

STATE OF CONNECTICUT  
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Internet: [ct.gov/csc](http://ct.gov/csc)

Daniel F. Caruso  
Chairman

June 23, 2008

Kenneth C. Baldwin, Esq.  
Robinson & Cole LLP  
280 Trumbull Street  
Hartford, CT 06103-3597

RE: **EM-VER-014-080519** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 180 North Main Street, Branford, Connecticut.

Dear Attorney Baldwin:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies.

The proposed modifications are to be implemented as specified here and in your notice dated May 19, 2008, including the placement of all necessary equipment and shelters within the tower compound. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,

S. Derek Phelps  
Executive Director

SDP/MP

- c: Honorable Anthony "Unk" DeRos, First Selectman, Town of Branford  
Diana Ross, Inland Wetland Enforcement Officer, Town of Branford  
Justine K. Gillen, Zoning Enforcement Officer, Town of Branford  
Crown Castle International



CONNECTIONS SITING COUNCIL  
Affirmative Action Equal Opportunity Employer



# STATE OF CONNECTICUT

## CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: [siting.council@ct.gov](mailto:siting.council@ct.gov)

Internet: [ct.gov/csc](http://ct.gov/csc)

Daniel F. Caruso

Chairman

May 20, 2008

The Honorable Anthony "Unk" DaRos  
First Selectman  
Town of Branford  
Town Hall  
1019 Main Street  
P. O. Box 150  
Branford, CT 06405-0150

RE: **EM-VER-014-080519** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 180 North Main Street, Branford, Connecticut.

Dear Mr. DaRos:

The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

If you have any questions or comments regarding this proposal, please call me or inform the Council by June 3, 2008.

Thank you for your cooperation and consideration.

Very truly yours,

S. Derek Phelps  
Executive Director

SDP/jb

Enclosure: Notice of Intent

c: Justine K. Gillen, Zoning Enforcement Officer, Town of Branford  
Diana Ross

280 Trumbull Street  
Hartford, CT 06103-3597  
Main (860) 275-8200  
Fax (860) 275-8299  
kbaldwin@rc.com  
Direct (860) 275-8345

EM-VER-014-080519

ORIGINAL May 19, 2008

*Via Hand Delivery*

RECEIVED  
MAY 19 2008

CONNECTICUT  
SITING COUNCIL

S. Derek Phelps  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: **Notice of Exempt Modification – Antenna Swap  
180 North Main Street, Branford, Connecticut**

Dear Mr. Phelps:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains a wireless telecommunications facility at the above referenced location. The Council approved Cellco’s shared use of this facility in Docket 122. On August 12, 2004 the Council granted Cellco’s request to replace three of its existing cellular antennas with three PCS antennas. Cellco now intends to modify its installation further by replacing the six (6) ALP 9212 cellular antennas with six (6) newer Model DB846F65ZAXY cellular antennas at the 96-foot level on the 110-foot tower. The tower is owned by Crown Castle International. Attached behind Tab 1 are the specifications for the proposed replacement antennas.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Anthony DaRos, First Selectman of the Town of Branford. Pursuant to a Council directive, a copy of this letter is being sent to Three M&M Ltd. Partnership, the owner of the property on which the tower is located.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).

1. The proposed modifications will not result in any increase in the overall height of the existing structure. Cellco’s replacement antennas will be located at the 96-foot level of the 110-foot tower.



Law Offices

BOSTON

HARTFORD

NEW LONDON

STAMFORD

WHITE PLAINS

NEW YORK CITY

SARASOTA

www.rc.com

HART1-1458383-1

S. Derek Phelps  
May 19, 2008  
Page 2

2. The proposed modifications will not involve any ground-mounted equipment and, therefore, will not require the extension of the site boundaries.

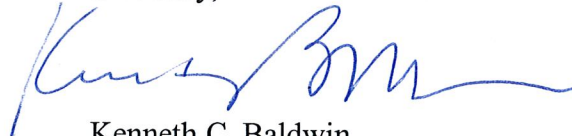
3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

4. The operation of the replacement antennas will not increase radio frequency (RF) power density levels at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative power density table for the facility is included behind Tab 2.

Also attached is a Structural Analysis Report confirming that the tower can support the proposed modifications. (See Tab 3).

For the foregoing reasons, Celco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

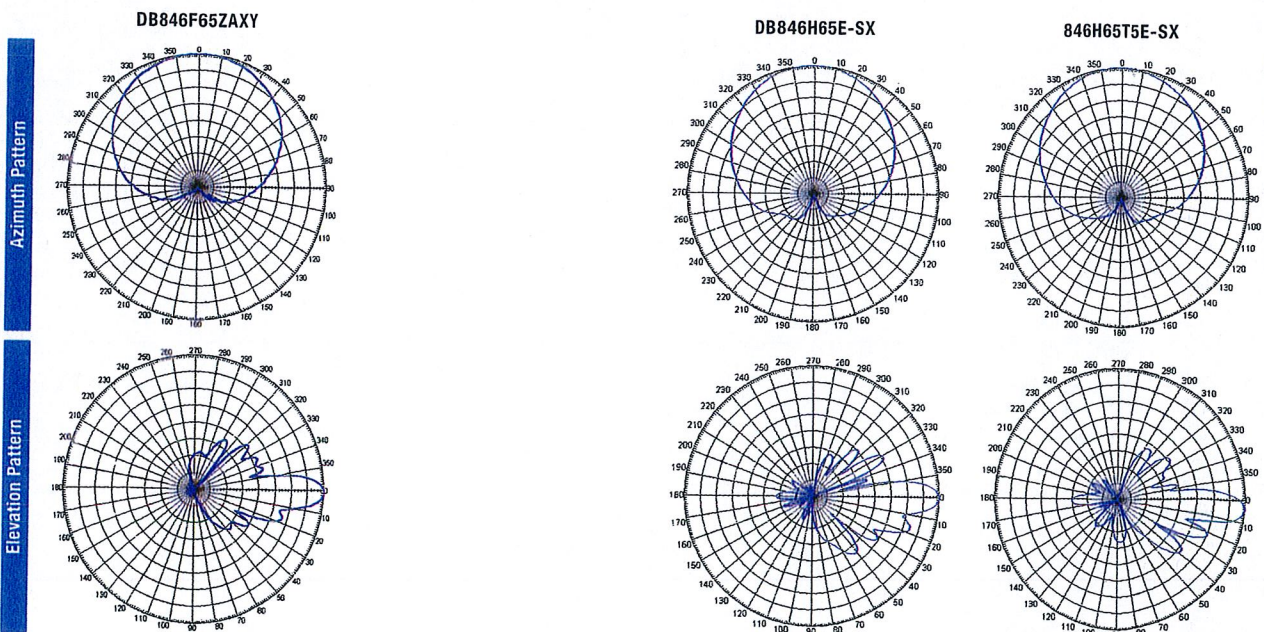
Copy to:

Anthony DaRos, Branford First Selectman  
Three M&M Ltd. Partnership  
Sandy M. Carter



HORIZONTAL BEAMWIDTH	65°	65°	65°
FREQUENCY RANGE	806-960 MHz	806-896 MHz	806-896 MHz
	14.5 & 14.8 dBd / 0° Tilt	14.5 dBd / 0° Tilt	14.3 dBd / 5° Tilt
MODEL	DB846F65ZAXY	DB846H65E-SX	846H65T5E-SX
TYPE	Directed Dipole®, No Screen	Directed Dipole®	Directed Dipole®
<b>ELECTRICAL SPECIFICATIONS</b>			
Frequency Range (MHz)	806-896	870-960	806-896
Gain (dBd/dBi)	14.5 / 16.6	14.8 / 16.9	14.5 / 16.6
Horizontal Beamwidth (Deg.)	65	60	65
Elevation Beamwidth (Deg.)	11	10.5	11
USLS (dB)	>15	>15	N/A
Null Fill (dB) – Below Peak	N/A	N/A	N/A
Beam Tilt (Deg.)	0	0	5
VSWR	<1.33:1	<1.33:1	<1.5:1
Front-To-Back Ratio (dB)	40	40	30
Isolation (dB)	N/A	N/A	N/A
Max. Input Power (Watts)	500	500	500
Polarization	Vertical	Vertical	Vertical
Connector Location	Back	Back	Back
Connector Type	7-16 DIN - Female	7-16 DIN - Female	7-16 DIN - Female
Optional Connectors	N/A	N/A	N/A
<b>MECHANICAL SPECIFICATIONS</b>			
Length (inch/mm)	72 / 1,829	72 / 1,829	72 / 1,829
Width (inch/mm)	10 / 254	10 / 254	20.5 / 521
Depth (inch/mm)	8.5 / 216	8.5 / 216	9 / 229
Net Weight (lbs/kg)	21 / 9.5	21 / 9.5	24 / 10.9
Max. Flat Plate Area (ft²/m²)	1.61 / 0.15	1.61 / 0.15	4.95 / 0.46
Max. Wind Load at 100 mph (lbf/N)	87 / 386	87 / 386	273 / 1,214
Max. Wind Speed (mph/kmh)	125 / 201	125 / 201	125 / 201
Radome Material	ABS, UV Resistant	ABS, UV Resistant	ABS, UV Resistant
Reflector Material	Pass. Aluminum	Pass. Aluminum	Pass. Aluminum
Radiator Material	Aluminum	Aluminum	Brass
Hardware Material	Galvanized Steel	Galvanized Steel	Galvanized Steel
Color	Light Gray	Light Gray	Light Gray
Std. Mounting Hardware	DB380	DB380	DB380
Optional Downtilt Kit	DB5083	DB5083	DB5083
Optional Special Mounting	DB5084-AZ	DB5084-AZ	DB5084-AZ

Specifications are subject to change. Please see our website for the latest information.



Scale: 10° radials, 5 dB per division

General Power Density

Site Name: Brandford, CT  
 Tower Height: 96' (Cellular) and (PCS) 105' to Rad Center (Verizon Platform)

Operator	Operating Frequency (MHz)	Number of Trans.	ERP Per Trans. (watts)	Total ERP (watts)	Distance to Target (feet)	Calculated Power Density (mW/cm <sup>2</sup> )	Maximum Permissible Exposure* (mW/cm <sup>2</sup> )	Fraction of MPE (%)
Verizon Cellular	880	9	365	3285	96	0.1282	0.5866	21.85%
Verizon PCS	1900	3	313	939	105	0.0306	1.0	3.06%
<b>Total Percentage of Maximum Permissible Exposure</b>								24.92%

\*Guidelines adopted by the FCC on August 1, 1996, 47 CFR Part 1 based on NCRP Report 86, 1986 and generally on ANSI/IEEE C95.1-1992

MHz = Megahertz

mW/cm<sup>2</sup> = milliwatts per square centimeter

ERP = Effective Radiated Power

Absolute worst case scenario, maximum values used.





