposed Bridge Stiffeners (texist)	1.25 in
Width of Proposed Bridge Stiffeners (wexist)	8.5 in
Existing Bridge Stiffener Grade:	
Fyex	65 ksi
Fuex	80 ksi
Diameter to the centroid of Existing Bridge Stiffeners (BCexist)	55 in
Gross Area of One Existing Bridge Stiffener (Ag_exist)	10.625 in ²
Radius of Gyration about x-axis (rx)	0.361 in
Moment of Inertia of Existing Bridge Stiffeners (Iexist)	12053 in ⁴

1.4 Flange Bolt Properties

Number of Flange Bolts (Nbolts)	36
Diameter of Flange Bolts	1 in
Bolt Circle of Flange Bolts (BCbolts)	51 in
Gross Area of One Flange Bolt (Ag_bolts)	0.785 in ²
Moment of Inertia of Flange Bolts (Ibolts)	9193 in ⁴

PROJECT	101126_004_01 - Branford/ I-95/ Intiti		
SUBJECT	Bridge Stiffener - 20 ft		
DATE	05-16-19	PAGE	2 OF 3



V1.0.1

1.5 Division of Forces

Total Gross Area (Ag_total)	60.15 in ²
Total Moment of Inertia (Itotal)	21245 in ⁴

1.6 Reactions to Existing Bridge Stiffeners

Moment Reaction to Existing Bridge Stiffeners (Mexist)	1129.8 kip-ft
Axial Reaction to Existing Bridge Stiffeners (Pexist)	0.0 kips
Shear Reaction to Flange Bolts (Vexist)	0.0 kips

1.7 Reactions to Flange Bolts

(It is assumed that all shear and axial loads are taken by the flange bolts)

Moment Reaction to Flange Bolts (Mbolts)	861.7 kip-ft	} Check Flange Connection with these Reactions
Axial Reaction to Flange Bolts (Pbolts)	39.4 kips	
Shear Reaction to Flange Bolts (Vbolts)	28.5 kips	

2. Existing Bridge Stiffener

2.1 Maximum Axial Forces in Single Existing

Outer Radius of Bolt Circle (C)	27.5 in
Critical Compression Bending Stress (Pcomp)	328.7 kips
Critical Tension Bending Stress (Ptens)	328.7 kips

2.2 Available Compression Strength

[AISC 15th Edition E3-1]

Resistance Factor (ϕ_c)	0.9	
Unbraced Length (Lu)	16 in	
Effective Length Factor (K)	1	
Strength of Bridge Stiffener:		
Fy	65 ksi	
Fu	80 ksi	
Effective Length of Member (Lc)	16.00 in	[AISC 15th Edition E3-2]
Elastic Buckling Stress (Fe)	145.6 ksi	[AISC 15th Edition, Eq. E3-4]
Limit	99.5	
Determination of Critical Stress (Fcr)	53.9 ksi	
[AISC 15th Edition, Eqs. E3-2 and E3-3]		

Allowable Compressive Strength (ϕP_n)	515.6 kips
[AISC 15th Ed., Eqs. J4-6 and E3-1]	

Check Compressive Strength (Checkcomp)

60.7% Pass

PROJECT	101126_004_01 - Branford/ I-95/ Intiti		
SUBJECT	Bridge Stiffener - 20 ft		
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V1.0.1

2.3 Available Tension Strength

Gross Section Yield

Available Tension Yield Strength (ϕP_t) [AISC 15th Ed., Ch.D2] 621.6 kips

Net Section Fracture

Bolt Hole Diameter (BH) 1.188 in
 Thickness (T) 1.25 in
 Net Area (A_{net}) 9.063 in²
 Net Area Limitation (A_e) 9.063 in²
 Available Fractile Strength (ϕP_{tr}) 543.8 kips

Tension Check

Controlling Mode of Failure (Check mode) (ϕP_{nt})	Fracture Controls
Controlling Tension Mode Check (Checktension)	543.8 kips
	57.6% Pass

SUMMARY

tnxTower Reactions

M 1991.5 kip-ft
 P 39.4 kip
 V 28.5 kip

Flange Bolts

Diameter of Flange Bolts (Dbolts) 1.00 in
 Bolt Circle of Flange Bolts (BCbolts) 51.0 in
 Loads to Flange Bolts

Mbolts	861.7 kip-ft	} See Flange tool for Flange Bolt and Plate Capacities
P	39.4 kip	
V	28.5 kip	

Existing Jump Plates

Moment to Existing Bridge Stiffeners (M_{exist})	1129.8 kip-ft
Number of Existing Bridge Stiffeners (N_{exist})	3
Thickness (t_{exist})	1.25 in
Width (w_{exist})	8.50 in
Controlling Capacity of Existing Bridge Stiffeners ($Capacity_{max}$)	60.7% Pass

Monopole Flange Plate Connection

Elevation = 100 ft.

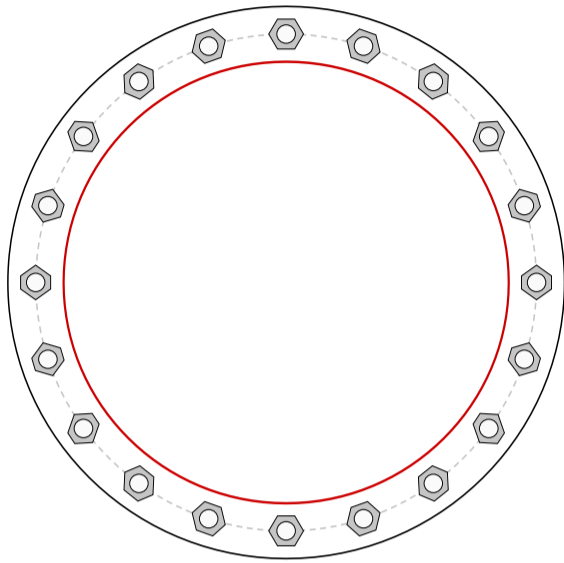


BU #	822765
Site Name	nford/ I-95/ X55/ Dtn1
Order #	479795, Rev. 0
TIA-222 Revision	H

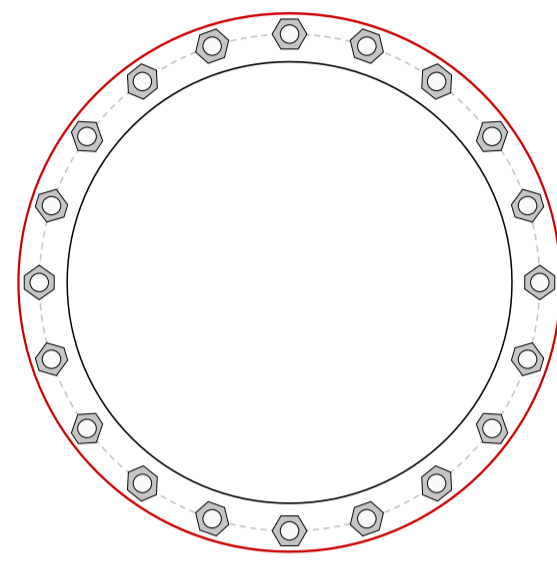
Applied Loads	
Moment (kip-ft)	188.90
Axial Force (kips)	10.10
Shear Force (kips)	11.15

*TIA-222-H Section 15.5 Applied

Top Plate - External



Bottom Plate - Internal



Connection Properties

Bolt Data

(20) 1" ϕ bolts (A325 N; Fy=92 ksi, Fu=120 ksi) on 27" BC

Top Plate Data

30" OD x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Bottom Plate Data

24" ID x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Top Stiffener Data

N/A

Bottom Stiffener Data

N/A

Top Pole Data

24" x 0.375" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Bottom Pole Data

30" x 0.375" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Analysis Results

Bolt Capacity

Max Load (kips)	16.28
Allowable (kips)	54.53
Stress Rating:	28.4% Pass

Top Plate Capacity

Max Stress (ksi):	17.32	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	50.9%	Pass
Tension Side Stress Rating:	18.3%	Pass

Bottom Plate Capacity

Max Stress (ksi):	16.93	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	49.8%	Pass
Tension Side Stress Rating:	N/A	

Monopole Flange Plate Connection

Elevation = 80 ft.

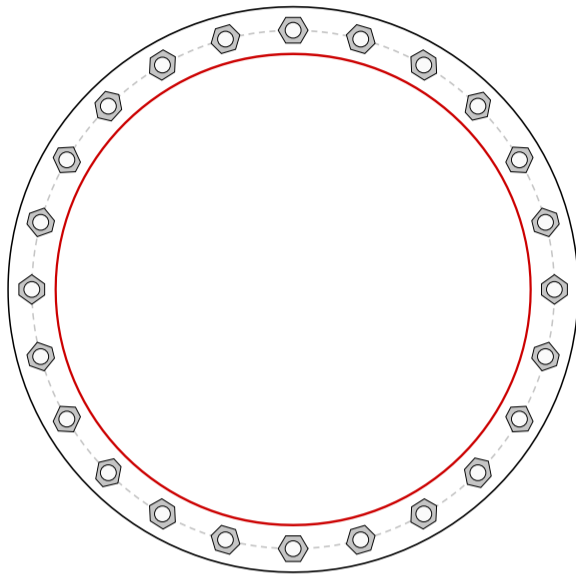


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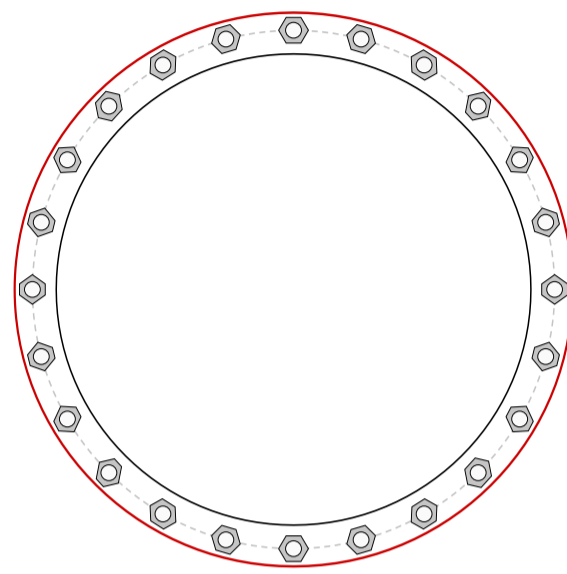
Applied Loads	
Moment (kip-ft)	286.60
Axial Force (kips)	20.10
Shear Force (kips)	20.10

*TIA-222-H Section 15.5 Applied

Top Plate - External



Bottom Plate - Internal



Connection Properties

Bolt Data

(24) 1" ϕ bolts (A325 N; Fy=92 ksi, Fu=120 ksi) on 33" BC

Top Plate Data

36" OD x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Bottom Plate Data

30" ID x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Top Stiffener Data

N/A

Bottom Stiffener Data

N/A

Top Pole Data

30" x 0.375" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Bottom Pole Data

36" x 0.375" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Analysis Results

Bolt Capacity

Max Load (kips)	16.53
Allowable (kips)	54.52
Stress Rating:	28.9% Pass

Top Plate Capacity

Max Stress (ksi):	17.55	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	51.6%	Pass
Tension Side Stress Rating:	17.7%	Pass

Bottom Plate Capacity

Max Stress (ksi):	17.75	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	52.2%	Pass
Tension Side Stress Rating:	N/A	

Monopole Flange Plate Connection

Elevation = 60 ft.

