



Daniel F. Caruso
Chairman

STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

Internet: ct.gov/csc

October 28, 2010

Thomas J. Regan, Esq.
Brown Rudnick LLP
CityPlace I, 185 Asylum Street
Hartford, CT 06103

RE: **EM-T-MOBILE-009-101007** - Omnipoint Communications, as subsidiary of T-Mobile USA, Inc., notice of intent to modify an existing telecommunications facility located at 8 Chimney Drive, Bethel, Connecticut.

Dear Attorney Regan:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

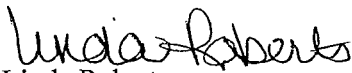
- That the tower's foundation be modified in accordance with the structural analysis prepared by Centek Engineering dated June 15, 2010 and stamped by Carlo Centore; and
- The tower and foundation shall not exceed 100 percent of the post-construction structural rating; and
- A signed letter from a Professional Engineer duly licensed in the State of Connecticut shall be submitted to the Council to certify that the necessary modifications have been properly completed and the tower and foundation do not exceed 100 percent of the post-construction structural rating.
- Any deviation from the proposed modification as specified in this notice and supporting materials with Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Not less than 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration;

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated October 7, 2010. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase

the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,



Linda Roberts
Executive Director

LR/CDM/laf

c: The Honorable Matthew S. Knickerbocker, First Selectman, Town of Bethel
Steve Palmer, Planning & Zoning Official, Town of Bethel
Daniel J. Garstka, Senior Engineer, Transmission Projects, Northeast Utilities Service Company



STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

Internet: ct.gov/csc

Daniel F. Caruso
Chairman

October 14, 2010

The Honorable Matthew S. Knickerbocker
First Selectman
Town of Bethel
1 School Street
Bethel Municipal Center
Bethel, CT 06801-2105

RE: **EM-T-MOBILE-009-101007** - Omnipoint Communications, as subsidiary of T-Mobile USA, Inc., notice of intent to modify an existing telecommunications facility located at 8 Chimney Drive, Bethel, Connecticut

Dear First Selectman Knickerbocker:

The Connecticut Siting Council (Council) received this request to modify an existing telecommunications facility, pursuant to Regulations of Connecticut State Agencies Section 16-50j-72.

If you have any questions or comments regarding this proposal, please call me or inform the Council by October 28, 2010.

Thank you for your cooperation and consideration.

Very truly yours,

Linda Roberts
Executive Director

LR/jbw

Enclosure: Notice of Intent

c: Steve Palmer, Planning & Zoning Official, Town of Bethel

THOMAS J. REGAN
Direct Dial: (860) 509-6522
tregan@brownrudnick.com

CityPlace I
185 Asylum
Street
Hartford
Connecticut
06103
tel 860.509.6500
fax 860.509.6501

ORIGINAL

Via Hand Delivery

October 7, 2010

RECEIVED
OCT - 7 2010

Daniel F. Caruso, Chairman
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

CONNECTICUT
SITING COUNCIL

RE: Notice of Exempt Modification / 8 Chimney Drive, Bethel

Dear Mr. Caruso:

On behalf of T-Mobile USA, Inc. ("T-Mobile"), enclosed for filing are an original and 5 copies of T-Mobile's Notice of Exempt Modification for the Facility located at 8 Chimney Drive in Bethel, Connecticut.

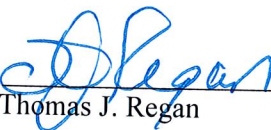
I also enclose herewith a check in the amount of \$625.00 representing the filing fee.

I would appreciate it if you would date-stamp the enclosed copy of this transmittal letter and return it to the courier delivering this package.

If you have any questions, please feel free to contact me.

Very truly yours,

BROWN RUDNICK LLP

By: 
Thomas J. Regan

Enclosures

cc w/ encl. via 1st Class Mail – First Selectman Matt Knickerbocker

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CONNECTICUT SITING COUNCIL

In re:

T-Mobile USA, Inc. Notice to Make an Exempt : **EXEMPT MODIFICATION NO.** _____
Modification to an Existing Facility at :
8 Chimney Drive, Bethel, Connecticut. : October 7, 2010

NOTICE OF EXEMPT MODIFICATION

Pursuant to Conn. Agencies Regs. §§ 16-50j-73 and 16-50j-72(b), T-Mobile USA, Inc. ("T-Mobile") hereby gives notice to the Connecticut Siting Council ("Council") and the Town of Bethel of T-Mobile's intent to make an exempt modification to the existing Connecticut Light & Power ("CL&P") transmission tower (the "Tower") located at 8 Chimney Drive in Bethel, Connecticut. Specifically, T-Mobile plans to upgrade its wireless system in Connecticut by implementing its Universal Mobile Telecommunications System ("UMTS"). UMTS is a third-generation ("3G") technology that utilizes a code division multiple access ("CDMA") base to allow for fast and large data transfers. To accomplish this upgrade, T-Mobile must modify its antenna and equipment configurations at many of its existing sites.

Once the UMTS upgrade is complete, T-Mobile will operate on a more unified communication system, allowing international wireless telephones to function world-wide. Furthermore, UMTS will enhance global positioning system ("GPS") navigation capabilities and provide emergency responders with more advanced tracking capabilities. The proposed UMTS technology is compatible with the existing second-generation ("2G") Global System for Mobile Communication ("GSM") currently on the Tower and the proposed upgrade is expected to enhance the existing 2G system. In order to accomplish the upgrade at this site, T-Mobile plans to add UMTS technology and install associated equipment at the base of the Tower.

Under the Council's regulations (Conn. Agencies Regs. § 16-50j-72(b)), T-Mobile's plans do not constitute a modification subject to the Council's review because T-Mobile will not

change the height of the Tower, will not extend the boundaries of the site, will not increase the noise levels at the site, and will not increase the total radio frequency electromagnetic radiation power density at the site to levels above applicable standards.

The Tower is a 145-foot CL&P pole (#10256), with an existing pipe mast extension located at 8 chimney Drive in Bethel, Connecticut (latitude N 41° 24' 38.9", longitude W -73° 24' 00.6"). The Tower is owned by CL&P. Currently, T-Mobile has 6 dual pole GSM panel antennas with centerlines at 162' and 154' on the Tower and 6 single Ground Mounted Antennas ("GMA") located in the equipment area. A site plan with Tower specifications is attached.

Specifically, T-Mobile plans to replace the existing 35-foot, 4-inch pipe mast with a 29-foot, 5-inch (approximately) mast in order to support its proposed installation. T-Mobile plans to remove the existing antennas and GMAs. T-Mobile will then install 3 new quad pole GSM panel antennas with a centerline at 162' and 3 new quad pole UMTS panel antennas with a centerline at 154'. In addition to the antennas, T-Mobile will also install 3 new twin PCS GMAs on the proposed equipment cabinet and run 6 new 1-5/8" coaxial cables which will be banded to the outside of the Tower with the existing cables. The 12 existing 1-5/8" coaxial cable will remain on the Tower.

To confirm the Tower can support these changes, T-Mobile commissioned Centek Engineering to perform a structural analysis of the Tower (attached). According to the structural assessment, dated June 15, 2010, "...the subject utility pole with the proposed foundation modifications **is adequate** to support the proposed T-Mobile equipment upgrade" (Section, 1-5, Structural Analysis, emphasis in original).

In addition, T-Mobile proposes to install 1 new UMTS equipment cabinet that will replace the existing equipment cabinet on the existing 6' x 12' concrete pad. T-Mobile will also install a new S12000 cabinet to replace the existing S8000 cabinet. Since T-Mobile is locating

its new equipment on the existing concrete pad adjacent to the Tower, no increase in the boundaries of the site will be necessary.

