

Crown Castle 3530 Toringdon Way Suite 300 Charlotte, NC 28277 Tel: 704-405-6600

www.crowncastle.com

June 16, 2014

Melanie A. Bachman Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Sprint PCS-Exempt Modification - Crown Site BU: 876329

Sprint PCS Site ID: CT03XC076

Located at: 28 Brewer Drive, Bloomfield, CT 06002

Dear Ms. Bachman:

This letter and exhibits are submitted on behalf of Sprint PCS (Sprint). Sprint is making modifications to certain existing sites in its Connecticut system in order to implement their 2.5GHz LTE technology. Please accept this letter and exhibits as notification, pursuant to § 16-50j-73 of the Regulations of Connecticut State Agencies ("R.C.S.A."), of construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In compliance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Mr. Philip K. Schenck, Jr., Manager for Town of Bloomfield.

Sprint plans to modify the existing wireless communications facility owned by Crown Castle and located at **28 Brewer Drive**, **Bloomfield**, **CT 06002**. Attached are a compound plan and elevation depicting the planned changes (Exhibit-1), and documentation of the structural sufficiency of the structure to accommodate the revised antenna configuration (Exhibit-2). Also included is a power density table report reflecting the modification to Sprint's operations at the site (Exhibit-3).

The changes to the facility do not constitute a modification as defined in Connecticut General Statutes ("C.G.S.") § 16-50i(d) because the general physical characteristics of the facility will not be significantly changed. Rather, the planned changes to the facility fall squarely within those activities explicitly provided for in the R.C.S.A. § 16-50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing tower. Sprint's additional antennas will be located at the same elevation on the existing tower.
- 2. There will be no proposed modifications to the ground and no extension of boundaries.
- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more.

- 4. A Structural Modification Report confirming that the tower and foundation can support Sprint's proposed modifications is included as Exhibit-2.
- 5. The operation of the additional antennas will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) adopted safety standard. A cumulative General Power Density table report for Sprint's modified facility is included as Exhibit-3.

For the foregoing reasons, Sprint respectfully submits the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Donna Neal.

Sincerely,

Jeff Barbadora

Real Estate Specialist

Jeff Barbla

# Enclosures

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. Philip K. Schenck, Jr., Town Manager Town of Bloomfield 800 Bloomfield Avenue, 2<sup>nd</sup> Floor

Bloomfield, CT 06002





SITE

SITE INFORMATION

**TOWER OWNER:** 

LATITUDE (NAD83):

**LONGITUDE (NAD83):** 

**ZONING JURISDICTION:** CONNECTICUT SITING COUNCIL

**ZONING DISTRICT:** RESIDENTIAL

**POWER COMPANY:** CONNECTICUT LIGHT & POWER

(800) 286-2000

**SPRINT CM:** 

PETER CULBERT (603) 203-6446 (603) 969-0686

peter.culbert@sprint.com

CROWN CASTLE CM: JASON D'AMICO (860) 209-0104

JASON.D'AMICO CROWNCASTLE.COM

**AAV PROVIDER:** 

41° 50′ 6.576″ N 41.83516°

72° 44' 28.0674" W -72.74113"

**COUNTY:** 

HARTFORD

CROWN ATLANTIC COMPANY LLC 2000 CORPORATE DRIVE CANONSBURG, PA 15317

PROJECT: 2.5 EQUIPMENT DEPLOYMENT

MTN. VIEW CEM (FILLEY PARK) SITE NAME:

SITE CASCADE: CT03XC076

876329 SITE NUMBER:

28 BREWER DR SITE ADDRESS:

BLOOMFIELD, CT 06002

**APPROVED** 

By Jeff Barbadora at 12:27 pm, Jun 05, 2014

Call before you dig.

SITE TYPE: MONOPOLE TOWER

MARKET: NORTHERN CONNECTICUT

#### DRAWING INDEX PROJECT DESCRIPTION AREA MAP SPRINT PROPOSES TO MODIFY AN EXISTING UNMANNED SHEET TITLE SHEET NO: TELECOMMUNICATIONS FACILITY. TITLE SHEET & PROJECT DATA INSTALL (1) 9929 EQUIPMENT CABINET Simsbury INSTALL (3) PANEL ANTENNAS SP-1 SPRINT SPECIFICATIONS SP-2 SPRINT SPECIFICATIONS Simsbury INSTALL (3) RRU'S TO TOWER SPRINT SPECIFICATIONS SP-3 INSTALL (27) JUMPER CABLES INSTALL (1) HYBRID CABLE TOWER ELEVATION & CABLE PLAN A-2 Windsor ANTENNA LAYOUT & MOUNTING DETAILS A-3 INSTALL (8) BATTERIES IN EXISTING BBU CABINET KING PHISITE COLOR CODING & NOTES A-4 REMOVE EXISTING CLEARWIRE CABINET, PANEL ANTENNAS AND RELOCATE RADIOS EQUIPMENT & MOUNTING DETAILS A-5 CIVIL DETAILS A-6 PLUMBING DIAGRAM A-7 ELECTRICAL & GROUNDING PLAN E-1 THESE PLANS HAVE BEEN DEVELOPED FOR THE MODIFICATION OF AN EXISTING UNMANNED TELECOMMUNICATIONS FACILITY OWNED OR LEASED BY SPRINT IN ACCORDANCE WITH THE SCOPE OF WORK PROVIDED BY SPRINT. INFINICY HAS INCORPORATED THIS SCOPE OF WORK IN THE PLANS. THESE PLANS ARE NOT FOR CONSTRUCTION UNLESS ACCOMPANIED BY A PASSING STRUCTURAL STABILITY ANALYSIS PREPARED BY A LICENSED STRUCTURAL ENGINEER. STRUCTURAL ANALYSIS MUST INCLUDE BOTH TOWER AND MOUNT ELECTRICAL & GROUNDING DETAILS E-2 Country Club Rd West Hartford LOCATION MAP APPLICABLE CODES ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALL IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES. INTERNATIONAL BUILDING CODE (2012 IBC) INIERNALIONAL BUILDING CODE (2012 IBC) TIA-EIA-222-F OR LATEST EDITION NFPA 780 - LIGHTNING PROTECTION CODE 2011 NATIONAL ELECTRIC CODE OR LATEST EDITION ANY OTHER NATIONAL OR LOCAL APPLICABLE CODES, MOST RECENT EDITIONS CT. BUILDING CODE

6. CT BUILDING CODE

8. CITY/COUNTY ORDINANCES



Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB NUMBER 353-XXX

MI A PARTNER:





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THESE DOCUMENTS ARE CONFIDENTIAL AND ARE THE SOLE PROPERTY OF SPRINT AND MAY NOT BE WITHOUT THE EXPRESS WRITTEN CONSENT OF

REVISIONS:		-	_
DESCRIPTION	DATE	BY	REV
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	_		$\vdash$
ISSUED FOR CONSTRUCTION	6/04/14	MAP	0

MTN. VIEW CEM (FILLEY PARK)

CT03XC076

28 BREWER DR BLOOMFIELD, CT 06002

TITLE SHEET & PROJECT DATA

T-1

THESE OUTLINE SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS, INCLUDING CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.

#### SECTION 01 100 - SCOPE OF WORK

#### PART 1 - GENERAL

1.1 THE WORK: THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT CONSTRUCTION STANDARDS FOR WIRELESS SITES, CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.

#### 1.2 RELATED DOCUMENTS:

- A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
- B. SPRINT "STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES" ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWITH.
- 1.3 PRECEDENCE: SHOULD CONFLICTS OCCUR BETWEEN THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES INCLUDING THE STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES AND THE CONSTRUCTION DRAWINGS, INFORMATION ON THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE. NOTIFY SPRINT CONSTRUCTION MANAGER IF THIS OCCURS

#### 1.4 NATIONALLY RECOGNIZED CODES AND STANDARDS:

- A. THE WORK SHALL COMPLY WITH APPLICABLE NATIONAL AND LOCAL CODES AND STANDARDS, LATEST EDITION, AND PORTIONS THEREOF, INCLUDED BUT NOT LIMITED TO THE FOIL OWING:
  - 1. GR-63-CORE NEBS REQUIREMENTS: PHYSICAL PROTECTION
  - GR-78-CORE GENERIC REQUIREMENTS FOR THE PHYSICAL DESIGN AND MANUFACTURE OF TELECOMMUNICATIONS EQUIPMENT.
- 3. GR-1089 CORE, ELECTROMAGNETIC COMPATIBILITY AND ELECTRICAL SAFETY
  -GENERIC CRITERIA FOR NETWORK TELECOMMUNICATIONS EQUIPMENT.
- NATIONAL FIRE PROTECTION ASSOCIATION CODES AND STANDARDS (NFPA) INCLUDING NFPA 70 (NATIONAL ELECTRICAL CODE — "NEC") AND NFPA 101 (LIFE SAFETY CODE).
- 5. AMERICAN SOCIETY FOR TESTING OF MATERIALS (ASTM)
- 6. INSTITUTE OF ELECTRONIC AND ELECTRICAL ENGINEERS (IEEE)
- 7. AMERICAN CONCRETE INSTITUTE (ACI)
- 8. AMERICAN WIRE PRODUCERS ASSOCIATION (AWPA)
- 9. CONCRETE REINFORCING STEEL INSTITUTE (CRSI)
- AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO)
- 11. PORTLAND CEMENT ASSOCIATION (PCA)
- 12. NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA)
- 13. BRICK INDUSTRY ASSOCIATION (BIA)
- 14. AMERICAN WELDING SOCIETY (AWS)
- 15. NATIONAL ROOFING CONTRACTORS ASSOCIATION (NRCA)
- SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION (SMACNA)
- 17. DOOR AND HARDWARE INSTITUTE (DHI)
- 18. OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA)
- 19. APPLICABLE BUILDING CODES INCLUDING UNIFORM BUILDING CODE, SOUTHERN BUILDING CODE, BOCA, AND THE INTERNATIONAL BUILDING CODE.

#### 1.5 DEFINITIONS

- A. WORK: THE SUM OF TASKS AND RESPONSIBILITIES IDENTIFIED IN THE CONTRACT DOCUMENTS.
- B. COMPANY: SPRINT CORPORATION
- C. ENGINEER: SYNONYMOUS WITH ARCHITECT & ENGINEER AND "A&E". THE DESIGN PROFESSIONAL HAVING PROFESSIONAL RESPONSIBILITY FOR DESIGN OF THE PROJECT.
- D. CONTRACTOR: CONSTRUCTION CONTRACTOR; CONSTRUCTION VENDOR; INDIVIDUAL OR ENTITY WHO AFTER EXECUTION OF A CONTRACT IS BOUND TO ACCOMPLISH THE WORK.
- E. THIRD PARTY VENDOR OR AGENCY: A VENDOR OR AGENCY ENGAGED SEPARATELY BY THE COMPANY, A&E, OR CONTRACTOR TO PROVIDE MATERIALS OR TO ACCOMPLISH SPECIFIC TASKS RELATED TO BUT NOT INCLUDED IN THE WORK.
- F. OFCI: OWNER FURNISHED, CONTRACTOR INSTALLED EQUIPMENT.
- G. CONSTRUCTION MANAGER ALL PROJECTS RELATED COMMUNICATION TO FLOW THROUGH SPRINT REPRESENTATIVE IN CHARGE OF PROJECT...

- 1.6 SITE FAMILIARITY: CONTRACTOR SHALL BE RESPONSIBLE FOR FAMILIARIZING HIMSELF WITH ALL CONTRACT DOCUMENTS, FIELD CONDITIONS AND DIMENSIONS PRIOR TO PROCEEDING WITH CONSTRUCTION. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE SPRINT CONSTRUCTION MANAGER PRIOR TO THE COMMENCEMENT OF WORK. NO COMPENSATION WILL BE AWARDED BASED ON CLAIM OF LACK OF KNOWLEDGE OR FIELD CONDITIONS.
- 1.7 POINT OF CONTACT: COMMUNICATION BETWEEN SPRINT AND THE CONTRACTOR SHALL FLOW THROUGH THE SINGLE SPRINT CONSTRUCTION MANAGER APPOINTED TO MANAGE THE PART OF SERVINT
- 1.8 ON—SITE SUPERVISION: THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY A COMPETENT SUPERINTENDENT WHO SHALL BE IN ATTENDANCE AT THE SITE AT ALL TIMES DURING PERFORMANCE OF THE WORK.
- 1.9 DRAWINGS, SPECIFICATIONS AND DETAILS REQUIRED AT JOBSITE: THE CONSTRUCTION CONTRACTOR SHALL MAINTAIN A FULL SET OF THE CONSTRUCTION DRAWINGS, STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES AND THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES AT THE JOBSITE FROM MOBILIZATION THROUGH CONSTRUCTION COMPLETION.
- A. THE JOBSITE DRAWINGS, SPECIFICATIONS AND DETAILS SHALL BE CLEARLY MARKED DAILY IN RED PENCIL WITH ANY CHANGES IN CONSTRUCTION OVER WHAT IS DEPICTED IN THE DOCUMENTS. AT CONSTRUCTION COMPLETION, THIS JOBSITE MARKUP SET SHALL BE DELIVERED TO THE COMPANY OR COMPANY'S DESIGNATED REPRESENTATIVE TO BE FORWARDED TO THE COMPANY'S A&E VENDOR FOR PRODUCTION OF "AS-RUILT DRAWINGS."
- B. DETAILS ARE INTENDED TO SHOW DESIGN INTENT. MODIFICATIONS MAY BE REQUIRED TO SUIT JOB DIMENSIONS OR CONDITIONS, AND SUCH MODIFICATIONS SHALL BE INCLUDED AS PART OF THE WORK. CONTRACTOR SHALL NOTIFY SPRINT CONSTRUCTION MANAGER OF ANY VARIATIONS PRIOR TO PROCEEDING WITH THE WORK.
- C. DIMENSIONS SHOWN ARE TO FINISH SURFACES UNLESS NOTED OTHERWISE.
  SPACING BETWEEN EQUIPMENT IS THE REQUIRED CLEARANCE. SHOULD THERE BE
  ANY QUESTIONS REGARDING THE CONTRACT DOCUMENTS, EXISTING CONDITIONS
  AND/OR DESIGN INTENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING
  A CLARIFICATION FROM THE SPRINT CONSTRUCTION MANAGER PRIOR TO
  BEDGEFORMS WITH THE WORK
- 1.10 USE OF JOB SITE: THE CONTRACTOR SHALL CONFINE ALL CONSTRUCTION AND RELATED OPERATIONS INCLUDING STAGING AND STORAGE OF MATERIALS AND EQUIPMENT, PARKING, TEMPORARY FACILITIES, AND WASTE STORAGE TO THE LEASE PARCEL UNLESS OTHERWISE PERMITTED BY THE CONTRACT DOCUMENTS.
- 1.11 UTILITIES SERVICES: WHERE NECESSARY TO CUT EXISTING PIPES, ELECTRICAL WIRES, CONDUITS, CABLES, ETC., OF UTILITY SERVICES, OR OF FIRE PROTECTION OR COMMUNICATIONS SYSTEMS, THEY SHALL BE CUT AND CAPPED AT SUITABLE PLACES OR WHERE SHOWN. ALL SUCH ACTIONS SHALL BE COORDINATED WITH THE UTILITY COMPANY INVOLVED:
- 1.12 PERMITS / FEES: WHEN REQUIRED THAT A PERMIT OR CONNECTION FEE BE PAID TO A PUBLIC UTILITY PROVIDER FOR NEW SERVICE TO THE CONSTRUCTION PROJECT, PAYMENT OF SUCH FEE SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- 1.13 CONTRACTOR SHALL TAKE ALL MEASURES AND PROVIDE ALL MATERIAL NECESSARY FOR PROTECTING EXISTING EQUIPMENT AND PROPERTY.
- 1.14 METHODS OF PROCEDURE (MOPS) FOR CONSTRUCTION: CONTRACTOR SHALL PERFORM WORK AS DESCRIBED IN THE FOLLOWING INSTALLATION AND COMMISSIONING MOPS.

NOTE: IN SHORT-FORM SPECIFICATIONS ON THE DRAWINGS, A/E TO INSERT LIST OF APPLICABLE MOPS INCLUDING EN-2012-001, EN-2013-002, EL-0568, AND TS-0193

1.15 USE OF ELECTRONIC PROJECT MANAGEMENT SYSTEMS:

#### PART 2 - PRODUCTS (NOT USED)

#### PART 3 - EXECUTION

- 3.1 TEMPORARY UTILITIES AND FACILITIES: THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY UTILITIES AND FACILITIES NECESSARY EXCEPT AS OTHERWISE INDICATED IN THE CONSTRUCTION DOCUMENTS. TEMPORARY UTILITIES AND FACILITIES INCLUDE POTABLE WATER, HEAT, HVAC, ELECTRICITY, SANITARY FACILITIES, WASTE DISPOSAL FACILITIES, AND TELEPHONE/COMMUNICATION SERVICES. PROVIDE TEMPORARY UTILITIES AND FACILITIES IN ACCORDANCE WITH OSHA AND THE AUTHORITY HAVING JURISDICTION. CONTRACTOR MAY UTILIZE THE COMPANY ELECTRICAL SERVICE IN THE COMPLETION OF THE WORK WHEN IT BECOMES AVAILABLE. USE OF THE LESSORS OR SITE OWNER'S UTILITIES OR FACILITIES IS EXPRESSLY FORBIDDEN EXCEPT AS OTHERWISE ALLOWED IN THE CONTRACT DOCUMENTS.
- 3.2 ACCESS TO WORK: THE CONTRACTOR SHALL PROVIDE ACCESS TO THE JOB SITE FOR AUTHORIZED COMPANY PERSONNEL AND AUTHORIZED REPRESENTATIVES OF THE ARCHITECT/ENGINEER DURING ALL PHASES OF THE WORK.
- 3.3 TESTING: REQUIREMENTS FOR TESTING BY THIS CONTRACTOR SHALL BE AS INDICATED HEREWITH, ON THE CONSTRUCTION DRAWINGS, AND IN THE INDIVIDUAL SECTIONS OF THESE SPECIFICATIONS. SHOULD COMPANY CHOOSE TO ENGAGE ANY THIRD—PARTY TO CONDUCT ADDITIONAL TESTING, THE CONTRACTOR SHALL COOPERATE WITH AND PROVIDE A WORK AREA FOR COMPANY'S TEST AGENCY.
- 3.4 DIMENSIONS: VERIFY DIMENSIONS INDICATED ON DRAWINGS WITH FIELD DIMENSIONS BEFORE FABRICATION OR ORDERING OF MATERIALS. DO NOT SCALE DRAWINGS.

3.5 EXISTING CONDITIONS: NOTIFY THE SPRINT CONSTRUCTION MANAGER OF EXISTING CONDITIONS DIFFERING FROM THOSE INDICATED ON THE DRAWINGS. DO NOT REMOVE OR ALTER STRUCTURAL COMPONENTS WITHOUT PRIOR WRITTEN APPROVAL FROM THE ARCHITECT AND ENGINEER.

# SECTION 01 200 - COMPANY FURNISHED MATERIAL AND EQUIPMENT PART 1 - GENERAL

1.1 THE WORK: THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.

#### 1.2 RELATED DOCUMENTS:

- A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
- B. SPRINT "STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES" ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWITH.

PART 2 - PRODUCTS (NOT USED)

3.1 RECEIPT OF MATERIAL AND EQUIPMENT:

# PART 3 - EXECUTION

- A. A COMPANY FURNISHED MATERIAL AND EQUIPMENT IS IDENTIFIED ON THE RF DATA SHEET IN THE CONSTRUCTION DOCUMENTS.
- B. THE CONTRACTOR IS RESPONSIBLE FOR SPRINT PROVIDED MATERIAL AND EQUIPMENT AND UPON RECEIPT SHALL:
- 1 ACCEPT DELIVERIES AS SHIPPED AND TAKE RECEIPT.
- 2. VERIFY COMPLETENESS AND CONDITION OF ALL DELIVERIES.
- TAKE RESPONSIBILITY FOR EQUIPMENT AND PROVIDE INSURANCE PROTECTION AS REQUIRED IN AGREEMENT.
- RECORD ANY DEFECTS OR DAMAGES AND WITHIN TWENTY—FOUR HOURS AFTER RECEIPT, REPORT TO SPRINT OR ITS DESIGNATED PROJECT REPRESENTATIVE OF SIGH.
- 5. PROVIDE SECURE AND NECESSARY WEATHER PROTECTED WAREHOUSING.
- COORDINATE SAFE AND SECURE TRANSPORTATION OF MATERIAL AND EQUIPMENT, DELIVERING AND OFF—LOADING FROM CONTRACTOR'S WAREHOUSE TO SITE

#### 3.2 DELIVERABLES:

- A. COMPLETE SHIPPING AND RECEIPT DOCUMENTATION IN ACCORDANCE WITH COMPANY PRACTICE.
- B. IF APPLICABLE, COMPLETE LOST/STOLEN/DAMAGED DOCUMENTATION REPORT AS NECESSARY IN ACCORDANCE WITH COMPANY PRACTICE, AND AS DIRECTED BY COMPANY.
- C. UPLOAD DOCUMENTATION INTO SPRINT SITE MANAGEMENT SYSTEM (SMS) AND/OR PROVIDE HARD COPY DOCUMENTATION AS REQUESTED.

# SECTION 01 300 - CELL SITE CONSTRUCTION CO. PART 1 - GENERAL

1.1 THE WORK: THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.

#### 1.2 RELATED DOCUMENTS:

- A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
- B. SPRINT 'STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES' ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWITH.

## 1.3 NOTICE TO PROCEED

- A. NO WORK SHALL COMMENCE PRIOR TO COMPANY'S WRITTEN NOTICE TO PROCEED AND THE ISSUANCE OF THE WORK ORDER.
- B. UPON RECEIVING NOTICE TO PROCEED, CONTRACTOR SHALL FULLY PERFORM ALL WORK NECESSARY TO PROVIDE SPRINT WITH AN OPERATIONAL WIRELESS FACILITY.

ONCE THE CONTRACTOR HAS RECEIVED AND ACCEPTED THE NOTICE TO PROCEED, CONTRACTOR WILL CONTACT THE CROWN CASTLE CONSTRUCTION MANAGER OF RECORD (NOTED ON THE FIRST PAGE ON THIS CONSTRUCTION DRAWING) A MINIMUM OF 48 HOURS PRIOR TO WORK START. UPON ARRIVAL TO THE JOB SITE, CONTRACTOR CREW IS REQUIRED CALL 1-800-788-7011 TO NOTIFY THE CROWN CASTLE NOC WORK HAS BEGUN.

# PART 2 - PRODUCTS (NOT USED) PART 3 - EXECUTION

#### 3.1 FUNCTIONAL REQUIREMENTS:

- A. THE ACTIVITIES DESCRIBED IN THIS PARAGRAPH REPRESENT MINIMUM ACTIONS AND PROCESSES REQUIRED TO SUCCESSFULLY COMPLETE THE WORK. THE ACTIVITIES DESCRIBED ARE NOT EXHAUSTIVE, AND CONTRACTOR SHALL TAKE ANY AND ALL ACTIONS AS NECESSARY TO SUCCESSFULLY COMPLETE THE CONSTRUCTION OF A FULLY FUNCTIONING WIRELESS FACILITY AT THE SITE IN ACCORDANCE WITH COMPANY PROCESSES.
- B. SUBMIT SPECIFIC DOCUMENTATION AS INDICATED HEREIN, AND OBTAIN REQUIRED APPROVALS WHILE THE WORK IS BEING PERFORMED.
- C. MANAGE AND CONDUCT ALL FIELD CONSTRUCTION SERVICE RELATED ACTIVITIES
- D. PROVIDE CONSTRUCTION ACTIVITIES TO THE EXTENT REQUIRED BY THE CONTRACT DOCUMENTS, INCLUDING BUT NOT LIMITED TO THE FOLLOWING:

Sprint Sprint

6580 Sprint Parkway Overland Park, Kansas 66251

PLANS PREPARED BY:

INFINIGY Build.

1033 Watervliet Shaker Rd Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB NUMBER 353-XXX

MLA PARTNER: --



ENGINEERING LICENSE:



DRAWING NOTICE:

THESE DOCUMENTS ARE CONFIDENTIAL AND ARE
THE SOLE PROPERTY OF SPRINT AND MAY NOT BE
REPRODUCED, DISSEMINATED OR REDISTRIBUTED
WITHOUT THE EXPRESS WRITTEN CONSENT OF
SPRINT

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SITE NAME:

MTN. VIEW CEM (FILLEY PARK)

SITE CASCADE

CT03XC076

SITE ADDRESS

28 BREWER DR BLOOMFIELD, CT 06002

- SHEET DESCRIPTION:

SPRINT SPECIFICATIONS

SHEET NUMBER:

SP-1

#### CONTINUE FROM SP-1

- 1. PERFORM ANY REQUIRED SITE ENVIRONMENTAL MITIGATION.
- PREPARE GROUND SITES; PROVIDE DE-GRUBBING; AND ROUGH AND FINAL GRADING, AND COMPOUND SURFACE TREATMENTS.
- 3. MANAGE AND CONDUCT ALL ACTIVITIES FOR INSTALLATION OF UTILITIES INCLUDING ELECTRICAL AND TELCO BACKHAUL.
- 4. INSTALL UNDERGROUND FACILITIES INCLUDING UNDERGROUND POWER AND COMMUNICATIONS CONDUITS, AND UNDERGROUND GROUNDING SYSTEM.
- 5. INSTALL ABOVE GROUND GROUNDING SYSTEMS.
- 6. PROVIDE NEW HVAC INSTALLATIONS AND MODIFICATIONS.
- 7. INSTALL "H-FRAMES". CABINETS AND SHELTERS AS INDICATED.
- 8. INSTALL ROADS, ACCESS WAYS, CURBS AND DRAINS AS INDICATED.
- 9. ACCOMPLISH REQUIRED MODIFICATION OF EXISTING FACILITIES.
- 10. PROVIDE ANTENNA SUPPORT STRUCTURE FOUNDATIONS.
- 11. PROVIDE SLABS AND EQUIPMENT PLATFORMS
- INSTALL COMPOUND FENCING, SIGHT SHIELDING, LANDSCAPING AND ACCESS BARRIERS.
- 13. PERFORM INSPECTION AND MATERIAL TESTING AS REQUIRED HEREINAFTER.
- 14. CONDUCT SITE RESISTANCE TO EARTH TESTING AS REQUIRED HEREINAFTER
- 15. INSTALL FIXED GENERATOR SETS AND OTHER STANDBY POWER SOLUTIONS.
- 16. INSTALL TOWERS, ANTENNA SUPPORT STRUCTURES AND PLATFORMS ON EXISTING TOWERS AS REQUIRED.
- 17. INSTALL CELL SITE RADIOS, MICROWAVE, GPS, COAXIAL MAINLINE, ANTENNAS, CROSS BAND COUPLERS, TOWER TOP AMPLIFIERS, LOW NOISE AMPLIFIERS AND RELATED EQUIPMENT.
- 18. PERFORM, DOCUMENT, AND CLOSE OUT ANY CONSTRUCTION CONTROL DOCUMENTS THAT MAY BE REQUIRED BY GOVERNMENT AGENCIES AND LANDLORDS
- 19. PERFORM ANTENNAL AND COAX SWEEP TESTING AND MAKE ANY AND ALL NECESSARY CORRECTIONS.
- 20. REMAIN ON SITE MOBILIZED THROUGHOUT HAND—OFF AND INTEGRATION TO ASSIST AS NEEDED UNTIL SITE IS DEEMED SUBSTANTIALLY COMPLETE AND PLACED "ON AIR."