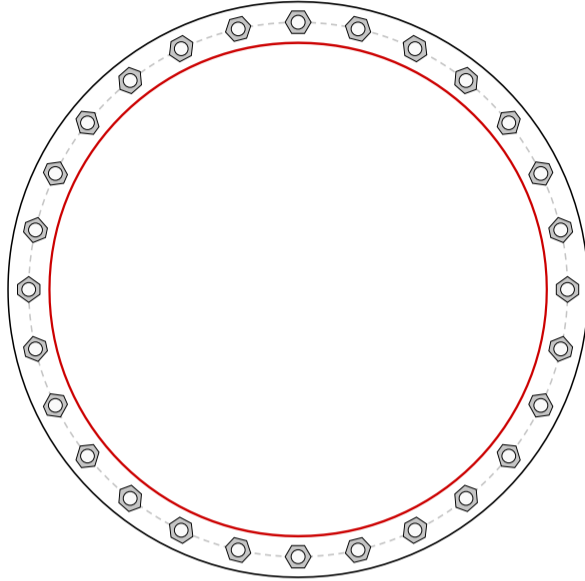


BU #	822765
Site Name	anford/ I-95/ X55/ Dtn1
Order #	479795, Rev. 0
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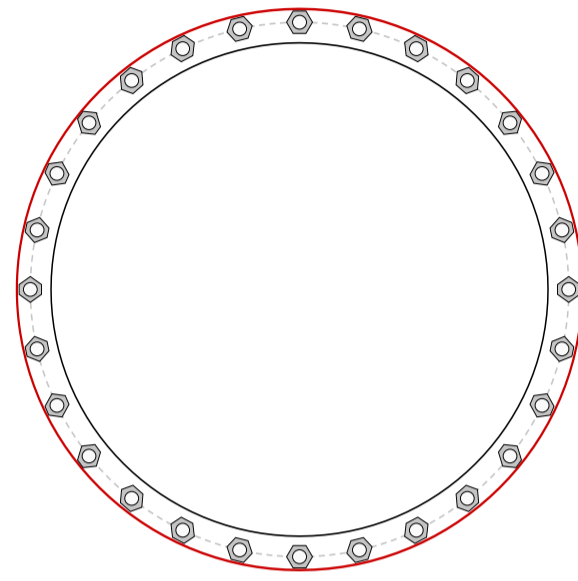
Applied Loads	
Moment (kip-ft)	485.40
Axial Force (kips)	25.40
Shear Force (kips)	22.60

*TIA-222-H Section 15.5 Applied

Top Plate - External



Bottom Plate - Internal



Connection Properties

Bolt Data

(28) 1" ϕ bolts (A325 N; Fy=92 ksi, Fu=120 ksi) on 39" BC

Top Plate Data

42" OD x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Bottom Plate Data

36" ID x 1.25" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Top Stiffener Data

N/A

Bottom Stiffener Data

N/A

Top Pole Data

36" x 0.5625" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Bottom Pole Data

42" x 0.525" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Analysis Results

Bolt Capacity

Max Load (kips)	20.42
Allowable (kips)	54.53
Stress Rating:	35.7% Pass

Top Plate Capacity

Max Stress (ksi):	20.79	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	61.1%	Pass
Tension Side Stress Rating:	21.2%	Pass

Bottom Plate Capacity

Max Stress (ksi):	12.08	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	35.5%	Pass
Tension Side Stress Rating:	N/A	

Monopole Flange Plate Connection

Elevation = 40 ft.

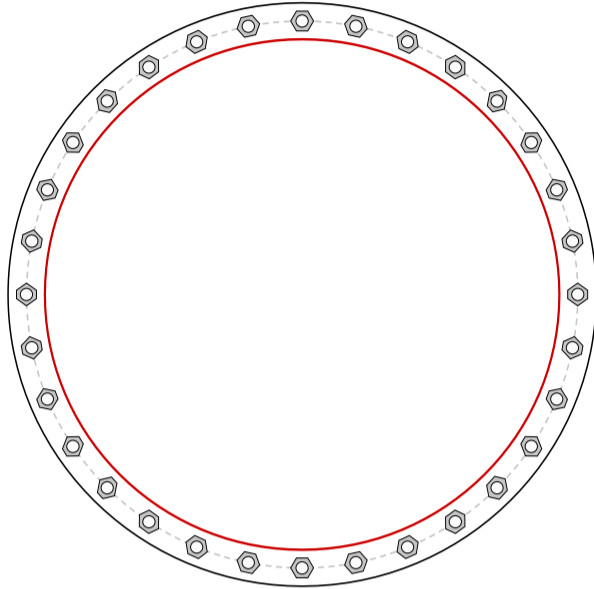


BU #	822765
Site Name	nford/ I-95/ X55/ Dtn1
Order #	479795, Rev. 0
TIA-222 Revision	H

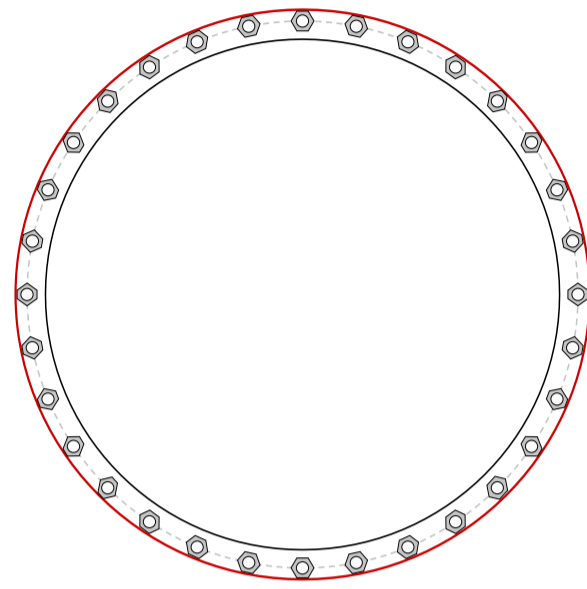
Applied Loads	
Moment (kip-ft)	674.70
Axial Force (kips)	31.90
Shear Force (kips)	25.60

*TIA-222-H Section 15.5 Applied

Top Plate - External



Bottom Plate - Internal



Connection Properties

Bolt Data

(32) 1" ϕ bolts (A325 N; Fy=92 ksi, Fu=120 ksi) on 45" BC

Top Plate Data

48" OD x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Bottom Plate Data

42" ID x 1.25" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Top Stiffener Data

N/A

Bottom Stiffener Data

N/A

Top Pole Data

42" x 0.525" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Bottom Pole Data

48" x 0.55625" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Analysis Results

Bolt Capacity

Max Load (kips)	21.49
Allowable (kips)	54.53
Stress Rating:	37.5% Pass

Top Plate Capacity

Max Stress (ksi):	22.03	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	64.8%	Pass
Tension Side Stress Rating:	21.8%	Pass

Bottom Plate Capacity

Max Stress (ksi):	12.32	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	36.2%	Pass
Tension Side Stress Rating:	N/A	

Monopole Flange Plate Connection

Elevation = 20 ft.

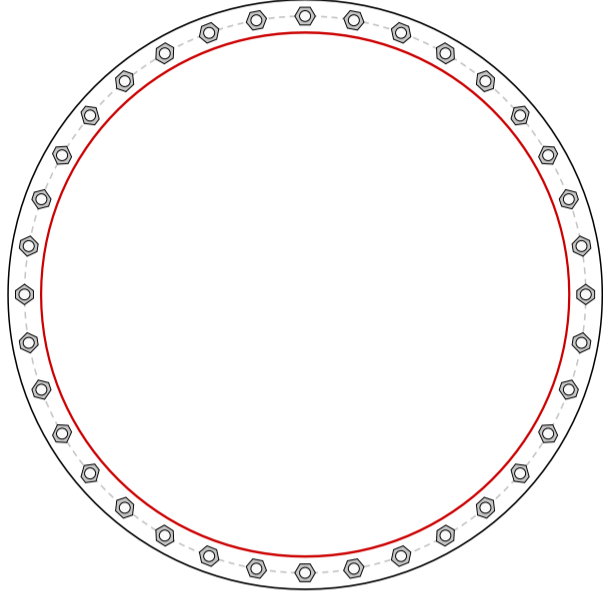


BU #	822765
Site Name	nford/ I-95/ X55/ Dtn1
Order #	479795, Rev. 0
TIA-222 Revision	H

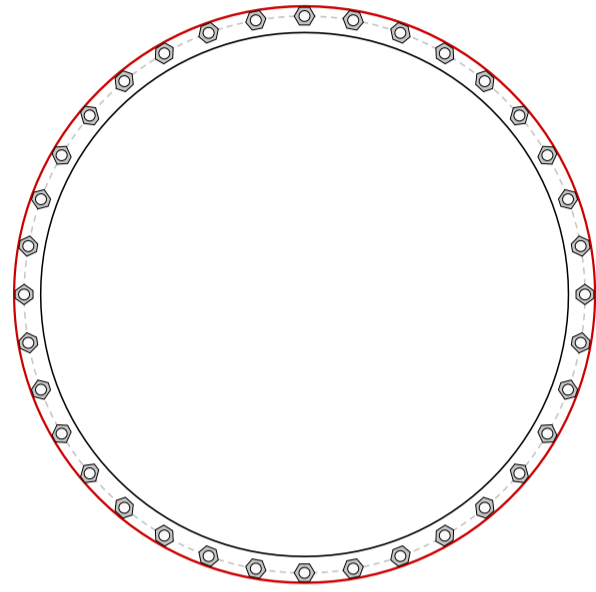
Applied Loads	
Moment (kip-ft)	861.70
Axial Force (kips)	39.40
Shear Force (kips)	28.50

*TIA-222-H Section 15.5 Applied

Top Plate - External



Bottom Plate - Internal



Connection Properties

Bolt Data

(36) 1" ϕ bolts (A325 N; Fy=92 ksi, Fu=120 ksi) on 51" BC

Top Plate Data

54" OD x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Bottom Plate Data

48" ID x 1.25" Plate (A36; Fy=36 ksi, Fu=58 ksi)

Top Stiffener Data

N/A

Bottom Stiffener Data

N/A

Top Pole Data

48" x 0.55625" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Bottom Pole Data

54" x 0.5875" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)