April 29, 2008

LaShay Holmes  
Crown Castle USA  
9105 Monroe Road, Suite 150  
Charlotte, NC 28270  
(704) 814-8311

Vertical Structures, Inc.  
309 Spangler Drive, Suite E  
Richmond, KY 40475  
(859) 624-8360  
acronin@verticalstructures.com

**Subject:** Structural Analysis Report

**Carrier Designation** Verizon Wireless Change-Out  
Carrier Site Number: NHV113  
Carrier Site Name: Branford, CT

**Crown Castle Designation** Crown Castle BU Number: 806360  
Crown Castle Site Name: NHV 113  
Crown Castle JDE Job Number: 102865

**Engineering Firm Designation** Vertical Structures Project Number: 2008-004-071

**Site Data** 180 & 184 North Main Street, Branford, CT, New Haven County  
Latitude 41°-17'-23.0", Longitude -72°-48'-43.0"  
110' Valmont Monopole Tower

Dear Ms. Holmes,

Vertical Structures is pleased to submit this structural analysis report to determine the structural integrity of the aforementioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 282654, and Application Number 59379, Revision 4. The purpose of the analysis is to determine the suitability of the tower for the following load case:

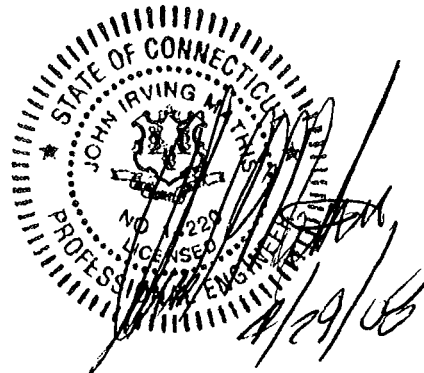
Load Case 1 (LC1): Proposed Equipment (Table 1) + Existing/Reserved Equipment (Table 2)

Based on our analysis we have determined the tower superstructure and foundation are sufficient for LC1. This analysis has been performed in accordance with the TIA/EIA-222-F standard and local code requirements based upon an 85 MPH basic "fastest mile" wind speed, equivalent to a 105 MPH basic "3-second gust" wind speed per IBC Table 1609.3.1.

Vertical Structures appreciates the opportunity of providing our continuing professional services to you and Crown Castle USA. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted,

Andy Cronin  
Project Engineer



## TABLE OF CONTENTS

### 1.) INTRODUCTION

### 2.) ANALYSIS CRITERIA

Table 1 – Proposed Antenna and Cable Information

Table 2 – Existing and Reserved Antenna and Cable Information

Table 3 – Design Antenna and Cable Information

### 3.) ANALYSIS PROCEDURE

Table 4 – Documents Provided

3.1) Analysis Methods

3.2) Assumptions

### 4.) ANALYSIS RESULTS

Table 5 – Tower Component Stresses vs. Capacity (LC1)

### 5.) APPENDIX A

RISA Tower Output

### 6.) APPENDIX B

Feedline Routing Drawing

### 7.) APPENDIX C

Additional Calculations



## 1.) INTRODUCTION

The 110' tall monopole tower was designed and manufactured by Valmont for Metro Mobile in 1990. The existing structure consists of three (3) 12-sided tapered polygonal tubes joined via slip joint connections and is founded on a 6' diameter by 6' deep drilled pier with eighteen (18) rock anchors embedded 12' into rock.

## 2.) ANALYSIS CRITERIA

The NHV 113 monopole tower was analyzed in accordance with the current EIA-222-F publication, "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures." The proposed, existing, and reserved antennas, cables, and mounts considered in this analysis are listed in Tables 1 and 2. Applied forces in this study were derived from an 85 MPH basic "fastest mile" wind speed with no ice and a reduced 74 MPH basic "fastest mile" wind speed with a 1/2" of radial ice accumulation. The tower was originally designed for a 90 MPH basic "fastest mile" wind speed with no ice and a reduced 78 MPH basic "fastest mile" wind speed with a 1/2" of radial ice accumulation. The original design loads are listed in Table 3. All cables are assumed to be routed in accordance with the drawing in Appendix B.

**Table 1 – Proposed Antenna and Cable Information**

Mount Center Line Elevation (feet)	Number Of Antenna	Antenna Manufacturer	Antenna Model	Mount Manufacturer	Mount Model	Number Of Feed Lines	Feed Line Size (inches)
96	6	Decibel	DB846F65ZAXY			6	1 5/8

**Table 2 – Existing and Reserved Antenna and Cable Information**

Mount Center Line Elevation (feet)	Number Of Antenna	Antenna Manufacturer	Antenna Model	Mount Manufacturer	Mount Model	Number Of Feed Lines	Feed Line Size (inches)
105	3	Decibel	932DG65T2E-M	Valmont	TEC COM. Platform w/ (3) 12' Knockdown Frames	6	1 1/4
96	6 + 6*	Swedcom	ALP 9212-N			6 + 6*	7/8
	1		E911 GPS			1*	1/2
83	1	Decibel	DB225-A			1	1/4
64	1	Decibel	DB225-A		(1) Pipe Mount	1	1 5/8

\*Indicates equipment to be removed.

**Table 3 – Design Antenna and Cable Information**

Mount Center Line Elevation (feet)	Number Of Antenna	Antenna Manufacturer	Antenna Model	Mount Manufacturer	Mount Model	Number Of Feed Lines	Feed Line Size (inches)
101	4	Celwave	PD10017	Valmont	TEC COM. Platform		
	12	Celwave	PD1132				
79	1	Decibel	DB-212-2	Valmont	(1) Pipe Mount		
60	1	Decibel	DB-212-2	Valmont	(1) Pipe Mount		

### 3.) ANALYSIS PROCEDURE

**Table 4 – Documents Provided**

<b>Document</b>	<b>Remarks</b>	<b>Reference</b>	<b>Source</b>
Online Application	Verizon Wireless Change-Out Revision #4	59379	CCI iSite
Tower Drawings	Valmont Order No. 10666-90	971913	CCI iSite
Foundation Drawing	SAC Engineering Job No. 1990-9	217660	CCI iSite
Geotechnical Report	Applied Earth Technologies Report Dated, "June 12, 1990"	262228	CCI iSite

#### 3.1) Analysis Methods

RISA Tower (Version 5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various dead, live, wind, and ice load cases. All loads were computed in accordance with the ANSI/TIA/EIA-222-F or the local building code requirements. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

1. Tower and structures were built in accordance with the manufacturer's specifications.
2. The tower and structures have been maintained in accordance with manufacturer's specifications.
3. The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and any referenced drawings.
4. When applicable, transmission cables are considered to be structural components for calculating wind loads, as allowed by TIA/EIA-222-F.