#### 3.2 GENERAL REQUIREMENTS FOR CIVIL CONSTRUCTION:

- A. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH. AT THE COMPLETION OF THE WORK, CONTRACTOR SHALL REMOVE FROM THE SITE ALL REMAINING RUBBISH, IMPLEMENTS, TEMPORARY FACILITIES, AND SURPLUS MATERIALS.
- B. EQUIPMENT ROOMS SHALL AT ALL TIMES BE MAINTAINED "BROOM CLEAN" AND CLEAR OF DEBRIS.
- C. CONTRACTOR SHALL TAKE ALL REASONABLE PRECAUTIONS TO DISCOVER AND LOCATE ANY HAZARDOUS CONDITION.
- 1. IN THE EVENT CONTRACTOR ENCOUNTERS ANY HAZARDOUS CONDITION WHICH HAS NOT BEEN ABATED OR OTHERWISE MITIGATED, CONTRACTOR AND ALL OTHER PERSONS SHALL IMMEDIATELY STOP WORK IN THE AFFECTED AREA AND NOTIFY COMPANY IN WRITING. THE WORK IN THE AFFECTED AREA SHALL NOT BE RESUMED EXCEPT BY WRITTEN NOTIFICATION BY COMPANY.
- CONTRACTOR AGREES TO USE CARE WHILE ON THE SITE AND SHALL NOT TAKE ANY ACTION THAT WILL OR MAY RESULT IN OR CAUSE THE HAZARDOUS CONDITION TO BE FURTHER RELEASED IN THE ENVIRONMENT, OR TO FURTHER EXPOSE INDIVIDUALS TO THE HAZARD.
- D. CONTRACTOR'S ACTIVITIES SHALL BE RESTRICTED TO THE PROJECT LIMITS. SHOULD AREAS OUTSIDE THE PROJECT LIMITS BE AFFECTED BY CONTRACTOR'S ACTIVITIES, CONTRACTOR SHALL IMMEDIATELY RETURN THEM TO ORIGINAL CONDITION
- E. CONDUCT TESTING AS REQUIRED HEREIN.

#### 3.3 DELIVERABLES:

- A. CONTRACTOR SHALL REVIEW, APPROVE, AND SUBMIT TO SPRINT SHOP DRAWINGS, PRODUCT DATA, SAMPLES, AND SIMILAR SUBMITTALS AS REQUIRED HEREINAFTER
- B. PROVIDE DOCUMENTATION INCLUDING, BUT NOT LIMITED TO, THE FOLLOWING. DOCUMENTATION SHALL BE FORWARDED IN ORIGINAL FORMAT AND/OR UPLOADED INTO SMS.
- 1. ALL CORRESPONDENCE AND PRELIMINARY CONSTRUCTION REPORTS.
- 2. PROJECT PROGRESS REPORTS.
- CML CONSTRUCTION START DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- ELECTRICAL SERVICE COMPLETION DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).

- LINES AND ANTENNA INSTALL DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- POWER INSTALL DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- TELCO READY DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- 8. PPC (OR SHELTER) INSTALL DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION)
- TOWER CONSTRUCTION START DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- 10. TOWER CONSTRUCTION COMPLETE DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION)
- 11. BTS AND RADIO EQUIPMENT DELIVERED AT SITE DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- 12. NETWORK OPERATIONS HANDOFF CHECKLIST (HOC WALK) COMPLETE (UPLOAD FORM IN SMS)
- CIVIL CONSTRUCTION COMPLETE DATE (POPULATE FIELD IN SMS AND/OR FORWARD NOTIFICATION).
- 14. SITE CONSTRUCTION PROGRESS PHOTOS UNLOADED INTO SMS.

#### SECTION 01 400 - SUBMITTALS & TESTS

#### PART 1 - GENERAL

1.1 THE WORK: THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.

#### 1.2 RELATED DOCUMENTS:

- A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
- B. SPRINT "STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES" ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWITH.

#### 1.3 SUBMITTALS:

- A. THE WORK IN ALL ASPECTS SHALL COMPLY WITH THE CONSTRUCTION DRAWINGS AND THESE SPECIFICATIONS.
- B. SUBMIT THE FOLLOWING TO COMPANY REPRESENTATIVE FOR APPROVAL.
  - CONCRETE MIX-DESIGNS FOR TOWER FOUNDATIONS, ANCHORS PIERS, AND CONCRETE PAVING.
  - 2. CONCRETE BREAK TESTS AS SPECIFIED HEREIN.
  - 3. SPECIAL FINISHES FOR INTERIOR SPACES, IF ANY.
  - 4. ALL EQUIPMENT AND MATERIALS SO IDENTIFIED ON THE CONSTRUCTION DRAWINGS.
  - 5. CHEMICAL GROUNDING DESIGN
- D. ALTERNATES: AT THE COMPANY'S REQUEST, ANY ALTERNATIVES TO THE MATERIALS OR METHODS SPECIFIED SHALL BE SUBMITTED TO SPRINT'S CONSTRUCTION MANAGER FOR APPROVAL PRIOR TO BEING SHIPPED TO SITE. SPRINT WILL REVIEW AND APPROVE ONLY THOSE REQUESTS MADE IN WRITING. NO VERBAL APPROVALS WILL BE CONSIDERED. SUBMITTAL FOR APPROVAL SHALL INCLUDE A STATEMENT OF COST REDUCTION PROPOSED FOR USE OF ALTERNATE PRODUCT.

#### 1.4 TESTS AND INSPECTIONS:

- A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROJECT DOCUMENTATION.
- B. CONTRACTOR SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
- COAX SWEEPS AND FIBER TESTS PER TS-0200 REV 4 ANTENNA LINE ACCEPTANCE STANDARDS.
- AGL, AZIMUTH AND DOWNTILT USING ELECTRONIC COMMERCIAL MADE—FOR—THE—PURPOSE ANTENNA ALIGNMENT TOOL
- CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL CORRECTIONS TO ANY WORK IDENTIFIED AS UNACCEPTABLE IN SITE INSPECTION ACTIVITIES AND/OR AS A RESULT OF TESTING.
- C. REQUIRED CLOSEOUT DOCUMENTATION INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING;
  - . AZIMUTH, DOWNTILT, AGL UPLOAD REPORT FROM ANTENNA ALIGNMENT TOOL TO SITERRA TASK 465. INSTALLED AZIMUTH, DOWNTILT, AND AGL MUST CONFORM TO THE RF DATA SHEETS. SWEEP AND FIBER TESTS
- 2. SCANABLE BARCODE PHOTOGRAPHS OF TOWER TOP AND INACCESSIBLE SERIALIZED EQUIPMENT
- 3. ALL AVAILABLE JURISDICTIONAL INFORMATION
- 4. PDF SCAN OF REDLINES PRODUCED IN FIELD

- 5. ELECTRONIC AS-BUILT DRAWINGS IN AUTOCAD AND PDF FORMATS. ANY FIELD CHANGE MUST BE REFLECTED BY MODIFYING THE PLANS, ELEVATIONS, AND DETAILS IN THE DRAWING SETS. GENERAL NOTES INDICATING MODIFICATIONS WILL NOT BE ACCEPTED. CHANGES SHALL BE HIGHLIGHTED AS "CLOUDS" IDENTIFIED AS THE "AS-BUILT" CONDITION.
- 6. LIEN WAIVERS
- 7. FINAL PAYMENT APPLICATION
- 8. REQUIRED FINAL CONSTRUCTION PHOTOS
- 9 . CONSTRUCTION AND COMMISSIONING CHECKLIST COMPLETE WITH NO DEFICIENT ITEMS
- ALL POST NTP TASKS INCLUDING DOCUMENT UPLOADS COMPLETED IN SITERRA (SPRINTS DOCUMENT REPOSITORY OF RECORD).
- 1.5 COMMISSIONING: PERFORM ALL COMMISSIONING AS REQUIRED BY APPLICABLE
- 1.6 INTEGRATION: PERFORM ALL INTEGRATION ACTIVITIES AS REQUIRED BY APPLICABLE

PART 2 - PRODUCTS (NOT USED)

#### PART 3 - EXECUTION

# 3.1 REQUIREMENTS FOR TESTING:

- A. THIRD PARTY TESTING AGENCY:
  - WHEN THE USE OF A THIRD PARTY INDEPENDENT TESTING AGENCY IS REQUIRED, THE AGENCY THAT IS SELECTED MUST PERFORM SUCH WORK ON A REGULAR BASIS IN THE STATE WHERE THE PROJECT IS LOCATED AND HAVE A THOROUGH UNDERSTANDING OF LOCAL AVAILABLE MATERIALS, INCLUDING THE SOIL, ROCK, AND GROUNDWATER CONDITIONS.
- 2. THE THIRD PARTY TESTING AGENCY IS TO BE FAMILIAR WITH THE APPLICABLE REQUIREMENTS FOR THE TESTS TO BE DONE, EQUIPMENT TO BE USED, AND ASSOCIATED HEALTH AND SAFETY ISSUES.
- EXPERIENCE IN SOILS, CONCRETE, MASONRY, AGGREGATE, AND ASPHALT TESTING USING ASTM, AASJTO, AND OTHER METHODS IS NEEDED.
- EXPERIENCE IN SOILS, CONCRETE, MASONRY, AGGREGATE, AND ASPHALT TESTING USING ASTM, AASJTO, AND OTHER METHODS IS NEEDED.

#### 3.2 REQUIRED TESTS:

- A. CONTRACTOR SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
  - CONCRETE CYLINDER BREAK TESTS FOR THE TOWER AND ANCHOR FOUNDATIONS AS SPECIFIED IN SECTION: PORTLAND CEMENT CONCRETE PAVING.
  - ASPHALT ROADWAY COMPACTED THICKNESS, SURFACE SMOOTHNESS, AND COMPACTED DENSITY TESTING AS SPECIFIED IN SECTION: HOT MIX ASPHALT PAYING
  - 3. FIELD QUALITY CONTROL TESTING AS SPECIFIED IN SECTION: PORTLAND CEMENT CONCRETE PAVING.
  - 4. TESTING REQUIRED UNDER SECTION: AGGREGATE BASE FOR ACCESS ROADS, PADS AND ANCHOR LOCATIONS
  - 5. STRUCTURAL BACKFILL COMPACTION TESTS FOR THE TOWER FOUNDATION.
  - SITE RESISTANCE TO EARTH TESTING PER EXHIBIT: CELL SITE GROUNDING SYSTEM DESIGN.
  - 7. ANTENNA AND COAX SWEEP TESTS PER EXHIBIT: ANTENNA TRANSMISSION LINE ACCEPTANCE STANDARDS.
  - 8. GROUNDING AT ANTENNA MASTS FOR GPS AND ANTENNAS
  - 9. ALL OTHER TESTS REQUIRED BY COMPANY OR JURISDICTION.

#### 3.3 REQUIRED INSPECTIONS

- A. SCHEDULE INSPECTIONS WITH COMPANY REPRESENTATIVE.
- B. CONDUCT INSPECTIONS INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
- GROUNDING SYSTEM INSTALLATION PRIOR TO EARTH CONCEALMENT DOCUMENTED WITH DIGITAL PHOTOGRAPHS BY CONTRACTOR, APPROVED BY A&E OR SPRINT REPRESENTATIVE.
- FORMING FOR CONCRETE AND REBAR PLACEMENT PRIOR TO POUR DOCUMENTED WITH DIGITAL PHOTOGRAPHS BY CONTRACTOR, APPROVED BY A&E OR SPRINT REPRESENTATIVE.
- COMPACTION OF BACKFILL MATERIALS; AGGREGATE BASE FOR ROADS, PADS, AND ANCHORS; ASPHALT PAVING; AND SHAFT BACKFILL FOR CONCRETE AND WOOD POLES, BY INDEPENDENT THIRD PARTY AGENCY.
- 4. PRE— AND POST—CONSTRUCTION ROOFTOP AND STRUCTURAL INSPECTIONS ON EXISTING FACILITIES.
- 5. TOWER ERECTION SECTION STACKING AND PLATFORM ATTACHMENT DOCUMENTED BY DIGITAL PHOTOGRAPHS BY THIRD PARTY AGENCY.
- ANTENNA AZIMUTH, DOWN TILT AND PER SUNLIGHT TOOL SUNSIGHT INSTRUMENTS — ANTENNALIGN ALIGNMENT TOOL (AAT)



PLANS PREPARED BY:

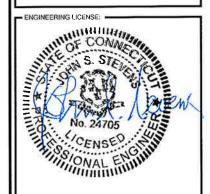
MLA PARTNER:

INFINIGY Build.

1033 Watervliet Shaker Rd Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB HUMBER 353-XXX

CROWN CASTLE



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DESCRIPTION	DATE	BY	REV
ISSUED FOR CONSTRUCTION	6/04/14	MAP	0

SITE NAME

MTN. VIEW CEM (FILLEY PARK)

CT03XC076

SITE CASCADE: -

28 BREWER DR BLOOMFIELD, CT 06002

DUEST DECODIDATION

- SITE ADDRESS:

SPRINT SPECIFICATIONS

SHEET NUMBER:

SP-2

#### CONTINUE FROM SP-2

- VERIFICATION DOCUMENTED WITH THE ANTENNA CHECKLIST REPORT, BY A&E, SITE DEVELOPMENT REP, OR RF REP.
- FINAL INSPECTION CHECKLIST AND HANDOFF WALK (HOC.). SIGNED FORM SHOWING ACCEPTANCE BY FIELD OPS IS TO BE UPLOADED INTO SMS.
- COAX SWEEP AND FIBER TESTING DOCUMENTS SUBMITTED VIA SMS FOR RF APPROVAL.
- 10. SCAN—ABLE BARCODE PHOTOGRAPHS OF TOWER TOP AND INACCESSIBLE SERIALIZED EQUIPMENT
- 11. ALL AVAILABLE JURISDICTIONAL INFORMATION
- 12. PDF SCAN OF REDLINES PRODUCED IN FIELD
- C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL CORRECTIONS TO ANY WORK IDENTIFIED AS UNACCEPTABLE IN SITE INSPECTION ACTIVITIES AND/OR AS A RESULT OF TESTING.
- D. CONSTRUCTION INSPECTIONS AND CORRECTIVE MEASURES SHALL BE DOCUMENTED BY THE CONTRACTOR WITH WRITTEN REPORTS AND PHOTOGRAPHS. PHOTOGRAPHS MUST BE DIGITAL AND OF SUFFICIENT QUALITY TO CLEARLY SHOW THE SITE CONSTRUCTION. PHOTOGRAPHS MUST CLEARLY IDENTIFY THE PHOTOGRAPHED ITEM AND BE LABELED WITH THE SITE CASCADE NUMBER, SITE NAME, DESCRIPTION, AND DATE.
- 3.4 DELIVERABLES: TEST AND INSPECTION REPORTS AND CLOSEOUT DOCUMENTATION SHALL BE UPLOADED TO THE SMS AND/OR FORWARDED TO SPRINT FOR INCLUSION INTO THE PERMANENT SITE FILES.
- A. THE FOLLOWING TEST AND INSPECTION REPORTS SHALL BE PROVIDED AS APPLICABLE.
- 1. CONCRETE MIX AND CYLINDER BREAK REPORTS.
- 2. STRUCTURAL BACKFILL COMPACTION REPORTS.
- 3. SITE RESISTANCE TO EARTH TEST.
- 4. ANTENNA AZIMUTH AND DOWN TILT VERIFICATION
- TOWER ERECTION INSPECTIONS AND MEASUREMENTS DOCUMENTING TOWER INSTALLED PER SUPPLIER'S REQUIREMENTS AND THE APPLICABLE SECTIONS HEREIN.
- COAX CABLE SWEEP TESTS PER COMPANY'S "ANTENNA LINE ACCEPTANCE STANDARDS".
- B. REQUIRED CLOSEOUT DOCUMENTATION INCLUDES THE FOLLOWING;
  - TEST WELLS AND TRENCHES: PHOTOGRAPHS OF ALL TEST WELLS; PHOTOGRAPHS SHOWING ALL OPEN EXCAVATIONS AND TRENCHING PRIOR TO BACKFILLING SHOWING A TAPE MEASURE VISIBLE IN THE EXCAVATIONS INDICATING DEPTH.
- CONDUITS, CONDUCTORS AND GROUNDING: PHOTOGRAPHS SHOWING TYPICAL INSTALLATION OF CONDUCTORS AND CONNECTORS; PHOTOGRAPHS SHOWING TYPICAL BEND RADIUS OF INSTALLED GROUND WIRES AND GROUND ROD SPACING:
- 3. CONCRETE FORMS AND REINFORCING: CONCRETE FORMING AT TOWER AND EQUIPMENT/SHELTER PAD/FOUNDATIONS PHOTOGRAPHS SHOWING ALL REINFORCING STEEL, UTILITY AND CONDUIT STUB OUTS; PHOTOGRAPHS SHOWING CONCRETE POUR OF SHELTER SLAB/FOUNDATION, TOWER FOUNDATION AND GUY ANCHORS WITH VIBRATOR IN USE; PHOTOGRAPHS SHOWING EACH ANCHOR ON GUYED TOWERS, BEFORE CONCRETE POUR.
- 4. TOWER, ANTENNAS AND MAINLINE: INSPECTION AND PHOTOGRAPHS OF SECTION STACKING; INSPECTION AND PHOTOGRAPHS OF PLATFORM COMPONENT ATTACHMENT POINTS; PHOTOGRAPHS OF TOWER TOP GROUNDING; PHOTOS OF TOWER COAX LINE COLOR CODING AT THE TOP AND AT GROUND LEVEL; INSPECTION AND PHOTOGRAPHS OF OPERATIONAL OF TOWER LIGHTING, AND PLACEMENT OF FAA REGISTRATION SIGN; PHOTOGRAPHS SHOWING ADDITIONAL GROUNDING POINTS FOR TOWERS GREATER THAN 200 FEET.; PHOTOS OF ANTENNA GROUND BAR, EQUIPMENT GROUND BAR, AND MASTER GROUND BAR; PHOTOS OF FACH SECTOR OF ANTENNAS; ONE PHOTOGRAPH LOCKING AT THE SECTOR AND ONE FROM BEHIND SHOWING THE PROJECTED COVERAGE AREA; PHOTOS OF COAX WEATHERPROOFING TOP AND BOTTOM; PHOTOS OF COAX GROUNDING—TOP AND BOTTOM; PHOTOS OF COAX GROUNDING—TOP AND BOTTOM; PHOTOS OF COAX GROUNDING—TOP AND BOTTOM; PHOTOS OF COAY GROUNDING—TOP AND BOTTOM; PHOTOS OF PLATFORM MECHANICAL CONNECTIONS TO TOWER/MONOPOLE.
- ROOF TOPS: PRE-CONSTRUCTION AND POST-CONSTRUCTION VISUAL INSPECTION AND PHOTOGRAPHS OF THE ROOF AND INTERIOR TO DETERMINE AND DOCUMENT CONDITIONS; ROOF TOP CONSTRUCTION INSPECTIONS AS REQUIRED BY THE JURISDICTION; PHOTOGRAPHS OF CABLE TRAY AND/OR ICE BRIDGE; PHOTOGRAPHS OF DOGHOUSE/CABLE EXIT FROM ROOF;
- SITE LAYOUT PHOTOGRAPHS OF THE OVERALL COMPOUND, INCLUDING EQUIPMENT PLATFORM FROM ALL FOUR CORNERS.
- 7. FINISHED UTILITIES: CLOSE—UP PHOTOGRAPHS OF THE PPC BREAKER PANEL; CLOSE—UP PHOTOGRAPH OF THE INSIDE OF THE TELCO PANEL AND NIU; CLOSE—UP PHOTOGRAPH OF THE POWER METER AND DISCONNECT; PHOTOS OF POWER AND TELCO ENTRANCE TO COMPANY ENCLOSURE; PHOTOGRAPHS AT METER BOX AND/OR FACILITY DISTRIBUTION PANEL.
- 8. REQUIRED MATERIALS CERTIFICATIONS: CONCRETE MIX DESIGNS; MILL CERTIFICATION FOR ALL REINFORCING AND STRUCTURAL STEEL; AND ASPHALT PAVING MIX DESIGN
- 9. ANY AND ALL SUBMITTALS BY THE JURISDICTION OR COMPANY.

#### SECTION 01 400 - SUBMITTALS & TESTS

#### PART 1 - GENERAL

- 1.1 THE WORK: THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.
- 1.2 RELATED DOCUMENTS:
- A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
- B. SPRINT "STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES" ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWITH.

#### PART 2 - PRODUCTS (NOT USED)

#### PART 3 - EXECUTION

#### 3.1 WEEKLY REPORTS:

- A. CONTRACTOR SHALL PROVIDE SPRINT WITH WEEKLY REPORTS SHOWING PROJECT STATUS. THIS STATUS REPORT FORMAT WILL BE PROVIDED TO THE CONTRACTOR BY SPRINT. THE REPORT WILL CONTAIN SITE ID NUMBER, THE MILESTONES FOR EACH SITE, INCLUDING THE BASELINE DATE, ESTIMATED COMPLETION DATE AND ACTUAL COMPLETION DATE.
- B. REPORT INFORMATION WILL BE TRANSMITTED TO SPRINT VIA ELECTRONIC MEANS AS REQUIRED. THIS INFORMATION WILL PROVIDE A BASIS FOR PROGRESS MONITORING AND PAYMENT
- 3.2 PROJECT CONFERENCE CALLS:
- A. SPRINT MAY HOLD WEEKLY PROJECT CONFERENCE CALLS. CONTRACTOR WILL BE REQUIRED TO COMMUNICATE SITE STATUS, MILESTONE COMPLETIONS AND UPCOMING MILESTONE PROJECTIONS, AND ANSWER ANY OTHER SITE STATUS QUESTIONS AS NECESSARY.
- 3.3 PROJECT TRACKING IN SMS:
- CONTRACTOR SHALL PROVIDE SCHEDULE UPDATES AND PROJECTIONS IN THE SMS SYSTEM ON A WEEKLY BASIS.
- 3.4 ADDITIONAL REPORTING:
  - A. ADDITIONAL OR ALTERNATE REPORTING REQUIREMENTS MAY BE ADDED TO THE REPORT AS DETERMINED TO BE REASONABLY NECESSARY BY COMPANY.

#### .5 PROJECT PHOTOGRAPHS:

- A. FILE DIGITAL PHOTOGRAPHS OF COMPLETED SITE IN JPEG FORMAT IN THE SMS PHOTO LIBRARY FOR THE RESPECTIVE SITE. PHOTOGRAPHS SHALL BE CLEARLY LABELED WITH SITE NUMBER, NAME AND DESCRIPTION, AND SHALL INCLUDE AT A MINIMUM THE FOLLOWING AS APPLICABLE:
- 1. 1SHELTER AND TOWER OVERVIEW.
- TOWER FOUNDATION(S) FORMS AND STEEL BEFORE POUR (EACH ANCHOR ON GUYED TOWERS).
- TOWER FOUNDATION(S) POUR WITH VIBRATOR IN USE (EACH ANCHOR ON GLYED TOWERS).
- TOWER STEEL AS BEING INSTALLED INTO HOLE (SHOW ANCHOR STEEL ON GUYED TOWERS).
- 5. PHOTOS OF TOWER SECTION STACKING.
- 6. CONCRETE TESTING / SAMPLES.
- 7. PLACING OF ANCHOR BOLTS IN TOWER FOUNDATION.
- 8. BUILDING/WATER TANK FROM ROAD FOR TENANT IMPROVEMENTS OR COMMENTS.
- 9. SHELTER FOUNDATION--FORMS AND STEEL BEFORE POURING.
- 10. SHELTER FOUNDATION POUR WITH VIBRATOR IN USE.
- 11. COAX CABLE ENTRY INTO SHELTER.
- 12. PLATFORM MECHANICAL CONNECTIONS TO TOWER/MONOPOLE.
- 13. ROOFTOP PRE AND POST CONSTRUCTION PHOTOS TO INCLUDE PENETRATIONS AND INTERIOR CEILING.
- 14. PHOTOS OF TOWER TOP COAX LINE COLOR CODING AND COLOR CODING AT GROUND LEVEL.
- 15. PHOTOS OF ALL APPROPRIATE COMPANY OR REGULATORY SIGNAGE.
- 16. PHOTOS OF EQUIPMENT BOLT DOWN INSIDE SHELTER.
- 17. POWER AND TELCO ENTRANCE TO COMPANY ENCLOSURE AND POWER AND TELCO SUPPLY LOCATIONS INCLUDING METER/DISCONNECT.
- 18. ELECTRICAL TRENCH(S) WITH ELECTRICAL / CONDUIT BEFORE BACKFILL.
- 19. ELECTRICAL TRENCH(S) WITH FOIL-BACKED TAPE BEFORE FURTHER BACKFILL.
- 20. TELCO TRENCH WITH TELEPHONE / CONDUIT BEFORE BACKFILL.
- 21. TELCO TRENCH WITH FOIL-BACKED TAPE BEFORE FURTHER BACKFILL.
- 22. SHELTER GROUND-RING TRENCH WITH GROUND-WIRE BEFORE BACKFILL (SHOW ALL CAD WELDS AND BEND RADII).
- 23. TOWER GROUND-RING TRENCH WITH GROUND-WIRE BEFORE BACKFILL (SHOW ALL CAD WELDS AND BEND RADII).

- 24. FENCE GROUND-RING TRENCH WITH GROUND-WIRE BEFORE BACKFILL (SHOW ALL CAD WELDS AND BEND RADII).
- 25. ALL BTS GROUND CONNECTIONS.
- 26. ALL GROUND TEST WELLS.
- 27. ANTENNA GROUND BAR AND EQUIPMENT GROUND BAR.
- 28. ADDITIONAL GROUNDING POINTS ON TOWERS ABOVE 200'.
- 29. HVAC UNITS INCLUDING CONDENSERS ON SPLIT SYSTEMS.
- 30, GPS ANTENNAS.
- 31. CABLE TRAY AND/OR WAVEGUIDE BRIDGE.
- 32. DOGHOUSE/CABLE EXIT FROM ROOF.
- 33. EACH SECTOR OF ANTENNAS; ONE PHOTOGRAPH LOOKING AT THE SECTOR AND ONE FROM BEHIND SHOWING THE PROJECTED COVERAGE AREA.
- 34. MASTER BUS BAR
- 35. TELCO BOARD AND NIU.
- 36. ELECTRICAL DISTRIBUTION WALL.
- 37. CABLE ENTRY WITH SURGE SUPPRESSION.
- 38. ENTRANCE TO EQUIPMENT ROOM.
- 39. COAX WEATHERPROOFING-TOP AND BOTTOM OF TOWER.
- 40. COAX GROUNDING -TOP AND BOTTOM OF TOWER.
- 41. ANTENNA AND MAST GROUNDING.
- 42. LANDSCAPING WHERE APPLICABLE.
- 3.6 FINAL PROJECT ACCEPTANCE: COMPLETE ALL REQUIRED REPORTING TASKS PER CONTRACT, CONTRACT DOCUMENTS OR THE SPRINT INTEGRATED CONSTRUCTION STANDARDS FOR WIRELESS SITES AND UPLOAD INTO SITERRA.