Analysis Results

Bolt Capacity

Max Load (kips)	21.43
Allowable (kips)	54.53
Stress Rating:	37.4% Pass

Top Plate Capacity

Max Stress (ksi):	22.02	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	64.7%	Pass
Tension Side Stress Rating:	21.4%	Pass

Bottom Plate Capacity

Max Stress (ksi):	11.97	(Flexural)
Allowable Stress (ksi):	32.40	
Stress Rating:	35.2%	Pass
Tension Side Stress Rating:	N/A	

PROJECT **101126_004_01 - Branford/ I-95/ X55/ Dtn1, CT**

SUBJECT **Anchor Rod Bracket Analysis**

DATE **05-16-19**



B+T GRP
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630

V3.2.1

Analysis Criteria	
Design or Analysis?	Analysis
AR Load Considered	Current Load
Current load	95.17 kips
AR Capacity	199.5 kips

Tower Type	Monopole
------------	----------

Post-Installed Adhesive AR Modification	
Size	1.75 in
Quantity	8
Bolt Circle	60.3 in
Grade	F1554-105
Fy	105 ksi
Fu	125 ksi

TIA-222 Rev.	H
Apply TIA-222-H Section 15.5?	Yes

Anchor Rod Bracket Analysis Checks		
Tube Bearing	33.3%	-
Tube Compression	N/A	
Gusset Shear Yield	8.9%	-
Gusset Shear Rupture	9.6%	-
Gusset Flexure	N/A	-
Welds	Gusset to Tower and BP	28.2%
	Gusset to Tube	16.9%
	Geometry	N/A
Tower Punching	3.6%	-
Tube Punching	5.5%	-
Utilization	33.3%	

Manufacturers Tower Properties	
Pole Thickness	0.375 in
Pole Grade	Custom
Fy	42 ksi
Fu	63 ksi
Base Plate Gr.	A36
Fy	36 ksi
Fu	58 ksi
Anchor Rods	
Size	1.75 in
Quantity	8
Bolt Circle	60.25 in
Grade	F1554-105
Fy	105 ksi
Fu	125 ksi

Bracket Properties			
Gusset		Pipe/Tube	
Thickness	1.25 in	Size	2.5 XXS Pipe
Width at Tube	3.125 in	Total Length	21 in
Height at Pole	60 in	Length above Gusset	0 in
Height at Tube	21 in	Length below Gusset	0 in
Grade	A572-65	Grade	Custom
Fy	65 ksi	Fy	50 ksi
Fu	80 ksi	Fu	65 ksi
Weld - Gusset to Tower		Weld - Gusset to Pipe/Tube	
FEXX	70 ksi	FEXX	70 ksi
Weld Type	Double Fillet	Weld Type	Double Bevel+Fillet
Fillet Size	3/8 in	Fillet Size	3/8 in
Length	60 in	Bevel Depth	3/8 in
Load Angle	45 deg.		
Weld - Gusset to Base Plate			
FEXX	70 ksi		
Weld Type	Double Bevel+Fillet		
Fillet Size	9/16 in		
Bevel Depth	9/16 in		
Gap	0 in		
Notch	0.75 in		
Pipe/Tube Welded to Base/Footpad?	Yes		
Fillet Size	1/2 in		

Monopole Base Plate Connection

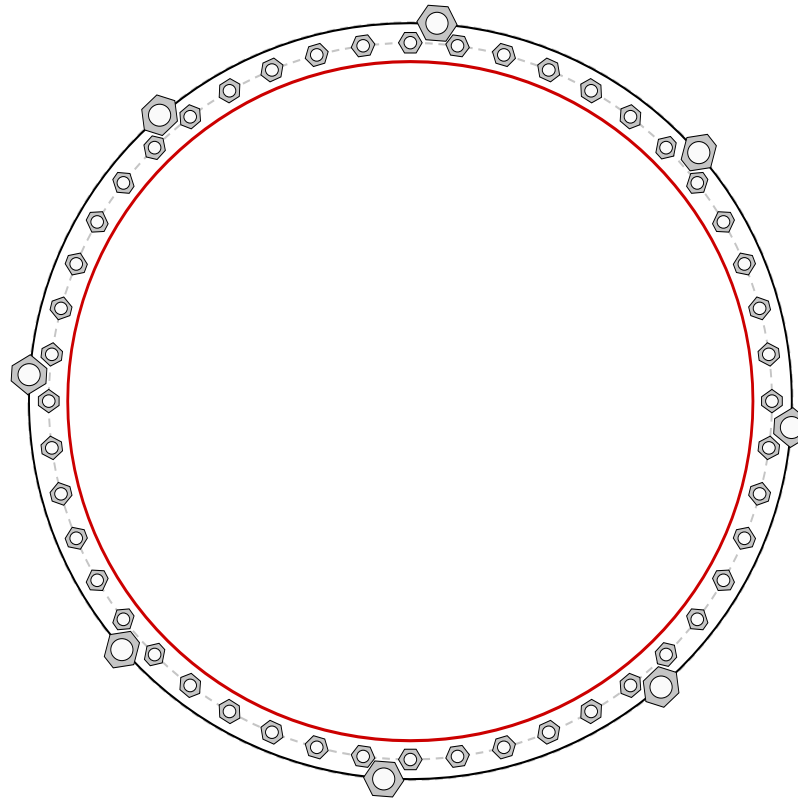


Site Info	
BU #	822765
Site Name	xford/ I-95/ X55/ Dtn1
Order #	479795, Rev. 0

Analysis Considerations	
TIA-222 Revision	H
Grout Considered:	No
l_{ar} (in)	2

Applied Loads	
Moment (kip-ft)	2593.31
Axial Force (kips)	48.24
Shear Force (kips)	31.62

*TIA-222-H Section 15.5 Applied



Connection Properties		Analysis Results		
Anchor Rod Data		Anchor Rod Summary (units of kips, kip-in)		
GROUP 1: (48) 1" \emptyset bolts (A687 N; Fy=105 ksi, Fu=125 ksi) on 57" BC		GROUP 1:		
GROUP 2: (8) 1-3/4" \emptyset bolts (A193 Gr. B7 N; Fy=105 ksi, Fu=125 ksi) on 60.25" BC		$Pu_c = 29.72$	$\phi Pn_c = 63.63$	Stress Rating
Base Plate Data		$Vu = 0.66$	$\phi Vn = 19.09$	52.2%
60.125" OD x 1" Plate (A36; Fy=36 ksi, Fu=58 ksi)		$Mu = 0.86$	$\phi Mn = 10.67$	Pass
Stiffener Data		GROUP 2:		
N/A		$Pu_c = 95.17$	$\phi Pn_c = 199.5$	Stress Rating
Pole Data		$Vu = 0$	$\phi Vn = 59.85$	45.4%
54" x 0.5875" round pole (A53-B-42; Fy=42 ksi, Fu=63 ksi)		$Mu = 0$	$\phi Mn = 59.26$	Pass
		Base Plate Summary		
		Max Stress (ksi):	32.67	(Flexural)
		Allowable Stress (ksi):	32.4	
		Stress Rating:	96.0%	Pass

Drilled Pier Foundation

BU #: 822765
 Site Name: Branford/ I-95/ X55/ Dtr
 Order Number: 479795, Rev. 0

TIA-222 Revisor: H
 Tower Type: Monopole



Applied Loads		
	Comp.	Uplift
Moment (kip-ft)	2593	
Axial Force (kips)	48	
Shear Force (kips)	32	

Material Properties		
Concrete Strength, f'c:	4	ksi
Rebar Strength, Fy:	60	ksi

Pier Design Data		
Depth	21	ft
Ext. Above Grade	0.5	ft
Pier Section 1		
<i>From 0.5' above grade to 12.547' below grade</i>		
Pier Diameter	6	ft
Rebar Quantity	24	
Rebar Size	9	
Clear Cover to Ties	3	in
Tie Size	5	
Rebar Quantity	2	
Rebar Size	14	
Rebar Cage Diameter	60.25	in
Pier Section 2		
<i>From 12.547' below grade to 21' below grade</i>		
Pier Diameter	6	ft
Rebar Quantity	24	
Rebar Size	9	
Clear Cover to Ties	3	in
Tie Size	5	

Analysis Results		
Soil Lateral Capacity		
	Compression	Uplift
D _{v=0} (ft from TOC)	5.79	-
Soil Safety Factor	2.01	-
Max Moment (kip-ft)	2750.06	-
Rating*	62.9%	-
Soil Vertical Capacity		
	Compression	Uplift
Skin Friction (kips)	366.71	-
End Bearing (kips)	254.47	-
Weight of Concrete (kips)	109.42	-
Total Capacity (kips)	621.18	-
Axial (kips)	157.42	-
Rating*	24.1%	-
Reinforced Concrete Capacity		
	Compression	Uplift
Critical Depth (ft from TOC)	5.77	-
Critical Moment (kip-ft)	2750.05	-
Critical Moment Capacity	3769.03	-
Rating*	69.5%	-
Soil Interaction Rating*		62.9%
Structural Foundation Rating*		69.5%

Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>

*Rating per TIA-222-H Section 15.5

Soil Profile			
Groundwater Depth	N/A	ft	# of Layers
			3

Layer	Top (ft)	Bottom (ft)	Thickness (ft)	γ _{soil} (pcf)	γ _{concrete} (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	3.333	3.333	120	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
2	3.333	11	7.667	120	150		34	0.979	0.979				20	Cohesionless
3	11	21	10	120	150		33	1.843	1.843			12	100	Cohesionless

Exhibit E

Mount Analysis



Date: May 9, 2019

Charles McGuirt
Crown Castle
2000 Corporate Drive.
Canonsburg, NC 15317
(704) 405-6607

B+T Group
1717 S. Boulder, Suite 300
Tulsa, OK 74119
(918) 587-4630
btwo@btgrp.com

Subject: Mount Analysis Report

Carrier Designation: T-Mobile Equipment Change-Out
Carrier Site Number: CT11025B
Carrier Site Name: Brandford/I-95/X55/Dtn1

Crown Castle Designation: **Crown Castle BU Number:** 822765
Crown Castle Site Name: Branford/ I-95/ X55/ Dtn1
Crown Castle JDE Job Number: 559227
Crown Castle Order Number: 479795, Rev.0

Engineering Firm Designation: **B+T Group Report Designation:** 101126.003.01

Site Data: 10 Sylvania St., Branford, CT, New Haven, 06405
Latitude 41° 17' 38.16" Longitude -72° 47' 8.54"

Structure Information: **Tower Height & Type:** 125 ft. Monopole
Mount Elevation: 122 ft.
Mount Type: 17 ft. Platform Mount

Dear Mr. McGuirt,

B+T Group is pleased to submit this "Mount Analysis Report" to determine the structural integrity of T-Mobile's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount's stress level. Based on our analysis we have determined the stress level to be:

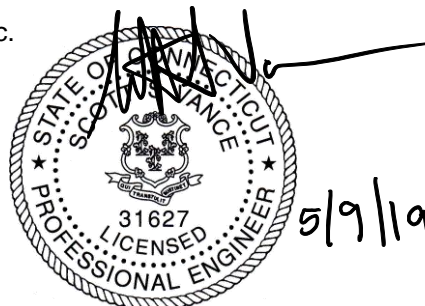
Platform Mount

Sufficient
*See Section 4.1 of this report for the structural modifications required in order for the mount to support the loading listed in Table 1

This analysis utilizes an ultimate 3-second gust wind speed of 130 mph as required by the 2015 International Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount structural analysis prepared by: Siva Tellakula, E.I.T.