Excluding brief, minor, construction-related noise during the addition of the antennas and the installation of the equipment cabinet, T-Mobile's changes to the Tower will not increase noise levels at the site.

The proposed antennas will not adversely impact the health and safety of the surrounding community or the people working on the Tower. The total radio frequency exposure measured around the Tower will be well below the National Council on Radiation Protection and Measurements' ("NCRP") standard adopted by the Federal Communications Commission ("FCC"). The worst-case power density analysis measured at the base of the Tower indicates that T-Mobile's antennas will emit 4.18% of the NCRP's standard for maximum permissible exposure. Therefore, the power density levels will be below the FCC mandated radio frequency exposure limits in all locations around the Tower, even with extremely conservative assumptions. The power density analysis is attached.

In conclusion, T-Mobile's proposed plan to add antennas and equipment at this site does not constitute a modification subject to the Council's jurisdiction because T-Mobile will not increase the height of the Tower, will not extend the boundaries of the site, will not increase the noise levels at the site, and the total radio frequency electromagnetic radiation power density will stay within all applicable standards. *See* Conn. Agencies Regs. § 16-50j-72.

T-Mobile USA, Inc.


By: 

Thomas J. Regan
Brown Rudnick LLP
185 Asylum Street, CityPlace I
Hartford, CT 06103-3402
Email - tregan@brownrudnick.com
Phone - 860.509.6522 /Fax - 860.509.6501

Certificate of Service

This is to certify that on this 7th day of October, 2010, the foregoing Notice of Exempt Modification was sent, via first class mail, to the following:

Town of Bethel
First Selectman Matt Knickerbocker
Clifford J. Hurgin Municipal Center
Bethel, CT 06801

By:  _____
Thomas J. Regan

40277173 v1 - MERCIECM - 025064/0016

TRANSCEND WIRELESS, LLC
 1400 INDUSTRIAL AVE
 OFFICE: (617) 486-9999
 FACSIMILE: (617) 486-9999

T-MOBILE NORTHEAST, LLC
 31 GARDEN ROAD SUITE 200
 OFFICE: (617) 486-9999
 FACSIMILE: (617) 486-9999

ATLANTIS GROUP
 1340 Centre Street Suite 200
 North Haven, CT 06460
 OFFICE: (817) 486-9999
 FAX: (817) 486-9999

AGE SEAL

APPROVALS

LANDLORD
 LEASING

R.F.

ZONING

CONSTRUCTION

A/E

DRAWN BY:

SB

CHECKED BY:

SM

SUBMITTALS

A 10/24/10 ISSUED FOR REVIEW

DESIGN FOR CONSTRUCTION AND CONSTRUCTION WORK OF T-MOBILE COMMUNICATIONS, WITHOUT EXPRESS WRITTEN CONSENT AND USE BY CONTRACTOR, APPLICATION FOR THE PURPOSES OF CONSTRUCTION REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

CT1110C
BETHEL UTILITY
POLE # 10256
 8 CHIMNEY DRIVE
 BETHEL, CT 06801

SHEET TITLE:

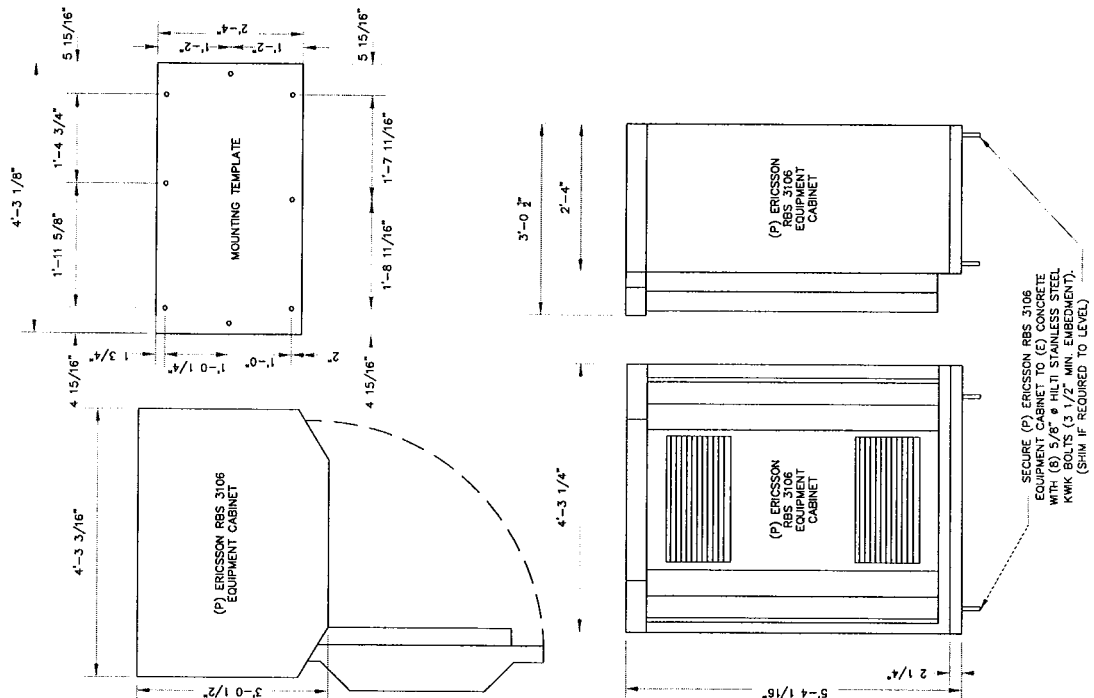
EQUIPMENT DETAILS

SHEET NUMBER:

04

STRUCTURAL NOTES

- DESIGN REQUIREMENTS ARE PER STATE BUILDING CODE AND APPLICABLE SUPPLEMENTS, ANS/MSC/E7, EAT/A-224-F STRUCTURAL STANDARDS FOR STEEL ANTENNA SUPPORTING STRUCTURES.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE CONSTRUCTION MANAGER.
- DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.
- STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A572 GRADE 50 STRUCTURAL STEEL UNLESS OTHERWISE INDICATED.
- STEEL PIPE SHALL CONFORM TO ASTM A500 COLD-FORMED WELDED & SEAMLESS CARBON STEEL. ALL STEEL SHALL BE BLACK AND HOT-DIPPED ZINC-COATED MEL AND EQUALS TYPE E OR S, GRADE 8. PIPE SIZES INDICATED ARE NOMINAL. ACTUAL OUTSIDE DIAMETER IS LARGER.
- STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLTS (BEARING TYPE) AND CONFORM TO ASTM A325 "HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS". ALL BOLTS SHALL 5/8" DIA UNLESS OTHERWISE NOTED.
- ALL STEEL MATERIAL SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS" UNLESS OTHERWISE NOTED.
- ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC COATING ON IRON AND STEEL HARDWARE" UNLESS OTHERWISE NOTED.
- FIELD WELDS, DRILL HOLES, SAW CUTS AND ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED. REPAIRS SHALL CONFORM WITH THE REQUIREMENTS OF ASTM 1293. GALVANIZING REPAIR PAINT SHALL HAVE 85 PERCENT ZINC BY WEIGHT. ZRP BY DUCAN GALVANIZING, GALVA BRIGHT PREMIUM BY CROWN OR EQUAL. THICKNESS OF APPLIED GALVANIZING REPAIR PAINT SHALL BE NOT LESS THAN 4 COATS (ALLOWED TIME TO DRY BETWEEN COATS) WITH A RESULTING COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.
- CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS AND METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS D1.1. ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AWS AND ALL WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PERTAINABLE IN THE AWS "MANUAL OF STEEL CONSTRUCTION" 9TH EDITION.
- NO WELDING, DRILLING, TAPPING OR CUTTING INTO CLIP OR WMECS STRUCTURES.
- INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE UNSUITABLE MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE CONSTRUCTION MANAGER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE CONSTRUCTION MANAGER APPROVAL.
- UNISTRUTS SHALL BE FORMED STEEL CHANNEL. STRUT FRAMING AS MANUFACTURED BY UNISTRUT CORP, WAYNE, MI OR EQUAL. UNISTRUT MEMBERS SHALL BE 1.56% 59%T300A UNLESS OTHERWISE SPECIFIED, AND SHALL BE HOT-RIIP GALVANIZED AFTER FABRICATION.
- EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF 1/2" DIAMETER EPOXY AND WIRE MESH. WIRE MESH SHALL BE 1/2" X 1/2" INTERNALLY THREADED WIRE. EPOXY SHALL BE ANCHORING ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HLT THAT MEETS OR EXCEEDS THE REQUIREMENTS OF THE HLT. HLT SHALL BE HY-20 AND HY-150 SYSTEMS (AS SPECIFIED ON DWG.) OR ENGINEERS APPROVED EQUAL WITH 4-1/4" MIN. EMBEDMENT DEPTH.
- EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION F5-328, GROUP II, TYPE 4, CLASS 1, HLT1 KWIK BOLTS II OR APPROVED EQUAL. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE THREE AND ONE HALF (3 1/2) INCHES.



