

CONNECTICUT SITING COUNCIL

ORIGINAL

In re:

T-Mobile USA, Inc. Notice to Make an Exempt
Modification to an Existing Facility,

142 Duncaster Road, Bloomfield, Connecticut.

: **EXEMPT MODIFICATION NO.** _____

:

: August 13, 2010

NOTICE OF EXEMPT MODIFICATION

Pursuant to Conn. Agencies Regs. §§ 16-50j-73 and 16-50j-72(b), T-Mobile USA, Inc. ("T-Mobile") hereby gives notice to the Connecticut Siting Council ("Council") and the Town of Bloomfield of T-Mobile's intent to make an exempt modification to the existing Connecticut Light & Power ("CL&P") pole (the "Tower") located at 142 Duncaster Road in Bloomfield, Connecticut. Specifically, T-Mobile plans to upgrade its wireless system in Connecticut by implementing its Universal Mobile Telecommunications System ("UMTS"). UMTS is a third-generation ("3G") technology that utilizes a code division multiple access ("CDMA") base to allow for fast and large data transfers. To accomplish this upgrade, T-Mobile must modify its antenna and equipment configurations at many of its existing sites.

Once the UMTS upgrade is complete, T-Mobile will operate on a more unified communication system, allowing international wireless telephones to function world-wide. Furthermore, UMTS will enhance GPS navigation capabilities and provide emergency responders with more advanced tracking capabilities. The proposed UMTS technology is compatible with the existing second-generation ("2G") Global System for Mobile Communication ("GSM") currently on the Tower and the proposed upgrade is expected to enhance the existing 2G system. In order to accomplish the upgrade at this site, T-Mobile plans to add UMTS technology and install associated equipment at the base of the Tower.

The Council initially approved T-Mobile's installation on this Tower under Petition No. 472. Under the Council's regulations (Conn. Agencies Regs. § 16-50j-72(b)), T-Mobile's plans

do not constitute a modification subject to the Council's review because T-Mobile will not change the height of the Tower, will not extend the boundaries of the compound, will not increase the noise levels at the site, and will not increase the total radio frequency electromagnetic radiation power density at the site to levels above applicable standards.

The Tower is an 87-foot CL&P pole (Pole No. 3147) with an existing extension for a total Tower height of 104 feet. The Tower is located at 142 Duncaster Road in Bloomfield, Connecticut (latitude 41° 51' 55.33" N, longitude -72° 45' 42.52" W). The Tower is owned by CL&P. Currently, T-Mobile has 3 antennas located on the Tower with a centerline of 100 feet and 3 Ground Mounted Amplifiers ("GMA"). A site plan with Tower specifications is attached.

T-Mobile plans to replace the existing 34-foot pipe mast with an upgraded 30-foot pipe mast in order to support its proposed modifications. The overall height of the Tower will remain the same, 104 feet, with the upgraded pipe mast. Specifically, T-Mobile plans to remove its 3 existing antennas and install 3 new GMS antennas. T-Mobile also plans to install 3 UMTS antennas for a total of 6 antennas to be located on the Tower. The 3 existing GMAs will remain. The proposed antennas will have centerlines of 101'- 8" and 95'- 3". To confirm the Tower can support these changes, T-Mobile commissioned Centek Engineering to perform a structural analysis of the Tower (attached). According to the structural assessment, dated April 27, 2010, "... the subject utility pole is adequate to support the proposed antenna mast and related appurtenances" (Section, 1-4, Structural Analysis, emphasis in original).

In addition, T-Mobile plans to utilize its 6 existing coax cables and install 6, 1-1/4 inch coax cables. T-Mobile also proposes to install its UTMS equipment cabinet on the existing 12' by 7' 6" (approximately) concrete pad. Hence, no increase in the boundaries of the site is necessary.

Therefore, excluding brief, minor, construction-related noise during the addition of the antennas and the installation of the equipment cabinet, T-Mobile's changes to the Tower will not increase noise levels at the site.

The proposed antennas will not adversely impact the health and safety of the surrounding community or the people working on the Tower. The total radio frequency exposure measured around the Tower will be well below the National Council on Radiation Protection and Measurements' ("NCRP") standard adopted by the Federal Communications Commission ("FCC"). The worst-case power density analysis measured at the base of the Tower indicates that T-Mobile's antennas will emit 11.74% of the NCRP's standard for maximum permissible exposure. Therefore, the power density levels will be below the FCC mandated radio frequency exposure limits in all locations around the Tower, even with extremely conservative assumptions. The power density analysis is attached.

In conclusion, T-Mobile's proposed plan to add antennas and equipment at this site does not constitute a modification subject to the Council's jurisdiction because T-Mobile will not increase the height of the Tower, will not extend the boundaries of the site, will not increase the noise levels at the site, and the total radio frequency electromagnetic radiation power density will stay within all applicable standards. *See* Conn. Agencies Regs. § 16-50j-72.

T-Mobile USA, Inc.

By: _____

Thomas J. Regan

Brown Rudnick LLP

185 Asylum Street, CityPlace I

Hartford, CT 06103-3402

Email - tregan@brownrudnick.com

Phone - 860.509.6522 /Fax - 860.509.6501

Certificate of Service

This is to certify that on this 13th day of August, 2010, the foregoing Notice of Exempt Modification was sent, via first class mail, to the following:

Town of Bloomfield
Mayor Sydney T. Schulman
Town Hall
800 Bloomfield Avenue
Bloomfield, CT 06002

By: _____
Thomas J. Regan

40274953 v1 025064/0016

**142 DUNCASTER ROAD
BLOOMFIELD, CT 06002**

SHEET NUMBER: 01

PROPOSED TELCO SERVICE WILL BE ROUTED ABOVE GRADE FROM PROPOSED JUNCTION BOX TO PROPOSED RES 3104 UNITS CABINET LOCATED WITHIN THE FENCED COMPOUND.