Respectfully submitted by: B&T Engineering, Inc.
COA: PEC.0001564 Expires: 02/10/2020



Scott S. Vance, P.E.

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6) APPENDIX B

Software Input Calculations

7) APPENDIX C

Software Analysis Output

1) INTRODUCTION

This is a 17' Platform Mount, Mapped by Pier Structural Engineering Corp.

2) ANALYSIS CRITERIA

Building Code:	2015 IBC
TIA-222 Revision:	TIA-222-H
Risk Category:	II
Ultimate Wind Speed:	130 mph
Exposure Category:	B
Topographic Factor at Base:	1
Topographic Factor at Mount:	1
Ice Thickness:	1.5 in
Wind Speed with Ice:	50 mph
Seismic S_s:	0.179
Seismic S_1:	0.061
Live Loading Wind Speed:	30 mph
Man Live Load at Mid/End-Points:	250 lb.
Man Live Load at Mount Pipes:	500 lb.

Table 1 - Proposed Equipment Configuration

Mount Centerline (ft.)	Antenna Centerline (ft.)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
122	122	3	Ericsson	AIR 32 B2A/B66AA	17' Platform Mount
		3	Ericsson	AIR 21 B2A B4P	
		3	RFS	APXVAARR24_43-U-NA20	
		3	Ericsson	KRY 112 144/1	
		3	Ericsson	RADIO 4449 B12/B71	

3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

Document	Remarks	Reference	Source
CCI Order	Existing Loading Proposed Loading	Date: 04/30/2019	Crown Castle
Mount Mapping	Pier Structural Engineering Corp.	Date: 04/17/2019	On File

3.1) Analysis Method

RISA-3D (Version 17.0.2), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed by B+T Group, was used to calculate wind loading on all appurtenances, dishes and mount members for various loading cases. Selected output from the analysis is included in Appendix B "Software Input Calculations".

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision C).

3.2) Assumptions

1. The mount was properly fabricated and installed in accordance with its original design and manufacturer's specifications.
2. The mount has been maintained in accordance with the manufacturer's specifications and is free of damage.
3. The configuration of antennas, mounts, and other appurtenances are as specified in Table-1.
4. All mount components have been assumed to be in sufficient condition to carry their full design capacity for the analysis.
5. Mount areas and weights are determined from field measurements, standard material properties, and/or manufacturer product data.
6. Serviceability with respect to antenna twist, tilt, roll or lateral translation is not checked and is left to the carrier or tower owner to ensure conformance.
7. All prior structural modifications, if any are assumed to be correctly installed and fully effective.
8. All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
9. The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
10. The following material grades were assumed (Unless Noted Otherwise):
 - (a) Connection Bolts : ASTM A325
 - (b) Steel Pipe : ASTM A53 (GR. 35)
 - (c) HSS (Round) : ASTM 500 (GR. B-42)
 - (d) HSS (Rectangular) : ASTM 500 (GR. B-46)
 - (e) Channel : ASTM A36 (GR. 36)
 - (f) Steel Solid Rod : ASTM A36 (GR. 36)
 - (g) Steel Plate : ASTM A36 (GR. 36)
 - (h) Steel Angle : ASTM A36 (GR. 36)
 - (i) UNISTRUT : ASTM A570 (GR. 33)

This analysis may be affected if any assumptions are not valid or have been made in error. B+T Group should be notified to determine the effect on the structural integrity of the antenna mounting system.

4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity (Platform Mount)

Notes	Component	Critical Member	Centerline (ft.)	% Capacity	Pass / Fail
1,2	Main Horizontals	M1	122	76.4	Pass
	Handrails	M14	122	54.8	Pass
	Support Tubes	M11	122	39.5	Pass
	Solid Rods	M92	122	32.6	Pass
	Cold Formed Support	M126	122	89.9	Pass
	Connection Plates	M2	122	24.1	Pass
	Connection Angles	M16	122	44.5	Pass
	Mount Pipes	M25	122	92.7	Pass

Structure Rating (max from all components) =	92.7%
---	--------------

Notes:

- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) All sectors are typical

4.1) Recommendations

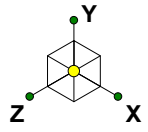
The mount has sufficient capacity to carry the proposed loading configuration. In order for the results of the analysis to be considered valid, the structural modification listed below must be completed.

1. Install new horizontal pipes 2" Std. x 17'-0" Long with SitePro1 Handrail Connection Assembly AHCP, 2'-0" above the main horizontals
2. Replace existing mount pipe at Position 1 with new 2.0" X-Strong. Pipe x 7'-0" Long

No further structural modifications are required at this time, provided that the above-listed structural modifications are implemented.

(Refer to SK-1 of Appendix-A for modification details)

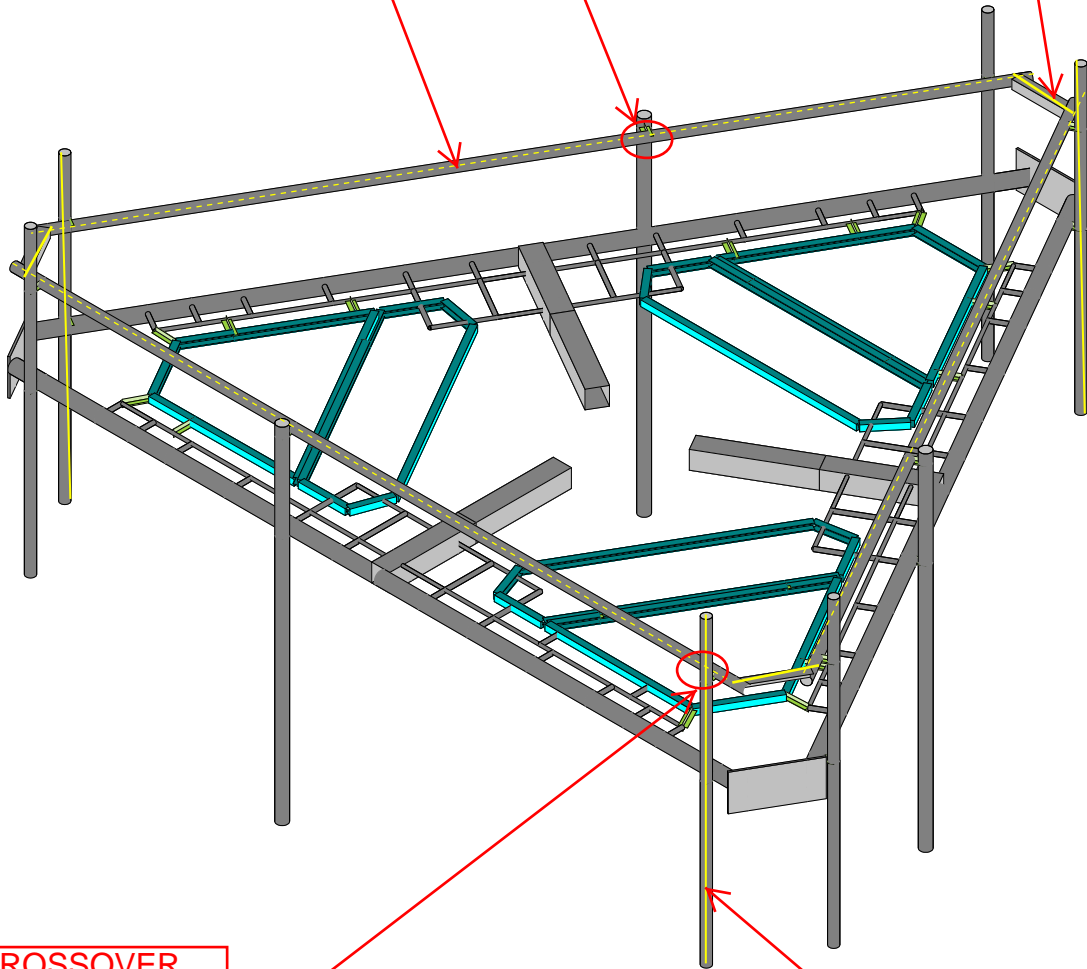
APPENDIX A
WIRE FRAME AND RENDERED MODELS



NEW 2" STD PIPE x 17'-0" LONG
TYP. 3 PLACES

NEW CROSSOVER
PLATE KIT
(SITEPRO #SCX2-K)
TYP.3 PLACES

NEW HANDRAIL
CORNER KIT,
SITEPRO #AHCP

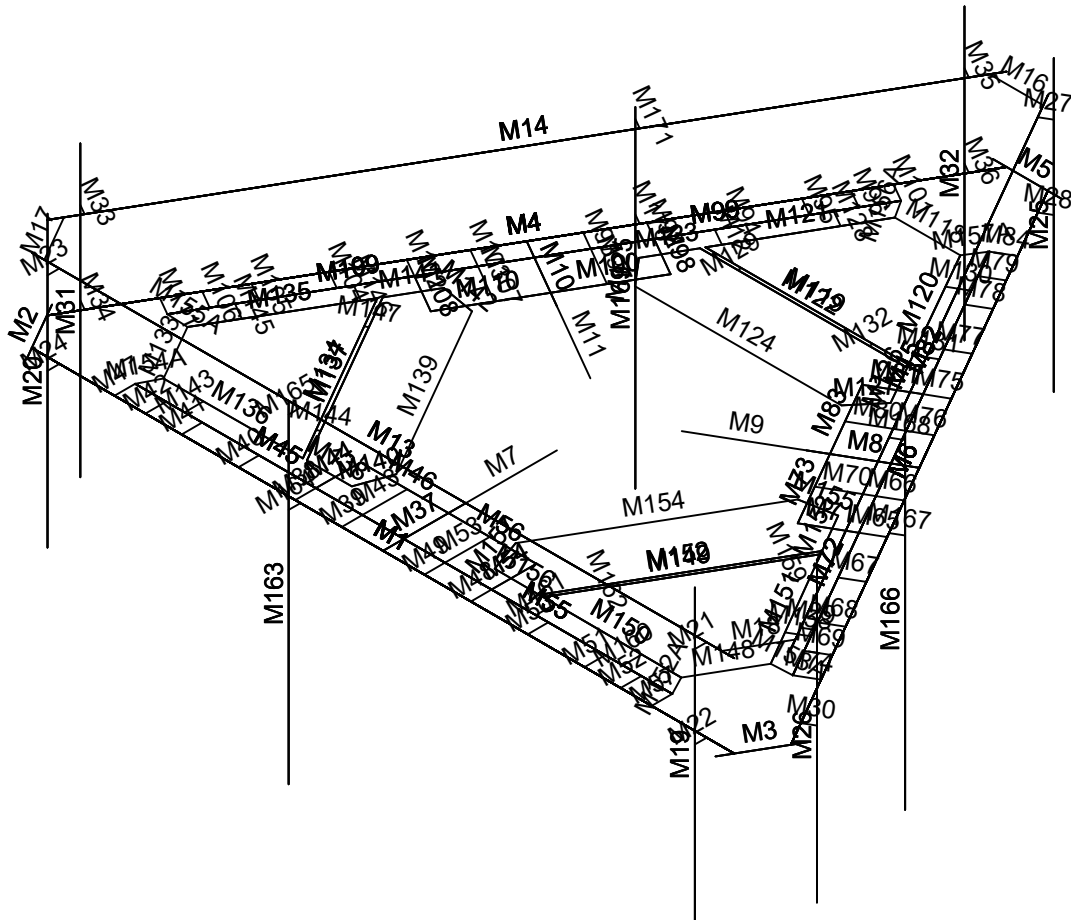
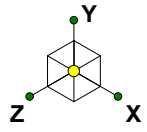