ERICSSON RBS 3106 EQUIPMENT CABINET
 SCALE: 1/2" = 1'-0" (11x17)
 SCALE: 1" = 1'-0" (24x36)

TRANSCEND WIRELESS, LLC
 1500 CENTRE STREET
 SUITE 200
 WILMINGTON, MA 01890
 PHONE: 617-465-7779
 FAX: 617-465-7779

T-MOBILE NORTHEAST, LLC
 1500 CENTRE STREET
 SUITE 200
 WILMINGTON, MA 01890
 PHONE: 617-465-7779
 FAX: 617-465-7779

ATLANTIS GROUP
 1500 CENTRE STREET
 SUITE 200
 WILMINGTON, MA 01890
 PHONE: 617-465-7779
 FAX: 617-465-7779

APPROVALS

LANDLORD _____
 LEASING _____
 R.F. _____
 ZONING _____
 CONSTRUCTION _____
 A/E _____

DRAWN BY: SB
 CHECKED BY: SM

SUBMITTALS

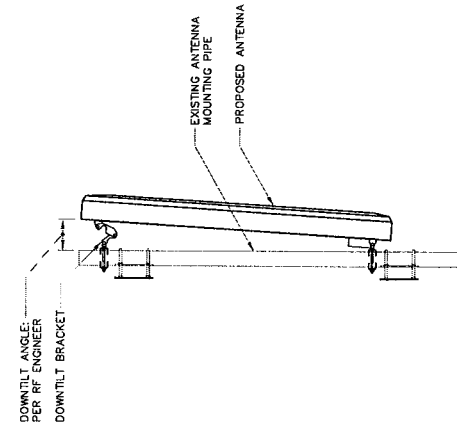
A 05/24/10 ISSUED FOR REVIEW

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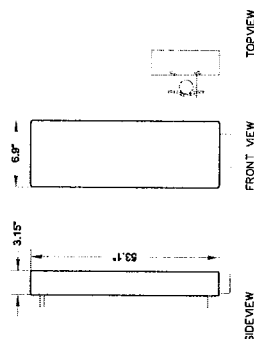
CT11100C
BETHEL UTILITY
POLE # 10256
 8 CHIMNEY DRIVE
 BETHEL, CT 06801

SHEET TITLE:
ANTENNA DETAILS

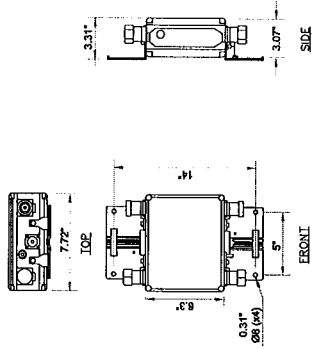
SHEET NUMBER:
05



MOUNTING ASSEMBLY (ALL ANTENNAS)
 N.T.S.



DUAL POLE ANTENNA DETAIL
 N.T.S.



TWIN GMA DETAIL
 N.T.S.

TRANSCEND WIRELESS, LLC
 1000 WASHINGTON STREET
 SUITE 200
 WESTPORT, MA 01886
 PHONE: (978) 335-1100
 FAX: (978) 335-1101

T-MOBILE NORTHEAST, LLC
 1000 WASHINGTON STREET
 SUITE 200
 WESTPORT, MA 01886
 PHONE: (978) 335-1100
 FAX: (978) 335-1101

ATLANTIS GROUP
 1940 Centre Street Suite 203
 Westport, MA 01886
 Phone: 617-462-0789
 Fax: 617-462-0022

DATE SEAL

APPROVALS
 LANDLORD _____
 LEASING _____
 R.F. _____
 ZONING _____
 CONSTRUCTION _____
 A/E _____

DRAWN BY: SB
 CHECKED BY: SM

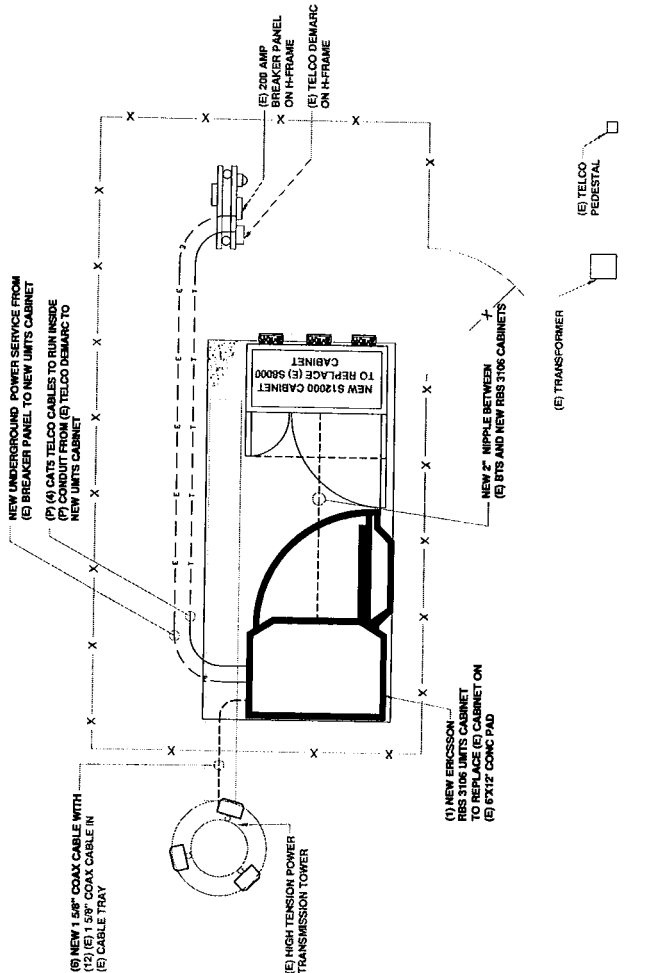
SUBMITTALS
 A. 05/24/10 ISSUED FOR REVIEW

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CT1110C
 BETHEL UTILITY
 POLE # 10256
 8 CHIMNEY DRIVE
 BETHEL, CT 06801