10. ALL UTILITY WORK SHALL BE IN ACCORDANCE WITH LOCAL UTILITY COMPANY REQUIREMENTS AND SPECIFICATIONS.

19. CONTRACTOR SHALL COMPLY WITH OWNERS ENVIRONMENTAL ENGINEER ON ALL METHODS AND PROVISIONS FOR ALL EXCAVATION ACTIVITIES INCLUDING SOIL DISPOSAL. ALL BACKFILL MATERIALS TO BE PROVIDED BY THE CONTRACTOR.

[illegible]

NOTE: NO STRUCTURAL EVALUATION HAS BEEN CONDUCTED BY ATLANTIS GROUP, INC. NO ADDITIONS OR MODIFICATIONS TO THE TOWER SHALL BE MADE WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

REFER TO STRUCTURAL ANALYSIS REPORT BY CENTEK ENGINEERING, INC. DATED APRIL 27, 2010

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ATLANTIS GROUP
1240 Centre Street, Suite 203
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APPROVALS

LANDLORD _____
LEASING _____
R.F. _____
ZONING _____
CONSTRUCTION _____
A/E _____

DRAWN BY: GC
CHECKED BY: SM

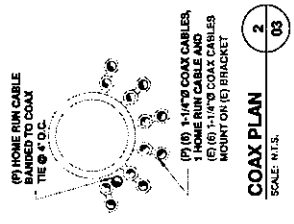
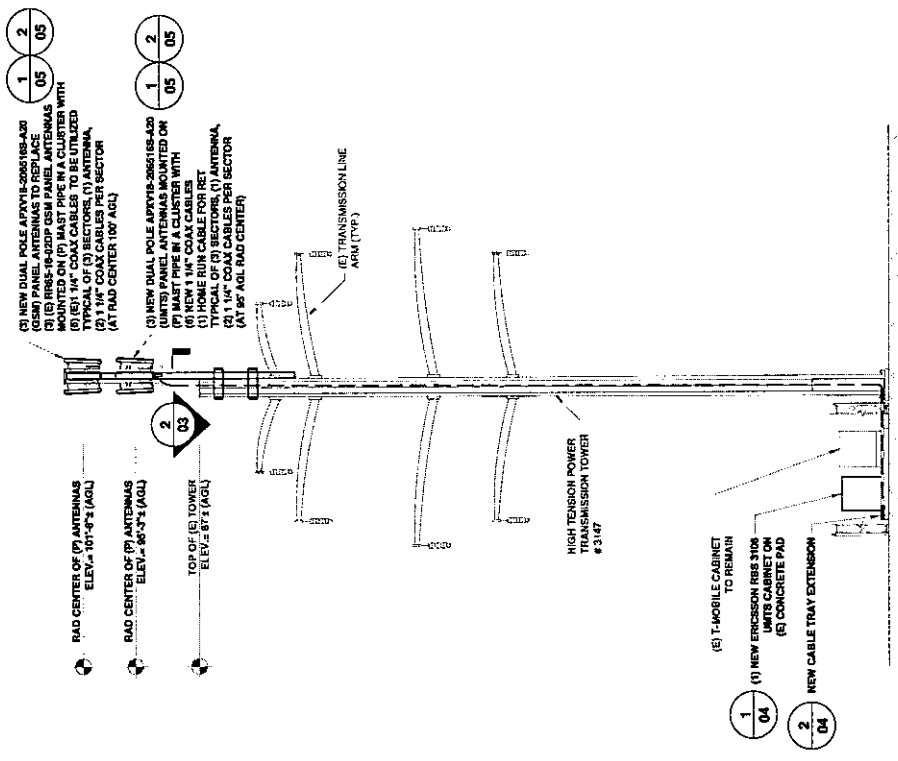
SUBMITTALS	
4	04/19/10 ISSUED FOR REVIEW
5	05/18/10 REVISED PER COMMENTS
1	06/24/10 REVISED PER COMMENTS

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CT11385A
BLOOMFIELD/
RT 187/RT 189
145 DANCASTER ROAD
BLOOMFIELD, CT 06002

SHEET TITLE:
ELEVATION

SHEET NUMBER:
03



ELEVATION
SCALE: 1/4\"/>



TRANSCEND WIRELESS, LLC

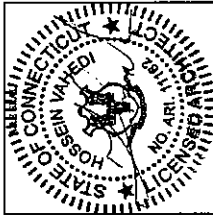
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APPROVALS

LANDLORD _____
LEASING _____
R.F. _____
ZONING _____
CONSTRUCTION _____
A/E _____

DRAWN BY: GC

CHECKED BY: SM

SUBMITTALS

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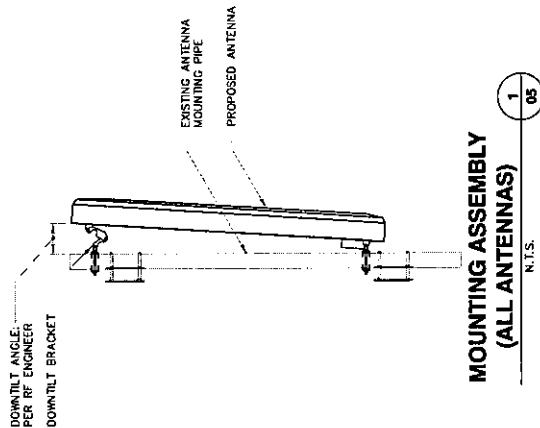
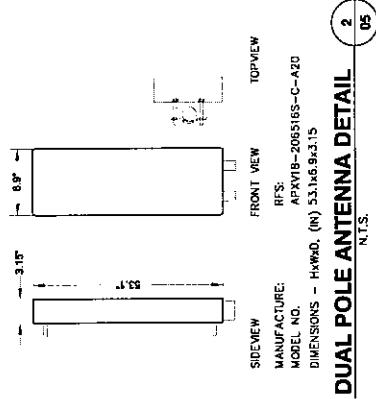
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CT11385A
BLOOMFIELD/
RT 187/RT 189
112 DANCASTER ROAD
BLOOMFIELD, CT 06002