NEW CROSSOVER
PLATE KIT
(SITEPRO #SCX1-K)
TYP.6 PLACES

REPLACE EXISTING AT POS. 1 IN
ALL SECTORS WITH
NEW 2" X-STRONG PIPE (2.38" x
0.218" thick) x 7'-0" LONG
TYP. 3 PLACES

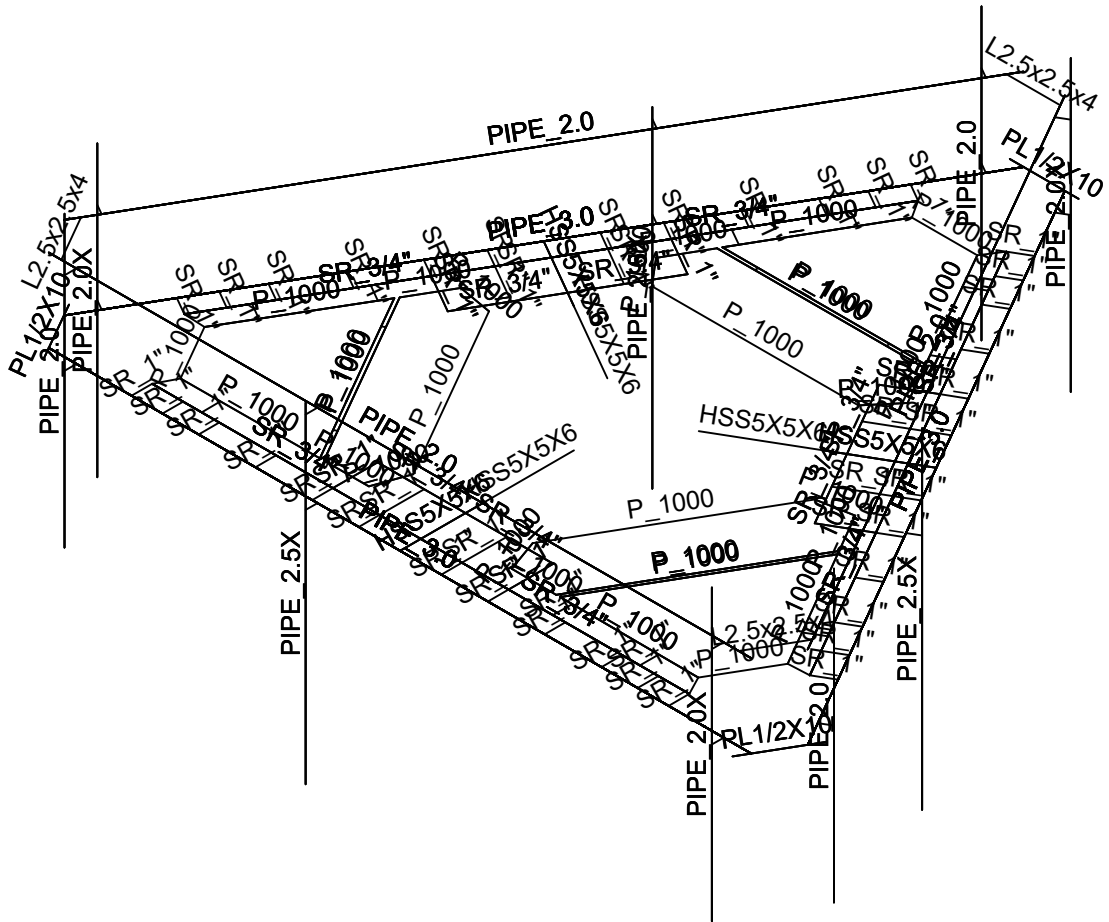
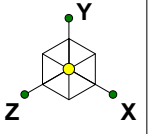
Envelope Only Solution

B+T Group	822765 - Branford/ I-95/ X55/ Dtn1	SK - 1
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101126.003.01		101126_003_01_Branford I-95 X5...



Envelope Only Solution

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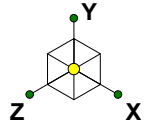


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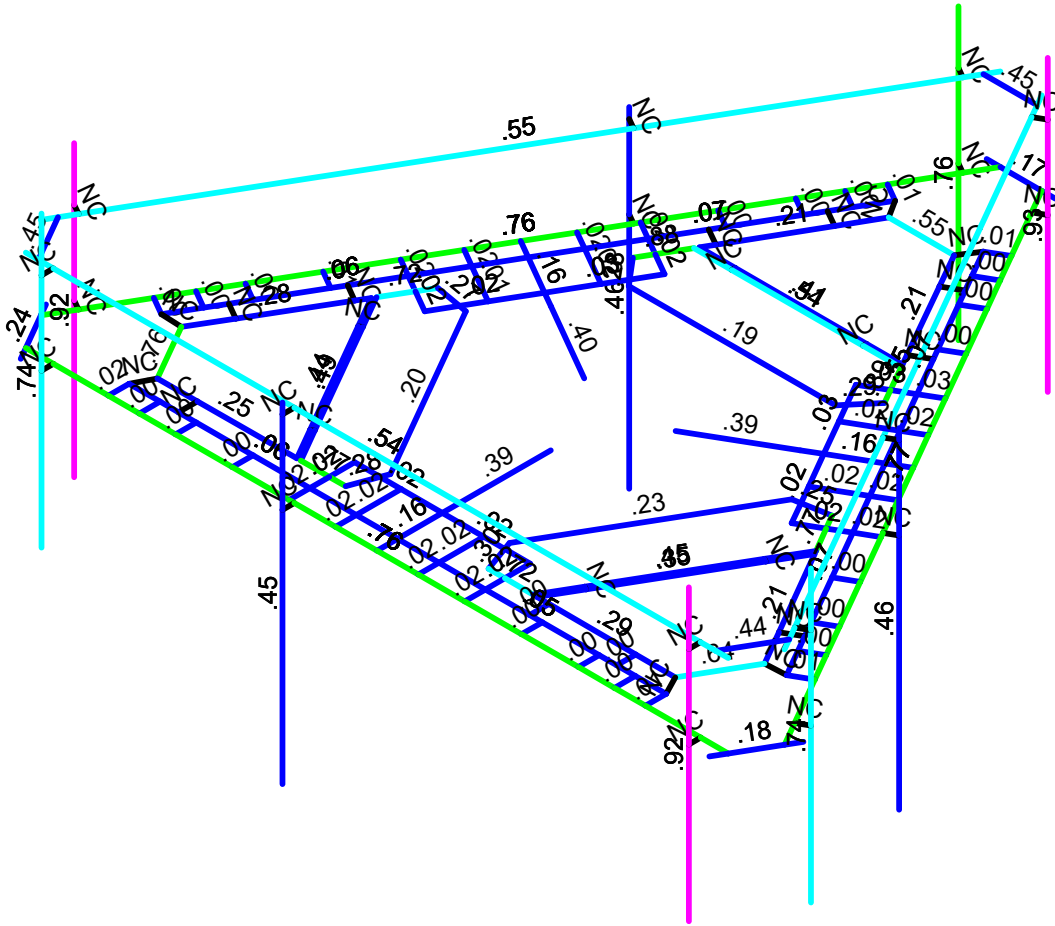
B+T Group
AS
101126.003.01

822765 - Branford/ I-95/ X55/ Dtn1

SK - 3
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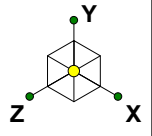


Code Check (Env)	
Black	No Calc
Red	> 1.0
Magenta	.90-1.0
Green	.75-.90
Cyan	.50-.75
Blue	0-.50

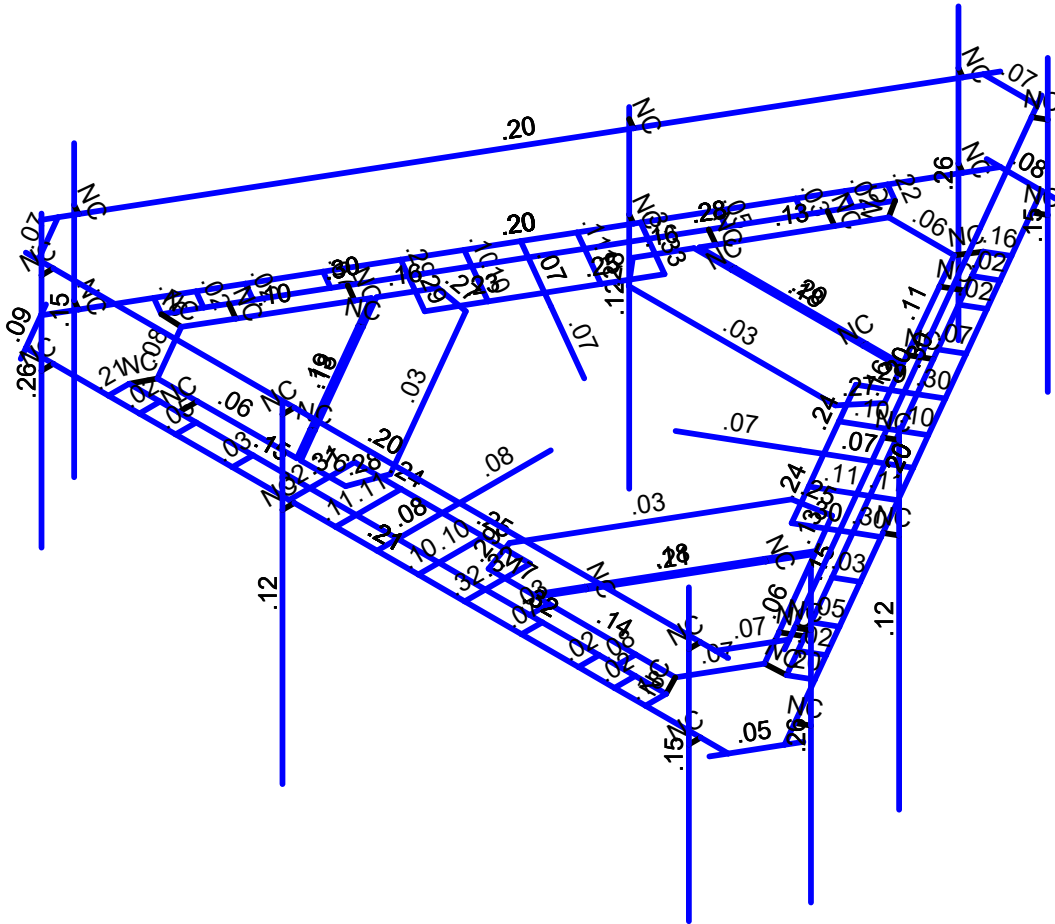


Member Code Checks Displayed (Enveloped)
Envelope Only Solution

B+T Group	822765 - Branford/ I-95/ X55/ Dtn1	SK - 4
AS		May 9, 2019 at 5:32 PM
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Shear Check (Env)	
	No Calc
	> 1.0
	.90-1.0
	.75-.90
	.50-.75
	0-.50