If any of these assumptions are not valid or have been made in error, this analysis may be affected, and Vertical Structures should be allowed to review any new information to determine its effect on the structural integrity of the tower.

#### 4.) ANALYSIS RESULTS

**Table 5 – Tower Component Stresses vs. Capacity (LC1)**

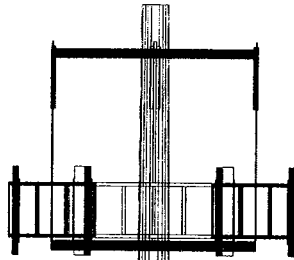
<b>Section Capacity Table</b>									
Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass/Fail	
L1	110 - 67.3333	Pole	TP30.45x21.91x0.2188	1	-6107.95	1013903.75	49.0	Pass	
L2	67.3333 - 29.4167	Pole	TP37.6x29.0784x0.3125	2	-11261.20	1892126.77	59.5	Pass	
L3	29.4167 - 0	Pole	TP42.85x35.8577x0.4063	3	-18694.00	2886411.43	58.7	Pass	
							Summary		
							Pole (L2)	59.5	Pass
							<b>RATING =</b>	<b>59.5</b>	<b>Pass</b>

Notes	Component	% Capacity	Pass/Fail
<b>Additional Component Analysis Summary:</b>			
1	Anchor Bolts (Tension)	64.1	Pass
1	Base Plate (Bending)	35.7	Pass
	Foundation (Compared to Design Loads)	62.5	Pass
<b>Structure Rating =</b>		<b>64.1</b>	<b>Pass</b>

1) Indicates calculations supporting % capacity are included in Appendix C.

## APPENDIX A

110.0 ft



**DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
Valmont 13'-5" L.P. Platform (VSI) (Verizon Wireless)	107	12' Knockdown T-Frame (VSI) (Verizon Wireless)	96
14' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	107 - 94	(2) ALP 9212-N w/Mount Pipe (Verizon Wireless)	96
14' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	107 - 94	(2) ALP 9212-N w/Mount Pipe (Verizon Wireless)	96
14' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	107 - 94	(2) ALP 9212-N w/Mount Pipe (Verizon Wireless)	96
932DG65T2E-M w/Mount Pipe (Verizon Wireless)	105	(2) DB846F65ZAXY w/Mount Pipe (Verizon Wireless)	96
932DG65T2E-M w/Mount Pipe (Verizon Wireless)	105	(2) DB846F65ZAXY w/Mount Pipe (Verizon Wireless)	96
932DG65T2E-M w/Mount Pipe (Verizon Wireless)	105	(2) DB846F65ZAXY w/Mount Pipe (Verizon Wireless)	96
(2) 6' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	105	Generic GPS (VSI) (Verizon Wireless)	96
(2) 6' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	105	Valmont 13'-5" L.P. Platform (VSI) (Verizon Wireless)	94
(2) 6' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	105	14' x 2" Antenna Mount Pipe (VSI)	83
12' Knockdown T-Frame (VSI) (Verizon Wireless)	96	DB225-A	83
12' Knockdown T-Frame (VSI) (Verizon Wireless)	96	14' x 2" Antenna Mount Pipe (VSI)	64
		DB225-A	64

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi			

**TOWER DESIGN NOTES**

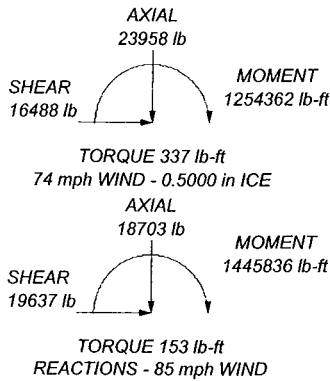
1. Tower is located in New Haven County, Connecticut.
2. Tower designed for a 85 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 74 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 59.5%

Section	1	2	3
Length (ft)	42.67	42.58	35.00
Number of Sides	12	12	12
Thickness (in)	0.2188	0.3125	0.4063
Lap Splice (ft)			
Top Dia (in)	21.9100	29.0784	35.8577
Bot Dia (in)	30.4500	37.6000	42.8500
Grade		A572-65	
Weight (lb)	2854.9	4815.6	6067.8

67.3 ft

29.4 ft

0.0 ft



<p><b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369</p>	<p>Job: <b>NHV 113, CT BU#806360</b></p>		
	<p>Project: <b>Vertical Structures Job No. 2008-004-071</b></p>		
	<p>Client: <b>Crown Castle</b></p>	<p>Drawn by: <b>Andy Cronin</b></p>	<p>App'd:</p>
	<p>Code: <b>TIA/EIA-222-F</b></p>	<p>Date: <b>04/29/08</b></p>	<p>Scale: <b>NTS</b></p>
	<p>Path: <small>\\nas1\acronin\2008-004-071-NHV 113 CTRISA\806360.dwg</small></p>		

<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> NHV 113, CT BU#806360	<b>Page</b> 1 of 6
	<b>Project</b> Vertical Structures Job No. 2008-004-071	<b>Date</b> 14:05:08 04/29/08
	<b>Client</b> Crown Castle	<b>Designed by</b> Andy Cronin

## Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in New Haven County, Connecticut.