LANS PREPARED FOR:

6580 Sprint Parkway Overland Park, Kansas 66251

PLANS PREPARED BY:

MI A PARTNER:

NFINIGY 25

1033 Watervliet Shaker Rd Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB NUMBER 353-XXX





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REVISIONS:		_	
DESCRIPTION	DATE	BY	REV
	_		$\vdash$
ISSUED FOR CONSTRUCTION	6/04/14	MAP	0

SITE NAME:

MTN. VIEW CEM (FILLEY PARK)

SITE CASCAD

CT03XC076

SITE ADDRESS:

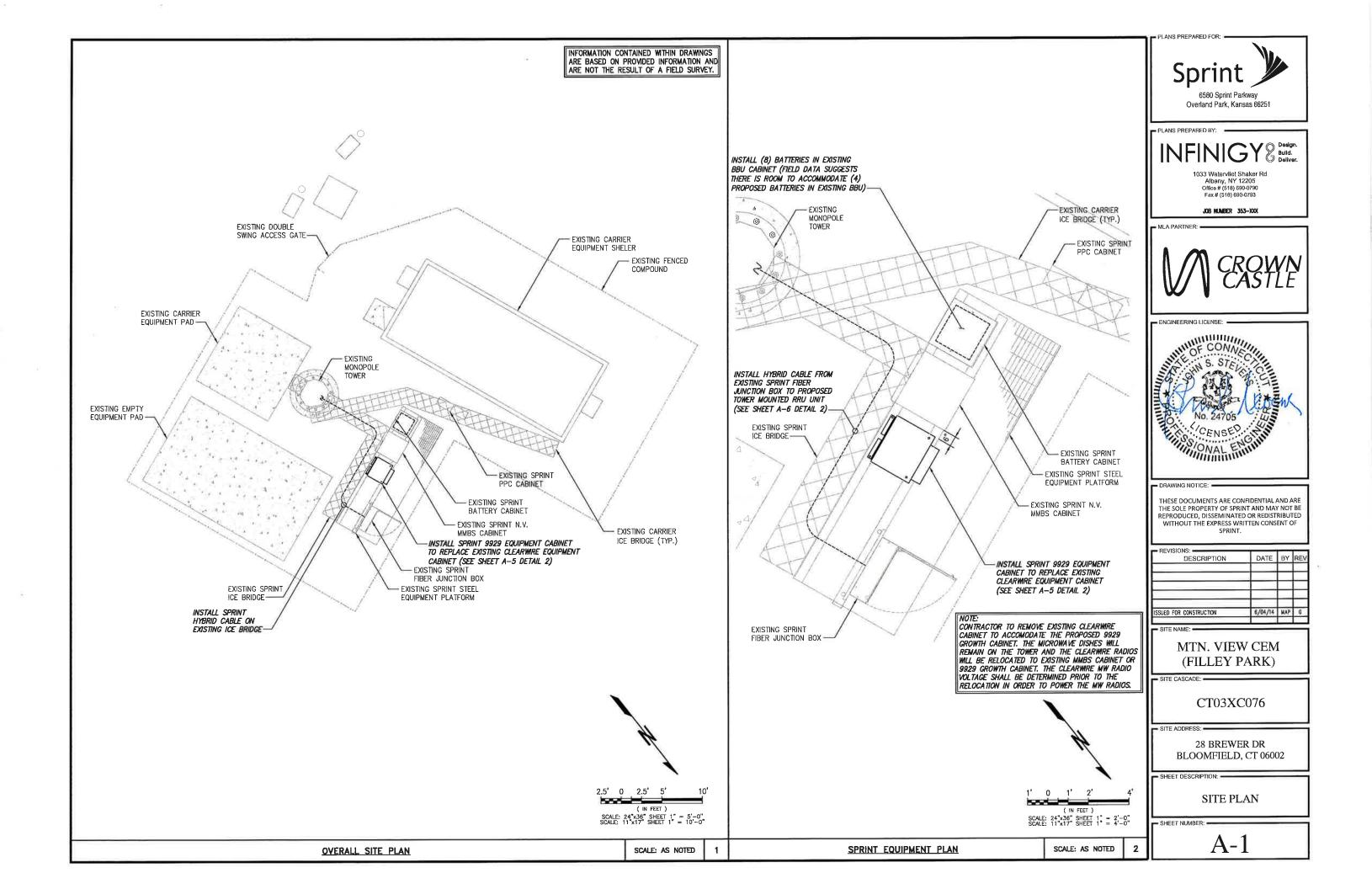
28 BREWER DR BLOOMFIELD, CT 06002

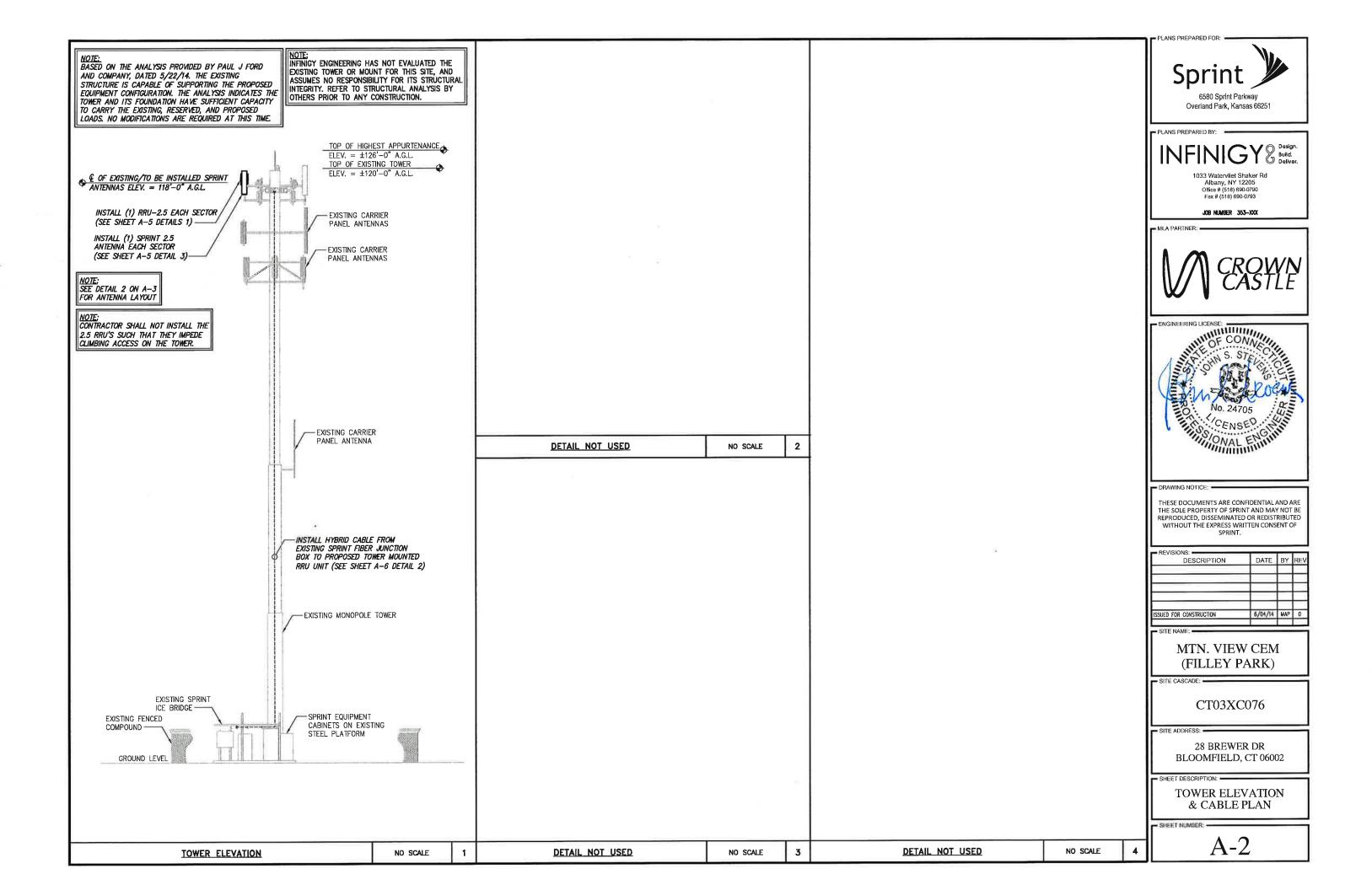
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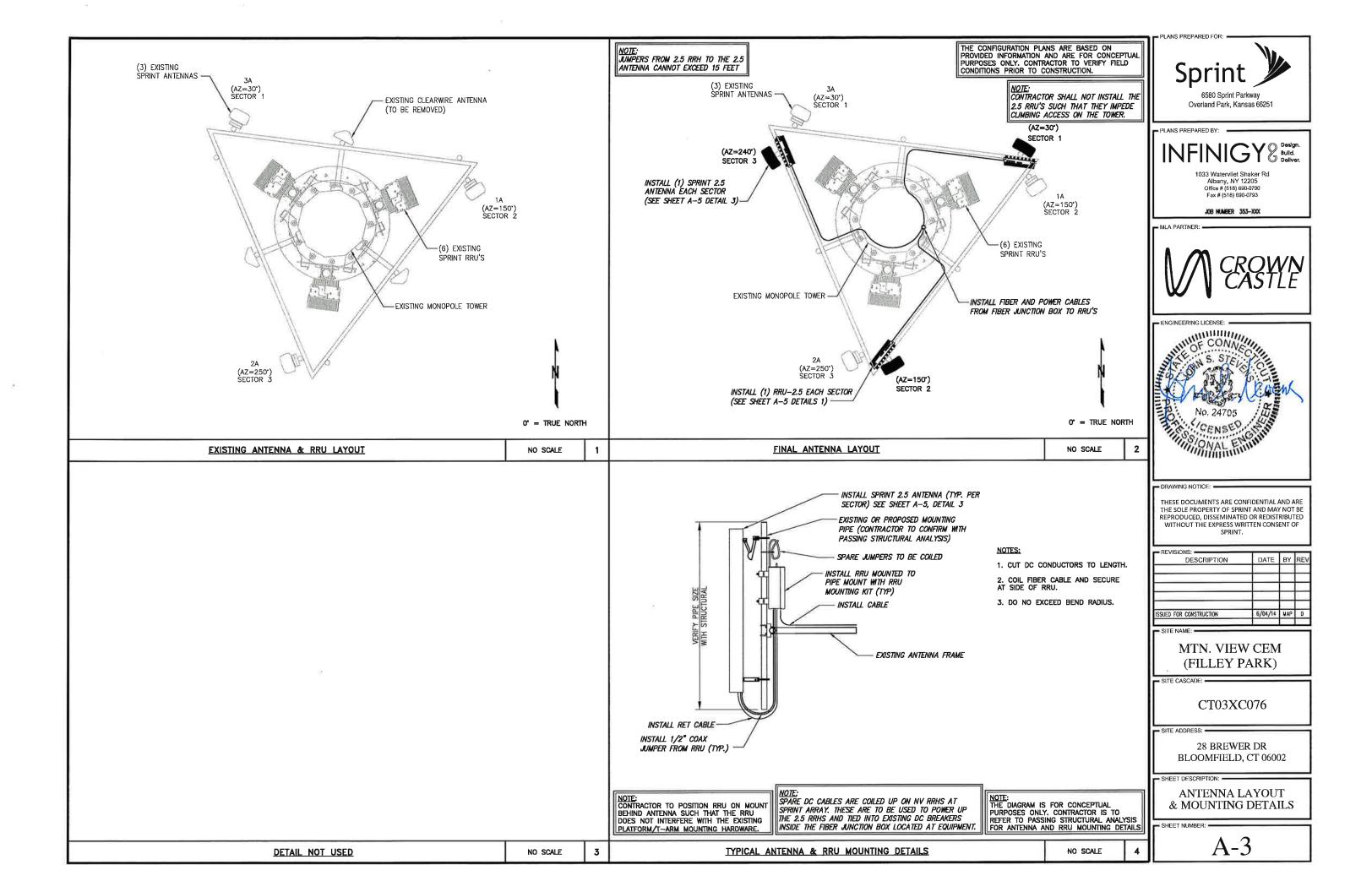
SPRINT SPECIFICATIONS

- SHEET NUMBER: -

SP-3



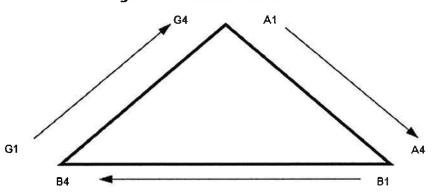




NV CABLES					
BAND	INDICATOR		PORT	COLOR	
800-1	YEL	GRN	NV-1	GRN	
1900-1	YEL	RED.	NV-2	BLU	
1900-2	YEL	BRN	NV-3	BRN	
1900-3	YEL	BLU	NV-4	WHT	
1900-4	YEL	SLT	NV-5	RED	
800-2	YEL	ORG	NV-6	SLT	
SPARE	YEL	WHT	NV-7	pp)_	
2500	YEL	PPL	NV-8	ORG	

HYBRID				
HYBRID	COLOR			
1	GRN			
2	BLU			
3	BRN			
4	WHT			
5	RED			
6	SLT			
7	EBT			
8	ORG			
tonna Orientation				

Figure 1: Antenna Orientation



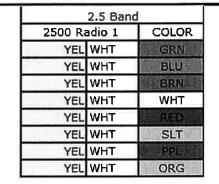
## NOTES:

- 1. ALL CABLES SHALL BE MARKED WITH 2" WIDE, UV STABILIZED, UL APPROVED TAPE.
- 2. THE FIRST RING SHALL BE CLOSEST TO THE END OF THE CABLE AND SPACED APPROXIMATELY 2" FROM THE END CONNECTOR, WEATHERPROOFING, OR BREAK-OUT CYLINDER. THERE SHALL BE A 1" SPACE BETWEEN EACH RING FOR THE CABLE IDENTIFIER, AND NO SPACES BETWEEN THE FREQUENCY BANDS.
- 3. A 2" GAP SHALL SEPARATE THE CABLE COLOR CODE FROM THE FREQUENCY COLOR CODE. THE 2" COLOR RINGS FOR THE FREQUENCY CODE SHALL BE PLACED NEXT TO EACH OTHER WITH NO
- 4. THE 2" COLORED TAPE(S) SHALL EACH BE WRAPPED A MINIMUM OF 3 TIMES AROUND THE INDIVIDUAL CABLES, AND THE TAPE SHALL BE KEPT IN THE SAME LOCATION AS MUCH AS POSSIBLE.
- 5. SITES WITH MORE THAN FOUR (4) SECTORS WILL REQUIRE ADDITIONAL RINGS FOR EACH SECTOR, FOLLOWING THE PATTERN. HIGH CAPACITY SITES WILL USE THE NEXT COLOR IN THE SEQUENCE FOR ADDITIONAL CABLES IN EACH SECTOR.
- 6. HYBRID FIBER CABLE SHALL BE SECTOR IDENTIFIED INSIDE THE CABINET ON FREQUENCY BUNDLES, ON THE SEALTITE, ON THE MAIN LINE UPON EXIT OF SEALTITE, AND BEFORE AND AFTER THE BREAKOUT UNIT (MEDUSA), AS WELL AS BEFORE AND AFTER ANY ENTRANCE OR EXIT.
- 7. HFC "MAIN TRUNK" WILL NOT BE MARKED WITH THE FREQUENCY CODES, AS IT CONTAINS ALL FREQUENCIES.
- 8. INDIVIDUAL POWER PAIRS AND FIBER BUNDLES SHALL BE LABELED WITH BOTH THE CABLE AND FREQUENCY.

Sector	Cable	First Ring	Second Ring	Third Ring
1 Alpha	1	Green	No Tape	No Tape
1	2	TO Alpay ITS	No Tape	No Tape
1	3	Brown	No Tape	No Tape
1	4	White	No Tape	No Tape
1	5	Red	No Tape	No Tape
1	6	Grey	No Tape	No Tape
1	7	Purple	No Tape	No Tape
1	8	Orange	No Tape	No Tape
2 Beta	1	in Green	Green	No Tape
2	2			No Tape
2	3	# Brown N	Brown	No Tape
2	4	White	White	No Tape
2	5	Pas Red and	<b>八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八八</b>	No Tape
2	6	Grey	Grey	No Tape
2	7	Purple	Purple	No Tape
2	8	Orange	Orange	No Tape
3 Gamma	1	in tigreen in	Green	Sreen.
3	2			
3	3	BROWN !	Brown	S. Biorn
3	4	White	White	White
3	5	<b>ELORED</b>	The Red	FR ed Mil
3	6	Grey	Grey	Grey
3	7	Purple	Purple :	Purple
3	8	Orange	Orange	Orange

NV FREQUENCY	INDICATOR	ID
800-1	YEL	GRN
1900-1	YEL	RED Hourd
1900-2	YEL	BRN
1900-3	YEL	BLU
1900-4	YEL	SLT
800-1	YEL	ORG
RESERVED	YEL	WHT
RESERVED	YEL	PPL /

2.5				
2.5			_	******
FREQUENCY		INDICATOR	₹ .	ID
2500 -1	YEL	W	HT	GRN
2500 -2	YEL	W	T	RED
2500 -3	YEL	W	HT	BRN
2500 -4	YEL	W	HT	BLU
2500 -5	YEL	W	HT	SLT
2500 -6	YEL	W	HT	ORG
2500 -7	YEL	W	HT	WHT
2500 -8	YEL	W	HT	PPL





PLANS PREPARED BY:

MLA PARTNER

Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB NUMBER 353-XXX

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DESCRIPTION	DATE	BY	REV
			H
ISSUED FOR CONSTRUCTION	6/04/14	MAP	0

Example - Sector 2, Cable 2, 800mhz Radio #1



Example - Sector 3, Cable 1, 1900mhz Radio #1

Example - Sector 1, Cable 4, 800 mhz Radio #1 and 1900mhz Radio #1



CT03XC076

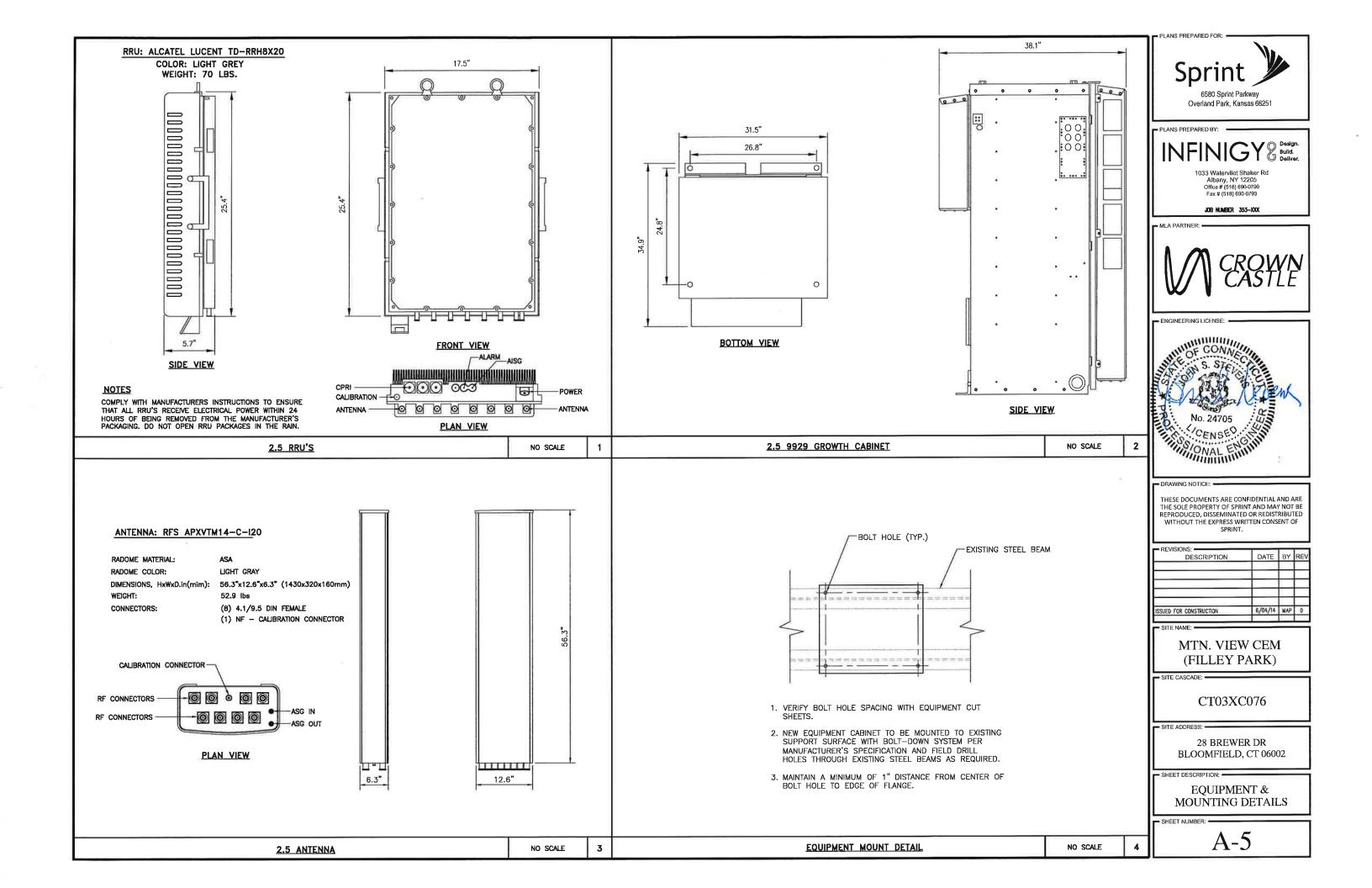
28 BREWER DR BLOOMFIELD, CT 06002

COLOR CODING AND NOTES

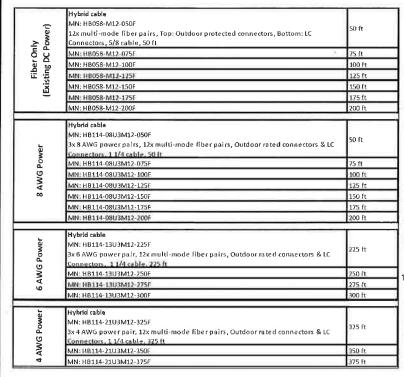
- SHEET NUMBER:

COLOR CODING AND NOTES

NO SCALE



#### RFS HYBRIFLEX RISER CABLE SCHEDULE

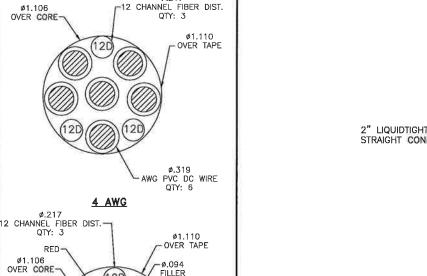


#### RFS HYBRIFLEX JUMPER CABLE SCHEDULE

MN: HBF012-M3-5F1 5 ft, 3x multi-mode fib MN: HBF012-M3-10F1

Fiber	MN: HBF012-M3-15F1	15 ft
歪	MN:HBF012-M3-20F1	20 ft
	MN:H8F012-M3-25F1	25 ft
	MN: HBF012-M3-30F1	30 ft
	Hybrid Jumper cable	
	MN: HBF058-08U1M3-5F1	= 6.
8 AWG Power	5 ft, 1x 8 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors,	5 ft
9	5/8 cable MN: HBF058-08U1M3-10F1	10 ft
	MN: HBF058-08U1M3-15F1	15 ft
8 ≯	MN: HBF058-08U1 M3-20F1	20 ft
	MN: HBF058-08U1M3-25F1	25 ft
	MN: HBF058-08U1M3-30F1	30 ft
	Hybrid Jumper cable	
	Hybrid Jumper cable	
Ε.	MN: HBF058-13U1M3-5F1	5 ft
6 AWG Power	5 ft, 1x 6 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	
d.	MN: HBF058-13U1M3-10F1	10 ft
3	MN: HBF058-13U1M3-15F1	15 ft
9	MN: HBF058-13U1M3-20F1	20 ft
	MN: HBF058-13U1M3-25F1	25 ft
	MN: HBF058-13U1M3-30F1	30 ft
	Hybrid Jumper cable	
Ε.	MN: HBF078-21U1M3-5F1 5 ft, 1x 4 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors,	5 ft
× ×	7/8 cable	
4 AWG Power	MN: HBF078-21U1M3-10F1	10 ft
×	MN: HBF078-21U1M3-15F1	15 ft
4 4	MN: HBF078-21U1M3-20F1	20 ft
	MN: H8F078-21U1M3-25F1	25 ft
	MN: HBE078-21U1M3-30E1	30 ft

NOTE: SPRINT CM TO CONFIRM HYBRID OR FIBER RISER CABLE AND HYBRID OR FIBER JUMPER CABLE MODEL NUMBERS IF HYBRID CABLES ARE REQUIRED BEFORE PREPARING BOM.



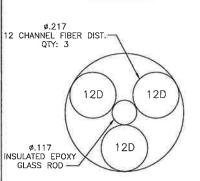
Ø.217

12D RED

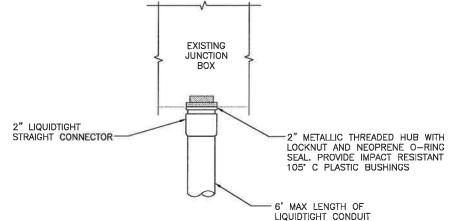
#### 8 & 6 AWG

INNER CORE-

5 ft



FIBER ONLY



FIBER JUNCTION BOX PENETRATION

NO SCALE



Overland Park, Kansas 66251

PLANS PREPARED BY:

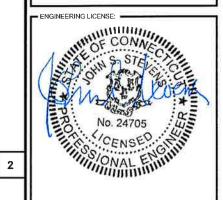
MLA PARTNER:

NFINIGY Build.
Deliver.