SHEET TITLE:
UTILITY ROUTING PLAN

SHEET NUMBER:
06



ELECTRICAL AND TELCO PLAN 1
 SCALE: 1/4" = 1'-0" (11x17)
 SCALE: 1/2" = 1'-0" (24x36)
 2 0 2 4 6

TRANSCEND WIRELESS, LLC
 140 INDUSTRIAL AVE
 SUITE 200
 OFFICE: (313) 638-0888
 FACILITY: (313) 638-0888

OR

T-MOBILE NORTHEAST, LLC
 11 GARDEN ROAD SUITE 200
 OFFICE: (603) 879-7399
 FACILITY: (603) 879-7399



ARE SEALS

APPROVALS

LANDLORD _____
 LEASING _____

R.F. _____
 ZONING _____

CONSTRUCTION _____
 A/E _____

DRAWN BY: _____ SB

CHECKED BY: _____ SM

SUBMITTALS

A	10/21/10	ISSUED FOR REVIEW

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CT1110C
BETHEL UTILITY
POLE # 10256
 8 CHIMNEY DRIVE
 BETHEL, CT 06801

GROUNDING NOTES
 SHEET TITLE:

SHEET NUMBER:
09

NORTHEAST UTILITIES - TOWER GROUNDING SYSTEM NOTES:

GENERAL:

1. T-MOBILE, NORTHEAST, LLC WILL FURNISH THE WIRE CONNECTORS, AND MISCELLANEOUS MATERIAL ASSOCIATED WITH THE COUNTERPOISE GROUNDING SYSTEM.
2. THE CONTRACTOR SHALL FURNISH ALL LABOR, MATERIALS AND EQUIPMENT NECESSARY TO INSTALL AND MAINTAIN THE TOWER GROUNDING SYSTEM. THE CONTRACTOR SHALL BE RESPONSIBLE TO REHABILITATE THE RIGHT-OF-WAY AS CLOSE AS POSSIBLE TO ITS ORIGINAL CONDITION.
3. THE CONTRACTOR SHALL HANDLE AND TRANSPORT THE OWNER SUPPLIED MATERIAL FROM THE TOWER SITES AND RETURN EXCESS MATERIAL TO THE OWNER'S OFFICE UPON COMPLETION OF THE CONTRACT.
4. NORTHEAST UTILITIES WILL BE RESPONSIBLE FOR PERFORMING TEST FOR SURGE IMPEDANCE AND WAVE IMPEDANCE.
5. AT STEEL POLE STRUCTURES, AN IF LENGTH OF PLASTIC MOLTING SHALL BE STAPLED OVER THE BOTTOM WITH 8" OF DOWNLAP.

- GROUND RODS:
1. WHERE GROUND RODS ARE REQUIRED, THEY SHALL BE SINGLE OR SECTIONAL WITH THE LENGTH OF EACH ROD TO BE 6 FEET MINIMUM. THE DEPTH OF EACH ROD SHALL BE 12" BELOW GRADE. ALL RODS SHALL BE CONNECTED TO COUNTERPOISE OR TO POLE GROUNDS USING BOLTED CONNECTORS.

- REHABILITATION:
1. SELECTIVE CLEARING PROCEDURES WERE USED IN THE DEVELOPMENT OF THE RIGHT-OF-WAY, AND GROWTH OF SELECTED SPECIES HAS BEEN SAVED. THE CONTRACTOR SHALL NOT VIOLATE THE OWNER'S INTENT TO SAVE SELECTED SPECIES AND IMPOSE THE MINIMUM ENVIRONMENTAL RESTRICTIONS OF THE RIGHT-OF-WAY. THE CONTRACTOR SHALL REVERSE THE RESULT OF EACH STEP OF THE CONSTRUCTION OF THE COUNTERPOISE SYSTEM REPRESENTATIVE PRIOR TO ITS FIELD SPECIFIED LOCATION. THE CONTRACTOR IS RESPONSIBLE TO THE OWNER FOR DAMAGES TO THE RIGHT-OF-WAY IN OTHER THAN THE FIELD SPECIFIED LOCATIONS.

2. ANY BRUSH ALONG THE FIELD SPECIFIED COUNTERPOISE ROUTES WHICH IS LEFT IN AN UNSIGHTLY CONDITION BY THE INSTALLATION WORK WILL BE CUT TO THE GROUND BY THE CONTRACTOR AND LEFT IN SMALL HEAT PILES IN PLACE WHERE CUT.
3. IN LOCATION WHERE EXCAVATION FOR THE INSTALLATION OF COUNTERPOISE BRINGS TO THE SURFACE ANY SMALL BOULDERS, THEY WILL BE BACKFILL BELOW GRADE OR DISPERSED ON THE RIGHT-OF-WAY AS THE OWNER'S REPRESENTATIVE MAY DIRECT. INSTALLATION OF THE COUNTERPOISE SHALL NOT RESULT IN A PATH OF SMALL BOULDERS ON THE FINISHED SURFACE.
4. THE OWNER ANTICIPATES THAT SEASONAL CONDITIONS MAY NOT ALLOW PERMANENT REHABILITATION OF WORK SITES AND THE RIGHT-OF-WAY UPON COMPLETION OF THE INSTALLATION OF THE COUNTERPOISE. WHERE TEMPORARY REHABILITATION HAS BEEN COMPLETED IN ADVERSE SEASON, THE CONTRACTOR SHALL TAKE THE FOLLOWING STEPS:
 - A. WATERBARS WILL BE CONSTRUCTED ON ACCESS ROADS AND TRENCH LINES TO SHUNT WATER OFF THIS LINE OF DISTURBED SURFACES AND CONTROL EROSION ALONG THE DISTURBED SURFACE.
 - B. ALL DISTURBED SURFACES OF FOUNDATION SITES OR LONG TRENCH LINES OR ACCESS ROADS WILL BE GRADED AND COVERED WITH MULCH. SUCH DISTURBED SURFACES ON SLOPES GREATER THAN ONE (VERTICAL) ON FOUR (HORIZONTAL) SHALL BE COVERED WITH WOOD CHIPS.

- A. LAWNS, GOLF COURSES, CEMETERIES AND OTHER SIMILAR OCCUPANCIES SHALL BE REHABILITATED TO RESTORE THE DEPTH OF FERTILE SOIL COMMON TO THE ADJACENT GROUND. WHERE APPROPRIATE, SEEDING SHALL BE DONE IN ACCORDANCE WITH STEP C BELOW.

- B. GARDENS, OTHER CULTIVATED AREAS AND PASTURES, SHALL BE GRADED AND SEEDING TO RESTORE THE DEPTH OF FERTILE SOIL COMMON TO THE ADJACENT GROUND. WHERE APPROPRIATE, SEEDING SHALL BE DONE IN ACCORDANCE WITH STEP C BELOW.

- C. THE CONTRACTOR SHALL SEED ALL DISTURBED AREAS ALONG THE NEW COUNTERPOISE ROUTES AT THE RATE OF 100 LBS. PER ACRE AND SHALL BE AS FOLLOWS ON APPROVED EQUAL:

CREATING RED FESCUE	% BY WEIGHT	% BY GERMINATION	% BY PURITY
CONNECTICUT TALL FESCUE	30	85	98
KENTUCKY TALL FESCUE	50	80	96

- D. ALL OTHER DISTURBED AREAS INCLUDING REMAINING FOUNDATION SITES, ACCESS ROADS, AND AREAS OF EROSION OF SLOPES SHALL BE SEED WITH MIXED SPECIES SEED AS APPROVED BY THE CONTRACTOR AND THE COMMISSION. THE CONNECTICUT STATE PARKS AND FOREST COMMISSION MAY BE SUBSTITUTED. ALL AREAS WHICH EXPERIENCED EROSION DAMAGE AND ALL SLOPES OVER ONE (VERTICAL) AND FOUR (HORIZONTAL) WHERE TEMPORARY REHABILITATION WORK HAS BEEN DONE SHALL BE REHABILITATED.