Basic wind speed of 85 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 74 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

## Options

- |  |  |   |
|--|--|---|
| <ul style="list-style-type: none"> <li>Consider Moments - Legs</li> <li>Consider Moments - Horizontals</li> <li>Consider Moments - Diagonals</li> <li>Use Moment Magnification</li> <li>√ Use Code Stress Ratios</li> <li>√ Use Code Safety Factors - Guys</li> <li>Escalate Ice</li> <li>Always Use Max Kz</li> <li>Use Special Wind Profile</li> <li>√ Include Bolts In Member Capacity</li> <li>√ Leg Bolts Are At Top Of Section</li> <li>√ Secondary Horizontal Braces Leg</li> <li>Use Diamond Inner Bracing (4 Sided)</li> <li>Add IBC .6D+W Combination</li> </ul> | <ul style="list-style-type: none"> <li>Distribute Leg Loads As Uniform</li> <li>Assume Legs Pinned</li> <li>√ Assume Rigid Index Plate</li> <li>√ Use Clear Spans For Wind Area</li> <li>√ Use Clear Spans For KL/r</li> <li>Retension Guys To Initial Tension</li> <li>√ Bypass Mast Stability Checks</li> <li>√ Use Azimuth Dish Coefficients</li> <li>√ Project Wind Area of Appurt.</li> <li>√ Autocalc Torque Arm Areas</li> <li>SR Members Have Cut Ends</li> <li>Sort Capacity Reports By Component</li> <li>√ Triangulate Diamond Inner Bracing</li> </ul> | <ul style="list-style-type: none"> <li>Treat Feedline Bundles As Cylinder</li> <li>Use ASCE 10 X-Brace Ly Rules</li> <li>√ Calculate Redundant Bracing Forces</li> <li>Ignore Redundant Members in FEA</li> <li>SR Leg Bolts Resist Compression</li> <li>√ All Leg Panels Have Same Allowable</li> <li>Offset Girt At Foundation</li> <li>√ Consider Feedline Torque</li> <li>Include Angle Block Shear Check</li> <li style="padding-left: 20px;">Poles</li> <li>Include Shear-Torsion Interaction</li> <li>Always Use Sub-Critical Flow</li> <li>Use Top Mounted Sockets</li> </ul> |
|--|--|---|

## Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	110.00-67.33	42.67	4.67	12	21.9100	30.4500	0.2188	0.8750	A572-65 (65 ksi)
L2	67.33-29.42	42.58	5.58	12	29.0784	37.6000	0.3125	1.2500	A572-65 (65 ksi)
L3	29.42-0.00	35.00		12	35.8577	42.8500	0.4063	1.6250	A572-65 (65 ksi)

<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> NHV 113, CT BU#806360	<b>Page</b> 2 of 6
	<b>Project</b> Vertical Structures Job No. 2008-004-071	<b>Date</b> 14:05:08 04/29/08
	<b>Client</b> Crown Castle	<b>Designed by</b> Andy Cronin

### Tapered Pole Properties

Section	Tip Dia. in	Area in <sup>2</sup>	I in <sup>4</sup>	r in	C in	I/C in <sup>3</sup>	J in <sup>4</sup>	I/Q in <sup>2</sup>	w in	w/I
L1	22.6829	15.2788	917.5793	7.7655	11.3494	80.8484	1859.2645	7.5197	5.2856	24.163
	31.5242	21.2941	2484.0378	10.8228	15.7731	157.4857	5033.3340	10.4803	7.5743	34.626
L2	31.0710	28.9457	3057.2251	10.2982	15.0626	202.9675	6194.7669	14.2462	6.9555	22.258
	38.9264	37.5205	6658.5803	13.3489	19.4768	341.8724	13492.0890	18.4665	9.2393	29.566
L3	38.2774	46.3749	7439.3822	12.6916	18.5743	400.5205	15074.2053	22.8243	8.5211	20.975
	44.3616	55.5217	12766.6349	15.1949	22.1963	575.1695	25868.6636	27.3261	10.3950	25.588

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals
ft	ft <sup>2</sup>	in					in	in
L1 110.00-67.33				1	1	1		
L2 67.33-29.42				1	1	1		
L3 29.42-0.00				1	1	1		

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement	Total Number	C <sub>AA</sub>	Weight
				ft		ft <sup>2</sup> /ft	plf
LDF6-50A (1-1/4 FOAM) (Verizon Wireless)	A	No	Inside Pole	105.00 - 5.00	6	No Ice	0.66
						1/2" Ice	0.66
LDF5-50A (7/8 FOAM) (Verizon Wireless)	A	No	Inside Pole	96.00 - 5.00	6	No Ice	0.33
						1/2" Ice	0.33
FLC 158-50J (1 5/8 FOAM) (Verizon Wireless)	A	No	Inside Pole	96.00 - 5.00	6	No Ice	0.92
						1/2" Ice	0.92
LDF1-50A (1/4 FOAM)	B	No	Inside Pole	83.00 - 5.00	1	No Ice	0.06
						1/2" Ice	0.06
LDF7-50A (1-5/8 FOAM)	C	No	Inside Pole	64.00 - 5.00	1	No Ice	0.82
						1/2" Ice	0.82

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation	Face	A <sub>R</sub>	A <sub>F</sub>	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face	Weight
	ft		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	lb
L1	110.00-67.33	A	0.000	0.000	0.000	0.000	364.16
		B	0.000	0.000	0.000	0.000	0.94
		C	0.000	0.000	0.000	0.000	0.00
L2	67.33-29.42	A	0.000	0.000	0.000	0.000	434.52
		B	0.000	0.000	0.000	0.000	2.27
		C	0.000	0.000	0.000	0.000	28.36
L3	29.42-0.00	A	0.000	0.000	0.000	0.000	279.82
		B	0.000	0.000	0.000	0.000	1.47
		C	0.000	0.000	0.000	0.000	20.02

<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> NHV 113, CT BU#806360	<b>Page</b> 3 of 6
	<b>Project</b> Vertical Structures Job No. 2008-004-071	<b>Date</b> 14:05:08 04/29/08
	<b>Client</b> Crown Castle	<b>Designed by</b> Andy Cronin

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight lb
L1	110.00-67.33	A	0.500	0.000	0.000	0.000	0.000	364.16
		B		0.000	0.000	0.000	0.000	0.94
		C		0.000	0.000	0.000	0.000	0.00
L2	67.33-29.42	A	0.500	0.000	0.000	0.000	0.000	434.52
		B		0.000	0.000	0.000	0.000	2.27
		C		0.000	0.000	0.000	0.000	28.36
L3	29.42-0.00	A	0.500	0.000	0.000	0.000	0.000	279.82
		B		0.000	0.000	0.000	0.000	1.47
		C		0.000	0.000	0.000	0.000	20.02