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ANTENNA DETAILS

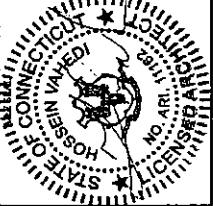
SHEET NUMBER
05



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APPROVALS
LANDLORD _____
LEASING _____
R.F. _____
ZONING _____
CONSTRUCTION _____
A/E _____

DRAWN BY: DC
CHECKED BY: SM

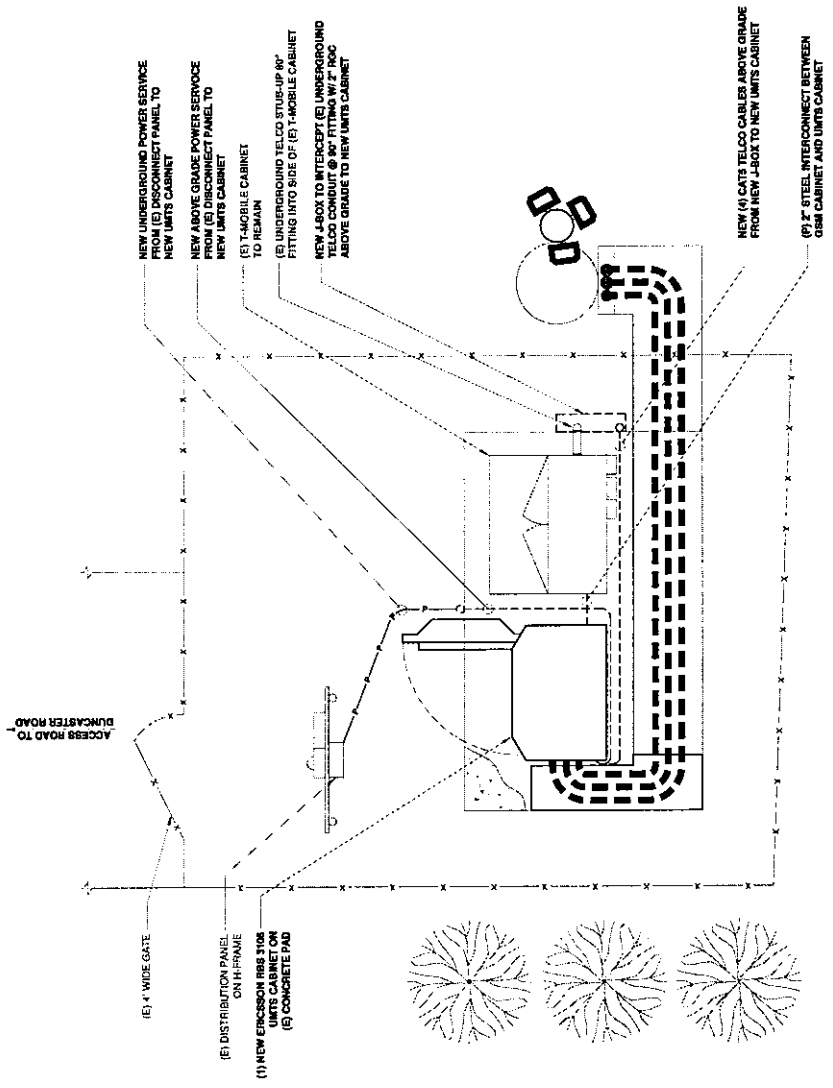
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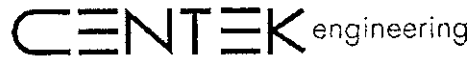
CT11385A
BLOOMFIELD/
RT 187/RT 189
123 BANCROFT RD
BLOOMFIELD, CT 06002

SHEET TITLE:
UTILITY ROUTING PLAN

SHEET NUMBER:
06



ELECTRICAL AND TELCO PLAN
1 06
SCALE: 1/4" = 1'-0" (1:48)
SCALE: 1/2" = 1'-0" (2:48)
2 0 2 4 6



Centered on Solutions™

Structural Analysis of PCS
Structure and CL&P Pole

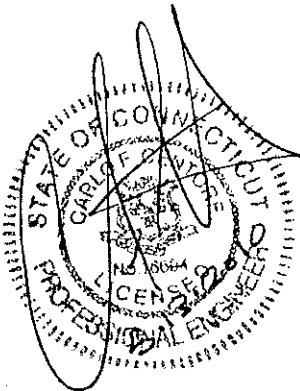
T-Mobile: CT11-385A; Bloomfield

CL&P Structure No. 3147
100' Electric Transmission Pole

142 Duncaster Road,
Bloomfield, CT

CENTEK Project No. 09045.CO1

~~Date: April 6, 2010~~
Rev 1: April 27, 2010



Prepared for:
Transcend Wireless, LLC
113 N Mountain Blvd.
Mountain Top, PA 18707

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Introduction

The purpose of this report is to analyze the existing 34" PCS mast and 100' CL&P pole located at 142 Duncaster Road Bloomfield, CT for the proposed antenna and equipment upgrade by T-Mobile.