033 Watervliet Shaker Rd Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB NUMBER 353-XXX

CROYYN CASTLE



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			_
REVISIONS: DESCRIPTION	DATE	ву	REV
	-	$\vdash$	
ISSUED FOR CONSTRUCTION	6/04/14	MAP	0

SITE NAME:

MTN. VIEW CEM (FILLEY PARK)

SITE CASCADE:

CT03XC076

- SITE ADDRESS: •

28 BREWER DR BLOOMFIELD, CT 06002

- SHEET DESCRIPTION: -

CIVIL DETAILS

- SHEET NUMBER

A-6

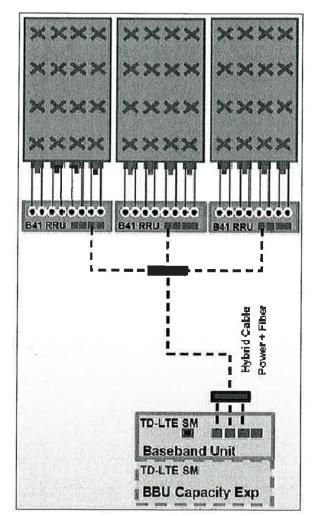
2.5 CABLE CROSS SECTION DATA

NO SCALE

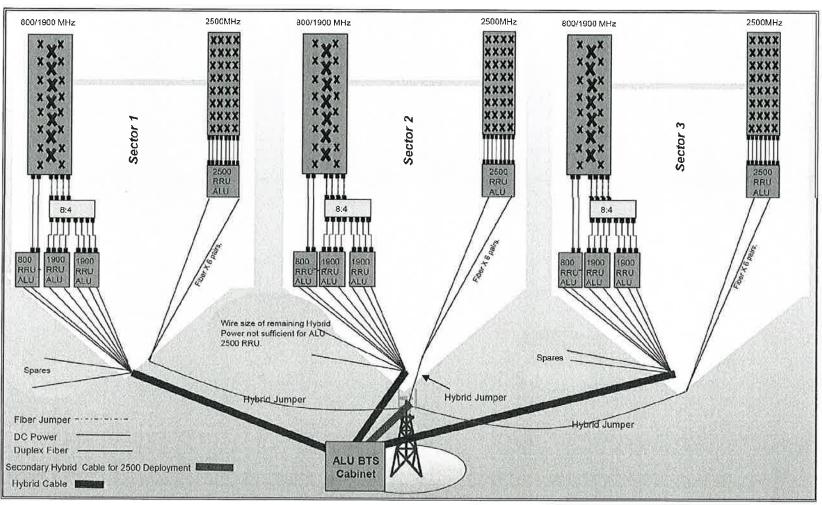
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DETAIL NOT USED

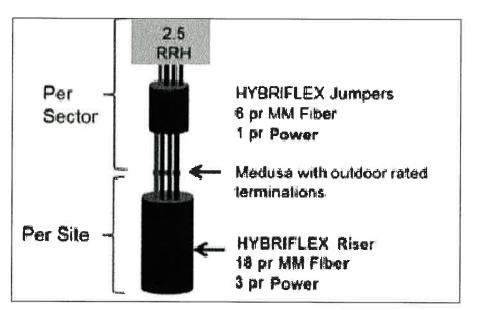
NO SCALE



ALU 2.5 ALU SCENARIO 1



RAN WIRING DIAGRAM



RF 2.5 ALU SCENARIO 1



PLANS PREPARED BY:

# NFINIGY Build. Deliver.

1033 Watervliet Shaker Rd Albany, NY 12205 Office # (518) 690-0790 Fax # (518) 690-0793

JOB HUMBER 353-XXX

MLA PARTNEI



ENGINEERING LICENSI



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DESCRIPTION	DATE	BY	REV
		_	
ISSUED FOR CONSTRUCTION	6/04/14	MAP	0

SITE NAME

MTN. VIEW CEM (FILLEY PARK)

SITE CASCADE

CT03XC076

SITE ADDRESS:

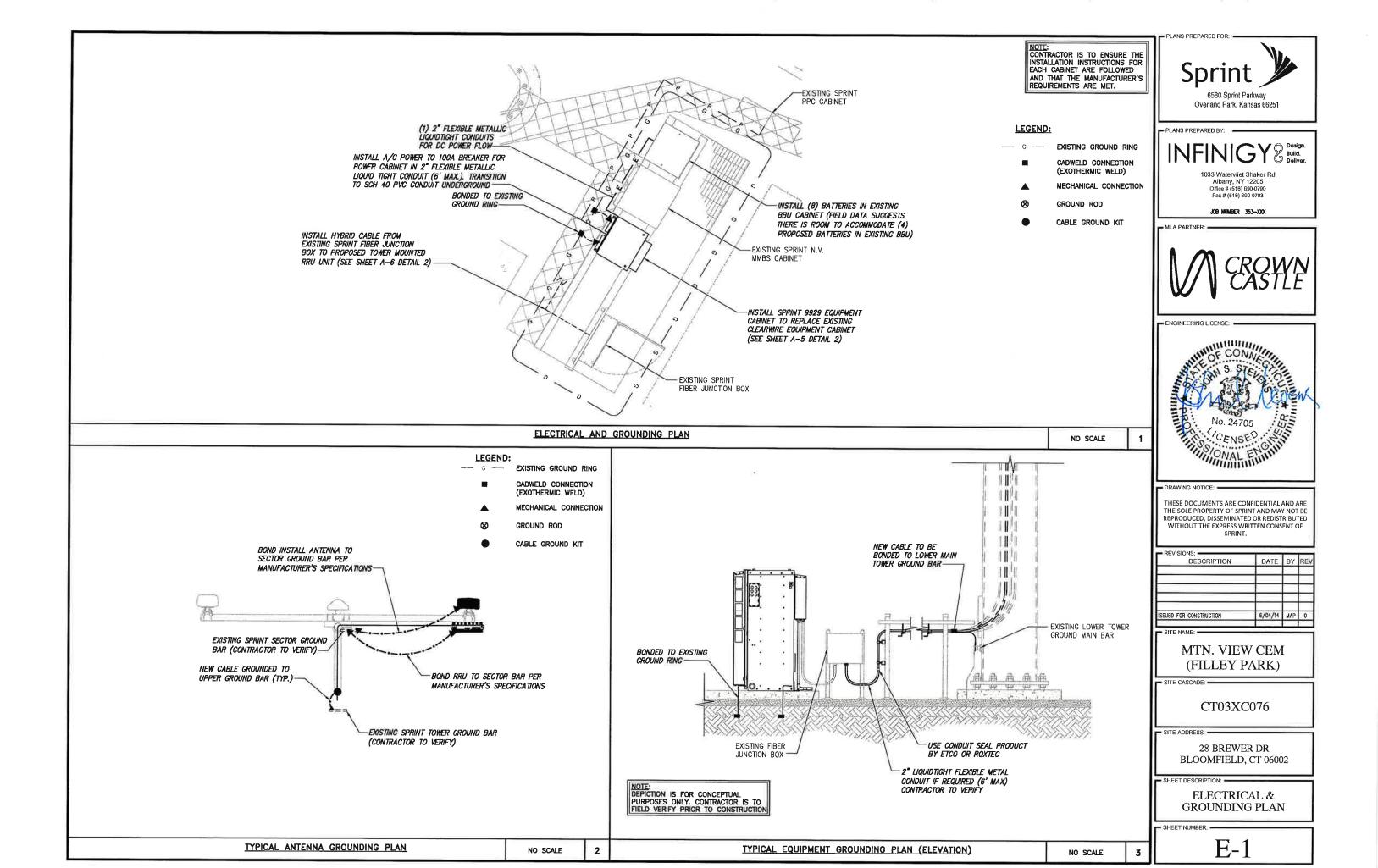
28 BREWER DR BLOOMFIELD, CT 06002

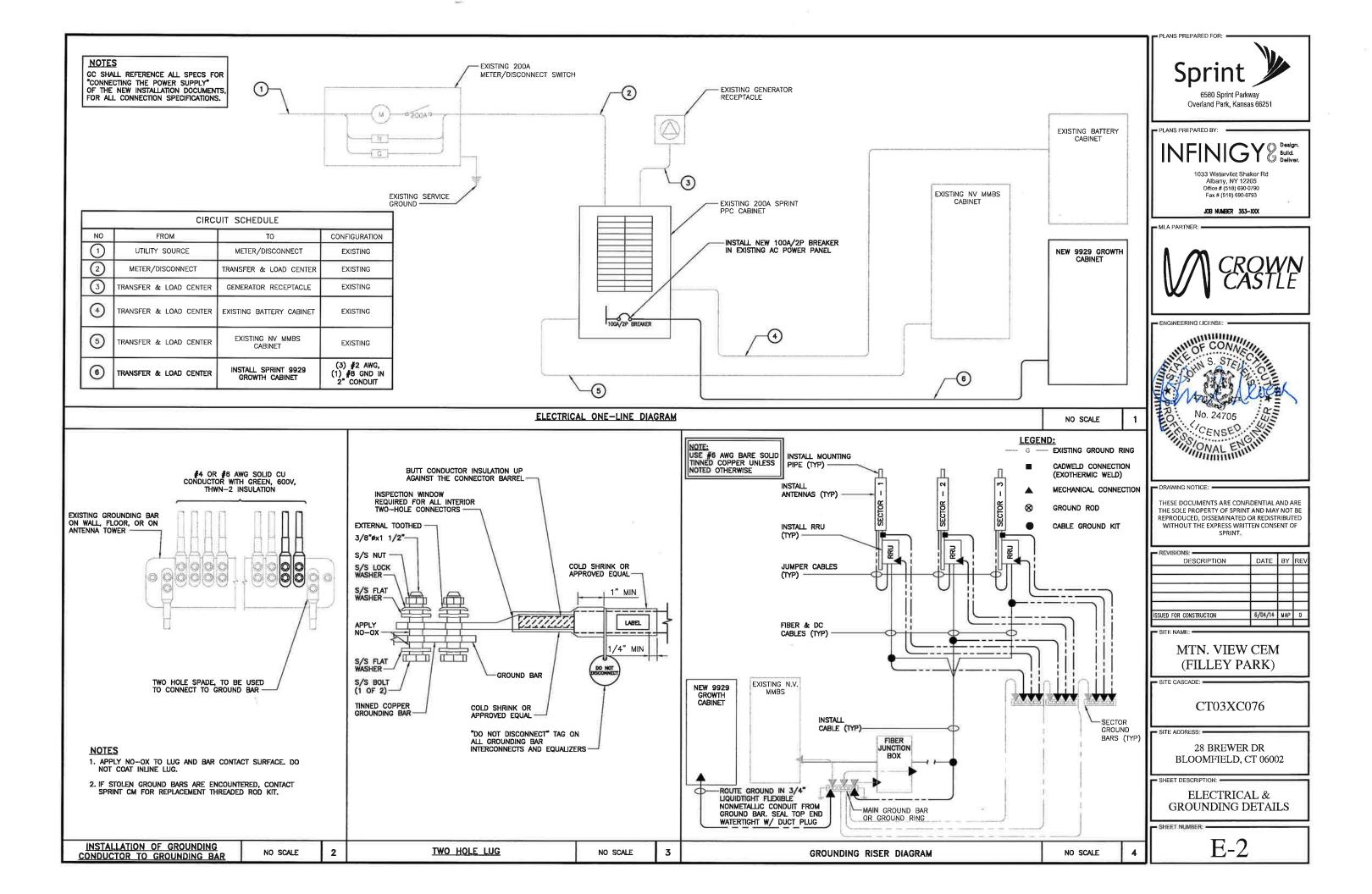
- SHEET DESCRIPTION:

PLUMBING DIAGRAM

SHEET NUMBER:

A-7







Date: May 22, 2014

Steve Tuttle Crown Castle 8 Parkmeadow Drive Pittsford, NY 14534

Paul J Ford and Company 250 E. Broad Street Suite 600 Columbus, OH 43215 mscroggy@pifweb.com

Subject:

Structural Analysis Report

Carrier Designation:

Sprint PCS Co-Locate Carrier Site Number: Carrier Site Name:

Scenario 2.5B CT03XC076 MTN. VIEW CEM. (FILLEY PARK)

Crown Castle Designation:

Crown Castle BU Number: Crown Castle Site Name:

876329 MTN. VIEW CEM.

Crown Castle JDE Job Number:

(FILLEY PARK)

Crown Castle Work Order Number: **Crown Castle Application Number:** 

764594 208258 Rev. 6

253010

Engineering Firm Designation:

Paul J Ford and Company Project Number: 37513-2485-R6

Site Data:

28 Brewer Dr., BLOOMFIELD, Hartford County, CT Latitude 41° 50' 6.57", Longitude -72° 44' 28.2"

120 Foot - Monopole Tower

Dear Steve Tuttle.

Paul J Ford and Company is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 646735, in accordance with application 208258, revision 6.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

**Sufficient Capacity** 

The structural analysis was performed for this tower in accordance with the requirements of the 2005 Connecticut Building Code and the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 37.6 mph with 1.0 inch ice thickness and 50 mph under service loads.

We at Paul J Ford and Company appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted by:

Morgan Scroggy, E.I. Structural Designer

tnxTower Report - version 6.1.4.1

MAY 2 2 2014



Date: May 22, 2014

Steve Tuttle Crown Castle 8 Parkmeadow Drive Pittsford, NY 14534 Paul J Ford and Company 250 E. Broad Street Suite 600 Columbus, OH 43215 mscroggy@pifweb.com

Subject: Structural Analysis Report

Carrier Designation: Sprint PCS Co-Locate
Carrier Site Number:

Carrier Site Number: CT03XC076
Carrier Site Name: MTN. VIEW CEM.
(FILLEY PARK)

Crown Castle Designation: Crown Castle BU Number: 876329

Crown Castle Site Name: MTN. VIEW CEM.

(FILLEY PARK)

Scenario 2.5B

Crown Castle JDE Job Number: 253010
Crown Castle Work Order Number: 764594

**Crown Castle Application Number:** 208258 Rev. 6

Engineering Firm Designation: Paul J Ford and Company Project Number: 37513-2485-R6

Site Data: 28 Brewer Dr., BLOOMFIELD, Hartford County, CT

Latitude 41° 50′ 6.57″, Longitude -72° 44′ 28.2″

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**Additional Calculations** 

## 1) INTRODUCTION

This tower is a 120 ft Monopole tower designed by ROHN in January of 1998. The tower was originally designed for a wind speed of 70 mph per TIA/EIA-222-F.

# 2) ANALYSIS CRITERIA

The structural analysis was performed for this tower in accordance with the requirements of the 2005 Connecticut Building Code and the TIA/EIA-222-F Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a fastest mile wind speed of 80 mph with no ice, 37.6 mph with 1.0 inch ice thickness and 50 mph under service loads.

**Table 1 - Proposed Antenna and Cable Information** 

Mounting Level (ft)	Elevetion	Number of Antennas	Antenna Manufacturer		Number of Feed Lines	Feed Line Size (in)	Note
		3	alcatel lucent	TD-RRH8x20-25			
118.0	120.0	3	rfs celwave	APXVTM14-C-120 w/ Mount Pipe	1	5/8	-

Table 2 - Existing and Reserved Antenna and Cable Information

Table 2 - E		Reserved	Antenna and C	Í.		ı	
Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		3	kathrein	840 10054 w/ Mount Pipe			
		3	samsung telecom	WIMAX DAP HEAD	-	-	3
	120.0	1	rfs celwave	APXV9ERR18-C-A20 w/ Mount Pipe			
118 0	118.0	2	rfs celwave	APXVSPP18-C-A20 w/ Mount Pipe			
110.0	118.0	1	tower mounts	Platform Mount [LP 501-1]	6	5/16	
		1	dragonwave	A-ANT-18G-1-C	2	1/2	1
		1	dragonwave	A-ANT-18G-2-C	3	1-1/4	
	116.0	2	dragonwave	HORIZON COMPACT			
		3	rfs celwave	IBC1900BB-1	-		
		3	rfs celwave	IBC1900HG-2A	-		
	115.0	3	alcatel lucent	PCS 1900MHz 4x45W-65MHz			
114.0	114.0		tower mounts	Pipe Mount [PM 601-3]	- -	_	1
ı	113.0	3	alcatel lucent	800MHz 2X50W RRH W/FILTER	-		
		3	ericsson	ERICSSON AIR 21 B2A B4P w/ Mount Pipe		1-5/8	
107.0	107.0	3	ericsson	ERICSSON AIR 21 B4A B2P w/ Mount Pipe	1		2
		3	ericsson	KRY 112 144/1			
	107.0	1	tower mounts	Platform Mount [LP 712-1]	18	1-5/8	1
		2	kmw communicatio	AM-X-CD-14-65-00T-RET w/ Mount Pipe			
		2		AM-X-CD-16-65-00T-RET w/ Mount Pipe			
		3	powerwave	7770.00 w/ Mount Pipe			
		2	technologies	P65-17-XLH-RR w/ Mount Pipe	10	7/0	
99.0	100.0	3	communicatio n components inc.	DTMABP7819VG12A	12 7/8 1 3/8 2 3/4		1
		6	ericsson	RRUS-11			
		6	powerwave technologies	LGP13519			
		1	raycap	DC6-48-60-18-8F			
	99.0 1		tower mounts	Platform Mount [LP 501-1]			
<b>50.0</b>	63.0	1	decibel	DB536	4	7/0	4
59.0	59.0	1	tower mounts	Side Arm Mount [SO 702-1]	1	7/8	1
	F0 0	1	lucent	KS24019-L112A			
48.0	50.0	1	unknown	GPS	2	1/2	1
	48.0	1	tower mounts	Pipe Mount [PM 601-1]			,

## Notes:

- Existing Equipment Reserved Equipment Equipment To Be Removed, Not Included in this SA 1) 2) 3)

# 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided** 

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	Dr. Clarence Welti, 08/09/96	1529722	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Rohn, 34738SW, 10/11/96	1616549	CCISITES
4-TOWER MANUFACTURER DRAWINGS	Rohn, 34738SW, 10/17/96	2158527	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	Semaan, CT03XC076, 08/25/03	3386189	Semaan
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	Vertical Solutions, 080063.01, 01/18/08	2205450	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	B&T, 79582, 11/03/08	2343687	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	GPD, 2011111.27, 05/31/11	2917489	CCISITES

## 3.1) Analysis Method

tnxTower (version 6.1.4.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

## 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) Monopole was reinforced in conformance with the referenced modification drawings.
- 5) The bridge stiffeners carry the entire load through the flange connection at 30'.
- 6) The existing "cut down" TS14x10 at 60' act as bridge stiffeners and carry the entire load through the flange connection.

This analysis may be affected if any assumptions are not valid or have been made in error. Paul J Ford and Company should be notified to determine the effect on the structural integrity of the tower.

# 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)** 

Section No.		Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	120 - 90	Pole	P24x1/4	1	-8.03	589.19	88.4	Pass
L2	90 - 80	Pole	P24x3/8	2	-9.33	934.94	79.2	Pass
L3	80 - 68.5	Pole	RPS 24" x 0.62517"	3	-11.52	1212.06	89.0	Pass
L4	68.5 - 68	Pole	RPS 24" x 0.88003"	4	-11.65	1685.53	66.2	Pass
L5	68 - 63.5	Pole	RPS 24" x 0.61306"	5	-12.50	1362.11	89.8	Pass
L6	63.5 - 60	Pole	RPS 24" x 1.13718"	6	-13.59	2216.54	62.5	Pass
L7	60 - 56.5	Pole	RPS 30" x 0.90733"	7	-14.72	2335.32	49.8	Pass
L8	56.5 - 45.417	Pole	RPS 30" x 0.55714"	8	-17.13	1565.42	90.6	Pass
L9	45.417 - 36.417	Pole	RPS 30" x 0.70733"	9	-19.47	1975.72	84.7	Pass
L10	36.417 - 33.5	Pole	RPS 30" x 0.86188"	10	-20.36	2395.96	74.0	Pass
L11	33.5 - 30	Pole	RPS 30" x 1.23648"	11	-21.81	3145.60	61.0	Pass
L12	30 - 26.5	Pole	RPS 36" x 0.7835"	12	-22.97	2484.77	65.4	Pass
L13	26.5 - 20.583	Pole	RPS 36" x 0.62423"	13	-24.58	1990.28	88.1	Pass
L14	20.583 - 2	Pole	RPS 36" x 0.8638"	14	-31.30	2733.17	82.4	Pass
L15	2 - 0	Pole	RPS 36" x 0.94668"	15	-32.09	2932.48	79.0	Pass
							Summary	
						Pole (L8)	90.6	Pass
						RATING =	90.6	Pass

Table 5 - Tower Component Stresses vs. Capacity

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1, 2	Anchor Rods	0	78.9	Pass
1	Base Plate	0	68.3	Pass
1	Base Foundation – Steel	0	73.6	Pass
1, 3	Base Foundation Soil Interaction	0	34.0	Pass
1	Flange Connection	30	96.8	Pass
1	Flange Connection	60	62.4	Pass
1	Flange Connection	90	59.6	Pass

Structure Rating (max from all components) =	96.8%

#### Notes:

- 1) See additional documentation in "Appendix C Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Worst case scenario between existing and post installed anchors.
- Foundation Analysis Notes: According to the procedures prescribed and agreed to by the Crown Castle Engineering Foundation Committee, held in January 2010, the existing caisson foundation was analyzed using the methodology in the software 'PLS-Caisson' (Version 8.10, or newer, by Power Line Systems, Inc.). Per the methods in PLS-Caisson, the soil reactions of cohesive soils are calculated using 8CD independent of the depth of the soil layer. The depth of soil to be ignored at the top of the caisson is the greater of the geotechnical report's recommendation, the frost depth of the site or half of the caisson diameter.

# APPENDIX A TNXTOWER OUTPUT

# **Tower Input Data**

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

- Tower is located in Hartford County, Connecticut.
- Basic wind speed of 80 mph. 2)
- 3) Nominal ice thickness of 1.0000 in.
- 4) Ice thickness is considered to increase with height.
- Ice density of 56 pcf. 5)
- A wind speed of 38 mph is used in combination with ice. 6)
- Temperature drop of 50 °F. 7)
- Deflections calculated using a wind speed of 50 mph. 8)
- A non-linear (P-delta) analysis was used. 9)
- Pressures are calculated at each section. 10)
- 11) Stress ratio used in pole design is 1.333.
- 12) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

# **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals **Use Moment Magnification** 

- √ Use Code Stress Ratios
- Use Code Safety Factors Guys
  - Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination

Distribute Leg Loads As Uniform Assume Legs Pinned

- Assume Rigid Index Plate
- Use Clear Spans For Wind Area
- Use Clear Spans For KL/r Retension Guys To Initial Tension
- Bypass Mast Stability Checks
- Use Azimuth Dish Coefficients
- Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Use TIA-222-G Tension Splice Capacity Exemption

Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

√ Consider Feedline Torque Include Angle Block Shear Check Poles

√ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

# Pole Section Geometry

Section	Elevation	Section	Pole	Pole	Socket
		Length	Size	Grade	Length
	ft	ft			ft
L1	120.00-90.00	30.00	P24x1/4	A36M-42	
				(42 ksi)	
L2	90.00-80.00	10.00	P24x3/8	A36M-42	
				(42 ksi)	
L3	80.00-68.50	11.50	RPS 24" x 0.62517"	Reinf 33.01 ksi	
				(33 ksi)	
L4	68.50-68.00	0.50	RPS 24" x 0.88003"	Reinf 32.97 ksi	
				(33 ksi)	
L5	68.00-63.50	4.50	RPS 24" x 0.61306"	Reinf 37.81 ksi	
				(38 ksi)	
L6	63.50-60.00	3.50	RPS 24" x 1.13718"	Reinf 33.93 ksi	
				(34 ksi)	
L7	60.00-56.50	3.50	RPS 30" x 0.90733"	Reinf 35.21 ksi	
				(35 ksi)	
L8	56.50-45.42	11.08	RPS 30" x 0.55714"	Reinf 37.98 ksi	
				(38 ksi)	
L9	45.42-36.42	9.00	RPS 30" x 0.70733"	Reinf 37.95 ksi	
				(38 ksi)	
L10	36.42-33.50	2.92	RPS 30" x 0.86188"	Reinf 37.97 ksi	
				(38 ksi)	
L11	33.50-30.00	3.50	RPS 30" x 1.23648"	Reinf 35.20 ksi	

Section	Elevation	Section Length	Pole Size	Pole Grade	Socket Length
	ft	fť			fť
				(35 ksi)	
L12	30.00-26.50	3.50	RPS 36" x 0.7835"	Reinf 35.84 ksi	
				(36 ksi)	
L13	26.50-20.58	5.92	RPS 36" x 0.62423"	Reinf 35.87 ksi	
				(36 ksi)	
L14	20.58-2.00	18.58	RPS 36" x 0.8638"	Reinf 35.84 ksi	
				(36 ksi)	
L15	2.00-0.00	2.00	RPS 36" x 0.94668"	Reinf 35.17 ksi	
				(35 ksi)	

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_A A_A$	Weight
***	Leg			ft			ft²/ft	plf
HB058-M12-	С	No	CaAa (Out Of	118.00 - 0.00	1	No Ice	0.08	0.24
XXXF(5/8")	C	INO	Face)	110.00 - 0.00	'	1/2" Ice	0.08	1.06
XXXI (3/0 )			i ace)			1" Ice	0.18	2.49
						2" Ice		2.49 7.18
							0.48	
ID4444 00114 ME 1/4	_			110.00 0.00		4" Ice	0.88	23.89
IB114-1-08U4-M5J(1	С	No	Inside Pole	118.00 - 0.00	3	No Ice	0.00	1.08
1/4'')						1/2" Ice	0.00	1.08
						1" Ice	0.00	1.08
						2" Ice	0.00	1.08
***						4" Ice	0.00	1.08
*** TCB-B01-001( 5/16)	С	No	Inside Pole	118.00 - 0.00	6	No Ice	0.00	0.07
(100-001-001(3/10)	C	INO	iliside i die	110.00 - 0.00	U	1/2" Ice	0.00	0.07
						1" Ice	0.00	0.07
						2" Ice	0.00	0.07
						4" Ice	0.00	0.07
FSJ4-50B(1/2")	С	No	Inside Pole	118.00 - 0.00	2	No Ice	0.00	0.14
						1/2" Ice	0.00	0.14
						1" Ice	0.00	0.14
						2" Ice	0.00	0.14
						4" Ice	0.00	0.14
2" Conduit	С	No	Inside Pole	118.00 - 0.00	2	No Ice	0.00	0.95
2 Goriaan	•		1110100 1 010	110.00 0.00	_	1/2" Ice	0.00	0.95
						1" Ice	0.00	0.95
						2" Ice	0.00	0.95
						4" Ice		0.95
***						4 ice	0.00	0.95
AL7-50(1 5/8)	С	No	CaAa (Out Of	107.00 - 0.00	2	No Ice	0.20	0.52
			Face)			1/2" Ice	0.30	2.02
			,			1" Ice	0.40	4.14
						2" Ice	0.60	10.20
						4" Ice	1.00	29.65
AL7-50(1 5/8)	С	No	CaAa (Out Of	107.00 - 0.00	4	No Ice	0.00	0.52
AL7-30(1 3/0)	C	INO	Face)	107.00 - 0.00	4	1/2" Ice	0.00	2.02
			race)			1" Ice		4.14
							0.00	
						2" Ice	0.00	10.20
	_					4" Ice	0.00	29.65
FLC 158-50J(1-5/8")	С	No	Inside Pole	107.00 - 0.00	12	No Ice	0.00	0.92
						1/2" Ice	0.00	0.92
						1" Ice	0.00	0.92
						2" Ice	0.00	0.92
						4" Ice	0.00	0.92
MLE Hybrid	С	No	CaAa (Out Of	107.00 - 0.00	1	No Ice	0.00	1.07
Power/18Fiber RL 2(	-		Face)			1/2" Ice	0.00	2.37
1 5/8)			,			1" Ice	0.00	4.28
. 5, 5,						2" Ice	0.00	9.93
						4" Ice	0.00	28.56
***						+ 10 <del>0</del>	0.00	20.30
	_			00.00 0.00	_	Mar Land	0.00	0.00
LDF5-50A(7/8")	С	No	Inside Pole	99.00 - 0.00	6	No Ice	0.00	0.33
LDF5-50A(7/8")	С	No	Inside Pole	99.00 - 0.00	6	1/2" Ice	0.00	0.33