Member Shear Checks Displayed (Enveloped)
Envelope Only Solution

B+T Group	822765 - Branford/ I-95/ X55/ Dtn1	SK - 5
AS		May 9, 2019 at 5:32 PM
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APPENDIX B
SOFTWARE INPUT CALCULATIONS

PROJECT	101126.003.01 - Branford, CT	SP
SUBJECT	Platform Mount Mount Analysis	
DATE	05/09/19	PAGE 1 OF 1



B+T Group
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 (918) 587-4630

INPUT

[REF: ANSI/TIA-222-H]

Tower Type	:	MP	
Tower Height	:	125	ft
Mount Elevation	:	122	ft
Antenna Elevation	:	122	ft
Crest Height	:	0	ft
Risk Category	:	II	[Table 2-1]
Exposure Category	:	B	[Sec. 2.6.5.1.2]
Topography Category	:	1	[Sec. 2.6.6.2]
Wind Velocity V	:	130	mph [Annex B]
Ice wind Velocity V _i	:	50	mph [Annex B]
Service Velocity V _s	:	30	mph [Annex B]
Base Ice thickness t _i	:	1.5	in [Annex B]
Ground Elevation z _s	:	56.16	ft [Sec. 2.6.8]

ANTENNAS

	Manufacturer	Model	Height (in)	Front Width (in)	Side Width (in)	Weight (lbs)	Shape	Quantity	Location (%)
Mount Pipe M19									
	Ericsson	AIR 21 B2A B4P	56.00	12.10	7.87	91.50	Flat	0.5	10
	Ericsson	AIR 21 B2A B4P	56.00	12.10	7.87	91.50	Flat	0.5	70
	Ericsson	RADIO 4449 B12/B71	14.95	13.19	9.25	75.00	Flat	1	30
Mount Pipe M163									
	Ericsson	AIR 32 B2A/B66AA	56.60	12.90	8.70	132.20	Flat	0.5	20
	Ericsson	AIR 32 B2A/B66AA	56.60	12.90	8.70	132.20	Flat	0.5	80
Mount Pipe M20									
	RFS	APXVAARR24_43-U-NA20	95.90	24.00	8.70	128.00	Flat	0.5	10
	RFS	APXVAARR24_43-U-NA20	95.90	24.00	8.70	128.00	Flat	0.5	90
	Ericsson	KRY 112 144/1	7.00	6.00	3.00	11.00	Flat	1	30
Mount Pipe M31									
	Ericsson	AIR 21 B2A B4P	56.00	12.10	7.87	91.50	Flat	0.5	10
	Ericsson	AIR 21 B2A B4P	56.00	12.10	7.87	91.50	Flat	0.5	70
	Ericsson	RADIO 4449 B12/B71	14.95	13.19	9.25	75.00	Flat	1	30
Mount Pipe M169									
	Ericsson	AIR 32 B2A/B66AA	56.60	12.90	8.70	132.20	Flat	0.5	20
	Ericsson	AIR 32 B2A/B66AA	56.60	12.90	8.70	132.20	Flat	0.5	80
Mount Pipe M32									
	RFS	APXVAARR24_43-U-NA20	95.90	24.00	8.70	128.00	Flat	0.5	10
	RFS	APXVAARR24_43-U-NA20	95.90	24.00	8.70	128.00	Flat	0.5	90
	Ericsson	KRY 112 144/1	7.00	6.00	3.00	11.00	Flat	1	30

PROJECT	101126.003.01 - Branford, CT	SP
SUBJECT	Platform Mount Mount Analysis	
DATE	05/09/19	PAGE 1 OF 1



B+T Group
 1717 S. Boulder, Suite 300
 Tulsa, OK 74119
 (918) 587-4630

B+T GRP

INPUT

[REF: ANSI/TIA-222-H]

Member Number	Section Set	Wind Projection (in)	Length (in)	Perimeter (in)	Shape	D _c (in)
M1	MF-H1	3.50	204.00	11.00	Round	3.50
M2	F1-C1	10.00	20.00	21.00	Flat	10.01
M3	F1-C1	10.00	20.00	21.00	Flat	10.01
M4	MF-H1	3.50	204.00	11.00	Round	3.50
M5	F1-C1	10.00	20.00	21.00	Flat	10.01
M6	MF-H1	3.50	204.00	11.00	Round	3.50
M7	F1-S1	5.00	26.50	20.00	Flat	7.07
M8	F1-S1	5.00	24.00	20.00	Flat	7.07
M9	F1-S1	5.00	26.50	20.00	Flat	7.07
M10	F1-S1	5.00	24.00	20.00	Flat	7.07
M11	F1-S1	5.00	26.50	20.00	Flat	7.07
M13	Handrail	2.38	204.00	7.48	Round	2.38
M14	Handrail	2.38	204.00	7.48	Round	2.38
M15	Handrail	2.38	204.00	7.48	Round	2.38
M16	F1-CA1	2.50	15.62	10.00	Flat	3.54
M17	F1-CA1	2.50	15.62	10.00	Flat	3.54
M18	F1-CA1	2.50	15.62	10.00	Flat	3.54
M19	MF-P3	2.38	84.00	7.48	Round	2.38
M20	MF-P1	2.38	84.00	7.48	Round	2.38
M25	MF-P3	2.38	84.00	7.48	Round	2.38
M26	MF-P1	2.38	84.00	7.48	Round	2.38
M31	MF-P3	2.38	84.00	7.48	Round	2.38
M32	MF-P1	2.38	84.00	7.48	Round	2.38
M37	F1-S1	5.00	24.00	20.00	Flat	7.07
M38	F1-SR1	1.00	6.00	3.14	Round	1.00
M39	F1-SR1	1.00	6.00	3.14	Round	1.00
M40	F1-SR1	1.00	6.00	3.14	Round	1.00
M41	F1-SR1	1.00	6.00	3.14	Round	1.00
M42	F1-SR1	1.00	6.00	3.14	Round	1.00
M43	F1-SR1	1.00	13.00	3.14	Round	1.00
M44	F1-SR1	1.00	13.00	3.14	Round	1.00
M45	F1-SR2	0.75	78.00	2.36	Round	0.75
M46	F1-SR2	0.75	25.50	2.36	Round	0.75
M47	F1-SR1	1.00	6.00	3.14	Round	1.00
M48	F1-SR1	1.00	6.00	3.14	Round	1.00
M49	F1-SR1	1.00	6.00	3.14	Round	1.00
M50	F1-SR1	1.00	6.00	3.14	Round	1.00
M51	F1-SR1	1.00	6.00	3.14	Round	1.00
M52	F1-SR1	1.00	6.00	3.14	Round	1.00
M53	F1-SR1	1.00	13.00	3.14	Round	1.00
M54	F1-SR1	1.00	13.00	3.14	Round	1.00
M55	F1-SR2	0.75	78.00	2.36	Round	0.75
M56	F1-SR2	0.75	25.50	2.36	Round	0.75
M57	F1-SR1	1.00	6.00	3.14	Round	1.00
M65	F1-SR1	1.00	6.00	3.14	Round	1.00
M66	F1-SR1	1.00	6.00	3.14	Round	1.00
M67	F1-SR1	1.00	6.00	3.14	Round	1.00
M68	F1-SR1	1.00	6.00	3.14	Round	1.00
M69	F1-SR1	1.00	6.00	3.14	Round	1.00
M70	F1-SR1	1.00	13.00	3.14	Round	1.00
M71	F1-SR1	1.00	13.00	3.14	Round	1.00
M72	F1-SR2	0.75	78.00	2.36	Round	0.75