**Feed Line Center of Pressure**

Section	Elevation ft	CP <sub>X</sub> in	CP <sub>Z</sub> in	CP <sub>X</sub> Ice in	CP <sub>Z</sub> Ice in
L1	110.00-67.33	0.0000	0.0000	0.0000	0.0000
L2	67.33-29.42	0.0000	0.0000	0.0000	0.0000
L3	29.42-0.00	0.0000	0.0000	0.0000	0.0000

**Discrete Tower Loads**

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight lb
Valmont 13'-5" L.P. Platform (VSI) (Verizon Wireless)	C	None		0.0000	107.00	No Ice	31.00	1260.00
						1/2" Ice	37.70	2110.00
Valmont 13'-5" L.P. Platform (VSI) (Verizon Wireless)	C	None		0.0000	94.00	No Ice	31.00	1260.00
						1/2" Ice	37.70	2110.00
14' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	A	From Centroid-Leg	8.00	0.0000	107.00 - 94.00	No Ice	3.33	52.00
			0.00			1/2" Ice	4.75	76.88
14' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	B	From Centroid-Leg	8.00	0.0000	107.00 - 94.00	No Ice	3.33	52.00
			0.00			1/2" Ice	4.75	76.88
14' x 2" Antenna Mount Pipe (VSI) (Verizon Wireless)	C	From Centroid-Leg	8.00	0.0000	107.00 - 94.00	No Ice	3.33	52.00
			0.00			1/2" Ice	4.75	76.88
932DG65T2E-M w/Mount Pipe (Verizon Wireless)	A	From Centroid-Leg	8.00	0.0000	105.00	No Ice	4.15	35.05
			0.00			1/2" Ice	4.79	69.93
932DG65T2E-M w/Mount Pipe (Verizon Wireless)	B	From Centroid-Leg	8.00	0.0000	105.00	No Ice	4.15	35.05
			0.00			1/2" Ice	4.79	69.93



<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b>		NHV 113, CT BU#806360		<b>Page</b>		4 of 6	
	<b>Project</b>		Vertical Structures Job No. 2008-004-071		<b>Date</b>		14:05:08 04/29/08	
	<b>Client</b>		Crown Castle		<b>Designed by</b>		Andy Cronin	

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Horz	Lateral Vert					
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	lb
932DG65T2E-M w/Mount Pipe	C	From	8.00	0.0000		105.00	No Ice	4.15	35.05
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	4.79	69.93
(2) 6' x 2" Antenna Mount Pipe (VSI)	A	From	8.00	0.0000		105.00	No Ice	1.43	23.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	1.92	33.83
(2) 6' x 2" Antenna Mount Pipe (VSI)	B	From	8.00	0.0000		105.00	No Ice	1.43	23.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	1.92	33.83
(2) 6' x 2" Antenna Mount Pipe (VSI)	C	From	8.00	0.0000		105.00	No Ice	1.43	23.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	1.92	33.83
12' Knockdown T-Frame (VSI)	A	From	8.00	0.0000		96.00	No Ice	6.00	125.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	8.50	195.00
12' Knockdown T-Frame (VSI)	B	From	8.00	0.0000		96.00	No Ice	6.00	125.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	8.50	195.00
12' Knockdown T-Frame (VSI)	C	From	8.00	0.0000		96.00	No Ice	6.00	125.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	8.50	195.00
(2) ALP 9212-N w/Mount Pipe	A	From	8.00	0.0000		96.00	No Ice	6.42	42.71
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	7.11	103.63
(2) ALP 9212-N w/Mount Pipe	B	From	8.00	0.0000		96.00	No Ice	6.42	42.71
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	7.11	103.63
(2) ALP 9212-N w/Mount Pipe	C	From	8.00	0.0000		96.00	No Ice	6.42	42.71
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	7.11	103.63
(2) DB846F65ZAXY w/Mount Pipe	A	From	8.00	0.0000		96.00	No Ice	7.27	46.55
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	7.88	111.10
(2) DB846F65ZAXY w/Mount Pipe	B	From	8.00	0.0000		96.00	No Ice	7.27	46.55
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	7.88	111.10
(2) DB846F65ZAXY w/Mount Pipe	C	From	8.00	0.0000		96.00	No Ice	7.27	46.55
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	7.88	111.10
Generic GPS (VSI)	C	From	8.00	0.0000		96.00	No Ice	1.40	25.00
(Verizon Wireless)		Centroid-Leg	0.00				1/2" Ice	1.70	30.00
**									
14' x 2" Antenna Mount Pipe (VSI)	B	From	2.25	0.0000		83.00	No Ice	3.33	52.00
		Centroid-Leg	0.00				1/2" Ice	4.75	76.88
DB225-A	B	From	2.75	-10.0000		83.00	No Ice	3.21	37.00
		Centroid-Leg	0.00				1/2" Ice	5.78	48.10
**									
14' x 2" Antenna Mount Pipe (VSI)	A	From	2.25	0.0000		64.00	No Ice	3.33	52.00
		Centroid-Leg	0.00				1/2" Ice	4.75	76.88
DB225-A	A	From	2.75	-30.0000		64.00	No Ice	3.21	37.00
		Centroid-Leg	0.00				1/2" Ice	5.78	48.10

<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> NHV 113, CT BU#806360	<b>Page</b> 5 of 6
	<b>Project</b> Vertical Structures Job No. 2008-004-071	<b>Date</b> 14:05:08 04/29/08
	<b>Client</b> Crown Castle	<b>Designed by</b> Andy Cronin