Description	Face or	Allow Shield	Component Type	Placement	Total Number		$C_AA_A$	Weight
	Leg		77-	ft			ft²/ft	plf
						2" Ice	0.00	0.33
						4" Ice	0.00	0.33
FB-L98B-002-75000(	С	No	Inside Pole	99.00 - 0.00	1	No Ice	0.00	0.06
3/8")						1/2" Ice	0.00	0.06
,						1" Ice	0.00	0.06
						2" Ice	0.00	0.06
						4" Ice	0.00	0.06
WR-VG86ST-BRD(	С	No	Inside Pole	99.00 - 0.00	2	No Ice	0.00	0.59
3/4)						1/2" Ice	0.00	0.59
,						1" Ice	0.00	0.59
						2" Ice	0.00	0.59
						4" Ice	0.00	0.59
LDF5-50A(7/8")	С	No	Inside Pole	99.00 - 0.00	6	No Ice	0.00	0.33
( ,						1/2" Ice	0.00	0.33
						1" Ice	0.00	0.33
						2" Ice	0.00	0.33
						4" Ice	0.00	0.33
***								
***								
LDF5-50A(7/8")	С	No	CaAa (Out Of	59.00 - 0.00	1	No Ice	0.11	0.33
( ,			Face)			1/2" Ice	0.21	1.30
			,			1" Ice	0.31	2.88
						2" Ice	0.51	7.88
						4" Ice	0.91	25.20
***								
LDF4-50A(1/2")	С	No	CaAa (Out Of	48.00 - 0.00	1	No Ice	0.00	0.15
			Face)			1/2" Ice	0.00	0.84
						1" Ice	0.00	2.14
						2" Ice	0.00	6.58
						4" Ice	0.00	22.78
LDF4-50A(1/2")	С	No	CaAa (Out Of	48.00 - 0.00	1	No Ice	0.00	0.15
			Face)			1/2" Ice	0.00	0.84
						1" Ice	0.00	2.14
						2" Ice	0.00	6.58
						4" Ice	0.00	22.78
***								
C8x18.75	С	No	CaAa (Out Of	33.50 - 0.00	2	No Ice	0.42	0.00
			Face)			1/2" Ice	0.53	0.00
						1" Ice	0.64	0.00
						2" Ice	0.87	0.00
						4" Ice	1.31	0.00
C8x11.5 brace	С	No	CaAa (Out Of	70.00 - 33.50	2	No Ice	0.42	0.00
			Face)			1/2" Ice	0.53	0.00
			,			1" Ice	0.64	0.00
						2" Ice	0.87	0.00
						4" Ice	1.31	0.00
***								

# Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
	. 3		Vert ft ft ft	0	ft		ft <sup>2</sup>	ft <sup>2</sup>	К
APXVTM14-C-120 w/ Mount Pipe	Α	From Leg	4.00 0.00 2.00	0.0000	118.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	7.13 7.66 8.18 9.26 11.53	4.96 5.75 6.47 8.01 11.41	0.08 0.13 0.19 0.34 0.75
APXVTM14-C-120 w/ Mount Pipe	В	From Leg	4.00 0.00 2.00	0.0000	118.00	No Ice 1/2" Ice	7.13 7.66 8.18	4.96 5.75 6.47	0.08 0.13 0.19

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	0	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
						1" Ice 2" Ice 4" Ice	9.26 11.53	8.01 11.41	0.34 0.75
APXVTM14-C-120 w/ Mount Pipe	С	From Leg	4.00 0.00 2.00	0.0000	118.00	No Ice 1/2" Ice 1" Ice 2" Ice	7.13 7.66 8.18 9.26 11.53	4.96 5.75 6.47 8.01 11.41	0.08 0.13 0.19 0.34 0.75
TD-RRH8x20-25	Α	From Leg	4.00 0.00 2.00	0.0000	118.00	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	4.72 5.01 5.32 5.95 7.31	1.70 1.92 2.15 2.62 3.68	0.07 0.10 0.13 0.20 0.40
TD-RRH8x20-25	В	From Leg	4.00 0.00 2.00	0.0000	118.00	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	4.72 5.01 5.32 5.95 7.31	1.70 1.92 2.15 2.62 3.68	0.07 0.10 0.13 0.20 0.40
TD-RRH8x20-25	С	From Leg	4.00 0.00 2.00	0.0000	118.00	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	4.72 5.01 5.32 5.95 7.31	1.70 1.92 2.15 2.62 3.68	0.07 0.10 0.13 0.20 0.40
APXV9ERR18-C-A20 w/ Mount Pipe	Α	From Leg	4.00 0.00 2.00	0.0000	118.00	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice	8.50 9.15 9.77 11.03 13.68	7.47 8.66 9.56 11.39 15.53	0.09 0.16 0.24 0.42 0.94
APXVSPP18-C-A20 w/ Mount Pipe	В	From Leg	4.00 0.00 2.00	0.0000	118.00	4" Ice No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.50 9.15 9.77 11.03 13.68	6.95 8.13 9.02 10.84 14.85	0.08 0.15 0.23 0.41 0.91
APXVSPP18-C-A20 w/ Mount Pipe	С	From Leg	4.00 0.00 2.00	0.0000	118.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.50 9.15 9.77 11.03 13.68	6.95 8.13 9.02 10.84 14.85	0.08 0.15 0.23 0.41 0.91
IBC1900HG-2A	Α	From Leg	4.00 0.00 -2.00	0.0000	118.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.13 1.27 1.43 1.76 2.53	0.53 0.65 0.77 1.04 1.69	0.02 0.03 0.04 0.06 0.15
IBC1900HG-2A	В	From Leg	4.00 0.00 -2.00	0.0000	118.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.13 1.27 1.43 1.76 2.53	0.53 0.65 0.77 1.04 1.69	0.02 0.03 0.04 0.06 0.15
IBC1900HG-2A	С	From Leg	4.00 0.00 -2.00	0.0000	118.00	No Ice 1/2" Ice 1" Ice 2" Ice	1.13 1.27 1.43 1.76 2.53	0.53 0.65 0.77 1.04 1.69	0.02 0.03 0.04 0.06 0.15
IBC1900BB-1	Α	From Leg	4.00 0.00	0.0000	118.00	4" Ice No Ice 1/2"	1.13 1.27	0.53 0.65	0.02 0.03

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Vert ft ft ft	٥	ft		ft <sup>2</sup>	ft²	К
			-2.00			Ice	1.43	0.77	0.04
						1" Ice 2" Ice 4" Ice	1.76 2.53	1.04 1.69	0.06 0.15
IBC1900BB-1	В	From Leg	4.00	0.0000	118.00	No Ice	1.13	0.53	0.02
			0.00 -2.00			1/2" Ice	1.27 1.43	0.65 0.77	0.03 0.04
			2.00			1" Ice	1.76	1.04	0.06
						2" Ice 4" Ice	2.53	1.69	0.15
IBC1900BB-1	С	From Leg	4.00	0.0000	118.00	No Ice	1.13	0.53	0.02
			0.00 -2.00			1/2" Ice	1.27 1.43	0.65 0.77	0.03 0.04
			2.00			1" Ice	1.76	1.04	0.06
						2" Ice 4" Ice	2.53	1.69	0.15
* (2) HORIZON COMPACT	В	From Leg	4.00	0.0000	118.00	No Ice	0.84	0.43	0.01
(2) HORIZON COMPACT	ь	r rom Leg	0.00	0.0000	116.00	1/2"	0.04	0.43	0.01
			-2.00			Ice	1.10	0.63	0.03
						1" Ice	1.39	0.86	0.05
						2" Ice 4" Ice	2.08	1.43	0.12
Platform Mount [LP 501-1]	С	None		0.0000	118.00	No Ice	32.04	32.04	0.98
						1/2"	45.28	45.28	1.28
						Ice	58.51	58.51	1.57
						1" Ice 2" Ice	84.98 137.92	84.98 137.92	2.16 3.33
***						4" Ice	107.02	107.02	0.00
Pipe Mount [PM 601-3]	С	None		0.0000	114.00	No Ice	4.39	4.39	0.20
						1/2"	5.48	5.48	0.24
						Ice 1" Ice	6.57 8.75	6.57 8.75	0.28 0.36
						2" Ice	13.11	13.11	0.53
***						4" Ice			
PCS 1900MHz 4x45W-	Α	From Leg	2.00	0.0000	114.00	No Ice	2.71	2.61	0.06
65MHz			0.00			1/2"	2.95	2.85	0.08
			1.00			Ice 1" Ice	3.20 3.72	3.09 3.61	0.11 0.17
						2" Ice	4.86	4.74	0.17
						4" Ice			
PCS 1900MHz 4x45W-	В	From Leg	2.00	0.0000	114.00	No Ice	2.71	2.61	0.06
65MHz			0.00 1.00			1/2" Ice	2.95 3.20	2.85 3.09	0.08 0.11
			1.00			1" Ice	3.72	3.61	0.17
						2" Ice 4" Ice	4.86	4.74	0.35
PCS 1900MHz 4x45W-	С	From Leg	2.00	0.0000	114.00	No Ice	2.71	2.61	0.06
65MHz			0.00			1/2"	2.95	2.85	0.08
			1.00			Ice 1" Ice	3.20 3.72	3.09 3.61	0.11 0.17
						2" Ice	4.86	4.74	0.17
OOOMUL OVEOM DDU	^	Francisco	2.00	0.0000	114.00	4" Ice	0.40	0.05	
800MHz 2X50W RRH W/FILTER	Α	From Leg	2.00 0.00	0.0000	114.00	No Ice 1/2"	2.40 2.61	2.25 2.46	0.06 0.09
VV/I ILILIX			-1.00			Ice	2.83	2.68	0.09
						1" Ice	3.30	3.13	0.17
						2" Ice 4" Ice	4.34	4.15	0.34
800MHz 2X50W RRH	В	From Leg	2.00	0.0000	114.00	No Ice	2.40	2.25	0.06
W/FILTER		3	0.00			1/2"	2.61	2.46	0.09
			-1.00			Ice	2.83	2.68	0.11
						1" Ice	3.30	3.13	0.17

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustmen	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
	Leg	Турс	Lateral Vert	t t			TTOIL	Olac	
			ft ft ft	0	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
			- IL			2" Ice	4.34	4.15	0.34
800MHz 2X50W RRH	С	From Leg	2.00	0.0000	114.00	4" Ice No Ice	2.40	2.25	0.06
W/FILTER			0.00			1/2"	2.61	2.46	0.09
			-1.00			Ice 1" Ice	2.83 3.30	2.68 3.13	0.11 0.17
						2" Ice 4" Ice	4.34	4.15	0.17
***									
Platform Mount [LP 712-1]	С	None		0.0000	107.00	No Ice 1/2"	24.53 29.94	24.53 29.94	1.34 1.65
						Ice	35.35	35.35	1.05
						1" Ice	46.17	46.17	2.58
						2" Ice	67.81	67.81	3.82
EDIO000N AID 04 D0A	^	E	4.00	0.0000	407.00	4" Ice	0.00	5.04	0.44
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	Α	From Leg	4.00 0.00	0.0000	107.00	No Ice 1/2"	6.83 7.35	5.64 6.48	0.11 0.17
64F W/ Mount Pipe			1.00			Ice	7.33 7.86	7.26	0.17
			1.00			1" Ice	8.93	8.86	0.38
						2" Ice 4" Ice	11.18	12.29	0.81
ERICSSON AIR 21 B2A	В	From Leg	4.00	0.0000	107.00	No Ice	6.83	5.64	0.11
B4P w/ Mount Pipe			0.00			1/2"	7.35	6.48	0.17
			1.00			Ice 1" Ice	7.86 8.93	7.26 8.86	0.23 0.38
						2" Ice	0.93 11.18	12.29	0.36
						4" Ice	11.10	12.20	0.01
ERICSSON AIR 21 B2A	С	From Leg	4.00	0.0000	107.00	No Ice	6.83	5.64	0.11
B4P w/ Mount Pipe			0.00			1/2''	7.35	6.48	0.17
			1.00			Ice 1" Ice	7.86 8.93	7.26 8.86	0.23 0.38
						2" Ice	0.93 11.18	12.29	0.36
						4" Ice		12.20	0.01
ERICSSON AIR 21 B4A	Α	From Leg	4.00	0.0000	107.00	No Ice	6.82	5.63	0.11
B2P w/ Mount Pipe			0.00			1/2"	7.34	6.47	0.17
			1.00			Ice 1" Ice	7.85	7.25	0.23
						2" Ice	8.92 11.17	8.85 12.28	0.38 0.81
						4" Ice		12.20	0.01
ERICSSON AIR 21 B4A	В	From Leg	4.00	0.0000	107.00	No Ice	6.82	5.63	0.11
B2P w/ Mount Pipe			0.00			1/2"	7.34	6.47	0.17
			1.00			Ice	7.85	7.25	0.23
						1" Ice 2" Ice	8.92 11.17	8.85 12.28	0.38 0.81
						4" Ice	11.17	12.20	0.01
ERICSSON AIR 21 B4A	С	From Leg	4.00	0.0000	107.00	No Ice	6.82	5.63	0.11
B2P w/ Mount Pipe			0.00			1/2"	7.34	6.47	0.17
			1.00			Ice	7.85	7.25	0.23
						1" Ice 2" Ice	8.92 11.17	8.85 12.28	0.38 0.81
						4" Ice	11.17	12.20	0.01
KRY 112 144/1	Α	From Leg	4.00	0.0000	107.00	No Ice	0.41	0.20	0.01
		_	0.00			1/2"	0.50	0.27	0.01
			1.00			Ice	0.59	0.35	0.02
						1" Ice 2" Ice	0.81 1.36	0.53 1.00	0.03
						4" Ice	1.30	1.00	0.08
KRY 112 144/1	В	From Leg	4.00	0.0000	107.00	No Ice	0.41	0.20	0.01
		3	0.00			1/2"	0.50	0.27	0.01
			1.00			Ice	0.59	0.35	0.02
						1" Ice 2" Ice	0.81 1.36	0.53 1.00	0.03 0.08
						4" Ice	1.30	1.00	0.00
KRY 112 144/1	С	From Leg	4.00	0.0000	107.00	No Ice	0.41	0.20	0.01
		-	0.00			1/2''	0.50	0.27	0.01

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			ft ft ft	0	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
			1.00			Ice	0.59	0.35	0.02
***						1" Ice 2" Ice 4" Ice	0.81 1.36	0.53 1.00	0.03 0.08
(2) P65-17-XLH-RR w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	11.82 12.59 13.38 14.94 18.33	9.06 10.62 12.21 14.70 19.64	0.09 0.18 0.28 0.51 1.14
(2) AM-X-CD-16-65-00T- RET w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	8.50 9.15 9.77 11.03 13.68	6.30 7.48 8.37 10.18 14.02	0.07 0.14 0.21 0.38 0.87
AM-X-CD-14-65-00T-RET w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.74 6.20 6.66 7.62 9.67	4.02 4.63 5.28 6.68 9.74	0.05 0.10 0.15 0.27 0.63
AM-X-CD-14-65-00T-RET w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	5.74 6.20 6.66 7.62 9.67	4.02 4.63 5.28 6.68 9.74	0.05 0.10 0.15 0.27 0.63
7770.00 w/ Mount Pipe	Α	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.12 6.63 7.13 8.16 10.36	4.25 5.01 5.71 7.16 10.41	0.06 0.10 0.16 0.29 0.66
7770.00 w/ Mount Pipe	В	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.12 6.63 7.13 8.16 10.36	4.25 5.01 5.71 7.16 10.41	0.06 0.10 0.16 0.29 0.66
7770.00 w/ Mount Pipe	С	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	6.12 6.63 7.13 8.16 10.36	4.25 5.01 5.71 7.16 10.41	0.06 0.10 0.16 0.29 0.66
(2) RRUS-11	Α	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.25 3.49 3.74 4.27 5.43	1.37 1.55 1.74 2.14 3.04	0.05 0.07 0.09 0.15 0.31
(2) RRUS-11	В	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.25 3.49 3.74 4.27 5.43	1.37 1.55 1.74 2.14 3.04	0.05 0.07 0.09 0.15 0.31
(2) RRUS-11	С	From Leg	4.00 0.00 1.00	0.0000	99.00	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.25 3.49 3.74 4.27 5.43	1.37 1.55 1.74 2.14 3.04	0.05 0.07 0.09 0.15 0.31

Description	<b>-</b>	0.551	0.55	A = '	D/			0.4	14/- 1-/-/
Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustmen t	Placement		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
	ŭ		Vert				•2	•2	
			ft ft ft	٥	ft		ft <sup>2</sup>	ft <sup>2</sup>	K
DTMABP7819VG12A	Α	From Leg	4.00	0.0000	99.00	No Ice	1.14	0.39	0.02
			0.00			1/2"	1.28	0.49	0.03
			1.00			Ice	1.44	0.59	0.04
						1" Ice 2" Ice	1.77 2.54	0.83 1.41	0.06 0.14
						4" Ice	2.34	1.41	0.14
DTMABP7819VG12A	В	From Leg	4.00	0.0000	99.00	No Ice	1.14	0.39	0.02
		Ü	0.00			1/2"	1.28	0.49	0.03
			1.00			Ice	1.44	0.59	0.04
						1" Ice	1.77	0.83	0.06
						2" Ice 4" Ice	2.54	1.41	0.14
DTMABP7819VG12A	С	From Leg	4.00	0.0000	99.00	No Ice	1.14	0.39	0.02
			0.00			1/2"	1.28	0.49	0.03
			1.00			Ice	1.44	0.59	0.04
						1" Ice	1.77	0.83	0.06
						2" Ice 4" Ice	2.54	1.41	0.14
(2) LGP13519	Α	From Leg	4.00	0.0000	99.00	No Ice	0.34	0.21	0.01
(=) ==: ::::::::::::::::::::::::::::::::	•		0.00	0.000	00.00	1/2"	0.42	0.28	0.01
			1.00			Ice	0.51	0.36	0.01
						1" Ice	0.73	0.55	0.02
						2" Ice 4" Ice	1.25	1.03	0.07
(2) LGP13519	В	From Leg	4.00	0.0000	99.00	No Ice	0.34	0.21	0.01
(2) 231 10010		r rom Log	0.00	0.0000	33.00	1/2"	0.42	0.28	0.01
			1.00			Ice	0.51	0.36	0.01
						1" Ice	0.73	0.55	0.02
						2" Ice 4" Ice	1.25	1.03	0.07
(2) LGP13519	С	From Leg	4.00	0.0000	99.00	No Ice	0.34	0.21	0.01
(2) 231 10010	Ū	r rom Log	0.00	0.0000	00.00	1/2"	0.42	0.28	0.01
			1.00			Ice	0.51	0.36	0.01
						1" Ice	0.73	0.55	0.02
						2" Ice 4" Ice	1.25	1.03	0.07
DC6-48-60-18-8F	Α	From Leg	4.00	0.0000	99.00	No Ice	2.57	2.57	0.02
200 10 00 10 01			0.00	0.000	00.00	1/2"	2.80	2.80	0.04
			1.00			Ice	3.04	3.04	0.07
						1" Ice	3.54	3.54	0.13
						2" Ice 4" Ice	4.66	4.66	0.30
Platform Mount [LP 501-1]	С	None		0.0000	99.00	No Ice	32.04	32.04	0.98
						1/2"	45.28	45.28	1.28
						Ice	58.51	58.51	1.57
						1" Ice 2" Ice	84.98 137.92	84.98 137.92	2.16 3.33
						4" Ice	137.92	137.92	3.33
***									
DB536	Α	From Leg	4.00	0.0000	59.00	No Ice	2.83	2.83	0.02
			0.00 4.00			1/2" Ice	3.99 5.16	3.99 5.16	0.04 0.06
			4.00			1" Ice	7.08	7.08	0.14
						2" Ice	9.94	9.94	0.40
0.1 4 44				0.000-	<b>F</b> • • •	4" Ice	4 2 2	,	
Side Arm Mount [SO 702-	Α	None		0.0000	59.00	No Ice 1/2"	1.00	1.43	0.03 0.04
1]						Ice	1.00 1.00	2.05 2.67	0.04
						1" Ice	1.00	3.91	0.07
						2" Ice	1.00	6.39	0.12
***						4" Ice			
GPS	Α	From Leg	4.00	0.0000	48.00	No Ice	0.17	0.17	0.00
5. 0			0.00	3.5555		1/2"	0.24	0.24	0.00
			2.00			Ice	0.31	0.31	0.00

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustmen	Placement		$C_A A_A$ Front	$C_A A_A$ Side	Weight
	Leg		Lateral Vert	t					
			ft		ft		$ft^2$	$ft^2$	K
			ft	0					
			ft						
						1" Ice	0.48	0.48	0.01
						2" Ice	0.92	0.92	0.05
						4" Ice			
KS24019-L112A	Α	From Leg	4.00	0.0000	48.00	No Ice	0.16	0.16	0.01
			0.00			1/2"	0.22	0.22	0.01
			2.00			Ice	0.30	0.30	0.01
						1" Ice	0.48	0.48	0.02
						2" Ice	0.95	0.95	0.06
						4" Ice			
Pipe Mount [PM 601-1]	Α	None		0.0000	48.00	No Ice	3.00	0.90	0.07
						1/2''	3.74	1.12	0.08
						Ice	4.48	1.34	0.09
						1" Ice	5.96	1.78	0.12
						2" Ice	8.92	2.66	0.18
						4" Ice			
***									

ח	ie	h	es
$\boldsymbol{\omega}$	13		<b>C</b> 3

Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	0	0	ft	ft		ft <sup>2</sup>	K
A-ANT-18G-1-C	В	Paraboloid w/o Radome	From Leg	4.00 0.00 -2.00	-6.0000		118.00	1.27	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	1.28 4.01 4.30 4.88 6.04	0.02 0.04 0.05 0.07 0.11
A-ANT-18G-2-C	В	Paraboloid w/o Radome	From Leg	4.00 0.00 -2.00	20.0000		118.00	2.17	No Ice 1/2" Ice 1" Ice 2" Ice 4" Ice	3.72 4.01 4.30 4.88 6.04	0.03 0.04 0.05 0.07 0.11

# **Tower Pressures - No Ice**

 $G_H = 1.690$ 

Section	Z	Kz	$q_z$	$A_{G}$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_AA_A$	$C_AA_A$
Elevation					а				%	In	Out
					С	_	_	_		Face	Face
ft	ft		psf	ft <sup>2</sup>	е	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
L1 120.00-	105.00	1.392	23	60.000	Α	0.000	60.000	60.000	100.00	0.000	0.000
90.00					В	0.000	60.000		100.00	0.000	0.000
					С	0.000	60.000		100.00	0.000	9.016
L2 90.00-	85.00	1.31	21	20.000	Α	0.000	20.000	20.000	100.00	0.000	0.000
80.00					В	0.000	20.000		100.00	0.000	0.000
					С	0.000	20.000		100.00	0.000	4.760
L3 80.00-	74.25	1.261	21	23.000	Α	0.000	23.000	23.000	100.00	0.000	0.000
68.50					В	0.000	23.000		100.00	0.000	0.000
					С	0.000	23.000		100.00	0.000	6.737
L4 68.50-	68.25	1.231	20	1.000	Α	0.000	1.000	1.000	100.00	0.000	0.000
68.00					В	0.000	1.000		100.00	0.000	0.000
					С	0.000	1.000		100.00	0.000	0.659
L5 68.00-	65.75	1.218	20	9.000	Α	0.000	9.000	9.000	100.00	0.000	0.000
63.50					В	0.000	9.000		100.00	0.000	0.000
					С	0.000	9.000		100.00	0.000	5.932
L6 63.50-	61.75	1.196	20	7.000	Α	0.000	7.000	7.000	100.00	0.000	0.000
60.00					В	0.000	7.000		100.00	0.000	0.000

Section	Z	K <sub>Z</sub>	$q_z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_AA_A$	$C_AA_A$
Elevation			-		а			_	%	In	Out
					С			_		Face	Face
ft	ft		psf	ft <sup>2</sup>	е	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
					С	0.000	7.000		100.00	0.000	4.614
L7 60.00-	58.25	1.176	19	8.750	Α	0.000	8.750	8.750	100.00	0.000	0.000
56.50					В	0.000	8.750		100.00	0.000	0.000
					С	0.000	8.750		100.00	0.000	4.887
L8 56.50-	50.96	1.132	19	27.708	Α	0.000	27.708	27.708	100.00	0.000	0.000
45.42					В	0.000	27.708		100.00	0.000	0.000
					С	0.000	27.708		100.00	0.000	15.819
L9 45.42-	40.92	1.063	17	22.500	Α	0.000	22.500	22.500	100.00	0.000	0.000
36.42					В	0.000	22.500		100.00	0.000	0.000
					С	0.000	22.500		100.00	0.000	12.846
L10 36.42-	34.96	1.017	17	7.293	Α	0.000	7.293	7.293	100.00	0.000	0.000
33.50					В	0.000	7.293		100.00	0.000	0.000
					С	0.000	7.293		100.00	0.000	4.163
L11 33.50-	31.75	1	16	8.750	Α	0.000	8.750	8.750	100.00	0.000	0.000
30.00					В	0.000	8.750		100.00	0.000	0.000
					С	0.000	8.750		100.00	0.000	4.996
L12 30.00-	28.25	1	16	10.500	Α	0.000	10.500	10.500	100.00	0.000	0.000
26.50					В	0.000	10.500		100.00	0.000	0.000
					С	0.000	10.500		100.00	0.000	4.996
L13 26.50-	23.54	1	16	17.751	Α	0.000	17.751	17.751	100.00	0.000	0.000
20.58					В	0.000	17.751		100.00	0.000	0.000
					С	0.000	17.751		100.00	0.000	8.445
L14 20.58-	11.29	1	16	55.749	Α	0.000	55.749	55.749	100.00	0.000	0.000
2.00					В	0.000	55.749		100.00	0.000	0.000
					С	0.000	55.749		100.00	0.000	26.524
L15 2.00-0.00	1.00	1	16	6.000	Α	0.000	6.000	6.000	100.00	0.000	0.000
					В	0.000	6.000		100.00	0.000	0.000
					С	0.000	6.000		100.00	0.000	2.855