6. IT IS IMPERATIVE THAT PERMANENT REHABILITATION BE ACCOMPLISHED IN GOOD TIME, WHICH WILL ALLOW THE OCCUPANT FULL AND UNDISTURBED USE OF THE SITE SUCCEEDING SEASON, AND TO PREVENT UNNECESSARY AND UNREASONABLE SPREADING OF CONTINUATION OF DISTURBED SURFACES.
7. ANY BRUSH ALONG THE ACCESS ROADS WHICH IS LEFT IN AN UNSIGHTLY CONDITION BY THE WORK CONDUCTED, SHALL BE CUT TO THE GROUND BY THE CONTRACTOR AND LEFT IN SMALL HEAT PILES IN PLACE WHERE CUT.

**Structural Analysis of PCS
Structure and CL&P Pole**

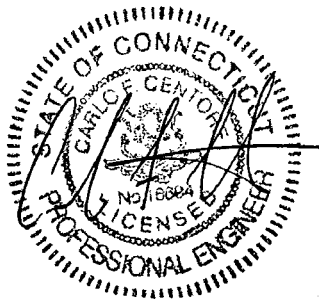
T-Mobile: CT11-110C

*CL&P Structure No. 10256
145' Electric Transmission Pole*

*8 Chimney Drive
Bethel, CT*

CEN TEK Project No. 10021.C05

Date: June 15, 2010



Prepared for:
*Transcend Wireless, LLC
113 N Mountain Blvd.
Mountain Top, PA 18707*

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Introduction

The purpose of this report is to analyze the existing 35-4" long PCS mast and 145' CL&P pole located at 8 Chimney Drive in Bethel, CT for the proposed antenna and equipment upgrade by T-Mobile.

The proposed loads consist of the following:

- **T-MOBILE (Existing to be removed):**
Antennas: Six (6) RR90-17-02DP panel antennas mounted in two (2) clusters on a PCS mast with RAD center elevations of 154-ft and 162-ft above tower base plate.
Mast: 4" Sch. 40 pipe mast.
- **T-MOBILE (Existing to remain):**
Coax Cables: Twelve (12) 1-5/8" \varnothing coax cables running on the outside of the tower as indicated in section 4 of this report.
- **T-MOBILE (Proposed):**
Antennas: Six (6) RFS APX16DWV-16DWVS-E-A20 panel antennas mounted in two (2) clusters on a PCS mast with RAD center elevations of 154-ft and 162-ft above tower base plate.
Coax Cables: Six (6) 1-5/8" \varnothing coax cables running on the outside of the tower as indicated in section 4 of this report.
Mast: 8" Sch. 80 pipe mast.

Primary assumptions used in the analysis

- Allowable steel stresses are defined by AISC-ASD 9th edition for design of the PCS Mast and antenna supporting elements.
- ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", defines allowable steel stresses for evaluation of the CL&P utility pole.
- All utility pole members are adequately protected to prevent corrosion of steel members.
- All proposed antenna mounts are modeled as listed above.
- All coaxial cable will be installed within the pipe mast unless specified otherwise.
- Pipe mast will be properly installed and maintained.
- No residual stresses exist due to incorrect pole erection.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds conform to the requirements of AWS D1.1.
- All proposed pipe mast members will be as specified in the construction documents to be prepared by CEN TEK engineering, Inc.
- Pipe mast and utility pole will be in plumb condition.
- Utility pole was properly installed and maintained and all members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
- Any deviation from the analyzed loading will require a new analysis for verification of structural adequacy.

A n a l y s i s

Structural analysis of the existing *PCS Mast Structure* was independently completed using the current version of RISA-3D computer program licensed to CEN TEK Engineering, Inc.

The existing mast consisting of a 4-in x 35-ft-4-in long SCH. 40 pipe (O.D. = 4.5") connected at two points to the existing tower was analyzed for its ability to resist loads prescribed by the TIA/EIA standard. Section 5 of this report details these gravity and lateral wind loads. NESC prescribed loads were also applied to the mast structure in order to obtain reactions needed for analyzing the CL&P pole structure. These loads are developed in Section 7 of this report. Load cases and combinations used in RISA-3D for TIA/EIA loading and for NESC/NU loading are listed in report Sections 6 and 8, respectively.

An envelope solution was first made to determine maximum and minimum forces, stresses, and deflections to confirm the selected section as adequate. Additional analyses were then made to determine the NESC forces to be applied to the CL&P pole structure.

The RISA-3D program contains a library of all AISC shapes and corresponding section properties are computed and applied directly within the program. The program's Steel Code Check option was also utilized. The forces calculated in RISA-3D using NESC guidelines were then applied to the CL&P pole using PLS-Pole. Maximum usage for the pole was calculated considering the additional forces from the mast and associated appurtenances.

D e s i g n B a s i s

Our analysis was performed in accordance with EIA-222-F-1996, ASCE Manual No. 72 – "Design of Steel Transmission Pole Structures Second Edition", NESC C2-2007 and Northeast Utilities Design Criteria.

The CL&P pole structure, considering existing and future conductor and shield wire loading, with the proposed antenna mast was analyzed under two conditions:

▪ UTILITY POLE ANALYSIS

The purpose of this analysis is to determine the adequacy of the existing utility pole to support the proposed antenna loads. The loading and design requirements were analyzed in accordance with the NU Design Criteria Table, NESC C2-2007 ~ Construction Grade B, and ASCE Manual No. 72.

Load cases considered:

Load Case 1: NESC Heavy

Wind Pressure.....	4.0 psf
Vertical Overload Capacity Factor.....	1.50
Wind Overload Capacity Factor.....	2.50
Wire Tension Overload Capacity Factor.....	1.65

Load Case 2: NESC Extreme

Wind Speed.....	110 mph ⁽¹⁾
Radial Ice Thickness.....	0"

Note 1: NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading, 1.25 x Gust Response Factor (wind speed: 3-second gust)

▪ **PCS MAST ANALYSIS**

The PCS mast, appurtenances and connections to the utility pole were analyzed and designed in accordance with the NU Design Criteria Table, TIA/EIA-222-F, and AISC-ASD standards.

Load cases considered:

Load Case 1:

Wind Speed..... 85 mph ⁽²⁾
 Radial Ice Thickness..... 0"

Load Case 2:

Wind Pressure..... 75% of 85 mph wind pressure
 Radial Ice Thickness..... 0.5"

| Note 2: Per NU Mast Design Criteria Exception 1.

Results

▪ **PCS MAST**

The existing pipe mast was determined to be structurally **inadequate**. Replacement of the existing antenna mast with a **8 SCH. 80 Pipe x 29.5-ft long (O.D. = 8.63")**, conforming to ASTM A53, Grade B, F_y = 35 ksi specifications will be required.

▪ **UTILITY POLE**

This analysis finds that the subject utility pole is adequate to support the proposed antenna mast and related appurtenances. The pole stresses meet the requirements set forth by the ASCE Manual No. 72, "Design of Steel Transmission Pole Structures Second Edition", for the applied NESC Heavy and Hi-Wind load cases. The detailed analysis results are provided in Section 9 of this report. The analysis results are summarized as follows:

A maximum usage of **95.5%** occurs in the utility pole under the **NESC Extreme** loading condition.

POLE SECTION:

The utility pole was found to be within allowable limits.

Tower Section	Elevation	Stress Ratio (% of capacity)	Result
Tube Number 3	0' -46.17' (AGL)	93.4%	PASS

BASE PLATE:

The base plate was found to be within allowable limits from the PLS output based on 24 bend lines.

Tower Component	Design Limit	Stress Ratio (percentage of capacity)	Result
Base Plate	Bending	95.5%	PASS

▪ FOUNDATION AND ANCHORS

The existing foundation consists of a 8-ft Ø x 18.0-ft long reinforced concrete caisson. The base of the tower is connected to the foundation by means of (24) 2.25"Ø, ASTM A615-75 anchor bolts embedded approximately 8-ft into the concrete foundation structure. Foundation information was obtained from NUSCO drawing # 01143-60001.