The proposed loads consist of the following:

- **T-MOBILE (Existing to be removed):**
Antennas: Three (3) RR65-18-02DP panel antennas mounted on a PCS mast with a RAD center elevation of 100-ft above grade level.
- **T-MOBILE (Existing to remain):**
Coax Cables: Six (6) 1-1/4" Ø coax cables.
- **T-MOBILE (Proposed):**
Antennas: Six (6) RFS APXV18-206516S-C-A20 panel antennas mounted on a proposed mast in two clusters with respective RAD center elevations of 95-ft-3-in and 101-ft-8-in above grade level.
Coax Cables: Six (6) 1-1/4" Ø coax cables and one (1) home run cable.

Primary assumptions used in the analysis

- Allowable steel stresses are defined by AISC-ASD 9th edition for design of the PCS Mast and antenna supporting elements.
- ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", defines allowable steel stresses for evaluation of the CL&P utility pole.
- All utility pole members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- All coaxial cable will be installed within the pipe mast unless specified otherwise.
- Pipe mast will be properly installed and maintained.
- No residual stresses exist due to incorrect pole erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- All proposed pipe mast members will be as specified in the construction documents to be prepared by CEN TEK engineering, Inc.
- Pipe mast and utility pole will be in plumb condition.
- Utility pole was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.

A n a l y s i s

Structural analysis of the existing *PCS Mast Structure* was independently completed using the current version of RISA-3D computer program licensed to CENTEK engineering, Inc.

The existing mast consisting of a 4.5-in dia. x 34-ft long SCH. 40 pipe connected at two points to the existing tower was analyzed for its ability to resist loads prescribed by the TIA/EIA standard. Section 5 of this report details these gravity and lateral wind loads. NESC prescribed loads were also applied to the mast structure in order to obtain reactions needed for analyzing the CL&P pole structure. These loads are developed in Section 7 of this report. Load cases and combinations used in RISA-3D for TIA/EIA loading and for NESC/NU loading are listed in report Sections 6 and 8, respectively.

An envelope solution was first made to determine maximum and minimum forces, stresses, and deflections to confirm the selected section as adequate. Additional analyses were then made to determine the NESC forces to be applied to the CL&P pole structure.

The RISA-3D program contains a library of all AISC shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized. The forces calculated in RISA-3D using NESC guidelines were then applied to the CL&P pole using PLS-Pole. Maximum usage for the pole was calculated considering the additional forces from the mast and associated appurtenances.

D e s i g n B a s i s

Our analysis was performed in accordance with EIA-222-F-1996, ASCE Manual No. 72 – "Design of Steel Transmission Pole Structures Second Edition", NESC C2-2007 and Northeast Utilities Design Criteria.

The CL&P pole structure, considering existing and future conductor and shield wire loading, with the proposed antenna mast was analyzed under two conditions:

▪ UTILITY POLE ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility pole to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the NU Design Criteria Table, NESC C2-2007 ~ Construction Grade B, and ASCE Manual No. 72.

Load cases considered:

Load Case 1: NESC Heavy

Wind Pressure.....	4.0 psf
Vertical Overload Capacity Factor.....	1.50
Wind Overload Capacity Factor.....	2.50
Wire Tension Overload Capacity Factor.....	1.65

Load Case 2: NESC Extreme

Wind Speed.....	110 mph ⁽¹⁾
Radial Ice Thickness.....	0"

Note 1: NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading.
1.25 x Gust Response Factor (wind speed: 3-second gust)

▪ PCS MAST ANALYSIS

The PCS mast, appurtenances and connections to the utility pole were analyzed and designed in accordance with the NU Design Criteria Table, TIA/EIA-222-F, and AISC-ASD standards.

Load cases considered:

Load Case 1:

Wind Speed..... 85 mph ⁽²⁾
 Radial Ice Thickness..... 0"

Load Case 2:

Wind Pressure..... 75% of 85 mph wind pressure
 Radial Ice Thickness..... 0.5"

| Note 2: Per NU Mast Design Criteria Exception 1.

Results

▪ PCS MAST

The existing pipe mast was determined to be structurally **inadequate**. Replacement of the existing antenna mast with a **6 SCH. 80 Pipe x 30-ft long (O.D. = 6.63")**, conforming to ASTM A53, Grade B, $F_y = 35$ ksi specifications will be required.

▪ UTILITY POLE

This analysis finds that the subject utility pole is **adequate** to support the proposed antenna mast and related appurtenances. The pole stresses meet the requirements set forth by the ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 9 of this report. The analysis results are summarized as follows:

A maximum usage of **98.10%** occurs in the utility pole under the NESC Extreme loading condition.

POLE SECTION:

The utility pole was found to be within allowable limits.

Tower Section	Component	Stress Ratio (percentage of capacity)	Result
Tube Number 2	0' -37.0' (AGL)	98.10%	PASS

CEN TEK
 Structural Analysis – T-Mobile: CT-11-385A Bloomfield
 CL&P Structure # 3147
 Bloomfield, CT
 April 27, 2010

The CL&P utility pole is embedded approximately 13-ft into the ground. Review of the utility pole embedment consisted of verification of applied loads obtained from the tower analysis calculations and a comparison against original tower design calculations.

BASE REACTIONS:

From PLS-Pole analysis of CL&P pole based on NESC/NU prescribed loads.

Load Case	Transverse	Axial	Overturning Moment
NESC Heavy Wind	10.0 kips	42.9 kips	748.20 ft-kips
NESC Extreme Wind	17.9 kips	19.8 kips	1126.66 ft-kips
NESC Heavy Wind (Broken Shield Wire)	42.5 kips	10.9 kips	867.46 kips

The direct embedment length was found to be within allowable limits based on original tower design reactions.

Reaction Type	Original Design Reaction ⁽¹⁾	Proposed Reaction	Result
Shear	17.3 kips	17.9 kips ⁽²⁾	PASS
Compression	11.3 kips	9.2 kips ⁽³⁾	PASS
Moment	1224.0 ft-kips	1126.7 ft-kips	PASS

Note 1: Original design reactions taken from NUSCO drawing no. 01123-50000 sh. 12 of 20.