### Compression Checks

### Pole Design Data

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KL/r	F <sub>a</sub> ksi	A in <sup>2</sup>	Actual P lb	Allow. P <sub>a</sub> lb	Ratio $\frac{P}{P_a}$
L1	110 - 67.3333 (1)	TP30.45x21.91x0.2188	42.67	0.00	0.0	36.858	20.6362	-6107.95	760618.00	0.008
L2	67.3333 - 29.4167 (2)	TP37.6x29.0784x0.3125	42.58	0.00	0.0	39.000	36.3963	-11261.20	1419450.00	0.008
L3	29.4167 - 0 (3)	TP42.85x35.8577x0.4063	35.00	0.00	0.0	39.000	55.5217	-18694.00	2165350.00	0.009

### Pole Bending Design Data

Section No.	Elevation ft	Size	Actual M <sub>x</sub> lb-ft	Actual f <sub>bx</sub> ksi	Allow. F <sub>bx</sub> ksi	Ratio $\frac{f_{bx}}{F_{bx}}$	Actual M <sub>y</sub> lb-ft	Actual f <sub>by</sub> ksi	Allow. F <sub>by</sub> ksi	Ratio $\frac{f_{by}}{F_{by}}$
L1	110 - 67.3333 (1)	TP30.45x21.91x0.2188	293070.00	-23.783	36.858	0.645	0.00	0.000	36.858	0.000
L2	67.3333 - 29.4167 (2)	TP37.6x29.0784x0.3125	820397.50	-30.611	39.000	0.785	0.00	0.000	39.000	0.000
L3	29.4167 - 0 (3)	TP42.85x35.8577x0.4063	1445833.33	-30.165	39.000	0.773	0.00	0.000	39.000	0.000

### Pole Interaction Design Data

Section No.	Elevation ft	Size	Ratio $\frac{P}{P_a}$	Ratio $\frac{f_{bx}}{F_{bx}}$	Ratio $\frac{f_{by}}{F_{by}}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	110 - 67.3333 (1)	TP30.45x21.91x0.2188	0.008	0.645	0.000	0.653 ✓	1.333	H1-3 ✓
L2	67.3333 - 29.4167 (2)	TP37.6x29.0784x0.3125	0.008	0.785	0.000	0.793 ✓	1.333	H1-3 ✓
L3	29.4167 - 0 (3)	TP42.85x35.8577x0.4063	0.009	0.773	0.000	0.782 ✓	1.333	H1-3 ✓

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail
L1	110 - 67.3333	Pole	TP30.45x21.91x0.2188	1	-6107.95	1013903.75	49.0	Pass
L2	67.3333 -	Pole	TP37.6x29.0784x0.3125	2	-11261.20	1892126.77	59.5	Pass

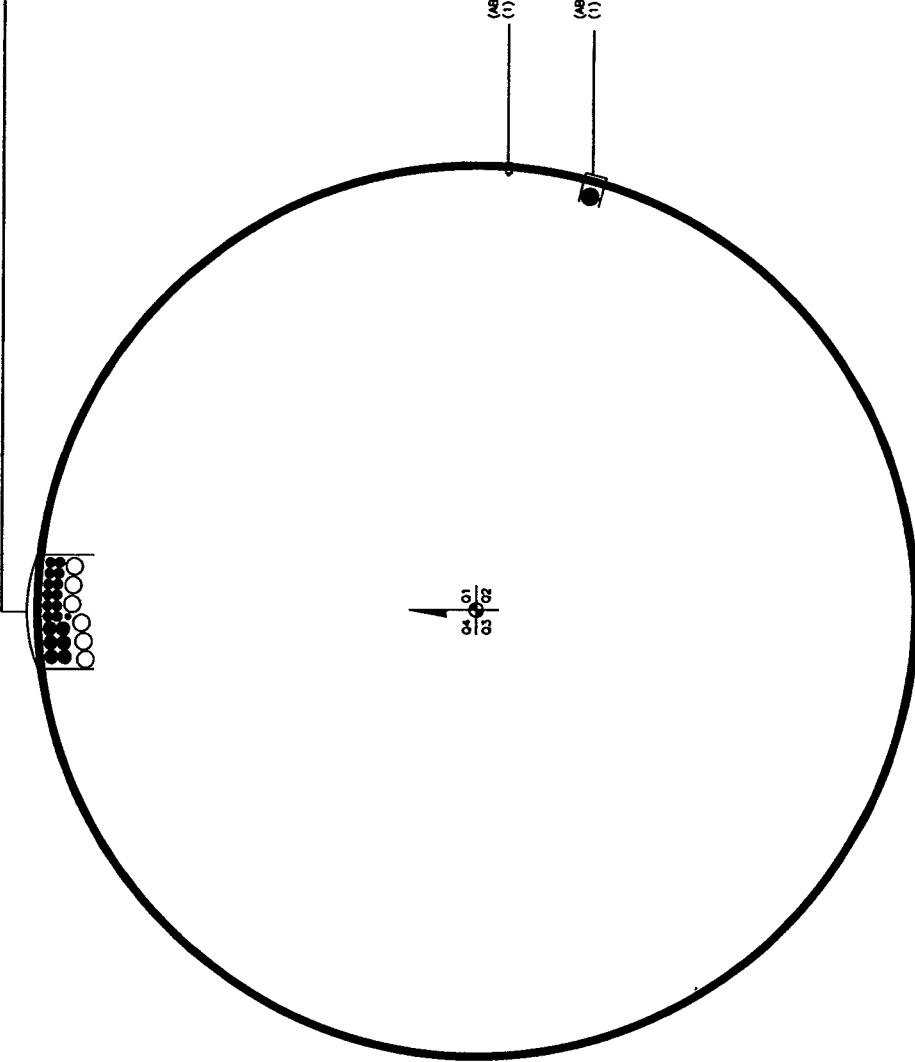
<b>RISATower</b>  <b>Vertical Structures, Inc.</b> 309 Spangler Drive, Suite E Richmond, Kentucky 40475 Phone: (859) 624-8360 FAX: (859) 624-8369	<b>Job</b> NHV 113, CT BU#806360	<b>Page</b> 6 of 6
	<b>Project</b> Vertical Structures Job No. 2008-004-071	<b>Date</b> 14:05:08 04/29/08
	<b>Client</b> Crown Castle	<b>Designed by</b> Andy Cronin