M73	F1-SR2	0.75	25.50	2.36	Round	0.75
M74	F1-SR1	1.00	6.00	3.14	Round	1.00
M75	F1-SR1	1.00	6.00	3.14	Round	1.00
M76	F1-SR1	1.00	6.00	3.14	Round	1.00
M77	F1-SR1	1.00	6.00	3.14	Round	1.00
M78	F1-SR1	1.00	6.00	3.14	Round	1.00
M79	F1-SR1	1.00	6.00	3.14	Round	1.00
M80	F1-SR1	1.00	13.00	3.14	Round	1.00
M81	F1-SR1	1.00	13.00	3.14	Round	1.00
M82	F1-SR2	0.75	78.00	2.36	Round	0.75
M83	F1-SR2	0.75	25.50	2.36	Round	0.75
M84	F1-SR1	1.00	6.00	3.14	Round	1.00
M92	F1-SR1	1.00	6.00	3.14	Round	1.00
M93	F1-SR1	1.00	6.00	3.14	Round	1.00
M94	F1-SR1	1.00	6.00	3.14	Round	1.00
M95	F1-SR1	1.00	6.00	3.14	Round	1.00
M96	F1-SR1	1.00	6.00	3.14	Round	1.00
M97	F1-SR1	1.00	13.00	3.14	Round	1.00
M98	F1-SR1	1.00	13.00	3.14	Round	1.00
M99	F1-SR2	0.75	78.00	2.36	Round	0.75
M100	F1-SR2	0.75	25.50	2.36	Round	0.75
M101	F1-SR1	1.00	6.00	3.14	Round	1.00
M102	F1-SR1	1.00	6.00	3.14	Round	1.00
M103	F1-SR1	1.00	6.00	3.14	Round	1.00
M104	F1-SR1	1.00	6.00	3.14	Round	1.00
M105	F1-SR1	1.00	6.00	3.14	Round	1.00
M106	F1-SR1	1.00	6.00	3.14	Round	1.00
M107	F1-SR1	1.00	13.00	3.14	Round	1.00
M108	F1-SR1	1.00	13.00	3.14	Round	1.00
M109	F1-SR2	0.75	78.00	2.36	Round	0.75
M110	F1-SR2	0.75	25.50	2.36	Round	0.75
M111	F1-SR1	1.00	6.00	3.14	Round	1.00
M160	F1-SR1	1.00	5.48	3.14	Round	1.00
M161	F1-SR1	1.00	5.60	3.14	Round	1.00
M163	MF-P2	2.88	96.00	9.05	Round	2.88
M166	MF-P2	2.88	96.00	9.05	Round	2.88
M169	MF-P2	2.88	96.00	9.05	Round	2.88
M118	CF1	1.63	19.00	9.75	Flat	2.30
M119	CF1	1.63	59.00	9.75	Flat	2.30
M120	CF1	1.63	40.31	9.75	Flat	2.30
M121	CF1	1.63	40.31	9.75	Flat	2.30
M122	CF1	1.63	60.14	9.75	Flat	2.30
M123	CF1	1.63	12.98	9.75	Flat	2.30
M124	CF1	1.63	60.00	9.75	Flat	2.30
M125	CF1	1.63	10.10	9.75	Flat	2.30
M126	CF1	1.63	12.98	9.75	Flat	2.30
M127	CF1	1.63	10.10	9.75	Flat	2.30
M133	CF1	1.63	19.00	9.75	Flat	2.30
M134	CF1	1.63	59.00	9.75	Flat	2.30
M135	CF1	1.63	40.31	9.75	Flat	2.30
M136	CF1	1.63	40.31	9.75	Flat	2.30
M137	CF1	1.63	60.14	9.75	Flat	2.30
M138	CF1	1.63	12.98	9.75	Flat	2.30
M139	CF1	1.63	60.00	9.75	Flat	2.30
M140	CF1	1.63	10.10	9.75	Flat	2.30
M141	CF1	1.63	12.98	9.75	Flat	2.30
M142	CF1	1.63	10.10	9.75	Flat	2.30
M148	CF1	1.63	19.00	9.75	Flat	2.30
M149	CF1	1.63	59.00	9.75	Flat	2.30
M150	CF1	1.63	40.31	9.75	Flat	2.30
M151	CF1	1.63	40.31	9.75	Flat	2.30
M152	CF1	1.63	60.14	9.75	Flat	2.30
M153	CF1	1.63	12.98	9.75	Flat	2.30
M154	CF1	1.63	60.00	9.75	Flat	2.30
M155	CF1	1.63	10.10	9.75	Flat	2.30
M156	CF1	1.63	12.98	9.75	Flat	2.30
M157	CF1	1.63	10.10	9.75	Flat	2.30

APPENDIX C
SOFTWARE ANALYSIS OUTPUT

A Ya Vy'Df]a Ufm8 UU'f7 cb]bi YXL

Saa\	U]ãc	R]ãc	S]ãc	Ü[æ^G^D Ü^&ç) DÜcç^	V]^(Ô•ã) Áçc	Tæ]æ	Ô•ã) ÁÜ]^(
F F	T FÍ F	þ G H	þ G H		Ø FÉ Ü F	Ô æ	Ó OÉ	OÉ H OÍ É	V] ææ
F I G	T FÍ G	þ G J	þ G €		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F I H	T FÍ H	þ G F	þ G G		T Ø É U G	Ô [] { }	Ü a ^	OÉ H OÍ É O	V] ææ
F I I	T FÍ I	þ G H	þ G I		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F I Í	T FÍ Í	þ G Í	þ G Í		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F I Î	T FÍ Î	þ G Î	þ G Î		T Ø É U G	Ô [] { }	Ü a ^	OÉ H OÍ É O	V] ææ
F I Ï	T FÍ Ï	þ G J	þ G €		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F I Ì	T FÍ Ì	þ G F	þ G G		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F I J	T FÍ J	þ G H	þ G I		T Ø É U G	Ô [] { }	Ü a ^	OÉ H OÍ É O	V] ææ
F Í €	T FÍ €	þ G Í	þ G Í		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í F	T FÍ F	þ G Î	þ G Î		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í G	T FÍ G	þ G G	þ J		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í H	T FÍ H	þ G H	þ F G		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í I	T FÍ I	þ G H	þ I G		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í Í	T FÍ Í	þ G F	þ F I		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í Î	T FÍ Î	þ F H	þ F Î		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ
F Í Ï	T FÍ Ï	þ F F	þ F H		Ü Ø Ø	þ] ^	þ] ^	Ü Ø Ø	V] ææ

6 Uq]W@ UX'7 UqYg

F	ÓSÓ/Ô•&ãç)	Oæ^[I^	YÁO]æç	YÁO]æç	ZÁO]æç	R]ãc	U]ãc	Öäã^çã OÉ^ç] ÆÜ]æ^ç	J
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I	É Á a á A] A^	Y S Z					Í	FFJ	
Í	J É Á a á A] A^	Y S Y					Í	FFJ	
Î	É Á a á A] A^ çæ	Y S Z					Í	FFJ	
Ï	J É Á a á A] A^ çæ	Y S Y					Í	FFJ	
ì	A^	U S F					Í	FFJ	J
J	Sã^ Á] ææ Ae	S S				F			
F €	Sã^ Á] ææ Á	S S				F			
F F	Sã^ Á] ææ &	S S				F			
F G	Sã^ Á] ææ Á	S S							
F H	T ææ Ö S S F	S S					F		
F I	T ææ Ö S S G	S S					F		
F Í	T ææ Ö S S H	S S					F		
F Î	T ææ Ö S S Á	S S					F		
F Ï	T ææ Ö S S Á	S S					F		
F J	T ææ Ö S S Á	S S					F		
F €	T ææ Ö S S Á	S S					F		
F F	T ææ Ö S S Á	S S					F		
F G	T ææ Ö S S Á €	S S					F		
F H	T ææ Ö S S Á F	S S					F		
F I	T ææ Ö S S Á G	S S					F		
F Î	T ææ Ö S S Á H	S S					F		
F Ï	T ææ Ö S S Á I	S S					F		
F J	T ææ Ö S S Á I	S S					F		
G	Ó S Ó Á (a) a) OÉ^æç ææ	þ] ^						Í H	
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A Ya Vyf'Dc]bhi@UXg'f6 @ ' : - \$'K]bX!'Bc ÷WV'L'f7 c b]jbi YXL

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Fì	THF	Ý	ĚĚ I	Á Ĥ
FJ	THF	Ý	€	€
Q€	THF	Ý	€	€
QF	TFÍJ	Ý	ĚĚJF	Á Ç
QG	TFÍJ	Ý	ĚĚJF	Á Ě
QH	TFÍJ	Ý	€	€
Q	TFÍJ	Ý	€	€
Q̇	TFÍJ	Ý	€	€
Q̈	THG	Ý	ĚĚ G	Á F
Q̉	THG	Ý	ĚĚ G	Á J
Q̊	THG	Ý	ĚĚ	Á Ĥ
GU	THG	Ý	€	€
H€	THG	Ý	€	€
HF	TĜ	Ý	ĚĚ H	Á F
HG	TĜ	Ý	ĚĚ H	Á Ě
HH	TĜ	Ý	ĚĚ I	Á Ĥ
H	TĜ	Ý	€	€
Ḣ	TĜ	Ý	€	€
Ḧ	TFÍĪ	Ý	ĚĚJF	Á Ç
H̉	TFÍĪ	Ý	ĚĚJF	Á Ě
H̊	TFÍĪ	Ý	€	€
HJ	TFÍĪ	Ý	€	€
I€	TFÍĪ	Ý	€	€
IF	TĜ	Ý	ĚĚ G	Á F
IG	TĜ	Ý	ĚĚ G	Á J
IH	TĜ	Ý	ĚĚ	Á Ĥ
I	TĜ	Ý	€	€
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H	TFJ	Z	ĚĚJ	Á Ĥ
I	TFJ	Z	€	€
í	TFJ	Z	€	€
î	TFÍH	Z	ĚĚJF	Á Ç
ï	TFÍH	Z	ĚĚJF	Á Ě
ì	TFÍH	Z	€	€
J	TFÍH	Z	€	€
F€	TFÍH	Z	€	€
FF	TÇ	Z	ĚĚ Í	Á F
FG	TÇ	Z	ĚĚ Í	Á J
FH	TÇ	Z	ĚĚG	Á Ĥ
FI	TÇ	Z	€	€
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F̉	THF	Z	ĚĚJ	Á Ĥ
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A Ya Vyf'8]gfv]vi hYX' @UXg'f6 @ '&.' \$'K]bX!'Bc =MYŁ

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H	TH	Z	ÆHJ	ÆHJ	€	€
I	TI	Z	ÆFH	ÆFH	€	€
Í	TÍ	Z	ÆHJ	ÆHJ	€	€
Ī	TĪ	Z	ÆFH	ÆFH	€	€
İ	Tİ	Z	ÆGF	ÆGF	€	€
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J	TJ	Z	ÆGF	ÆGF	€	€
F€	TFE	Z	ÆGF	ÆGF	€	€
FF	TFE	Z	ÆGF	ÆGF	€	€
FG	TFH	Z	ÆEJ	ÆEJ	€	€
FH	TFI	Z	ÆEJ	ÆEJ	€	€
FI	TFÍ	Z	ÆEJ	ÆEJ	€	€
FÍ	TFĪ	Z	ÆFF	ÆFF	€	€
FĪ	TFİ	Z	ÆFF	ÆFF	€	€
Fì	TFì	Z	ÆFF	ÆFF	€	€
Fì	TFJ	Z	ÆEJ	ÆEJ	€	€
FJ	TGE	Z	ÆEJ	ÆEJ	€	€

A Ya Vyf'8]gfh]Vi hYX' @ UXg'f6 @' " : - \$ 'K]bX'! 'Bc =WYLF'7 c bh]bi YXL

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FÉ	T FHJ	Y	FFFF	FFFF	€	€
FÉ	T FI €	Y	FFÉ	FFÉ	€	€
FÉ	T FIF	Y	FFÉ	FFÉ	€	€
FÉ	T FIG	Y	FFÉ	FFÉ	€	€
FFÉ	T FI Ì	Y	FFÉ	FFÉ	€	€
FFF	T FI J	Y	FFFF	FFFF	€	€
FFG	T FI €	Y	FFÉ	FFÉ	€	€
FFH	T FI F	Y	FFÉ	FFÉ	€	€
FFI	T FI G	Y	FFFF	FFFF	€	€
FFÍ	T FI H	Y	FFÉ	FFÉ	€	€
FFÎ	T FI Ì	Y	FFFF	FFFF	€	€
FFÏ	T FI Í	Y	FFÉ	FFÉ	€	€
FFÏ	T FI Î	Y	FFÉ	FFÉ	€	€
FFJ	T FI Ì	Y	FFÉ	FFÉ	€	€