# **Tower Pressure - With Ice**

 $G_H = 1.690$ 

Section	Z	Κz	$q_z$	$t_Z$	$A_{G}$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
Elevation						а				%	In	Out
						С	_		_		Face	Face
ft	ft		psf	in	ft <sup>2</sup>	е	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		f <del>t²</del>	ft <sup>2</sup>
L1 120.00-	105.00	1.392	5	1.1490	65.745	Α	0.000	65.745	65.745	100.00	0.000	0.000
90.00						В	0.000	65.745		100.00	0.000	0.000
						С	0.000	65.745		100.00	0.000	23.264
L2 90.00-80.00	85.00	1.31	5	1.1202	21.867	Α	0.000	21.867	21.867	100.00	0.000	0.000
						В	0.000	21.867		100.00	0.000	0.000
						С	0.000	21.867		100.00	0.000	11.481
L3 80.00-68.50	74.25	1.261	5	1.1022	25.113	Α	0.000	25.113	25.113	100.00	0.000	0.000
						В	0.000	25.113		100.00	0.000	0.000
						С	0.000	25.113		100.00	0.000	15.078
L4 68.50-68.00	68.25	1.231	4	1.0911	1.091	Α	0.000	1.091	1.091	100.00	0.000	0.000
						В	0.000	1.091		100.00	0.000	0.000
						С	0.000	1.091		100.00	0.000	1.229
L5 68.00-63.50	65.75	1.218	4	1.0862	9.815	Α	0.000	9.815	9.815	100.00	0.000	0.000
						В	0.000	9.815		100.00	0.000	0.000
						С	0.000	9.815		100.00	0.000	11.038
L6 63.50-60.00	61.75	1.196	4	1.0781	7.629	Α	0.000	7.629	7.629	100.00	0.000	0.000
						В	0.000	7.629		100.00	0.000	0.000
						С	0.000	7.629		100.00	0.000	8.555
L7 60.00-56.50	58.25	1.176	4	1.0706	9.374	Α	0.000	9.374	9.374	100.00	0.000	0.000
						В	0.000	9.374		100.00	0.000	0.000
						С	0.000	9.374		100.00	0.000	9.336
L8 56.50-45.42	50.96	1.132	4	1.0535	29.654	Α	0.000	29.654	29.654	100.00	0.000	0.000
						В	0.000	29.654		100.00	0.000	0.000
						С	0.000	29.654		100.00	0.000	30.350
L9 45.42-36.42	40.92	1.063	4	1.0261	24.039	Α	0.000	24.039	24.039	100.00	0.000	0.000
						В	0.000	24.039		100.00	0.000	0.000
						С	0.000	24.039		100.00	0.000	24.339
L10 36.42-	34.96	1.017	4	1.0069	7.782	Α	0.000	7.782	7.782	100.00	0.000	0.000
33.50						В	0.000	7.782		100.00	0.000	0.000

Section	Z	Κz	$q_z$	$t_Z$	$A_{G}$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_AA_A$	$C_AA_A$
Elevation						а				%	In	Out
					_	С		_			Face	Face
ft	ft		psf	in	ft <sup>2</sup>	е	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	f <del>t²</del>
						С	0.000	7.782		100.00	0.000	7.819
L11 33.50-	31.75	1	4	1.0000	9.333	Α	0.000	9.333	9.333	100.00	0.000	0.000
30.00						В	0.000	9.333		100.00	0.000	0.000
						С	0.000	9.333		100.00	0.000	9.351
L12 30.00-	28.25	1	4	1.0000	11.083	Α	0.000	11.083	11.083	100.00	0.000	0.000
26.50						В	0.000	11.083		100.00	0.000	0.000
						С	0.000	11.083		100.00	0.000	9.351
L13 26.50-	23.54	1	4	1.0000	18.737	Α	0.000	18.737	18.737	100.00	0.000	0.000
20.58						В	0.000	18.737		100.00	0.000	0.000
						С	0.000	18.737		100.00	0.000	15.809
L14 20.58-2.00	11.29	1	4	1.0000	58.846	Α	0.000	58.846	58.846	100.00	0.000	0.000
						В	0.000	58.846		100.00	0.000	0.000
						С	0.000	58.846		100.00	0.000	49.650
L15 2.00-0.00	1.00	1	4	1.0000	6.333	Α	0.000	6.333	6.333	100.00	0.000	0.000
						В	0.000	6.333		100.00	0.000	0.000
						С	0.000	6.333		100.00	0.000	5.344

# **Tower Pressure - Service**

 $G_H = 1.690$ 

Section	Z	Kz	$q_z$	$A_{G}$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
Elevation					а			.5	%	In	Out
					С					Face	Face
ft	ft		psf	ft <sup>2</sup>	е	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
L1 120.00-	105.00	1.392	9	60.000	Α	0.000	60.000	60.000	100.00	0.000	0.000
90.00					В	0.000	60.000		100.00	0.000	0.000
					С	0.000	60.000		100.00	0.000	9.016
L2 90.00-	85.00	1.31	8	20.000	Α	0.000	20.000	20.000	100.00	0.000	0.000
80.00					В	0.000	20.000		100.00	0.000	0.000
					С	0.000	20.000		100.00	0.000	4.760
L3 80.00-	74.25	1.261	8	23.000	Α	0.000	23.000	23.000	100.00	0.000	0.000
68.50					В	0.000	23.000		100.00	0.000	0.000
					С	0.000	23.000		100.00	0.000	6.737
L4 68.50-	68.25	1.231	8	1.000	Α	0.000	1.000	1.000	100.00	0.000	0.000
68.00					В	0.000	1.000		100.00	0.000	0.000
					С	0.000	1.000		100.00	0.000	0.659
L5 68.00-	65.75	1.218	8	9.000	Α	0.000	9.000	9.000	100.00	0.000	0.000
63.50					В	0.000	9.000		100.00	0.000	0.000
1000 50	04.75	4 400	•	7.000	Ç	0.000	9.000	7 000	100.00	0.000	5.932
L6 63.50-	61.75	1.196	8	7.000	Α	0.000	7.000	7.000	100.00	0.000	0.000
60.00					В	0.000	7.000		100.00	0.000	0.000
17.00.00	50.05	4 470		0.750	C	0.000	7.000	0.750	100.00	0.000	4.614
L7 60.00-	58.25	1.176	8	8.750	Α	0.000	8.750	8.750	100.00	0.000	0.000
56.50					ВС	0.000 0.000	8.750 8.750		100.00 100.00	0.000 0.000	0.000 4.887
L8 56.50-	50.96	1.132	7	27.708		0.000	27.708	27.708	100.00	0.000	0.000
45.42	50.96	1.132	1	21.106	A B	0.000	27.708	21.106	100.00	0.000	0.000
45.42					С	0.000	27.708		100.00	0.000	15.819
L9 45.42-	40.92	1.063	7	22.500	A	0.000	22.500	22.500	100.00	0.000	0.000
36.42	40.92	1.003	,	22.300	В	0.000	22.500	22.300	100.00	0.000	0.000
30.42					C	0.000	22.500		100.00	0.000	12.846
L10 36.42-	34.96	1.017	7	7.293	A	0.000	7.293	7.293	100.00	0.000	0.000
33.50	04.00	1.017	,	7.200	В	0.000	7.293	7.200	100.00	0.000	0.000
00.00					C	0.000	7.293		100.00	0.000	4.163
L11 33.50-	31.75	1	6	8.750	Ä	0.000	8.750	8.750	100.00	0.000	0.000
30.00	00	·	ŭ	0.7.00	В	0.000	8.750	000	100.00	0.000	0.000
30.00					Č	0.000	8.750		100.00	0.000	4.996
L12 30.00-	28.25	1	6	10.500	Ä	0.000	10.500	10.500	100.00	0.000	0.000
26.50		•			В	0.000	10.500		100.00	0.000	0.000
					C	0.000	10.500		100.00	0.000	4.996
L13 26.50-	23.54	1	6	17.751	Α	0.000	17.751	17.751	100.00	0.000	0.000
20.58					В	0.000	17.751		100.00	0.000	0.000
					С	0.000	17.751		100.00	0.000	8.445
L14 20.58-	11.29	1	6	55.749	Α	0.000	55.749	55.749	100.00	0.000	0.000
2.00					В	0.000	55.749		100.00	0.000	0.000

Section Elevation	Z	Kz	$q_z$	$A_G$	F a	$A_F$	$A_R$	$A_{leg}$	Leg %	C <sub>A</sub> A <sub>A</sub> In	$C_A A_A$ Out
Lievation	6			s-2	С	•2	•2	•2	70	Face	Face
ft	ft		psf	ft <sup>2</sup>	е	f <del>t'</del>	f <del>t'</del>	f <del>t</del>		f <del>t'</del>	ft
					С	0.000	55.749		100.00	0.000	26.524
L15 2.00-0.00	1.00	1	6	6.000	Α	0.000	6.000	6.000	100.00	0.000	0.000
					В	0.000	6.000		100.00	0.000	0.000
					С	0.000	6.000		100.00	0.000	2.855

# **Load Combinations**

Dead Only	0	Description	_
1 Dead-Wind 0 deg - No Ice 3 Dead+Wind 30 deg - No Ice 4 Dead+Wind 30 deg - No Ice 5 Dead+Wind 90 deg - No Ice 6 Dead+Wind 120 deg - No Ice 7 Dead+Wind 150 deg - No Ice 8 Dead+Wind 150 deg - No Ice 9 Dead+Wind 210 deg - No Ice 10 Dead+Wind 210 deg - No Ice 11 Dead+Wind 220 deg - No Ice 12 Dead+Wind 270 deg - No Ice 13 Dead+Wind 330 deg - No Ice 14 Dead+Wind 330 deg - No Ice 15 Dead+Wind 330 deg - No Ice 16 Dead+Wind 330 deg - No Ice 17 Dead+Wind 330 deg - No Ice 18 Dead+Wind 300 deg - No Ice 19 Dead+Wind 300 deg+Ice+Temp 10 Dead+Wind 0 deg+Ice+Temp 11 Dead+Wind 0 deg+Ice+Temp 12 Dead+Wind 10 deg+Ice+Temp 13 Dead+Wind 10 deg+Ice+Temp 14 Dead+Wind 150 deg+Ice+Temp 15 Dead+Wind 150 deg+Ice+Temp 16 Dead+Wind 150 deg+Ice+Temp 17 Dead+Wind 150 deg+Ice+Temp 18 Dead+Wind 150 deg+Ice+Temp 20 Dead+Wind 310 deg+Ice+Temp 21 Dead+Wind 310 deg+Ice+Temp 22 Dead+Wind 310 deg+Ice+Temp 23 Dead+Wind 300 deg+Ice+Temp 24 Dead+Wind 300 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg+Ice+Temp 27 Dead+Wind 300 deg - Service 28 Dead+Wind 30 deg - Service 30 Dead+Wind 10 deg - Service 31 Dead+Wind 150 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 180 deg - Service 35 Dead+Wind 240 deg - Service	Comb.	Description	
Dead+Wind 0 deg - No Ice Dead+Wind 60 deg - No Ice Dead+Wind 60 deg - No Ice Dead+Wind 120 deg - No Ice Dead+Wind 150 deg - No Ice Dead+Wind 210 deg - No Ice Dead+Wind 270 deg - No Ice Dead+Wind 300 deg - No Ice Dead+Wind 0 deg+Ice+Temp Dead+Wind 100 deg+Ice+Temp Dead+Wind 100 deg+Ice+Temp Dead+Wind 100 deg+Ice+Temp Dead+Wind 150 deg+Ice+Temp Dead+Wind 130 deg+Ice+Temp Dead+Wind 300 deg - Service Dead+Wind 300 deg - Service Dead+Wind 50 deg - Service Dead+Wind 50 deg - Service Dead+Wind 150 deg - Service Dead+Wind 160 deg - Service		Dood Only	—
Dead+Wind 30 deg - No Ice Dead+Wind 90 deg - No Ice Dead+Wind 90 deg - No Ice Dead+Wind 120 deg - No Ice Dead+Wind 150 deg - No Ice Dead+Wind 150 deg - No Ice Dead+Wind 180 deg - No Ice Dead+Wind 210 deg - No Ice Dead+Wind 210 deg - No Ice Dead+Wind 270 deg - No Ice Dead+Wind 270 deg - No Ice Dead+Wind 300 deg - No Ice Dead+Wind 0 deg+Ice+Temp Dead+Wind 0 deg+Ice+Temp Dead+Wind 0 deg+Ice+Temp Dead+Wind 0 deg+Ice+Temp Dead+Wind 150 deg+Ice+Temp Dead+Wind 200 deg+Ice+Temp Dead+Wind 200 deg+Ice+Temp Dead+Wind 200 deg+Ice+Temp Dead+Wind 270 deg+Ice+Temp Dead+Wind 270 deg+Ice+Temp Dead+Wind 300 deg - Service Dead+Wind 30 deg - Service Dead+Wind 30 deg - Service Dead+Wind 150 deg - Service			
4 Dead+Wind 60 deg - No Ice 5 Dead+Wind 90 deg - No Ice 6 Dead+Wind 120 deg - No Ice 7 Dead+Wind 150 deg - No Ice 8 Dead+Wind 150 deg - No Ice 9 Dead+Wind 210 deg - No Ice 9 Dead+Wind 240 deg - No Ice 11 Dead+Wind 270 deg - No Ice 12 Dead+Wind 300 deg - No Ice 13 Dead+Wind 300 deg - No Ice 14 Dead+Wind 300 deg - No Ice 15 Dead+Wind 0 deg+Ice+Temp 16 Dead+Wind 0 deg+Ice+Temp 17 Dead+Wind 60 deg+Ice+Temp 18 Dead+Wind 90 deg+Ice+Temp 19 Dead+Wind 120 deg+Ice+Temp 20 Dead+Wind 150 deg+Ice+Temp 21 Dead+Wind 180 deg+Ice+Temp 22 Dead+Wind 210 deg+Ice+Temp 23 Dead+Wind 210 deg+Ice+Temp 24 Dead+Wind 270 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg+Ice+Temp 27 Dead+Wind 300 deg+Ice+Temp 28 Dead+Wind 300 deg+Ice+Temp 29 Dead+Wind 300 deg+Ice+Temp 20 Dead+Wind 300 deg+Ice+Temp 21 Dead+Wind 300 deg+Ice+Temp 22 Dead+Wind 300 deg+Ice+Temp 23 Dead+Wind 300 deg+Ice+Temp 24 Dead+Wind 300 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg - Service 27 Dead+Wind 300 deg - Service 38 Dead+Wind 300 deg - Service 39 Dead+Wind 150 deg - Service 30 Dead+Wind 150 deg - Service 31 Dead+Wind 150 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 150 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 210 deg - Service			
5 Dead+Wind 90 deg - No Ice 6 Dead+Wind 120 deg - No Ice 7 Dead+Wind 150 deg - No Ice 8 Dead+Wind 180 deg - No Ice 9 Dead+Wind 210 deg - No Ice 10 Dead+Wind 210 deg - No Ice 11 Dead+Wind 270 deg - No Ice 12 Dead+Wind 300 deg - No Ice 13 Dead+Wind 300 deg - No Ice 14 Dead+Ima 300 deg - No Ice 15 Dead+Wind 300 deg - No Ice 16 Dead+Wind 300 deg - No Ice 17 Dead+Wind 300 deg - No Ice 18 Dead+Wind 300 deg+Ice+Temp 19 Dead+Wind 30 deg+Ice+Temp 10 Dead+Wind 30 deg+Ice+Temp 11 Dead+Wind 30 deg+Ice+Temp 12 Dead+Wind 30 deg+Ice+Temp 13 Dead+Wind 120 deg+Ice+Temp 14 Dead+Wind 150 deg+Ice+Temp 15 Dead+Wind 150 deg+Ice+Temp 16 Dead+Wind 150 deg+Ice+Temp 17 Dead+Wind 150 deg+Ice+Temp 18 Dead+Wind 150 deg+Ice+Temp 19 Dead+Wind 150 deg+Ice+Temp 20 Dead+Wind 300 deg+Ice+Temp 21 Dead+Wind 210 deg+Ice+Temp 22 Dead+Wind 300 deg+Ice+Temp 23 Dead+Wind 300 deg+Ice+Temp 24 Dead+Wind 300 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg+Ice+Temp 27 Dead+Wind 300 deg - Service 28 Dead+Wind 30 deg - Service 29 Dead+Wind 60 deg - Service 30 Dead+Wind 150 deg - Service 31 Dead+Wind 150 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 150 deg - Service 34 Dead+Wind 150 deg - Service 35 Dead+Wind 210 deg - Service	_	•	
6 Dead+Wind 120 deg - No Ice 7 Dead+Wind 150 deg - No Ice 8 Dead+Wind 180 deg - No Ice 9 Dead+Wind 210 deg - No Ice 10 Dead+Wind 240 deg - No Ice 11 Dead+Wind 270 deg - No Ice 12 Dead+Wind 300 deg - No Ice 13 Dead+Wind 300 deg - No Ice 14 Dead+Wind 300 deg - No Ice 15 Dead+Wind 0 deg+Ice+Temp 16 Dead+Wind 0 deg+Ice+Temp 17 Dead+Wind 0 deg+Ice+Temp 18 Dead+Wind 90 deg+Ice+Temp 19 Dead+Wind 90 deg+Ice+Temp 20 Dead+Wind 120 deg+Ice+Temp 21 Dead+Wind 150 deg+Ice+Temp 22 Dead+Wind 180 deg+Ice+Temp 23 Dead+Wind 210 deg+Ice+Temp 24 Dead+Wind 270 deg+Ice+Temp 25 Dead+Wind 270 deg+Ice+Temp 26 Dead+Wind 300 deg+Ice+Temp 27 Dead+Wind 300 deg+Ice+Temp 28 Dead+Wind 300 deg+Ice+Temp 29 Dead+Wind 300 deg+Ice+Temp 20 Dead+Wind 300 deg+Ice+Temp 21 Dead+Wind 300 deg+Ice+Temp 22 Dead+Wind 300 deg+Ice+Temp 23 Dead+Wind 300 deg+Ice+Temp 24 Dead+Wind 300 deg-Ice+Temp 25 Dead+Wind 300 deg-Ice+Temp 26 Dead+Wind 30 deg - Service 27 Dead+Wind 100 deg - Service 28 Dead+Wind 100 deg - Service 30 Dead+Wind 100 deg - Service 31 Dead+Wind 100 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 150 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 210 deg - Service 36 Dead+Wind 210 deg - Service 37 Dead+Wind 210 deg - Service 38 Dead+Wind 210 deg - Service			
7 Dead+Wind 150 deg - No Ice 8 Dead+Wind 180 deg - No Ice 9 Dead+Wind 210 deg - No Ice 10 Dead+Wind 240 deg - No Ice 11 Dead+Wind 270 deg - No Ice 12 Dead+Wind 330 deg - No Ice 13 Dead+Wind 330 deg - No Ice 14 Dead+Ice+Temp 15 Dead+Wind 0 deg+Ice+Temp 16 Dead+Wind 30 deg+Ice+Temp 17 Dead+Wind 30 deg+Ice+Temp 18 Dead+Wind 90 deg+Ice+Temp 19 Dead+Wind 120 deg+Ice+Temp 20 Dead+Wind 150 deg+Ice+Temp 21 Dead+Wind 180 deg+Ice+Temp 22 Dead+Wind 210 deg+Ice+Temp 23 Dead+Wind 210 deg+Ice+Temp 24 Dead+Wind 270 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg+Ice+Temp 27 Dead+Wind 300 deg+Ice+Temp 28 Dead+Wind 300 deg+Ice+Temp 29 Dead+Wind 300 deg+Ice+Temp 20 Dead+Wind 300 deg+Ice+Temp 21 Dead+Wind 300 deg+Ice+Temp 22 Dead+Wind 300 deg+Ice+Temp 23 Dead+Wind 300 deg+Ice+Temp 24 Dead+Wind 300 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg - Service 27 Dead+Wind 300 deg - Service 38 Dead+Wind 30 deg - Service 39 Dead+Wind 10 deg - Service 30 Dead+Wind 10 deg - Service 31 Dead+Wind 10 deg - Service 32 Dead+Wind 110 deg - Service 33 Dead+Wind 210 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 210 deg - Service			
8 Dead+Wind 180 deg - No Ice 9 Dead+Wind 210 deg - No Ice 10 Dead+Wind 240 deg - No Ice 11 Dead+Wind 270 deg - No Ice 12 Dead+Wind 300 deg - No Ice 13 Dead+Wind 330 deg - No Ice 14 Dead+Ice+Temp 15 Dead+Wind 30 deg+Ice+Temp 16 Dead+Wind 30 deg+Ice+Temp 17 Dead+Wind 60 deg+Ice+Temp 18 Dead+Wind 60 deg+Ice+Temp 19 Dead+Wind 120 deg+Ice+Temp 20 Dead+Wind 150 deg+Ice+Temp 21 Dead+Wind 180 deg+Ice+Temp 22 Dead+Wind 180 deg+Ice+Temp 23 Dead+Wind 210 deg+Ice+Temp 24 Dead+Wind 270 deg+Ice+Temp 25 Dead+Wind 300 deg+Ice+Temp 26 Dead+Wind 300 deg+Ice+Temp 27 Dead+Wind 300 deg+Ice+Temp 28 Dead+Wind 300 deg+Ice+Temp 29 Dead+Wind 300 deg+Ice+Temp 20 Dead+Wind 300 deg+Ice+Temp 21 Dead+Wind 300 deg+Ice+Temp 22 Dead+Wind 300 deg+Ice+Temp 23 Dead+Wind 300 deg-Ice+Temp 24 Dead+Wind 300 deg-Ice+Temp 25 Dead+Wind 300 deg-Ice+Temp 26 Dead+Wind 30 deg - Service 27 Dead+Wind 60 deg - Service 28 Dead+Wind 60 deg - Service 60 Dead+Wind 160 deg - Service 61 Dead+Wind 160 deg - Service 62 Dead+Wind 160 deg - Service 63 Dead+Wind 180 deg - Service 64 Dead+Wind 180 deg - Service 65 Dead+Wind 180 deg - Service 66 Dead+Wind 210 deg - Service 67 Dead+Wind 180 deg - Service 68 Dead+Wind 180 deg - Service 68 Dead+Wind 210 deg - Service 79 Dead+Wind 180 deg - Service 80 Dead+Wind 210 deg - Service 81 Dead+Wind 210 deg - Service 82 Dead+Wind 210 deg - Service 83 Dead+Wind 210 deg - Service 84 Dead+Wind 210 deg - Service 85 Dead+Wind 210 deg - Service			
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Dead+Wind 180 deg+lce+Temp Dead+Wind 210 deg+lce+Temp Dead+Wind 240 deg+lce+Temp Dead+Wind 270 deg+lce+Temp Dead+Wind 300 deg+lce+Temp Dead+Wind 300 deg+lce+Temp Dead+Wind 330 deg+lce+Temp Dead+Wind 0 deg - Service Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service Dead+Wind 60 deg - Service Dead+Wind 90 deg - Service Dead+Wind 120 deg - Service Dead+Wind 120 deg - Service Dead+Wind 150 deg - Service Dead+Wind 150 deg - Service Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service Dead+Wind 210 deg - Service	19	Dead+Wind 120 deg+lce+Temp	
Dead+Wind 210 deg+lce+Temp Dead+Wind 240 deg+lce+Temp Dead+Wind 270 deg+lce+Temp Dead+Wind 300 deg+lce+Temp Dead+Wind 300 deg+lce+Temp Dead+Wind 0 deg - Service Dead+Wind 30 deg - Service Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service Dead+Wind 90 deg - Service Dead+Wind 120 deg - Service Dead+Wind 120 deg - Service Dead+Wind 120 deg - Service Dead+Wind 150 deg - Service Dead+Wind 150 deg - Service Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service Dead+Wind 240 deg - Service	20	Dead+Wind 150 deg+lce+Temp	
Dead+Wind 240 deg+lce+Temp Dead+Wind 270 deg+lce+Temp Dead+Wind 300 deg+lce+Temp Dead+Wind 300 deg+lce+Temp Dead+Wind 300 deg - Service Dead+Wind 0 deg - Service Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service Dead+Wind 90 deg - Service Dead+Wind 120 deg - Service Dead+Wind 120 deg - Service Dead+Wind 150 deg - Service Dead+Wind 150 deg - Service Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service Dead+Wind 240 deg - Service	21	Dead+Wind 180 deg+lce+Temp	
24 Dead+Wind 270 deg+lce+Temp 25 Dead+Wind 300 deg+lce+Temp 26 Dead+Wind 330 deg+lce+Temp 27 Dead+Wind 0 deg - Service 28 Dead+Wind 30 deg - Service 29 Dead+Wind 60 deg - Service 30 Dead+Wind 90 deg - Service 31 Dead+Wind 120 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 150 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service	22	Dead+Wind 210 deg+lce+Temp	
Dead+Wind 300 deg+lce+Temp Dead+Wind 330 deg+lce+Temp Dead+Wind 0 deg - Service Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service Dead+Wind 60 deg - Service Dead+Wind 90 deg - Service Dead+Wind 120 deg - Service Dead+Wind 150 deg - Service Dead+Wind 150 deg - Service Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service Dead+Wind 210 deg - Service	23	Dead+Wind 240 deg+lce+Temp	
Dead+Wind 330 deg+lce+Temp Dead+Wind 0 deg - Service Dead+Wind 30 deg - Service Dead+Wind 60 deg - Service Dead+Wind 90 deg - Service Dead+Wind 120 deg - Service Dead+Wind 150 deg - Service Dead+Wind 150 deg - Service Dead+Wind 180 deg - Service Dead+Wind 210 deg - Service Dead+Wind 210 deg - Service Dead+Wind 210 deg - Service	24	Dead+Wind 270 deg+lce+Temp	
27 Dead+Wind 0 deg - Service 28 Dead+Wind 30 deg - Service 29 Dead+Wind 60 deg - Service 30 Dead+Wind 90 deg - Service 31 Dead+Wind 120 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service	25	Dead+Wind 300 deg+lce+Temp	
28 Dead+Wind 30 deg - Service 29 Dead+Wind 60 deg - Service 30 Dead+Wind 90 deg - Service 31 Dead+Wind 120 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service	26	Dead+Wind 330 deg+lce+Temp	
29 Dead+Wind 60 deg - Service 30 Dead+Wind 90 deg - Service 31 Dead+Wind 120 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service		Dead+Wind 0 deg - Service	
30 Dead+Wind 90 deg - Service 31 Dead+Wind 120 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service	28	Dead+Wind 30 deg - Service	
31 Dead+Wind 120 deg - Service 32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service	-	Dead+Wind 60 deg - Service	
32 Dead+Wind 150 deg - Service 33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service			
33 Dead+Wind 180 deg - Service 34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service	-		
34 Dead+Wind 210 deg - Service 35 Dead+Wind 240 deg - Service			
35 Dead+Wind 240 deg - Service			
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	36	Dead+Wind 270 deg - Service	
37 Dead+Wind 300 deg - Service			
38 Dead+Wind 330 deg - Service	38	Dead+Wind 330 deg - Service	

# **Maximum Member Forces**

Sectio	Elevation	Component	Condition	Gov.	Force	Major Axis	Minor Axis
n	ft	Туре		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
L1	120 - 90	Pole	Max Tension	27	0.00	0.00	-0.00
			Max. Compression	14	-17.14	-0.71	0.75
			Max. Mx	11	-8.04	247.25	6.43
			Max. My	2	-8.04	5.83	247.18
			Max. Vy	11	-13.52	247.25	6.43
			Max. Vx	2	-13.54	5.83	247.18
			Max. Torque	12			-2.55
L2	90 - 80	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-19.03	-0.39	0.56
			Max. Mx	11	-9.33	385.48	8.96
			Max. My	2	-9.33	8.36	385.48
			Max. Vy	11	-14.12	385.48	8.96