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	SF*P <sub>allow</sub> lb	% Capacity	Pass Fail	
L3	29.4167 29.4167 - 0	Pole	TP42.85x35.8577x0.4063	3	-18694.00	2886411.43	58.7	Pass	
							Summary		
							Pole (L2)	59.5	Pass
							<b>RATING =</b>	<b>59.5</b>	<b>Pass</b>

## APPENDIX B

COLUMBIAN TELEPHONE ADDRESS  
 100A

(PROPOSED-IN ADDITION TO INSTALLED)  
 (6) 1--5/8" TO 101 FT LEVEL  
 (INSTALLED--TO BE REMOVED)  
 (1) 1/2" TO 101 FT LEVEL  
 (6) 7/8" TO 101 FT LEVEL  
 (INSTALLED)  
 (6) 1-1/4" TO 101 FT LEVEL  
 (6) 7/8" TO 101 FT LEVEL  
 (VERIZON WIRELESS)



01  
 02  
 03

(ABANDONED)  
 (1) 1/4" TO 63 FT LEVEL

(ABANDONED)  
 (1) 1--5/8" TO 64 FT LEVEL



DRAWN BY: KENNETH  
 CHECKED BY: JIM  
 DRAWING DATE: 2/20/03

SITE NUMBER:  
 SITE NAME:  
 NEW YORSHIP:  
 BUSINESS UNIT NUMBER:

LETTER ADDRESS:  
 BUSINESS UNIT NAME STREET:  
 CITY STATE ZIP:  
 COUNTY:  
 STATE:  
 SHEET TITLE:  
 BASE LEVEL:  
 SHEET NUMBER:

LEGEND: FEEDLINES  
 ● SOLID BLUE CIRCLE DENOTES EXISTING FEEDLINE  
 ○ OPEN RED CIRCLE DENOTES PROPOSED FEEDLINE  
 ○ OPEN BLUE CIRCLE DENOTES RESERVED FEEDLINE  
 X BLUE "X" DENOTES LOCATION NOT GIVEN  
 NOTE: ASSUME FEEDLINE ATTACHMENT HEIGHT TO TOWER STEEL  
 AT 8- FEET ABOVE FINISHED GRADE UNLESS OTHERWISE SPECIFIED

BUSINESS UNIT: 806360 TOWER ID: C\_BASELEVEL



BASE LEVEL DRAWING

PLOT DATE: 2/20/03 FILE NAME: 806360\_BASELEVEL.DWG

1

A1-0

## APPENDIX C



## ANCHOR BOLT CALCULATIONS

**Customer:** Crown Castle  
**Site Name:** NHV 113, CT BU#806360  
**Job Number:** 2008-004-071  
**Tower Model:** 110' Valmont Monopole Tower  
**Date:** 4/29/2008

<i>Input Information:</i>	<i>Existing Bolts</i>	
# Bolts, <b>n</b>	12	
Bolt Diameter, <b>d</b>	2.25	in
Bolt Circle Diameter, <b>D</b>	50.86	in
Bolt Ultimate Tensile Stress, <b>F<sub>u</sub></b>	100	ksi
Applied Vertical Load <b>P</b>	18.70	kips
Applied Shear <b>S</b>	19.64	kips
Applied Moment <b>M</b>	17350.03	kip-in
Steel Grade	A615 Gr 75	

---

Bolt Cross-Sectional Area, <b>A</b>	3.976	in <sup>2</sup> (each)
Bolt Group Moment of Inertia, <b>I</b>	15427.61841	in <sup>4</sup>
Maximum Tensile Stress (outer bolt), <b>σ<sub>y</sub></b>	28.21	ksi
Maximum Shear Stress (any bolt), <b>τ<sub>xy</sub></b>	0.412	ksi
Maximum Allowable Stress (per bolt), <b>F<sub>t</sub></b>	44.00	ksi
<b>% Capacity</b>	<b>64.1%</b>	

**The Bolt Group is Adequate for Loading**

Maximum Allowable Stress (per bolt), **F<sub>t</sub>**

$$0.43F_u - 1.8f_v \leq 0.33F_u$$

This equation is for threaded parts, A449 bolts over 1 1/2" dia. (threads included in shear plane) Manual of Steel Construction ASD, 9th Edition, pg. 5-74, Table J3.3



## BASE PLATE CALCULATIONS

**Customer:** Crown Castle  
**Site Name:** NHV 113, CT BU#806360  
**Job Number:** 2008-004-071  
**Tower Model:** 110' Valmont Monopole Tower  
**Date:** 4/29/2008

### ***FOR BASE PLATES WITH EQUALLY DISTRUBUTED ANCHOR BOLTS WITHOUT GUSSET PLATE STIFFENERS***

Maximum Tensile Bolt Load	112.15	kip
Number of Sides of Pole	12	
Diameter of Pole at Base	42.85	in
Thickness of Pole at Base	0.40625	in
Area of Pole at Base	55.44	in <sup>2</sup>
Circumference of Pole at Base	137.78	in
Anchor Bolt Circle	50.86	in
Anchor Bolt Diameter	2.25	in
Base Plate Section Length	2.88	in
Anchor Bolt Quantity	12	
Base Plate Section Width	11.97	in
Moment on Base Plate Section	323.00	kip-in
Base Plate Thickness	2.75	in
Base Plate Bending Stress	21.41	ksi
Base Plate Yield Strength	60	ksi
Allowable Bending Stress (with 4/3 Increase)	<b>60</b>	ksi
<b>% Capacity</b>	<b>35.7%</b>	

**Base Plate is Adequate for Loading**