Review of the foundation and anchor design consisted of verification of applied loads obtained from the tower design calculations and code checks of allowable stresses:

BASE REACTIONS:

From PLS-Pole analysis of CL&P pole based on NESC/NU prescribed loads.

Load Case	Transverse	Axial	Overturning Moment
NESC Heavy Wind	31.2 kips	107.6 kips	3510.2 ft-kips
NESC Extreme Wind	58.1 kips	54.7 kips	5636.4 ft-kips
NESC Heavy Broken Wire	31.1 kips	107.3 kips	3534.3 ft-kips

ANCHOR BOLTS:

The anchor bolts were found to be within allowable limits.

Tower Component	Design Limit	Stress Ratio (% of capacity)	Result
Anchor Bolts	Tension	53.78%	PASS

FOUNDATION:

The foundation was **not found** to be structural adequate based on the original design reactions.

Foundation	Design Limit	Original Design Reaction ⁽¹⁾	Proposed Loading ⁽²⁾	Result
Reinf. Conc. Pad and Pier	OTM ⁽³⁾	4977.9 ft-kips	6200 ft-kips	FAIL

Note: 1. Original design reaction taken from NU drawing 01143-50004.
 2. 10% increase to PLS base reactions per OTRM 051.
 2. OTM denotes Overturning Moment

The foundation with modifications per drawing S-3 was found to be within allowable limits.

Foundation	Design Limit	OTRM 051 FS ⁽¹⁾	Proposed Loading FS ⁽¹⁾	Result
Reinf. Conc. Pad and Pier	OTM ⁽²⁾	1.5	1.57	PASS

Note: 1. FS denote Factor of Safety
 2. OTM denotes Overturning Moment

CENTEK
Structural Analysis – T-Mobile: CT-11-110C
CL&P Structure # 10256
Bethel, CT
Rev 1 ~ June 15, 2010

Conclusions and Recommendations

The existing caisson foundation was found to be structurally inadequate. A proposed 26-ft square by 4-ft deep reinforced concrete mat installed at the periphery of the existing caisson is required to limit the deflection to within the allowable limit. Refer to the structural design drawings located within Section 4 of this report for details of the proposed foundation reinforcements.

This analysis shows that the subject utility pole with the proposed foundation modifications is adequate to support the proposed T-Mobile equipment upgrade.

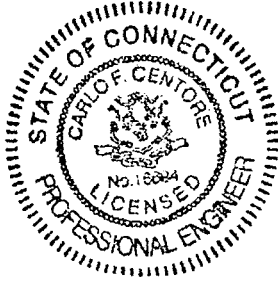
The analysis is based, in part on the information provided to this office by Northeast Utilities and Transcend Wireless. If the existing conditions are different than the information in this report, CENTEK engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

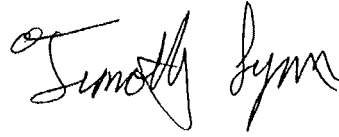
Respectfully Submitted by:



Carlo F. Centore, PE
Principal ~ Structural Engineer



Prepared by:



Timothy J. Lynn, EIT
Structural Engineer

STANDARD CONDITIONS FOR FURNISHING OF
PROFESSIONAL ENGINEERING SERVICES ON
EXISTING STRUCTURES

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CEN TEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provided to CEN TEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the "as new" condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222.
- All services are performed, results obtained, and recommendations made in accordance with generally accepted engineering principles and practices. CEN TEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ RISA - 3 D

RISA-3D Structural Analysis Program is an integrated structural analysis and design software package for buildings, bridges, tower structures, etc.

Modeling Features:

- Comprehensive CAD-like graphic drawing/editing capabilities that let you draw, modify and load elements as well as snap, move, rotate, copy, mirror, scale, split, merge, mesh, delete, apply, etc.
- Versatile drawing grids (orthogonal, radial, skewed)
- Universal snaps and object snaps allow drawing without grids
- Versatile general truss generator
- Powerful graphic select/unselect tools including box, line, polygon, invert, criteria, spreadsheet selection, with locking
- Saved selections to quickly recall desired selections
- Modification tools that modify single items or entire selections
- Real spreadsheets with cut, paste, fill, math, sort, find, etc.
- Dynamic synchronization between spreadsheets and views so you can edit or view any data in the plotted views or in the spreadsheets
- Simultaneous view of multiple spreadsheets
- Constant in-stream error checking and data validation
- Unlimited undo/redo capability
- Generation templates for grids, disks, cylinders, cones, arcs, trusses, tanks, hydrostatic loads, etc.
- Support for all units systems & conversions at any time
- Automatic interaction with RISASection libraries
- Import DXF, RISA-2D, STAAD and ProSteel 3D files
- Export DXF, SDNF and ProSteel 3D files

Analysis Features:

- Static analysis and P-Delta effects
- Multiple simultaneous dynamic and response spectra analysis using Gupta, CQC or SRSS mode combinations
- Automatic inclusion of mass offset (5% or user defined) for dynamic analysis
- Physical member modeling that does not require members to be broken up at intermediate joints
- State of the art 3 or 4 node plate/shell elements
- High-end automatic mesh generation — draw a polygon with any number of sides to create a mesh of well-formed quadrilateral (NOT triangular) elements.
- Accurate analysis of tapered wide flanges - web, top and bottom flanges may all taper independently
- Automatic rigid diaphragm modeling
- Area loads with one-way or two-way distributions
- Multiple simultaneous moving loads with standard AASHTO loads and custom moving loads for bridges, cranes, etc.
- Torsional warping calculations for stiffness, stress and design
- Automatic Top of Member offset modeling
- Member end releases & rigid end offsets
- Joint master-slave assignments
- Joints detachable from diaphragms
- Enforced joint displacements
- 1-Way members, for tension only bracing, slipping, etc.

- 1-Way springs, for modeling soils and other effects
- Euler members that take compression up to their buckling load, then turn off.
- Stress calculations on any arbitrary shape
- Inactive members, plates, and diaphragms allows you to quickly remove parts of structures from consideration
- Story drift calculations provide relative drift and ratio to height
- Automatic self-weight calculations for members and plates
- Automatic subgrade soil spring generator

Graphics Features:

- Unlimited simultaneous model view windows
- Extraordinary "true to scale" rendering, even when drawing
- High-speed redraw algorithm for instant refreshing
- Dynamic scrolling stops right where you want
- Plot & print virtually everything with color coding & labeling
- Rotate, zoom, pan, scroll and snap views
- Saved views to quickly restore frequent or desired views
- Full render or wire-frame animations of deflected model and dynamic mode shapes with frame and speed control
- Animation of moving loads with speed control
- High quality customizable graphics printing

Design Features:

- Designs concrete, hot rolled steel, cold formed steel and wood
- ACI 1999/2002, BS 8110-97, CSA A23.3-94, IS456:2000, EC 2-1992 with consistent bar sizes through adjacent spans
- Exact integration of concrete stress distributions using parabolic or rectangular stress blocks
- Concrete beam detailing (Rectangular, T and L)
- Concrete column interaction diagrams
- Steel Design Codes: AISC ASD 9th, LRFD 2nd & 3rd, HSS Specification, CAN/CSA-S16.1-1994 & 2004, BS 5950-1-2000, IS 800-1984, Euro 3-1993 including local shape databases
- AISI 1999 cold formed steel design
- NDS 1991/1997/2001 wood design, including Structural Composite Lumber, multi-ply, full sawn
- Automatic spectra generation for UBC 1997, IBC 2000/2003
- Generation of load combinations: ASCE, UBC, IBC, BOCA, SBC, ACI
- Unbraced lengths for physical members that recognize connecting elements and full lengths of members
- Automatic approximation of K factors
- Tapered wide flange design with either ASD or LRFD codes
- Optimization of member sizes for all materials and all design codes, controlled by standard or user-defined lists of available sizes and criteria such as maximum depths
- Automatic calculation of custom shape properties
- Steel Shapes: AISC, HSS, CAN, ARBED, British, Euro, Indian, Chilean
- Light Gage Shapes: AISI, SSMA, Dale / Incor, Dietrich, MarinoWARE
- Wood Shapes: Complete NDS species/grade database
- Full seamless integration with RISAFoot (Ver 2 or better) for advanced footing design and detailing
- Plate force summation tool

Results Features:

- Graphic presentation of color-coded results and plotted designs
- Color contours of plate stresses and forces with quadratic smoothing, the contours may also be animated
- Spreadsheet results with sorting and filtering of: reactions, member & joint deflections, beam & plate forces/stresses, optimized sizes, code designs, concrete reinforcing, material takeoffs, frequencies and mode shapes
- Standard and user-defined reports
- Graphic member detail reports with force/stress/deflection diagrams and detailed design calculations and expanded diagrams that display magnitudes at any dialed location
- Saved solutions quickly restore analysis and design results.

GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM ~ PLS-POLE

PLS-POLE provides all of the capabilities a structural engineer requires to design transmission, substation or communications structures. It does so using a simple easy to use graphical interface that rests upon our time tested finite element engine. Regardless of whether you want to model a simple wood pole or a guyed steel X-Frame; PLS-POLE can handle the job simply, reliably and efficiently.

Modeling Features:

- Structures are made of standard reusable components that are available in libraries. You can easily create your own libraries or get them from a manufacturer
- Structure models are built interactively using interactive menus and graphical commands
- Automatic generation of underlying finite element model of structure
- Steel poles can have circular, 4, 6, 8, 12, 16, or 18-sided, regular, elliptical or user input cross sections (flat-to-flat or tip-to-tip orientations)
- Steel and concrete poles can be selected from standard sizes available from manufacturers
- Automatic pole class selection
- Cross brace position optimizer
- Capability to specify pole ground line rotations
- Capability to model foundation displacements
- Can optionally model foundation stiffness
- Guys are easily handled (modeled as exact cable elements in nonlinear analysis)
- Powerful graphics module (members color-coded by stress usage)
- Graphical selection of joints and components allows graphical editing and checking
- Poles can be shown as lines, wire frames or can be rendered as 3-d polygon surfaces

Analysis Features:

- Automatic distribution of loads in 2-part suspension insulators (v-strings, horizontal vees, etc.)
- Design checks for ASCE, ANSI/TIA/EIA 222 (Revisions F and G) or other requirements
- Automatic calculation of dead and wind loads
- Automated loading on structure (wind, ice and drag coefficients) according to:
 - ASCE 74-1991
 - NESC 2002
 - NESC 2007
 - IEC 60826:2003
 - EN50341-1:2001 (CENELEC)
 - EN50341-3-9:2001 (UK NNA)
 - EN50341-3-17:2001 (Portugal NNA)
 - ESAA C(b)1-2003 (Australia)
 - TPNZ (New Zealand)
 - REE (Spain)
 - EIA/TIA 222-F
 - ANSI/TIA 222-G
 - CSA S37-01
- Automated microwave antenna loading as per EIA/TIA 222-F and ANSI/TIA 222-G
- Detects buckling by nonlinear analysis

CENTEK Engineering, Inc.
Structural Analysis – T-Mobile: CT-11-110C
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Bethel, CT
Rev 1 ~ June 15, 2010

Results Features:

- Detects buckling by nonlinear analysis
- Easy to interpret text, spreadsheet and graphics design summaries
- Automatic determination of allowable wind and weight spans
- Automatic determination of interaction diagrams between allowable wind and weight spans
- Automatic tracking of part numbers and costs

Criteria for Design of PCS Facilities On or
Extending Above Metal Electric Transmission
Towers & Analysis of Transmission Towers
Supporting PCS Masts ⁽¹⁾

Introduction

This criteria is the result from an evaluation of the methods and loadings specified by the separate standards, which are used in designing telecommunications towers and electric transmission towers. That evaluation is detailed elsewhere, but in summary; the methods and loadings are significantly different. This criteria specifies the manner in which the appropriate standard is used to design PCS facilities including masts and brackets (hereafter referred to as "masts"), and to evaluate the electric transmission towers to support PCS masts. The intent is to achieve an equivalent level of safety and security under the extreme design conditions expected in Connecticut and Massachusetts.

ANSI Standard TIA/EIA-222 covering the design of telecommunications structures specifies a working strength/allowable stress design approach. This approach applies the loads from extreme weather loading conditions, and designs the structure so that it does not exceed some defined percentage of failure strength (allowable stress).

ANSI Standard C2-2007 (National Electrical Safety Code) covering the design of electric transmission metal structures is based upon an ultimate strength/yield stress design approach. This approach applies a multiplier (overload capacity factor) to the loads possible from extreme weather loading conditions, and designs the structure so that it does not exceed its ultimate strength (yield stress).

Each standard defines the details of how loads are to be calculated differently. Most of the NU effort in "unifying" both codes was to establish what level of strength each approach would provide, and then increasing the appropriate elements of each to achieve a similar level of security under extreme weather loadings.

Two extreme weather conditions are considered. The first is an extreme wind condition (hurricane) based upon a 50-year recurrence (2% annual probability). The second is a winter condition combining wind and ice loadings.

The following sections describe the design criteria for any PCS mast extending above the top of an electric transmission tower, and the analysis criteria for evaluating the loads on the transmission tower from such a mast from the lower portions of such a mast, and loads on the pre-existing electric lower portions of such a mast, and loads on the pre-existing electric transmission tower and the conductors it supports.

| Note 1: Prepared from documentation provide from Northeast Utilities.

P C S M a s t

The PCS facility (mast, external cable/trays, including the initial and any planned future support platforms, antennas, etc. extending the full height above the top level of the electric transmission structure) shall be designed in accordance with the provisions of TIA/EIA Standard 222 with two exceptions:

1. An 85 mph extreme wind speed shall be used for locations in all counties throughout the NU system.
2. The stress increase of TIA Section 3.1.1.1 is disallowed. The combined wind and ice condition shall consider ½" radial ice in combination with the wind load (0.75 W_i) as specified in TIA section 2.3.16.

E L E C T R I C T R A N S M I S S I O N T O W E R

The electric transmission tower shall be analyzed using yield stress theory in accordance with the attached table titled "NU Design Criteria". This specifies uniform loadings (different from the TIA loadings) on the each of the following components of the installed facility:

- PCS mast for its total height above ground level, including the initial and planned future support platforms, antennas, etc. above the top of an electric transmission structure.
- Conductors are related devices and hardware.
- Electric transmission structure. The loads from the PCS facility and from the electric conductors shall be applied to the structure at conductor and PCS mast attachment points, where those load transfer to the tower.

The uniform loadings and factors specified for the above components in the table are based upon the National Electrical Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to TIA and its loads and factors with the exceptions noted above. (Note that the NESC does not require the projected wind surfaces of structures and equipment to be increased by the ice covering.)

In the event that the electric transmission tower is not sufficient to support the additional loadings of the PCS mast, reinforcement will be necessary to upgrade the strength of the overstressed members.