Note 2: After consultation with Northeast Utilities on 3/17/10 proposed shear force deemed acceptable.

Note 3: Original compression reaction does not include weight of utility structure. Proposed reaction reduced by weight of structure (10.6 kips) for comparison.

Conclusions

This analysis shows that the subject utility pole is adequate to support the proposed antenna mast and related appurtenances.

The analysis is based, in part on the information provided to this office by Northeast Utilities and Transcend Wireless. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:

Carlo F. Centore, PE
 Principal ~ Structural Engineer



Prepared by:

Timothy J. Lynn, EIT
 Structural Engineer

CEN TEK engineering, INC.
Structural Analysis – T-Mobile: CT-11-385A Bloomfield
CL&P Structure # 3147
Bloomfield, CT
April 27, 2010

STANDARD CONDITIONS FOR FURNISHING OF
PROFESSIONAL ENGINEERING SERVICES ON
EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ RISA - 3 D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- Modification tools that modify single items or entire selections
- Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation — draw a polygon with any number of sides to create a mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges - web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

CEN TEK engineering, INC.
Structural Analysis – T-Mobile: CT-11-385A Bloomfield
CL&P Structure # 3147
Bloomfield, CT
April 27, 2010

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

Graphics Features:

- Unlimited simultaneous model view windows
- Extraordinary “true to scale” rendering, even when drawing
- High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000, EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, MarinoWARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

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April 27, 2010

Results Features:

- Graphic presentation of color-coded results and plotted designs
- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes
- Standard and user-defined reports
- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ PLS - POLE

PLS-POLE provides all of the capabilities a structural engineer requires to design transmission, substation or communications structures. It does so using a simple easy to use graphical interface that rests upon our time tested finite element engine. Regardless of whether you want to model a simple wood pole or a guyed steel X-Frame; PLS-POLE can handle the job simply, reliably and efficiently.

Modeling Features:

- Structures are made of standard reusable components that are available in libraries. You can easily create your own libraries or get them from a manufacturer
- Structure models are built interactively using interactive menus and graphical commands
- Automatic generation of underlying finite element model of structure
- Steel poles can have circular, 4, 6, 8, 12, 16, or 18-sided, regular, elliptical or user input cross sections (flat-to-flat or tip-to-tip orientations)
- Steel and concrete poles can be selected from standard sizes available from manufacturers
- Automatic pole class selection
- Cross brace position optimizer
- Capability to specify pole ground line rotations
- Capability to model foundation displacements
- Can optionally model foundation stiffness
- Guys are easily handled (modeled as exact cable elements in nonlinear analysis)
- Powerful graphics module (members color-coded by stress usage)
- Graphical selection of joints and components allows graphical editing and checking
- Poles can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces

Analysis Features:

- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Design checks for ASCE, ANSI/TIA/EIA 222 (Revisions F and G) or other requirements
- Automatic calculation of dead and wind loads
- Automated loading on structure (wind, ice and drag coefficients) according to:
 - ASCE 74-1991
 - NESC 2002
 - NESC 2007
 - IEC 60826:2003
 - EN50341-1:2001 (CENELEC)
 - EN50341-3-9:2001 (UK NNA)
 - EN50341-3-17:2001 (Portugal NNA)
 - ESAA C(b)1-2003 (Australia)
 - TPNZ (New Zealand)
 - REE (Spain)
 - EIA/TIA 222-F
 - ANSI/TIA 222-G
 - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Detects buckling by nonlinear analysis

CEN TEK engineering. INC.
Structural Analysis – T-Mobile: CT-11-385A Bloomfield
CL&P Structure # 3147
Bloomfield, CT
April 27, 2010

Results Features:

- Detects buckling by nonlinear analysis
- Easy to interpret text, spreadsheet and graphics design summaries
- Automatic determination of allowable wind and weight spans
- Automatic determination of interaction diagrams between allowable wind and weight spans
- Automatic tracking of part numbers and costs

CEN TEK engineering, INC.
Structural Analysis – T-Mobile: CT-11-385A Bloomfield
CL&P Structure # 3147
Bloomfield, CT
April 27, 2010

Criteria for Design of PCS Facilities On or
Extending Above Metal Electric Transmission
Towers & Analysis of Transmission Towers
Supporting PCS Masts ⁽¹⁾

Introduction

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as "masts"), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA/EIA-222 covering the design of telecommunications structures specifies a working strength/allowable stress design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed some defined percentage of failure strength (allowable stress).

ANSI Standard C2-2007 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the NU effort in "unifying" both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 50-year recurrence (2% annual probability). The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

| Note 1: Prepared from documentation provide from Northeast Utilities.

P C S M a s t

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA/EIA Standard 222 with two exceptions:

1. An 85 mph extreme wind speed shall be used for locations in all counties throughout the NU system.
2. The stress increase of TIA Section 3.1.1.1 is disallowed. The combined wind and ice condition shall consider ½" radial ice in combination with the wind load (0.75 Wi) as specified in TIA section 2.3.16.

E L E C T R I C T R A N S M I S S I O N T O W E R

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled "NU Design Criteria". This specifies uniform loadings (different from the TIA loadings) on the each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors are related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those load transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.