Sectio	Elevation	Component	Condition	Gov.	Force	Major Axis	Minor Axis
n	ft	Type		Load		Moment	Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. Vx	2	-14.13	8.36	385.48
L3	80 - 68.5	Pole	Max. Torque Max Tension	12 1	0.00	0.00	-2.53 0.00
LJ	00 - 00.5	Fole	Max. Compression	14	-21.88	-0.03	0.00
			Max. Mx	11	-11.52	552.02	11.86
			Max. My	2	-11.52	11.27	552.10
			Max. Vy	11	-14.84	552.02	11.86
			Max. Vx	2	-14.85	11.27	552.10
			Max. Torque	12			-2.51
L4	68.5 - 68	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-22.04	-0.01	0.35
			Max. Mx Max. My	11 2	-11.65 -11.65	559.45 11.39	11.99 559.53
			Max. Vy	11	-11.03	559.45	11.99
			Max. Vx	2	-14.89	11.39	559.53
			Max. Torque	12			-2.47
L5	68 - 63.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-23.14	0.13	0.27
			Max. Mx	11	-12.51	627.21	13.12
			Max. My	2	-12.50	12.53	627.33
			Max. Vy	11	-15.24	627.21	13.12
			Max. Vx	2 12	-15.25	12.53	627.33
L6	63.5 - 60	Pole	Max. Torque Max Tension	1	0.00	0.00	-2.47 0.00
LO	03.3 - 00	Fole	Max. Compression	14	-24.43	0.24	0.00
			Max. Mx	11	-13.59	681.07	14.00
			Max. My	2	-13.59	13.41	681.21
			Max. Vy	11	-15.54	681.07	14.00
			Max. Vx	2	-15.55	13.41	681.21
			Max. Torque	12			-2.43
L7	60 - 56.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-25.88	0.38	0.49
			Max. Mx Max. My	11 2	-14.73 -14.73	736.79 14.30	14.97 737.05
			Max. Vy	11	-16.02	736.79	14.97
			Max. Vx	2	-16.04	14.30	737.05
			Max. Torque	12			-2.83
L8	56.5 -	Pole	Max Tension	1	0.00	0.00	0.00
	45.417						
			Max. Compression	14	-29.05	0.84	0.30
			Max. Mx	11	-17.14	920.00	17.76
			Max. My Max. Vy	2 11	-17.14 -17.08	17.12	920.34 17.76
			Max. Vx	2	-17.06	920.00 17.12	920.34
			Max. Torque	12	-17.05	17.12	-2.81
L9	45.417 -	Pole	Max Tension	1	0.00	0.00	0.00
	36.417						
			Max. Compression	14	-31.96	1.23	0.08
			Max. Mx	11	-19.47	1076.89	19.99
			Max. My	2	-19.47	19.39	1077.27
			Max. Vy	11	-17.78	1076.89	19.99
			Max. Vx Max. Torque	2 12	-17.79	19.39	1077.27 -2.75
L10	36.417 -	Pole	Max Tension	1	0.00	0.00	0.00
LIO	33.5	1 010	Wax Tollololl	•	0.00	0.00	0.00
	00.0		Max. Compression	14	-33.03	1.35	0.00
			Max. Mx	11	-20.36	1129.07	20.70
			Max. My	2	-20.36	20.13	1129.46
			Max. Vy	11	-18.00	1129.07	20.70
			Max. Vx	2	-18.01	20.13	1129.46
1.44	00.5.00	Data	Max. Torque	11	0.00	0.00	-2.69
L11	33.5 - 30	Pole	Max Tension Max. Compression	1 14	0.00 -34.70	0.00 1.50	0.00
			Max. Mx	14	-34.70 -21.82	1.50	-0.08 21.57
			Max. My	2	-21.82	21.01	1192.94
			Max. Vy	11	-18.27	1192.53	21.57
			Max. Vx	2	-18.28	21.01	1192.94
			Max. Torque	11			-2.68
L12	30 - 26.5	Pole	Max Tension	1	0.00	0.00	0.00

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Force	Major Axis Moment	Minor Axis Moment
No.				Comb.	K	kip-ft	kip-ft
			Max. Compression	14	-36.11	1.68	-0.18
			Max. Mx	11	-22.97	1256.97	22.42
			Max. My	2	-22.97	21.89	1257.39
			Max. Vy	11	-18.56	1256.97	22.42
			Max. Vx	2	-18.57	21.89	1257.39
			Max. Torque	11			-2.66
L13	26.5 - 20.583	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-38.13	1.97	-0.35
			Max. Mx	11	-24.58	1368.14	23.87
			Max. My	2	-24.58	23.39	1368.57
			Max. Vý	11	-19.02	1368.14	23.87
			Max. Vx	2	-19.03	23.39	1368.57
			Max. Torque	11			-2.64
L14	20.583 - 2	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-46.13	2.91	-0.89
			Max. Mx	11	-31.30	1734.93	28.36
			Max. My	2	-31.30	28.03	1735.41
			Max. Vy	11	-20.43	1734.93	28.36
			Max. Vx	2	-20.44	28.03	1735.41
			Max. Torque	11			-2.61
L15	2 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-47.05	3.01	-0.95
			Max. Mx	11	-32.09	1775.93	28.84
			Max. My	2	-32.09	28.52	1776.42
			Max. Vy	11	-20.57	1775.93	28.84
			Max. Vx	2	-20.59	28.52	1776.42
			Max. Torque	11			-2.51

# **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	120 - 90	20.900	38	1.5599	0.0098
L2	90 - 80	11.499	38	1.3277	0.0064
L3	80 - 68.5	8.896	38	1.1453	0.0046
L4	68.5 - 68	6.356	38	0.9530	0.0033
L5	68 - 63.5	6.256	38	0.9458	0.0032
L6	63.5 - 60	5.410	38	0.8487	0.0027
L7	60 - 56.5	4.805	38	0.8008	0.0025
L8	56.5 - 45.417	4.230	38	0.7691	0.0023
L9	45.417 - 36.417	2.650	38	0.5852	0.0016
L10	36.417 - 33.5	1.679	38	0.4413	0.0011
L11	33.5 - 30	1.423	38	0.3983	0.0009
L12	30 - 26.5	1.145	38	0.3590	0.0008
L13	26.5 - 20.583	0.895	38	0.3233	0.0007
L14	20.583 - 2	0.543	38	0.2433	0.0005
L15	2 - 0	0.005	38	0.0245	0.0000

# **Critical Deflections and Radius of Curvature - Service Wind**

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
118.00	APXVTM14-C-120 w/ Mount	38	20.238	1.5526	0.0096	20510
	Pipe					
116.00	A-ANT-18G-1-C	38	19.578	1.5450	0.0095	20510
114.00	Pipe Mount [PM 601-3]	38	18.919	1.5370	0.0093	17091
107.00	Platform Mount [LP 712-1]	38	16.641	1.5025	0.0087	7888
99.00	(2) P65-17-XLH-RR w/ Mount	38	14.131	1.4412	0.0078	4883
	Pipe					
59.00	DB536	38	4.638	0.7918	0.0025	4763
48.00	GPS	38	2.982	0.6327	0.0018	3477

# **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	120 - 90	53.407	13	3.9896	0.0249
L2	90 - 80	29.396	13	3.3946	0.0164
L3	80 - 68.5	22.745	13	2.9286	0.0117
L4	68.5 - 68	16.252	13	2.4371	0.0083
L5	68 - 63.5	15.998	13	2.4185	0.0082
L6	63.5 - 60	13.834	13	2.1704	0.0069
L7	60 - 56.5	12.288	13	2.0478	0.0063
L8	56.5 - 45.417	10.817	13	1.9669	0.0060
L9	45.417 - 36.417	6.778	13	1.4968	0.0040
L10	36.417 - 33.5	4.295	13	1.1286	0.0027
L11	33.5 - 30	3.639	13	1.0187	0.0024
L12	30 - 26.5	2.929	13	0.9183	0.0021
L13	26.5 - 20.583	2.289	13	0.8270	0.0018
L14	20.583 - 2	1.389	13	0.6223	0.0013
L15	2 - 0	0.013	13	0.0627	0.0001

# Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
118.00	APXVTM14-C-120 w/ Mount	13	51.717	3.9707	0.0245	8084
	Pipe					
116.00	A-ANT-18G-1-C	13	50.030	3.9513	0.0241	8084
114.00	Pipe Mount [PM 601-3]	13	48.347	3.9308	0.0237	6736
107.00	Platform Mount [LP 712-1]	13	42.529	3.8421	0.0222	3108
99.00	(2) P65-17-XLH-RR w/ Mount	13	36.119	3.6850	0.0199	1923
	Pipe					
59.00	DB536	13	11.861	2.0249	0.0062	1868
48.00	GPS	13	7.626	1.6181	0.0045	1362

# **Compression Checks**

## **Pole Design Data**

Section	Elevation	Size	L	$L_u$	KI/r	$F_a$	Α	Actual	Allow.	Ratio
No.								P	$P_a$	P
	ft		ft	ft		ksi	in <sup>2</sup>	K	K	$P_a$
L1	120 - 90 (1)	P24x1/4	30.00	0.00	0.0	23.696	18.6532	-8.03	442.00	0.018
L2	90 - 80 (2)	P24x3/8	10.00	0.00	0.0	25.200	27.8325	-9.33	701.38	0.013
L3	80 - 68.5 (3)	RPS 24" x 0.62517"	11.50	0.00	0.0	19.806	45.9089	-11.52	909.27	0.013
L4	68.5 - 68 (4)	RPS 24" x 0.88003"	0.50	0.00	0.0	19.782	63.9197	-11.65	1264.46	0.009
L5	68 - 63.5 (5)	RPS 24" x 0.61306"	4.50	0.00	0.0	22.686	45.0429	-12.50	1021.84	0.012
L6	63.5 - 60 (6)	RPS 24" x 1.13718"	3.50	0.00	0.0	20.358	81.6787	-13.59	1662.82	0.008
L7	60 - 56.5 (7)	RPS 30" x 0.90733"	3.50	0.00	0.0	21.126	82.9275	-14.72	1751.93	0.008
L8	56.5 - 45.417	RPS 30" x 0.55714"	11.08	0.00	0.0	22.788	51.5340	-17.13	1174.36	0.015
	(8)									
L9	45.417 -	RPS 30" x 0.70733"	9.00	0.00	0.0	22.770	65.0925	-19.47	1482.16	0.013
	36.417 (9)									
L10	36.417 - 33.5	RPS 30" x 0.86188"	2.92	0.00	0.0	22.782	78.8966	-20.36	1797.42	0.011
	(10)									
L11	33.5 - 30 (11)	RPS 30" x 1.23648"	3.50	0.00	0.0	21.120	111.732	-21.81	2359.79	0.009
							0			
L12	30 - 26.5 (12)	RPS 36" x 0.7835"	3.50	0.00	0.0	21.504	86.6832	-22.97	1864.04	0.012
L13	26.5 - 20.583	RPS 36" x 0.62423"	5.92	0.00	0.0	21.522	69.3746	-24.58	1493.08	0.016
	(13)									
L14	20.583 - 2 (14)	RPS 36" x 0.8638"	18.58	0.00	0.0	21.504	95.3494	-31.30	2050.39	0.015
L15	2 - 0 (15)	RPS 36" x 0.94668"	2.00	0.00	0.0	21.102	104.251	-32.09	2199.91	0.015
	` '						0			

Section	Elevation	Size	L	Lu	KI/r	Fa	Α	Actual	Allow.	Ratio
No.								P	$P_a$	P
	ft		ft	ft		ksi	in <sup>2</sup>	K	K	$\overline{P_a}$

		Pol	e Bend	ding C	)esigi	n Dat	a			
Section	Elevation	Size	Actual	Actual	Allow.	Ratio	Actual	Actual	Allow.	Ratio
No.			$M_{x}$	$f_{bx}$	$F_{bx}$	$f_{bx}$	$M_y$	$f_{by}$	$F_{by}$	$f_{by}$
	ft		kip-ft	ksi	ksi	F <sub>bx</sub>	kip-ft	ksi	ksi	F <sub>by</sub>
L1	120 - 90 (1)	P24x1/4	249.15	27.276	23.696	1.151	0.00	0.000	23.696	0.000
L2	90 - 80 (2)	P24x3/8	388.22	28.782	27.720	1.038	0.00	0.000	27.720	0.000
L3	80 - 68.5 (3)	RPS 24" x 0.62517"	555.83	25.509	21.787	1.171	0.00	0.000	21.787	0.000
L4	68.5 - 68 (4)	RPS 24" x 0.88003"	563.30	18.965	21.760	0.872	0.00	0.000	21.760	0.000
L5	68 - 63.5 (5)	RPS 24" x 0.61306"	631.47	29.508	24.955	1.182	0.00	0.000	24.955	0.000
L6	63.5 - 60 (6)	RPS 24" x 1.13718"	685.65	18.455	22.394	0.824	0.00	0.000	22.394	0.000
L7	60 - 56.5 (7)	RPS 30" x 0.90733"	741.78	15.204	23.239	0.654	0.00	0.000	23.239	0.000
L8	56.5 - 45.417	RPS 30" x 0.55714"	926.01	29.838	25.067	1.190	0.00	0.000	25.067	0.000
	(8)									
L9	45.417 -	RPS 30" x 0.70733"	1083.7	27.923	25.047	1.115	0.00	0.000	25.047	0.000
	36.417 (9)		0							
L10	36.417 - 33.5	RPS 30" x 0.86188"	1136.1	24.402	25.060	0.974	0.00	0.000	25.060	0.000
	(10)		3							
L11	33.5 - 30 (11)	RPS 30" x 1.23648"	1199.9	18.657	23.232	0.803	0.00	0.000	23.232	0.000
			1							
L12	30 - 26.5 (12)	RPS 36" x 0.7835"	1264.6	20.318	23.654	0.859	0.00	0.000	23.654	0.000
			5							
L13	26.5 - 20.583	RPS 36" x 0.62423"	1376.3	27.386	23.674	1.157	0.00	0.000	23.674	0.000
	(13)		3							
L14	20.583 - 2	RPS 36" x 0.8638"	1744.7	25.597	23.654	1.082	0.00	0.000	23.654	0.000
	(14)		3							
L15	2 - 0 (15)	RPS 36" x 0.94668"	1785.9	24.074	23.212	1.037	0.00	0.000	23.212	0.000
			1							

		Po	ole She	ear De	esign	Data				
Section	Elevation	Size	Actual	Actual	Allow.	Ratio	Actual	Actual	Allow.	Ratio
No.			V	$f_{\nu}$	$F_{v}$	$f_{_{V}}$	Τ	$f_{vt}$	$F_{vt}$	$f_{vt}$
	ft		K	ksi	ksi	$\overline{F_{v}}$	kip-ft	ksi	ksi	$F_{vt}$
L1	120 - 90 (1)	P24x1/4	13.61	1.459	16.800	0.087	2.54	0.139	11.901	0.012
L2	90 - 80 (2)	P24x3/8	14.20	1.021	16.800	0.061	2.51	0.093	16.800	0.006
L3	80 - 68.5 (3)	RPS 24" x 0.62517"	14.93	0.650	13.204	0.049	1.96	0.045	13.204	0.003
L4	68.5 - 68 (4)	RPS 24" x 0.88003"	14.98	0.469	13.188	0.036	1.96	0.033	13.188	0.003
L5	68 - 63.5 (5)	RPS 24" x 0.61306"	15.34	0.681	15.124	0.045	1.93	0.045	15.124	0.003
L6	63.5 - 60 (6)	RPS 24" x 1.13718"	15.63	0.383	13.572	0.028	1.90	0.026	13.572	0.002
L7	60 - 56.5 (7)	RPS 30" x 0.90733"	16.12	0.389	14.084	0.028	2.11	0.022	14.084	0.002
L8	56.5 - 45.417 (8)	RPS 30" x 0.55714"	17.17	0.666	15.192	0.044	2.02	0.033	15.192	0.002
L9	45.417 - 36.417 (9)	RPS 30" x 0.70733"	17.88	0.549	15.180	0.036	1.93	0.025	15.180	0.002
L10	36.417 - 33.5 (10)	RPS 30" x 0.86188"	18.09	0.459	15.188	0.030	1.91	0.021	15.188	0.001
L11	33.5 - 30 (11)	RPS 30" x 1.23648"	18.36	0.329	14.080	0.023	1.87	0.015	14.080	0.001
L12	30 - 26.5 (12)	RPS 36" x 0.7835"	18.65	0.430	14.336	0.030	1.84	0.015	14.336	0.001
L13	26.5 - 20.583 (13)	RPS 36" x 0.62423"	19.11	0.551	14.348	0.038	1.78	0.018	14.348	0.001
L14	20.583 - 2 (14)	RPS 36" x 0.8638"	20.52	0.430	14.336	0.030	1.57	0.012	14.336	0.001
L15	2 - 0 (15)	RPS 36" x 0.94668"	20.67	0.396	14.068	0.028	1.55	0.010	14.068	0.001

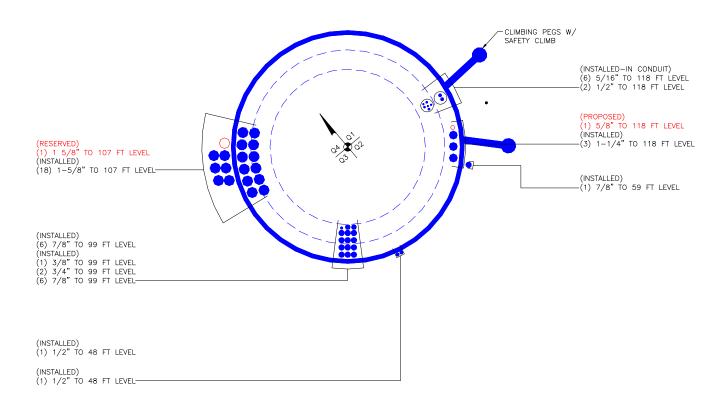
**Pole Interaction Design Data** 

Section No.	Elevation	Ratio P	Ratio f <sub>bx</sub>	Ratio f <sub>by</sub>	Ratio f <sub>v</sub>	Ratio f <sub>vt</sub>	Comb. Stress	Allow. Stress	Criteria
	ft	Pa	$F_{bx}$	$\overline{F_{by}}$	$\overline{F_{v}}$	$\overline{F_{vt}}$	Ratio	Ratio	
L1	120 - 90 (1)	0.018	1.151	0.000	0.087	0.012	1.179	1.333	H1-3+VT 🗸
L2	90 - 80 (2)	0.013	1.038	0.000	0.061	0.006	1.056	1.333	H1-3+VT 🗸
L3	80 - 68.5 (3)	0.013	1.171	0.000	0.049	0.003	1.186	1.333	H1-3+VT 🗸
L4	68.5 - 68 (4)	0.009	0.872	0.000	0.036	0.003	0.882	1.333	H1-3+VT 🗸
L5	68 - 63.5 (5)	0.012	1.182	0.000	0.045	0.003	1.197	1.333	H1-3+VT 🖊
L6	63.5 - 60 (6)	0.008	0.824	0.000	0.028	0.002	0.833	1.333	H1-3+VT 🖊
L7	60 - 56.5 (7)	0.008	0.654	0.000	0.028	0.002	0.663	1.333	H1-3+VT 🖊
L8	56.5 - 45.417 (8)	0.015	1.190	0.000	0.044	0.002	1.207	1.333	H1-3+VT 🗸
L9	45.417 - 36.417 (9)	0.013	1.115	0.000	0.036	0.002	1.129	1.333	H1-3+VT 🗸
L10	36.417 - 33.5 (10)	0.011	0.974	0.000	0.030	0.001	0.986	1.333	H1-3+VT 🗸
L11	33.5 - 30 (11)	0.009	0.803	0.000	0.023	0.001	0.813	1.333	H1-3+VT 🗸
L12	30 - 26.5 (12)	0.012	0.859	0.000	0.030	0.001	0.872	1.333	H1-3+VT 🗸
L13	26.5 - 20.583 (13)	0.016	1.157	0.000	0.038	0.001	1.175	1.333	H1-3+VT 🗸
L14	20.583 - 2 (14)	0.015	1.082	0.000	0.030	0.001	1.098	1.333	H1-3+VT 🗸
L15	2 - 0 (15)	0.015	1.037	0.000	0.028	0.001	1.053	1.333	H1-3+VT 🗸

# **Section Capacity Table**

Section	Elevation	Component	Size	Critical	Р	SF*P <sub>allow</sub>	%	Pass
No.	ft	Type		Element	K	K	Capacity	Fail
L1	120 - 90	Pole	P24x1/4	1	-8.03	589.19	88.4	Pass
L2	90 - 80	Pole	P24x3/8	2	-9.33	934.94	79.2	Pass
L3	80 - 68.5	Pole	RPS 24" x 0.62517"	3	-11.52	1212.06	89.0	Pass
L4	68.5 - 68	Pole	RPS 24" x 0.88003"	4	-11.65	1685.53	66.2	Pass
L5	68 - 63.5	Pole	RPS 24" x 0.61306"	5	-12.50	1362.11	89.8	Pass
L6	63.5 - 60	Pole	RPS 24" x 1.13718"	6	-13.59	2216.54	62.5	Pass
L7	60 - 56.5	Pole	RPS 30" x 0.90733"	7	-14.72	2335.32	49.8	Pass
L8	56.5 - 45.417	Pole	RPS 30" x 0.55714"	8	-17.13	1565.42	90.6	Pass
L9	45.417 - 36.417	Pole	RPS 30" x 0.70733"	9	-19.47	1975.72	84.7	Pass
L10	36.417 - 33.5	Pole	RPS 30" x 0.86188"	10	-20.36	2395.96	74.0	Pass
L11	33.5 - 30	Pole	RPS 30" x 1.23648"	11	-21.81	3145.60	61.0	Pass
L12	30 - 26.5	Pole	RPS 36" x 0.7835"	12	-22.97	2484.77	65.4	Pass
L13	26.5 - 20.583	Pole	RPS 36" x 0.62423"	13	-24.58	1990.28	88.1	Pass
L14	20.583 - 2	Pole	RPS 36" x 0.8638"	14	-31.30	2733.17	82.4	Pass
L15	2 - 0	Pole	RPS 36" x 0.94668"	15	-32.09	2932.48	79.0	Pass
							Summary	
						Pole (L8)	90.6	Pass
						RATING =	90.6	Pass

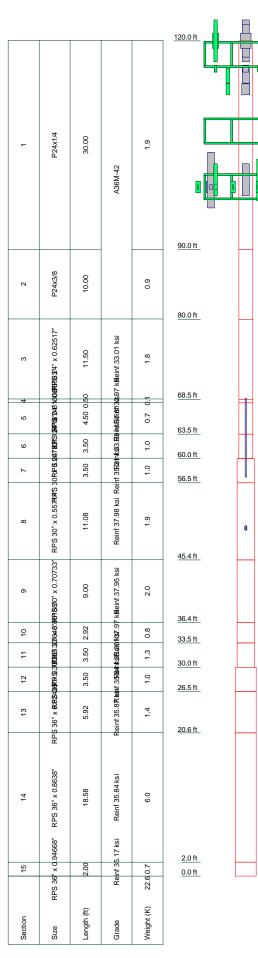
# APPENDIX B BASE LEVEL DRAWING



#### **APPENDIX C**

#### **ADDITIONAL CALCULATIONS**

Program Version 6.1.4.1 - 12/17/2013 File:G:/TOWER/375\_Crown\_Castle/2013/37513-2485 BU 876329/WO 764594 BU 876329 (Phase 7811)/37513-2485-R6.eri



#### **DESIGNED APPURTENANCE LOADING**

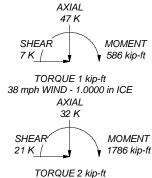
TYPE	ELEVATION	TYPE	ELEVATION
APXVTM14-C-120 w/ Mount Pipe	118	ERICSSON AIR 21 B4A B2P w/ Mount	107
APXVTM14-C-120 w/ Mount Pipe	118	Pipe	
APXVTM14-C-120 w/ Mount Pipe	118	KRY 112 144/1	107
TD-RRH8x20-25	118	KRY 112 144/1	107
TD-RRH8x20-25	118	KRY 112 144/1	107
TD-RRH8x20-25	118	Platform Mount [LP 712-1]	107
APXV9ERR18-C-A20 w/ Mount Pipe	118	ERICSSON AIR 21 B2A B4P w/ Mount	107
APXVSPP18-C-A20 w/ Mount Pipe	118	Pipe	
APXVSPP18-C-A20 w/ Mount Pipe	118	AM-X-CD-14-65-00T-RET w/ Mount Pipe	99
IBC1900HG-2A	118	AM-X-CD-14-65-00T-RET w/ Mount	99
IBC1900HG-2A	118	Pipe	99
IBC1900HG-2A	118	7770.00 w/ Mount Pipe	99
IBC1900BB-1	118	7770.00 w/ Mount Pipe	99
IBC1900BB-1	118	7770.00 w/ Mount Pipe	99
IBC1900BB-1	118	(2) RRUS-11	99
(2) HORIZON COMPACT	118	(2) RRUS-11	99
Platform Mount [LP 501-1]	118	(2) RRUS-11	99
A-ANT-18G-1-C	118	DTMABP7819VG12A	99
A-ANT-18G-2-C	118	DTMABP7819VG12A	99
PCS 1900MHz 4x45W-65MHz	114	DTMABP7819VG12A	99
PCS 1900MHz 4x45W-65MHz	114	(2) LGP13519	99
800MHz 2X50W RRH W/FILTER	114	(2) LGP13519	99
800MHz 2X50W RRH W/FILTER	114	(2) LGP13519	99
800MHz 2X50W RRH W/FILTER	114	DC6-48-60-18-8F	99
Pipe Mount [PM 601-3]	114	Platform Mount [LP 501-1]	99
PCS 1900MHz 4x45W-65MHz	114	(2) P65-17-XLH-RR w/ Mount Pipe	99
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	107	(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	99
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	107	DB536	59
ERICSSON AIR 21 B4A B2P w/ Mount	107	Side Arm Mount [SO 702-1]	59
Pipe	107	Pipe Mount [PM 601-1]	48
ERICSSON AIR 21 B4A B2P w/ Mount	107	GPS	48
Pipe		KS24019-L112A	48

#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A36M-42	42 ksi	60 ksi	Reinf 37.95 ksi	38 ksi	48 ksi
Reinf 33.01 ksi	33 ksi	42 ksi	Reinf 37.97 ksi	38 ksi	48 ksi
Reinf 32.97 ksi	33 ksi	42 ksi	Reinf 35.20 ksi	35 ksi	44 ksi
Reinf 37.81 ksi	38 ksi	48 ksi	Reinf 35.84 ksi	36 ksi	45 ksi
Reinf 33.93 ksi	34 ksi	43 ksi	Reinf 35.87 ksi	36 ksi	45 ksi
Reinf 35.21 ksi	35 ksi	45 ksi	Reinf 35.17 ksi	35 ksi	44 ksi
Reinf 37.98 ksi	38 ksi	48 ksi			

#### **TOWER DESIGN NOTES**

- Tower is located in Hartford County, Connecticut.
   Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
   Tower is also designed for a 38 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
   Deflections are based upon a 50 mph wind.
   TOWER RATING: 90.6%



REACTIONS - 80 mph WIND



Paul J Ford and Company 250 E. Broad Street Suite 600 Columbus, OH 43215 Phone: 614.221.6679 FAX: 614.448.4105

Job:	Existing 120 ft. I	Monopole / Mtn. Vie	w Cem. (Filley Park)
Proje	ect: PJF: 37513-2485-	R6 / BU876329	
Clien	t: Crown Castle USA	Drawn by: Morgan Scroggy	App'd:
	TIA/EIA-222-F	Date: 05/22/14	Scale: NTS
Path:		76329/WO 764594 BU 876329 (Phase 7811)/37513-2485-R6.eri	Dwg No. E-1

# Stiffened or Unstiffened, Exterior Flange Plate - Any Bolt Material TIA Rev F

#### Site Data

BU#: 876329

Site Name: App #:

Pole Manufacturer:	Other

Reactions		
Moment:	249.15	ft-kips
Axial:	8.02	kips
Shear:	13.61	kips
Elevation:	90	feet

В			
Qty:	20		
Diameter (in.):	1	Bolt Fu:	120
Bolt Material:	A325	Bolt Fy:	92
N/A:		< Disregard	Bolt Fty
N/A:		< Disregard	44.00
Circle (in.):	29		

11 140 30110	JIICIO,	Ontena.	
Flange	Bolt	Result	ts

Bolt Tension Capacity, **B**: 46.07 kips Max Bolt <u>directly</u> applied T: 20.22 Kips

 Min. PL "tc" for B cap. w/o Pry:
 2.018 in

 Min PL "treq" for actual T w/ Pry:
 1.020 in

 Min PL "t1" for actual T w/o Pry:
 1.337 in

T allowable with Prying: 35.75 kips 0≤α'≤1 case

AISC ASD <-Only Applicable to Unstiffened Cases

Rigid

Service, ASD

Fty\*ASIF

Rigid

Service ASD

0.75\*Fy\*ASIF

Comp. Y.L. Length:

16.28

 $\begin{array}{ccc} & \text{Prying Force, Q:} & 0.00 \text{ kips} \\ & \text{Total Bolt Tension=T+Q:} & 20.22 \text{ kips} \\ & \text{Prying Bolt Stress Ratio=(T+Q)/(B):} & 43.9\% \text{ Pass} \end{array}$ 

Plate Data									
Diam:	32	in							
Thick, t:	1.5	in							
Grade (Fy):	36	ksi							
Strength, Fu:	58	ksi							
Single-Rod B-eff:	3.77	in							

**Exterior Flange Plate Results** Flexural Check Compression Side Plate Stress: 21.5 ksi

Allowable Plate Stress: 36.0 ksi Compression Plate Stress Ratio: 59.6% Pass

No Prying

Tension Side Stress Ratio, (treq/t)^2: 46.3% Pass

Stiffener Data (Welding at Both Sides)								
Config:	0	*						
Weld Type:								
Groove Depth:		in **						
Groove Angle:		degrees						
<u>Fillet</u> H. Weld:		< Disregard						
<u>Fillet</u> V. Weld:		in						
Width:		in						
Height:		in						
Thick:		in						
Notch:		in						
Grade:		ksi						
Weld str.:		ksi						

rd	

Pole Data										
Diam:	24	in								
Thick:	0.25	in								
Grade:	42	ksi								
# of Sides:	0	"0" IF Round								
Fu	60	ksi								
Reinf. Fillet Weld	0	"0" if None								

Stress Increase Factor								
ASIF:	1.333							

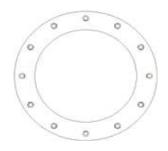
#### <u>n/a</u>

## Stiffener Results

Horizontal Weld: n/a
Vertical Weld: n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a
Plate Comp. (AISC Bracket): n/a

#### **Pole Results**

Pole Punching Shear Check: n/a





<sup>\* 0 =</sup> none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

<sup>\*\*</sup> Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes



v4.1 - Effective 7-3-12

Date: 5/22/2014

PJF Project: 37513-2485 R6 (60' Flange)

Client Ref. # 876329

Site Name: MTN. View Cem. (Filley Park)

Description: 60' Flange Owner: CCI Engineer: MLS

Asymmetric Bolt Analysis

685.65 Moment = k-ft TIA Ref. Axial = 13.59 kips ASIF = 1.3333 Shear = 15.63 kips Max Ratio = 100.0% 16 Anchor Qty

Location = Flange Plate η = N/A Threads = N/A

N/A for BP, Rev. G Sect. 4.9.9 for FP, Rev. G

\*\* For Flange Plates: Prying action is not considered in the bolt loads. \*\*

Item	Nominal Bolt Dia, in	Spec	Fy, ksi	Fu, ksi	Location, degrees	Bolt Circle,	Area Override, in <sup>2</sup>	Area, in <sup>2</sup>	Max Net Compressi on, kips	Max Net Tension, kips	Load for Capacity Calc, kips	Capacity Override, kips	Capacity, kips	Capacity Ratio
1	0.000				0.0	46.19	8.00	8.00	181.54	174.74	181.54	290.77	290.77	62.4%
2	0.000				90.0	46.19	8.00	8.00	181.54	174.74	181.54	290.77	290.77	62.4%
3	0.000				180.0	46.19	8.00	8.00	181.54	174.74	181.54	290.77	290.77	62.4%
4	0.000				270.0	46.19	8.00	8.00	181.54	174.74	181.54	290.77	290.77	62.4%
5	0.000	A325	0	0	0.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
6	0.000	A325	0	0	30.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
7	0.000	A325	0	0	60.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
8	0.000	A325	0	0	90.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
9	0.000	A325	0	0	120.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
10	0.000	A325	0	0	150.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
11	0.000	A325	0	0	180.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
12	0.000	A325	0	0	210.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
13	0.000	A325	0	0	240.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
14	0.000	A325	0	0	270.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
15	0.000	A325	0	0	300.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
16	0.000	A325	0	0	330.0	35.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
								32.00						



v4.1 - Effective 7-3-12

UL J. FURD AND COMPANY

Client Bef # 83

PJF Project: 37513-2485 R6 (30' Flange) Client Ref. # 876329

Date: 5/22/2014

Site Name: MTN. View Cem. (Filley Park)

Description: 30' Flange Owner: CCI Engineer: MLS

#### Asymmetric Bolt Analysis

Moment = 1199.91 k-ft TIA Ref. Axial = 21.81 ASIF = 1.3333 kips Shear = 18.36 kips Max Ratio = 100.0% 20 Anchor Qty

 $\begin{array}{ll} \text{Location} = & \hline{\text{Flange Plate}} \\ \eta = & N/A \\ \hline{\text{Threads}} = & N/A \\ \end{array}$ 

N/A for BP, Rev. G Sect. 4.9.9 N/A for FP, Rev. G

#### \*\* For Flange Plates: Prying action is not considered in the bolt loads. \*\*

	Item	Nominal Bolt Dia, in	Spec	Fy, ksi	Fu, ksi	Location, degrees	Bolt Circle,	Area Override, in <sup>2</sup>	Area, in <sup>2</sup>	Max Net Compressi on, kips	Max Net Tension, kips	Load for Capacity Calc, kips	Capacity Override, kips	Capacity,	Capacity Ratio
⊩	1	0.000	Opec	ı y, kəi	i u, koi	0.0	52.19	8.00	8.00	281.36	270.45	281.36	290.77	290.77	96.8%
	2	0.000				90.0	52.19	8.00	8.00	281.36	270.45	281.36	290.77	290.77	96.8%
lh	3	0.000				180.0	52.19	8.00	8.00	281.36	270.45	281.36	290.77	290.77	96.8%
	4	0.000				270.0	52.19	8.00	8.00	281.36	270.45	281.36	290.77	290.77	96.8%
	5	0.000	A325	0	0	0.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	6	0.000	A325	0	0	22.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
П	7	0.000	A325	0	0	45.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	8	0.000	A325	0	0	67.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	9	0.000	A325	0	0	90.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	10	0.000	A325	0	0	112.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	11	0.000	A325	0	0	135.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	12	0.000	A325	0	0	157.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	13	0.000	A325	0	0	180.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	14	0.000	A325	0	0	202.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	15	0.000	A325	0	0	225.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	16	0.000	A325	0	0	247.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
IĽ	17	0.000	A325	0	0	270.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	18	0.000	A325	0	0	292.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
ΙĹ	19	0.000	A325	0	0	315.0	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
	20	0.000	A325	0	0	337.5	41.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0%
									32.00						



v4.1 - Effective 7-3-12

Date: 5/22/2014 PJF Project: 37513-2485 R6

Client Ref. # 876329

Site Name: MTN. View CEM. (Filley Park)

for BP, Rev. G Sect. 4.9.9

Description: 120' Pole Owner: CCI Engineer: MLS

### Asymmetric Anchor Rod Analysis

1786 Moment = k-ft TIA Ref. Axial = 32.0 ASIF = 1.3333 kips Shear = 21.0 kips Max Ratio = 100.0% Anchor Qty 24

Base Plate Location = η= N/A Threads = N/A for FP, Rev. G

\*\* For Post Installed Anchors: Check anchors for embedment, epoxy/grout bond, and capacity based on proof load. \*\*

Item	Nominal Anchor Dia, in	Spec	Fy, ksi	Fu, ksi	Location, degrees	Anchor Circle, in	Area Override, in <sup>2</sup>	Area, in <sup>2</sup>	Max Net Compressi on, kips	Max Net Tension, kips	Load for Capacity Calc, kips	Capacity Override, kips	Capacity, kips	Capacity Ratio
1	1.500	A354 Gr BC	109	125	0.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
2	1.500	A354 Gr BC	109	125	22.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
3	1.500	A354 Gr BC	109	125	45.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
4	1.500	A354 Gr BC	109	125	67.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
5	1.500	A354 Gr BC	109	125	90.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
6	1.500	A354 Gr BC	109	125	112.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
7	1.500	A354 Gr BC	109	125	135.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
8	1.500	A354 Gr BC	109	125	157.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
9	1.500	A354 Gr BC	109	125	180.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
10	1.500	A354 Gr BC	109	125	202.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
11	1.500	A354 Gr BC	109	125	225.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
12	1.500	A354 Gr BC	109	125	247.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
13	1.500	A354 Gr BC	109	125	270.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
14	1.500	A354 Gr BC	109	125	292.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
15	1.500	A354 Gr BC	109	125	315.0	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
16	1.500	A354 Gr BC	109	125	337.5	41.00	0.00	1.77	78.11	75.40	75.40	0.00	97.19	77.6%
17	1.375	Williams R71	127.7	150	33.8	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
18	1.375	Williams R71	127.7	150	56.3	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
19	1.375	Williams R71	127.7	150	123.8	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
20	1.375	Williams R71	127.7	150	146.3	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
21	1.375	Williams R71	127.7	150	213.8	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
22	1.375	Williams R71	127.7	150	236.3	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
23	1.375	Williams R71	127.7	150	303.8	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
24	1.375	Williams R71	127.7	150	326.3	49.88	0.00	1.68	89.95	87.37	87.37	0.00	110.75	78.9%
								41.70						

#### Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

## TIA Rev F

Site Data

BU#: 876329

Site Name: MTN View CEM (Filley Parl

App #:

Qty:

Diam:

Rod Material:

Strength (Fu):

Yield (Fy):

**Bolt Circle:** 

Weld str.:

Pole Manufacturer: Other

**Anchor Rod Data** 

16

1.5

Other

125

109

41

in

ksi

ksi

in

Reactions			Moment adjusted to
Moment:	1049	ft-kips	account for post
Axial:	21.7	kips	installed anchor
Shear:	14.2	kips	rods.

	If No stiffeners, Criteria:
•	

AISC ASD <-Only Applicable to Unstiffened Cases

**Anchor Rod Results** 

Maximum Rod Tension: 75.4 Kips Allowable Tension: 97.2 Kips Anchor Rod Stress Ratio: 77.6% Pass

Stiffened Service, ASD Fty\*ASIF

Plate Data						
Diam:	47	in				
Thick:	2	in				
Grade:	36	ksi				
Single-Rod B-eff:	7.07	in				

Base Plate Results	Flexural Check
Base Plate Stress:	24.6 ksi
Allowable Plate Stress:	36.0 ksi
Base Plate Stress Ratio:	68.3% Pass

Stiffened
Service, ASD
0.75*Fy*ASIF
Y.L. Length:
N/A, Roark

Stiffener Data (Welding at both sides) Config:

Weld Type: Fillet Groove Depth: 0.5 <-- Disregard Groove Angle: 45 <-- Disregard Fillet H. Weld: 0.75 in Fillet V. Weld: 0.375 in 5.5 Width: in Height: 20.5 in Thick: 0.75 in 0.75 Notch: in Grade: 50 ksi

Stiffener Results	iffener Re	esults
-------------------	------------	--------

Horizontal Weld: 36.0% Pass Vertical Weld: 18.5% Pass Plate Flex+Shear, fb/Fb+(fv/Fv)^2: 5.3% Pass Plate Tension+Shear, ft/Ft+(fv/Fv)^2: 36.2% Pass Plate Comp. (AISC Bracket): 33.9% Pass

**Pole Results** 

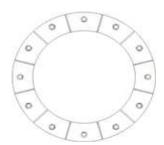
Pole Punching Shear Check: 6.6% Pass

Pole Data									
Diam:	36	in							
Thick:	0.375	in							
Grade:	42	ksi							
# of Sides:	0	"0" IF Round							
Fu	60	ksi							
Reinf. Fillet Weld	0	"0" if None							

70

ksi

Stress Increase Factor							
ASIF:	1.333						





<sup>\* 0 =</sup> none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

<sup>\*\*</sup> Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

Job Number: 37513-2485 R6 Site Number: 876329 Site Name:

MTN. View CEM. (Filley Park)

#### DRILLED PIER SOIL AND STEEL ANALYSIS - TIA/EIA-222-F

Unfactored Base Reactions from RISA

Comp. (+) Tension (-) Moment, M = 1786.0 Shear, V = 21.0 kips Axial Load, P = 32 0 kips

0.0 k-ft @ Ground OTM = 1796.5

Safety Factors / Load Factors / Φ Factors

Page:

Date:

Safety Factor

2.00

2.00

2.00

MI S

5/22/2014

Φ Factor

0.75

0.75

0.75

Bv:

Tower Type = Monopole DP ACI Code = CI 318-02 Seismic Design Category = Reference Standard = TA/EIA-222-F Use 1.3 Load Factor? Yes Load Factor = 1.30

**Drilled Pier Parameters** 

Diameter = 6 ft 0.5 ft Height Above Grade = Depth Below Grade = 25 ft fc' = ksi 0.003 in/in εc =

Mat Ftdn. Cap Width = Mat Ftdn. Cap Length = Depth Below Grade =

Load Combinations Checked per TIA/EIA-222-F

1. Ult. Skin Friction/2.00 + Ult. End Bearing/2.00 + Effective Soil Wt. - Buoyant Conc. Wt. ≥ Comp.

2. Ult. Skin Friction/2.00 + Buoyant Conc. Wt./1.25 ≥ Uplift

3. Ult. Skin Friction/1.50 + Buoyant Conc. Wt./1.50 ≥ Uplift

Steel Parameters

Number of Bars = Rebar Size = Rebar Fy = 60 ksi Rebar MOE = 29000 ksi Tie Size = Side Clear Cover to Ties =

Soil Parameters

Soil Lateral Resistance =

Concrete Wt. Resist Uplift =

Skin Friction =

End Bearing =

Water Table Depth = 15.00 f Depth to Ignore Soil = 3.00 Depth to Full Cohesion = 0 1 Full Cohesion Starts at? Ground

Above Full Cohesion Lateral Resistance = 4(Cohesion)(Dia)(H) Below Full Cohesion Lateral Resistance = 8(Cohesion)(Dia)(H)

**Direct Embed Pole Shaft Parameters** 

Dia @ Grade = Dia @ Depth Below Grade = Number of Sides = Thickness = Fv = ks Backfill Condition =

**Maximum Capacity Ratios** 

Maximum Soil Ratio = 100.0% Maximum Steel Ratio = 100.0%

#### **Define Soil Layers**

Note: Cohesion = Undrained Shear Strengh = Unconfined Compressive Strength / 2

	Thickness	Unit Weight	Cohesion	Friction Angle		Ultimate End Bearing	Comp. Ult. Skin Friction	Tension Ult. Skin Friction	Depth
Layer	ft	pcf	psf	degrees	Soil Type	psf	psf	psf	ft
1	15	135		36	Sand	8000			15
2	15	137.4		36	Sand	8000			30
3									
4									
5									
6									
7									
8									
9									
10									
11									
12									

Soil Results: Overturning

Depth to COR = 17.64 ft, from Grade Shear, V = Bending Moment, M = 2166.89 k-ft, from COR Resisting Shear, Va = 6378.30 k-ft, from COR Resisting Moment, Ma =

SHEAR RATIO = 34.0% **MOMENT RATIO =** 34.0% OK OK

Soil Results: Uplift

0.00 kips Allowable Uplift Cap., Ta = 72.40 kips **UPLIFT RATIO =** 0.0% OK Soil Results: Compression

Compression, C = 32.00 kips Allowable Comp. Cap., Ca = 101.05 kips **COMPRESSION RATIO =** 31.7% OK

Steel Results (ACI 318-02):

Minimum Steel Area = 13.57 sq in Actual Steel Area = 24.00 sq in

Allowable Min Axial, Pa = -996.92 kips, Where Ma = 0 k-ft Allowable Max Axial, Pa = 4704.45 kips, Where Ma = 0 k-ft Axial Load, P = Moment, M = Allowable Moment, Ma =

56.39 kips @ 5.25 ft Below Grade 1891.66 k-ft @ 5.25 ft Below Grade 2568.68 k-ft

21.00 kips

61.81 kips

MOMENT RATIO = 73.6% OK

#### Moment Capacity of Drilled Concrete Shaft (Caisson) for TIA Rev F or G

Note: Shaft assumed to have ties, not spiral, transverse reinforcing

#### Site Data

BU#: 876329

Site Name: MTN. View CEM. (Filley Park)

App #:

Enter Load Factors Below:							
For M (WL)	1.3	< Enter Factor					
For P (DL)	1.3	< Enter Factor					

Pier Properties						
Concrete:						
Pier Diameter =	ft					
Concrete Area =	4071.5	in <sup>2</sup>				
Reinforcement:		_				
Clear Cover to <b>Tie</b> =	3.00	in				
Horiz. <b>Tie</b> Bar Size=	5					
Vert. Cage Diameter =	5.30	ft				
Vert. Cage Diameter =	63.62	in				
Vertical Bar Size =	9					
Bar Diameter =	1.13	in				
Bar Area =	1	in <sup>2</sup>				
Number of Bars =	24					
As Total=	24	in <sup>2</sup>				
A s/ Aconc, Rho:	0.0059	0.59%				

ACI 10.5, ACI 21.10.4, and IBC 1810.

Min As for Flexural, Tension Controlled, Shafts:

(3)\*(Sqrt(f'c)/Fy: 0.0027

200 / Fy: 0.0033

#### Minimum Rho Check:

Actual Req'd Min. Rho:	0.33%	Flexural
Provided Rho:	0.59%	OK

Ref. Shaft Max Axial Capacities, φ Max(Pn or Tn):						
Max Pu = $(\phi = 0.65)$ Pn.						
Pn per ACI 318 (10-2)	6115.79	kips				
at Mu=(φ=0.65)Mn=	3187.43	ft-kips				
Max Tu, (φ=0.9) Tn =	1296	kips				
at Mu=φ=(0.90)Mn=	0.00	ft-kips				

Maximum Shaft Superimposed Forces					
TIA Revision:	F				
Max. Service Shaft M:	1891.66	ft-kips (* Note)			
Max. Service Shaft P:	56.39	kips			
Max Axial Force Type:					
(4) NI 4 NI OI (4 O :					

(\*) Note: Max Shaft Superimposed Moment does not necessarily equal to the shaft top reaction moment

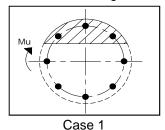
Load Factor	Shaft Factored Loads						
1.30	Mu: 2459.158 ft-kips						
1.30	Pu:	73.307	kips				

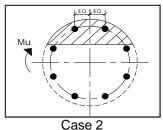
Material Properties					
Concrete Comp. strength, f'c =	3000	psi			
Reinforcement yield strength, Fy =	60	ksi			
Reinforcing Modulus of Elasticity, E =	29000	ksi			
Reinforcement yield strain =	0.00207	_			
Limiting compressive strain =	0.003				
ACI 318 Cod	е				
Select Analysis ACI Code=	2002				
Seismic Proper	ties	_			
Seismic Design Category =	D				
Seismic Risk =	High				

Solve	< Press Upon Completing All Input
(Run)	

#### Results:

#### Governing Orientation Case: 2





Dist. From Edge to Neutral Axis: 12.09 in Extreme Steel Strain, et: 0.0138

et > 0.0050, Tension Controlled

Reduction Factor, $\varphi$ : **0.900** 

Output Note: Negative Pu=Tension

For Axial Compression,  $\varphi$  Pn = Pu: 73.31 kips Drilled Shaft Moment Capacity,  $\varphi$ Mn: 3339.29 ft-kips Drilled Shaft Superimposed Mu: 2459.16 ft-kips

(Mu/φMn, Drilled Shaft Flexure CSR: 73.6%



# RADIO FREQUENCY FCC REGULATORY COMPLIANCE MAXIMUM PERMISSIBLE EXPOSURE (MPE) ASSESSMENT

**Sprint Existing Facility** 

Site ID: CT03XC076

Mtn. View Cem. (Filley Park)

28 Brewer Drive Bloomfield, CT 06002

June 10, 2014

EBI Project Number: 62143354

21 B Street Burlington, MA 01803 Tel: (781) 273.2500 Fax: (781) 273.3311



June 10, 2014

Sprint Attn: RF Engineering Manager 1 International Boulevard, Suite 800 Mahwah, NJ 07495

Re: Radio Frequency Maximum Permissible Exposure (MPE) Assessment for Site: CT03XC076 - Mtn. View Cem. (Filley Park)

Site Total: 77.37% - MPE% in full compliance

EBI Consulting was directed to analyze the proposed upgrades to the existing Sprint facility located at 28 Brewer Drive, Bloomfield, CT, for the purpose of determining whether the radio frequency (RF) exposure levels from the proposed Sprint equipment upgrades on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm2). The number of  $\mu$ W/cm2 calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general public may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general public would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm<sup>2</sup>). The general population exposure limit for the cellular band (850 MHz Band) is approximately 567  $\mu$ W/cm<sup>2</sup>, and the general population exposure limit for the 1900 MHz and 2500 MHz bands is 1000  $\mu$ W/cm<sup>2</sup>. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed upgrades to the existing Sprint Wireless antenna facility located at 28 Brewer Drive, Bloomfield, CT, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. All calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6 foot person standing at the base of the tower.

For all calculations, all emissions were calculated using the following assumptions:

- 1) 6 channels in the 1900 MHz Band were considered for each sector of the proposed installation.
- 2) 1 channel in the 800 MHz Band was considered for each sector of the proposed installation
- 3) 2 channels in the 2500 MHz Band were considered for each sector of the proposed installation.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 5) For the following calculations the sample point was the top of a six foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.



- 6) The antennas used in this modeling are the RFS APXVSPP18-C-A20, The RFS APXV9ERR18-C-A20 and the RFS APXVTM14-C-I20. This is based on feedback from the carrier with regards to anticipated antenna selection. The RFS APXVSPP18-C-A20 has a 15.9 dBd gain value at its main lobe at 1900 MHz and 13.4 dBd at its main lobe for 850 MHz. The RFS APXV9ERR18-C-A20 has a 14.9 dBd gain value at its main lobe at 1900 MHz and 11.9 dBd at its main lobe for 850 MHz. The RFS APXVTM14-C-I20 has a 15.9 dBd gain value at its main lobe at 2500 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antenna mounting height centerline for the proposed antennas is **120 feet** above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculation were done with respect to uncontrolled / general public threshold limits

	Site ID	CT03XC076 -	Mtn. View Cem	n. (Filley Park)												
	Site Addresss	28 Brewer D	rive, Bloomfiel	d, CT, 06002												
	Site Type		Monopole													
	Sector 1															
						D										
						Power Out Per			Antenna Gain							Power
Antenna							Number of	Composite	(10 db	Antenna	analysis		Cable Loss	Additional		Density
	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	(Watts)	Channels	Power	,	Height (ft)	height	Cable Size	(dB)	Loss (dB)	ERP	Percentage
1a	RFS	APXV9ERR18-C-A20	RRH	1900 MHz	CDMA / LTE	20	6	120	4.9	120	114	1/2 "	0.5	3	165.65	0.46%
1a	RFS	APXV9ERR18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	1.9	120	114	1/2 "	0.5	3	13.84	0.40%
1B	RFS	APXVTMM14-C-120	RRH	2500 MHz	CDMA / LTE	20	2	40	5.9	120	114	1/2 "	0.5	3	69.51	0.34%
					,							Sector to	otal Power D	ensity Value:	0.86%	
							Sector 2							·		
							Sector 2									
						Power										
						Out Per			Antenna Gain					A 1 100		Power
Antenna Number	A A 4-1	Antenna Model	Dadia Torra	Farance David	Tarkardan.			Composite	(10 db	Antenna	analysis	C-bl- C:	Cable Loss	Additional	ERP	Density
2a	Antenna Make RFS	APXVSPP18-C-A20	Radio Type RRH	Frequency Band 1900 MHz	Technology CDMA / LTE	(Watts)	Channels 6	Power 120	reduction) 5.9	Height (ft) 120	height 114	Cable Size	(dB) 0.5	Loss (dB)	208.54	Percentage 0.58%
2a 2a	RFS	APXVSPP18-C-A20 APXVSPP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	3.4	120	114	1/2 "	0.5	3	19.54	0.58%
2B	RFS	APXVTMM14-C-120	RRH	2500 MHz	CDMA / LTE	20	2	40	5.9	120	114	1/2 "	0.5	3	69.51	0.34%
20	NI 3	AFXVIIVIIVI14-C-120	KKIT	2300 1411 12	CDIVIA / LIL	20		40	3.3	120	114			ensity Value:	1.01%	0.3470
												Sector to	, ca. i owel b	c.isity value.	1.01/0	
							Sector 3									
						Power										
						Out Per			Antenna Gain							Power
Antenna							Number of	Composite	(10 db	Antenna	analysis		Cable Loss	Additional		Density
	Antenna Make	Antenna Model	Radio Type	Frequency Band	Technology	(Watts)	Channels	Power		Height (ft)	height	Cable Size	(dB)	Loss (dB)	ERP	Percentage
3a	RFS	APXVSPP18-C-A20	RRH	1900 MHz	CDMA / LTE	20	6	120	5.9	120	114	1/2 "	0.5	3	208.54	0.58%
3a	RFS	APXVSPP18-C-A20	RRH	850 MHz	CDMA / LTE	20	1	20	3.4	120	114	1/2 "	0.5	3	19.54	0.10%
3B	RFS	APXVTMM14-C-120	RRH	2500 MHz	CDMA / LTE	20	2	40	5.9	120	114	1/2 "	0.5	3	69.51	0.34%
												Sector to	otal Power D	ensity Value:	1.01%	

Site Composite MPE %						
Carrier MPE %						
Sprint	2.89%					
T-Mobile	0.30%					
Verizon Wireless	28.70%					
AT&T	40.74%					
Clearwire	1.33%					
Town of Bloomfield	3.41%					
Total Site MPE %	77.37%					



### **Summary**

All calculations performed for this analysis yielded results that were well within the allowable limits for general public Maximum Permissible Exposure (MPE) to radio frequency energy.

The anticipated Maximum Composite contributions from the Sprint facility are 2.89% (0.86% from sector 1, 1.01% from sector 2 and 1.01% from sector 3) of the allowable FCC established general public limit considering all three sectors simultaneously sampled at the ground level.

The anticipated composite MPE value for this site assuming all carriers present is 77.37% of the allowable FCC established general public limit sampled at 6 feet above ground level. This total composite site value is based upon MPE values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan

RF Engineering Director

**EBI Consulting** 

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