A Ya Vyf'8]gfh]Vi hYX' @ UXg'f6 @' (: - \$ 'K]bX'! =WYLF

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F	TF	Z	FFEG	FFEG	€	€
G	TG	Z	FFÉJ	FFÉJ	€	€
H	TH	Z	FFÉJ	FFÉJ	€	€
I	TI	Z	FFEG	FFEG	€	€
Í	TÍ	Z	FFÉJ	FFÉJ	€	€
Î	TÎ	Z	FFEG	FFEG	€	€
Ï	TÏ	Z	FFÉ	FFÉ	€	€
Ì	TÌ	Z	FFÉ	FFÉ	€	€
J	TJ	Z	FFÉ	FFÉ	€	€
F€	T F€	Z	FFÉ	FFÉ	€	€
FF	T FF	Z	FFÉ	FFÉ	€	€
FG	T FH	Z	FFEG	FFEG	€	€
FH	T FI	Z	FFEG	FFEG	€	€
FI	T FÍ	Z	FFEG	FFEG	€	€
FÍ	T FÍ	Z	FFÉ	FFÉ	€	€
FÎ	T FÎ	Z	FFÉ	FFÉ	€	€
FÏ	T FÏ	Z	FFÉ	FFÉ	€	€
FJ	T FJ	Z	FFEG	FFEG	€	€
FJ	T F€	Z	FFEG	FFEG	€	€
G€	T G€	Z	FFEG	FFEG	€	€
GF	T GÍ	Z	FFEG	FFEG	€	€
GG	T HF	Z	FFEG	FFEG	€	€
GH	T HG	Z	FFEG	FFEG	€	€
G	TH	Z	FFÉ	FFÉ	€	€
G	TH	Z	FFEG	FFEG	€	€
G	THU	Z	FFEG	FFEG	€	€
G	TI €	Z	FFEG	FFEG	€	€
G	TIF	Z	FFEG	FFEG	€	€
GJ	TIG	Z	FFEG	FFEG	€	€
H€	TIH	Z	FFÉF	FFÉF	€	€
HF	TII	Z	FFÉF	FFÉF	€	€
HG	TÍI	Z	FFEG	FFEG	€	€
HH	TÏI	Z	FFEG	FFEG	€	€



Ó[{] æ ^ K ÓÉV/Ó:[^]
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 T [à ^ ¡ Á æ ^ K Í G G Í Í Á Ó : æ { ¡ á Á Ú Í Á Ó G F

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A Ya Vyf'8]gfl]Vi hYX' @ UXg'f6 @' (: \$ 'K]bX!' =MYL'f'7 cb]hbi YXL

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Ë Ë	T F Î Î	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
Ì J	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
J€	T F Î Æ	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JF	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JG	T F Î G	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JH	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JI	T F Î G	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JÍ	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JÏ	T F Î Î	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JÌ	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
JJ	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€€	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€F	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€G	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€H	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€Î	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€J	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€Í	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€Ï	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€Ì	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
F€J	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FF€	T F Î Î	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFF	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFG	T F Î G	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFH	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFI	T F Î G	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFÍ	T F Î H	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFÏ	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFÌ	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€
FFJ	T F Î J	Z	ÏÏÏÏ	ÏÏÏÏ	€	€

A Ya Vyf'8]gfl]Vi hYX' @ UXg'f6 @' () : - \$ 'K]bX!' =MYL

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J	T J	Y	ÏÏÏÏ	ÏÏÏÏ	€	€
F€	T F€	Y	ÏÏÏÏ	ÏÏÏÏ	€	€
FF	T FF	Y	ÏÏÏÏ	ÏÏÏÏ	€	€
FG	T FH	Y	ÏÏÏÏ	ÏÏÏÏ	€	€
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Exhibit F

Power Density/RF Emissions Report

Transcom Engineering, Inc.

Wireless Network Design and Deployment

Radio Frequency Emissions Analysis Report

T-MOBILE Existing Facility

Site ID: CT11025B

Branford/ I-95/ X55/ Dtn1
10 Sylvia St
Branford, CT 06405

May 20, 2019

Transcom Engineering Project Number: 737001-0028

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	52.66 %

Transcom Engineering, Inc.

Wireless Network Design and Deployment

May 20, 2019

T-MOBILE

Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, CT 6009

Emissions Analysis for Site: **CT11025B – Branford/ I-95/ X55/ Dtn1**

Transcom Engineering, Inc (“Transcom”) was directed to analyze the proposed upgrades to the T-MOBILE facility located at **10 Sylvia St, Branford, CT**, for the purpose of determining whether the emissions from the Proposed T-MOBILE Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The number of $\mu\text{W}/\text{cm}^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu\text{W}/\text{cm}^2$). The general population exposure limits for the 600 & 700 MHz bands are approximately $400 \mu\text{W}/\text{cm}^2$ and $467 \mu\text{W}/\text{cm}^2$ respectively. The general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is $1000 \mu\text{W}/\text{cm}^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Transcom Engineering, Inc.

Wireless Network Design and Deployment

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

Transcom Engineering, Inc.

Wireless Network Design and Deployment

CALCULATIONS

Calculations were performed for the proposed upgrades to the T-MOBILE antenna facility located at **10 Sylvia St, Branford, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-MOBILE is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
LTE	1900 MHz (PCS)	4	40
LTE	2100 MHz (AWS)	2	60
UMTS	1900 MHz (PCS)	1	40
GSM	1900 MHz (PCS)	1	15
UMTS	2100 MHz (AWS)	1	40
LTE / 5G NR	600 MHz	2	40
LTE	700 MHz	2	20

Table 1: Channel Data Table

Transcom Engineering, Inc.

Wireless Network Design and Deployment

The following antennas listed in *Table 2* were used in the modeling for transmission in the 600, 700 MHz, 1900 MHz (PCS) and 2100 MHz (AWS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

Sector	Antenna Number	Antenna Make / Model	Antenna Centerline (ft)
A	1	Ericsson AIR32 B66A / B2A	122
A	2	Ericsson AIR21 B2A/B4P	122
A	3	RFS APXVAARR24_43-U-NA20	120
B	1	Ericsson AIR32 B66A / B2A	122
B	2	Ericsson AIR21 B2A/B4P	122
B	3	RFS APXVAARR24_43-U-NA20	120
C	1	Ericsson AIR32 B66A / B2A	122
C	2	Ericsson AIR21 B2A/B4P	122
C	3	RFS APXVAARR24_43-U-NA20	120

Table 2: Antenna Data

All calculations were done with respect to uncontrolled / general population threshold limits.

Cable losses were factored in the calculations for this site. Since all **2100 MHz (AWS) UMTS** radios are ground mounted the following cable loss values were used. For each ground mounted **2100 MHz (AWS) UMTS** radio there was **1.59 dB** of cable loss calculated into the system gains / losses for this site. These values were calculated based upon the manufacturers specifications for **150 feet** of **1-5/8"** coax.

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RESULTS

Per the calculations completed for the proposed T-MOBILE configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBi)	Channel Count	Total TX Power (W)	ERP (W)	MPE %
Antenna A1	Ericsson AIR32 B66A / B2A	1900 MHz (PCS) / 2100 MHz (AWS)	15.85	6	280	10,768.57	2.88
Antenna A2	Ericsson AIR21 B2A/B4P	1900 MHz (PCS) / 2100 MHz (AWS)	15.9	3	95	3,218.84	0.86
Antenna A3	RFS APXVAARR24_43-U-NA20	600 MHz / 700 MHz	12.95 / 13.35	4	200	4,173.20	2.63
Sector A Composite MPE%							6.37
Antenna B1	Ericsson AIR32 B66A / B2A	1900 MHz (PCS) / 2100 MHz (AWS)	15.85	6	280	10,768.57	2.88
Antenna B2	Ericsson AIR21 B2A/B4P	1900 MHz (PCS) / 2100 MHz (AWS)	15.9	3	95	3,218.84	0.86
Antenna B3	RFS APXVAARR24_43-U-NA20	600 MHz / 700 MHz	12.95 / 13.35	4	200	4,173.20	2.63
Sector B Composite MPE%							6.37
Antenna C1	Ericsson AIR32 B66A / B2A	1900 MHz (PCS) / 2100 MHz (AWS)	15.85	6	280	10,768.57	2.88
Antenna C2	Ericsson AIR21 B2A/B4P	1900 MHz (PCS) / 2100 MHz (AWS)	15.9	3	95	3,218.84	0.86
Antenna C3	RFS APXVAARR24_43-U-NA20	600 MHz / 700 MHz	12.95 / 13.35	4	200	4,173.20	2.63
Sector C Composite MPE%							6.37

Table 3: T-MOBILE Emissions Levels

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The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum T-MOBILE MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. *Table 5* below shows a summary for each T-MOBILE Sector as well as the composite MPE value for the site.

Site Composite MPE%	
Carrier	MPE%
T-MOBILE – Max Per Sector Value	6.37 %
AT&T	29.73 %
Verizon Wireless	4.24 %
Clearwire	12.32 %
Site Total MPE %:	52.66 %

Table 4: All Carrier MPE Contributions

T-MOBILE Sector A Total:	6.37 %
T-MOBILE Sector B Total:	6.37 %
T-MOBILE Sector C Total:	6.37 %
Site Total:	52.66 %

Table 5: Site MPE Summary

Transcom Engineering, Inc.

Wireless Network Design and Deployment

FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated T-MOBILE sector(s). For this site, all three sectors have the same configuration yielding the same results on all three sectors.

T-MOBILE _ Frequency Band / Technology Max Power Values (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ($\mu\text{W}/\text{cm}^2$)	Frequency (MHz)	Allowable MPE ($\mu\text{W}/\text{cm}^2$)	Calculated % MPE
T-Mobile 1900 MHz (PCS) LTE	4	1,538.37	122	16.45	1900 MHz (PCS)	1000	1.64%
T-Mobile 2100 MHz (AWS) LTE	2	2,307.55	122	12.33	2100 MHz (AWS)	1000	1.23%
T-Mobile 1900 MHz (PCS) UMTS	1	1,556.18	122	4.16	1900 MHz (PCS)	1000	0.42%
T-Mobile 1900 MHz (PCS) GSM	1	583.57	122	1.56	1900 MHz (PCS)	1000	0.16%
T-Mobile 2100 MHz (AWS) UMTS	1	1,079.10	122	2.88	2100 MHz (AWS)	1000	0.29%
T-Mobile 600 MHz LTE / 5G NR	2	788.97	120	4.37	600 MHz	400	1.09%
T-Mobile 700 MHz LTE	2	1,297.63	120	7.18	700 MHz	467	1.54%
						Total:	6.37%

Table 6: T-MOBILE Maximum Sector MPE Power Values

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Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-MOBILE facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-MOBILE Sector	Power Density Value (%)
Sector A:	6.37 %
Sector B:	6.37 %
Sector C:	6.37 %
T-MOBILE Maximum Total (per sector):	6.37 %
Site Total:	52.66 %
Site Compliance Status:	COMPLIANT

The anticipated composite MPE value for this site assuming all carriers present is **52.66 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.



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