Northeast Utilities Overhead Transmission Standards

Attachment A

NU Design Criteria

			Basic Wind Speed	Pressure	Height Factor	Gust Factor	Load or Stress Factor	Force Coef - Shape Factor	
			V (MPH)	Q (PSF)	Kz	Gh			
Ice Condition	TIA/EIA	Antenna Mount	TIA	TIA (75W)	TIA	TIA	TIA - Section 3.1.1.1 disallowed for connection design	TIA	
	NESC Heavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)	----	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces	
		Tower/Pole Analysis with Antennas below top of Tower/Pole (on two faces)			1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces	
	Conductors:		Conductor loads provided by NU						
High Wind Condition	TIA/EIA	Antenna Mount	85	TIA	TIA	TIA	TIA - Section 3.1.1.1 disallowed for connection design	TIA	
	NESC Extreme Wind	Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna						1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading Height above ground level based on top of Tower/Pole						1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU						
NESC Extreme Ice with Wind Condition*		Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna						1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load Height above ground level based on top of Tower/Pole						1.6 Flat Surfaces 1.3 Round Surfaces
	Conductors:		Conductor loads provided by NU						
* Only for Structures Installed after 2007									

Communication Antennas on Transmission Structures (CL&P & WMECo Only)

Northeast Utilities
Approved by: DEH (NU)

Design

OTRM 059
Page 7 of 9

Rev.0
11/17/2009

Northeast Utilities Overhead Transmission Standards

- 2) STEP 2 - The electric transmission structure analysis and evaluation shall be performed in accordance with NESC requirements and shall include the mast and antenna loads determined from NESC applied loading conditions (not TIA/EIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "NU Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by NU).
- c) Electric Transmission Structure
 - i) The loads from the wireless communication equipment components based on NESC and NU Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower.
 - ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2

- iii) When Coaxial Cables are mounted along side the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.3

- d) The uniform loadings and factors specified for the above components in Attachment A, "NU Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

Note: The NESC does not require ice load be included in the supporting structure. (Ice on conductors and shield wire only, and NU will provide these loads).

- e) Mast reaction loads shall be evaluated for local effects on the transmission structure members at the attachment points.

If the electric transmission structure is not sufficient to support the additional loadings of the wireless communication mast, reinforcement will be required to upgrade the strength of the overstressed members. Any reinforcement design will be reviewed by NU TL&CE to determine the feasibility of construction and its impact on the use of the structure as a transmission structure.

Communication Antennas on Transmission Structures (CL&P & WMECo Only)			
Northeast Utilities Approved by: DEH (NU)	Design	OTRM 059	Rev.0
		Page 3 of 9	11/17/2009

CT11385A



Northeast
Utilities System

107 Seiden Street, Berlin, CT 06037
 Northern Utilities Service Company
 P.O. Box 270
 Hartford, CT 06141-0270
 (860) 665-5000

Transmittal Cover Sheet

DATE	21-Aug-09	WORK ORDER
------	-----------	------------

TO	Copies	
Hans Fielder	1	Set
T-Mobile		
860-692-7123		
FROM		
Robert Gray		
Transmission Engineering		

WE ARE SENDING YOU:

☒ ATTACHED
☐ UNDER SEPARATE COVER

☐ SPECIFICATIONS
☐ PLANS
☒ PRINTS
☐ SHOP DRAWINGS
☐ COPY OF LETTER
☐ OTHER

COPIES	DATE	DWG #	DESCRIPTION
Noted		01123-S0000	Structure Drawing
			Wire loads

THESE ARE TRANSMITTED AS CHECKED BELOW

☐ FOR APPROVAL
☒ FOR YOUR USE
☐ AS REQUESTED
☐ FOR REVIEW AND COMMENT
☐ FOR BIDS DUE

☐ APPROVED AS SUBMITTED
☐ APPROVED AS NOTED
☐ RETURNED FOR CORRECTIONS
☐ RESUBMIT FOR APPROVAL
☐ SUBMIT FOR DISTRIBUTION

REMARKS:
Bloomfield, Str #4770, NOTE: There are no other drawings for this structure 311
SIGNED: <i>Robert Gray</i> Telephone: 860-665-3175

ENCLOSURES ARE NOT AS NOTED KINDLY NOTIFY US AT ONCE



Job : T-Mobile, Bloomfield
Description:

Spec. Number
Computed by
Checked by
Date
Page
of
Sheet
Date
8/21/09

INPUT DATA

TOWER ID: 3147

Structure Height (ft) : 100

Wind Zone : Central CT (green)

Wind Speed : 90.5711047 mph

Tower Type : ☒ Suspension
☐ Strain

Extreme Wind Model : PCS Addition

Shield Wire Properties:

	BACK	AHEAD
NAME =	3/8 AW	3/8 AW
DESCRIPTION =	3/8	3/8
STRANDING =	7 #8 Al Weld	7 #8 Al Weld
DIAMETER =	0.385 in	0.385 in
WEIGHT =	0.262 lb/ft	0.262 lb/ft

Conductor Properties:

	BACK	AHEAD
NAME =	BITTERN	BITTERN
Number of Conductors per phase	1	1
DIAMETER =	45/7 ACSR	45/7 ACSR
WEIGHT =	1.345 lb/ft	1.345 lb/ft

Insulator Weight = 0 lbs Broken Wire Side = AHEAD SPAN

Horizontal Line Tensions:

	BACK	Shield	Conductor	AHEAD
NESC HEAVY =	4,200	10,000	10,000	
EXTREME WIND =	3,412	9,929	9,929	
LONG. WIND =	na	na	na	
2500 COMBINED =	na	na	na	
NESC W/O OLF =	na	na	na	
60 DEG F NO WIND =	1,643	4,509	4,509	

Line Geometry:

	BACK	0	AHEAD	0	SUM
LINE ANGLE (deg) =	BACK: 250	0	AHEAD: 241	0	0
WIND SPAN (ft) =	BACK: 601	0	AHEAD: 371	0	973



Job : T-Mobile, Bloomfield
Description:

Spec. Number
Computed by
Checked by
Page of
Sheet of
Date 8/21/09
Date

WIRE LOADING AT ATTACHMENTS

TOWER ID: 3147

Wind Span = 491 ft
Weight Span = 973 ft
Total Angle = 0 degrees
Broken Wire Span = AHEAD SPAN
Type of Insulator Attachment = SUSPENSION

1. NESC RULE 250B Heavy Loading:

	INTACT CONDITION		BROKEN WIRE CONDITION	
	Horizontal	Longitudinal	Horizontal	Longitudinal
Shield Wire =	557 lb	0 lb	289 lb	733 lb
Conductor =	960 lb	0 lb	489 lb	2,327 lb

2. NESC RULE 250C Transverse Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	371 lb	0 lb	293 lb
Conductor =	1,298 lb	0 lb	1,602 lb

3. NESC RULE 250C Longitudinal Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	293 lb
Conductor =	#VALUE!	#VALUE!	1,602 lb

4. NESC RULE 250D Extreme Ice & Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	1,930 lb
Conductor =	#VALUE!	#VALUE!	4,229 lb

5. NESC RULE 250B w/o O.L.F.'s

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	790 lb
Conductor =	#VALUE!	#VALUE!	2,509 lb

6. 50 Deg. E. No Wind

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	255 lb
Conductor =	0 lb	0 lb	1,393 lb

7. Construction

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	382 lb
Conductor =	0 lb	0 lb	2,089 lb

[NOTE: All loads include required overload factors (O.L.F.'s).]

MC-1

GENERAL NOTES:

1. ALL WORK SHALL BE IN ACCORDANCE WITH TIA/EIA-222 REVISION "E," "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND SUPPORTING STRUCTURES," ASCE MANUAL, NO. 72, DESIGN OF STEEL TRANSMISSION POLE STRUCTURES SECOND EDITION; NESC C2-2002 AND NORTHEAST UTILITIES DESIGN CRITERIA.
2. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHOPPING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTH-EAST UTILITIES.
3. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
4. THE CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS AT THE SITE, PRIOR TO FABRICATION AND/OR INSTALLATION OF ANY WORK IN THE CONTRACT AREA.
5. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
6. NO DRILLING, TAPPING OR WELDING ON CL&P OWNED EQUIPMENT

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL IS DESIGNED BY ALLOWABLE STRESS DESIGN (ASD)
 - A. STRUCTURAL STEEL (W SHAPES)---ASTM A992 (FY = 50 KSI)
 - B. STRUCTURAL STEEL (OTHER SHAPES)---ASTM A36 (FY = 36 KSI)
 - C. STRUCTURAL HSS (RECTANGULAR SHAPES)---ASTM A500 GRADE B, (FY = 46 KSI)
 - D. STRUCTURAL HSS (ROUND SHAPES)---ASTM A500 GRADE B, (FY = 42 KSI)
 - E. PIPE---ASTM A53 (FY = 35 KSI)
 - F. CONNECTION BOLTS---ASTM A325-N
 - G. ANCHOR BOLTS---ASTM A307
 - H. ANCHOR RODS---ASTM F 1554
 - I. WELDING ELECTRODE---ASTM E 70XX
2. CONTRACTOR TO REVIEW ALL SHOP DRAWINGS AND SUBMIT COPY TO ENGINEER FOR APPROVAL. DRAWINGS MUST BEAR THE CHECKER'S INITIALS BEFORE SUBMITTING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL INCLUDE THE FOLLOWING: SECTION PROFILES, SIZES, DIMENSIONS, ELEVATIONS, ANGLES, AND DETAILS OF FASTENERS AND ACCESSORIES. INCLUDE ERECTION DRAWINGS, ELEVATIONS AND DETAILS.
3. LATEST PROVISIONS OF AISC MANUAL OF STEEL CONSTRUCTION.
4. PROVIDE ALL PLATES, CLIP ANGLES, CLOSURE PIECES, STRAP ANCHORS, MISCELLANEOUS PIECES AND HOLES REQUIRED TO COMPLETE THE STRUCTURE.
5. FIT AND SHOP ASSEMBLE FABRICATIONS IN THE LARGEST PRACTICAL SECTIONS FOR DELIVERY TO SITE.
6. INSTALL FABRICATIONS PLUMB AND LEVEL, ACCURATELY FITTED, AND FREE FROM DISTORTIONS OR DEFECTS.
7. AFTER ERECTION OF STRUCTURES, TOUCHUP ALL WELDS, ABRASIONS AND NON-GALVANIZED SURFACES WITH A 95% ORGANIC ZINC RICH PAINT IN ACCORDANCE WITH ASTM 780.
8. ALL STEEL MATERIAL (EXPOSED TO WEATHER) SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT DIPPED GALVANIZED) COATINGS" ON IRONS AND STEEL PRODUCTS.
9. ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING (HOT-DIP) ON IRON AND STEEL HARDWARE."
10. CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES APPEARANCE AND QUALITY OF WELDS, AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND D1.1 WHERE FILLET WELD SIZES ARE NOT SHOWN. PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC MANUAL OF STEEL CONSTRUCTION" SPECIFIED AT THE COMPLETION OF WELDING. ALL DAMAGE TO GALVANIZED COATING SHALL BE REPAIRED.
11. THE ENGINEER SHALL BE NOTIFIED OF ANY INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON CONFORMING MATERIALS OR CONDITIONS TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE ENGINEER REVIEW.
12. CONNECTION ANGLES SHALL HAVE A MINIMUM THICKNESS OF 1/4 INCHES.
13. STRUCTURAL CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ALL BOLTS SHALL BE 3/4" DIAMETER MINIMUM AND SHALL HAVE A MINIMUM OF TWO BOLTS, UNLESS OTHERWISE ON THE DRAWINGS.
14. LOCK WASHER ARE NOT PERMITTED FOR A325 STEEL ASSEMBLIES.
15. SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.
16. MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.
17. FABRICATE BEAMS WITH MILL CAMBER UP.
18. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF THE STRUCTURE TO AN ACCURACY OF 1:500, BUT NOT TO EXCEED 1/4" IN THE FULL HEIGHT OF THE COLUMN.
19. COMMENCEMENT OF STRUCTURAL STEEL WORK WITHOUT NOTIFYING THE ENGINEER OF ANY DISCREPANCIES WILL BE CONSIDERED ACCEPTANCE OF PRECEDING WORK.
20. INSPECTION AND TESTING OF ALL WELDING AND HIGH STRENGTH BOLTING SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY.
21. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE ENGINEER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

DESIGNED BY:	TIA
CHECKED BY:	CTC
DATE:	04/08/02
SCALE:	AS SHOWN
JOB NO:	000143.001
T-MOBILE CL&P POLE # 3147 CT11385A REVISION NO. 01 REVISION DATE 04/08/02	
STRUCTURAL NOTES	
SHEET NO. 1 OF 1	

Technical Memo

To: Transcend
From: Amir Uzzaman - Radio Frequency Engineer
cc: Jason Overbey
Subject: Power Density Report for CT11385A
Date: July 1, 2010

1. Introduction:

This report is the result of an Electromagnetic Field Intensities (EMF - Power Densities) study for the T-Mobile antenna installation on a Utility Monopole at 142 Duncaster Road, Bloomfield, CT. This study incorporates the most conservative consideration for determining the practical combined worst case power density levels that would be theoretically encountered from locations surrounding the transmitting location.

2. Discussion:

The following assumptions were used in the calculations:

- 1) The emissions from T-Mobile transmitters are in the (1935-1944.8), (2140-2145), (2110-2120)MHz frequency Band.
- 2) The antenna array consists of three sectors, with 2 antennas per sector.
- 3) The model number of the antennas are APXV18-206516, APXV18-206517-C, APXV18-206517-C
- 3) The model number for UMTS antenna is APXV18-206516.
- 4) GSM antenna center line height is 101 ft.
- 4) UMTS antenna center line height is 95 ft.
- 5) The maximum transmit power from any GSM sector is 2134.62 Watts Effective Radiated Power (EIRP) assuming 8 channels per sector.
- 5) The maximum transmit power from any UMTS sector is 2365.32 Watts Effective Radiated Power (EIRP) assuming 2 channels per sector.
- 6) All the antennas are simultaneously transmitting and receiving, 24 hours a day.
- 7) Power levels emitting from the antennas are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 8) The average ground level of the studied area does not change significantly with respect to the transmitting location.

Equations given in "FCC OET Bulletin 65, Edition 97-01" were then used with the above information to perform the calculations.

3. Conclusion:

Based on the above worst case assumptions, the power density calculation from the T-Mobile antenna installation on a Utility Monopole at 142 Duncaster Road, Bloomfield, CT, is 0.1174 mW/cm². This value represents 11.74% of the Maximum Permissible Exposure (MPE) standard of 1 milliwatt per square centimeter (mW/cm²) set forth in the FCC/ANSI/IEEE C95.1-1991. Furthermore, the proposed antenna location for T-Mobile will not interfere with existing public safety communications, AM or FM radio broadcasts, TV, Police Communications, HAM Radio communications or any other signals in the area.

Connecticut Market**T-Mobile****Worst Case Power Density**

Site: CT11385A
 Site Address: 142 Duncaster Road
 Town: Bloomfield
 Tower Height: 104 ft.
 Tower Style: Utility Monopole

GSM Data		UMTS Data	
Base Station TX output	20 W	Base Station TX output	40 W
Number of channels	8	Number of channels	2
Antenna Model	APXV18-206516	Antenna Model	APXV18-206516
Cable Size	1 1/4 in.	Cable Size	1 1/4 in.
Cable Length	120 ft.	Cable Length	120 ft.
Antenna Height	101.0 ft.	Antenna Height	95.0 ft.
Ground Reflection	1.6	Ground Reflection	1.6
Frequency	1945.0 MHz	Frequency	2.1 GHz
Jumper & Connector loss	4.50 dB	Jumper & Connector loss	1.50 dB
Antenna Gain	17.6 dBi	Antenna Gain	17.6 dBi
Cable Loss per foot	0.0154 dB	Cable Loss per foot	0.0116 dB
Total Cable Loss	1.8480 dB	Total Cable Loss	1.3920 dB
Total Attenuation	6.3480 dB	Total Attenuation	2.8920 dB
Total EIRP per Channel (In Watts)	54.26 dBm 266.83 W	Total EIRP per Channel (In Watts)	60.73 dBm 1182.66 W
Total EIRP per Sector (In Watts)	63.29 dBm 2134.62 W	Total EIRP per Sector (In Watts)	63.74 dBm 2365.32 W
nsg	11.2520	nsg	14.7080
Power Density (S) = 0.051891 mW/cm ²		Power Density (S) = 0.065513 mW/cm ²	
T-Mobile Worst Case % MPE =		11.7405%	

Equation Used :

$$S = \frac{(1000)(grf)^2 (Power) 10^{(nsg/10)}}{4\pi(R)^2}$$

Office of Engineering and Technology (OET) Bulletin 65, Edition 97-01, August 1997

Co-Location Total

Carrier	% of Standard
Sprint/Nextel WiMax	
Cingular	
Sprint CDMA	
AT&T Wireless	
Pocket	
MetroPCS	
Nextel iDen	
Other Antenna Systems	
Total Excluding T-Mobile	0.0000 %
T-Mobile	11.7405
Total % MPE for Site	11.7405%