Northeast Utilities Overhead Transmission Standards

Attachment A

NU Design Criteria

		Basic Wind Speed	Pressure	Height Factor	Gust Factor	Load or Stress Factor	Force Coef. - Shape Factor	
		V (MPH)	Q (PSF)	Kz	Gh			
Ice Condition	TIA/EIA	Antenna Mount	TIA	TIA (75W)	TIA	TIA	TIA Section 23.1.1 disallowed for connection design	
	NESC Heavy	Tower/Pole Analysis with antennas extending above top of Tower/Pole (Yield Stress)	---	4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
		Tower/Pole Analysis with Antennas below top of Tower/Pole (on two faces)		4	1.00	1.00	2.50	1.6 Flat Surfaces 1.3 Round Surfaces
Conductors:		Conductor loads provided by NU						
High Wind Condition	TIA/EIA	Antenna Mount	BS	TIA	TIA	TIA	TIA Section 23.1.1 disallowed for connection design	
	NESC Extreme Wind	Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna				1.6 Flat Surfaces 1.3 Round Surfaces	
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250C: Extreme Wind Loading Height above ground level based on top of Tower/Pole				1.6 Flat Surfaces 1.3 Round Surfaces	
Conductors:		Conductor loads provided by NU						
NESC Extreme Ice with Wind Condition*		Tower/Pole Analysis with antennas extending above top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load 1.25 x Gust Response Factor Height above ground level based on top of Mast/Antenna				1.6 Flat Surfaces 1.3 Round Surfaces	
		Tower/Pole Analysis with Antennas below top of Tower/Pole	Use NESC C2-2007, Section 25, Rule 250D: Extreme Ice with Wind Loading 4PSF Wind Load Height above ground level based on top of Tower/Pole				1.6 Flat Surfaces 1.3 Round Surfaces	
	Conductors:		Conductor loads provided by NU					

* Only for Structures Installed after 2007

Northeast Utilities Overhead Transmission Standards

- 2) STEP 2 - The electric transmission structure analysis and evaluation shall be performed in accordance with NESC requirements and shall include the mast and antenna loads determined from NESC applied loading conditions (not TIA/EIA Loads) on the structure and mount as specified below, and shall include the wireless communication mast and antenna loads per NESC criteria)

The structure shall be analyzed using yield stress theory in accordance with Attachment A, "NU Design Criteria." This specifies uniform loadings (different from the TIA loadings) on each of the following components of the installed facility:

- a) Wireless communication mast for its total height above ground level, including the initial and any planned future equipment (Support Platforms, Antennas, TMA's etc.) above the top of an electric transmission structure.
- b) Conductors and related devices and hardware (wire loads will be provided by NU).
- c) Electric Transmission Structure
 - i) The loads from the wireless communication equipment components based on NESC and NU Criteria in Attachment A, and from the electric conductors shall be applied to the structure at conductor and wireless communication mast attachment points, where those loads transfer to the tower.
 - ii) Shape Factor Multiplier:

NESC Structure Shape	Cd
Polyround (for polygonal steel poles)	1.3
Flat	1.6
Open Lattice	3.2

- iii) When Coaxial Cables are mounted along side the pole structure, the shape multiplier shall be:

Mount Type	Cable Cd	Pole Cd
Coaxial Cables on outside periphery (One layer)	1.45	1.45
Coaxial Cables mounted on stand offs	1.6	1.3

- d) The uniform loadings and factors specified for the above components in Attachment A, "NU Design Criteria" are based upon the National Electric Safety Code 2007 Edition Extreme Wind (Rule 250C) and Combined Ice and Wind (Rule 250B-Heavy) Loadings. These provide equivalent loadings compared to the TIA and its loads and factors with the exceptions noted above.

Note: The NESC does not require ice load be included in the supporting structure. (Ice on conductors and shield wire only, and NU will provide these loads).

- e) Mast reaction loads shall be evaluated for local effects on the transmission structure members at the attachment points.

If the electric transmission structure is not sufficient to support the additional loadings of the wireless communication mast, reinforcement will be required to upgrade the strength of the overstressed members. Any reinforcement design will be reviewed by NU TL&CE to determine the feasibility of construction and its impact on the use of the structure as a transmission structure.

Communication Antennas on Transmission Structures (CL&P & WMECo Only)

Northeast Utilities
Approved by: DEH (NU)

Design

OTRM 059
Page 3 of 9

Rev.0
11/17/2009



Job :
Description:

Spec. Number
Computed by
Checked by

* Page of
Sheet of
Date 9/25/09
Date

INPUT DATA

TOWER ID: 10256

Structure Height (ft) : 145

Wind Zone : Central CT (green)

Wind Speed : 90.5711047 mph

Tower Type : Suspension
 Strain

Extreme Wind Model : PCS Addition

Shield Wire Properties:

	BACK	AHEAD
NAME =	3/8 AW	3/8 AW
DESCRIPTION =	3/8	3/8
STRANDING =	7 #8 Al Weld	7 #8 Al Weld
DIAMETER =	0.385 in	0.385 in
WEIGHT =	0.262 lb/ft	0.262 lb/ft

Conductor Properties:

		BACK	AHEAD		
NAME =		BITTERN	BITTERN		
Number of Conductors per phase	2	1272.000	1272.000	2	Number of Conductors per phase
		45/7 ACSR	45/7 ACSR		
DIAMETER =		1.345 in	1.345 in		
WEIGHT =		1.432 lb/ft	1.432 lb/ft		

Insulator Weight = 0 lbs

Broken Wire Side = AHEAD SPAN

Horizontal Line Tensions:

	BACK		AHEAD	
	Shield	Conductor	Shield	Conductor
NESC HEAVY =	4,200	10,000	4,200	10,000
EXTREME WIND =	5,616	12,705	5,616	12,705
LONG. WIND =	na	na	na	na
250D COMBINED =	na	na	na	na
NESC W/O OLF =	na	na	na	na
60 DEG F NO WIND =	1,239	4,616	1,239	4,616

Line Geometry:

	BACK:	0	AHEAD:	0	SUM
LINE ANGLE (deg) =	BACK:	493	AHEAD:	358	851
WIND SPAN (ft) =	BACK:	613	AHEAD:	300	913
WEIGHT SPAN (ft) =					



Job :
Description:

Spec. Number
Computed by
Checked by

Page of
Sheet of
Date 9/25/09
Date

WIRE LOADING AT ATTACHMENTS

TOWER ID:

Wind Span =
Weight Span =
Total Angle =

Broken Wire Span =
Type of Insulator Attachment =

1. NESC RULE 250B Heavy Loading:

	INTACT CONDITION			BROKEN WIRE CONDITION		
	Horizontal	Longitudinal	Vertical	Horizontal	Longitudinal	Vertical
Shield Wire =	982 lb	0 lb	1,112 lb	569 lb	4,830 lb	747 lb
Conductor =	3,326 lb	0 lb	7,066 lb	1,927 lb	23,000 lb	4,746 lb

2. NESC RULE 250C Transverse Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	641 lb	0 lb	275 lb
Conductor =	4,479 lb	0 lb	3,008 lb

3. NESC RULE 250C Longitudinal Extreme Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	275 lb
Conductor =	#VALUE!	#VALUE!	3,008 lb

4. NESC RULE 250D Extreme Ice & Wind Loading:

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	1,812 lb
Conductor =	#VALUE!	#VALUE!	7,941 lb

5. NESC RULE 250B w/o OLF's

	Horizontal	Longitudinal	Vertical
Shield Wire =	#VALUE!	#VALUE!	742 lb
Conductor =	#VALUE!	#VALUE!	4,711 lb

6. 60 Deg. F, No Wind

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	239 lb
Conductor =	0 lb	0 lb	2,615 lb

7. Construction

	Horizontal	Longitudinal	Vertical
Shield Wire =	0 lb	0 lb	359 lb
Conductor =	0 lb	0 lb	3,923 lb

NOTE: All loads include required overload factors (OLF's).

- ☉ T-MOBILE ANTENNAS
EL. ±162'-0" ABP
- ☉ T-MOBILE ANTENNAS
EL. ±154'-0" ABP

- ☉ TOP CONNECTION
EL. ±143'-0" ABP
- ☉ BOTTOM CONNECTION
EL. ±135'-0" ABP

NOTE: ABP DENOTES ABOVE OF BASE PLATE

PROPOSED SIX (6) RFS APX16DWV-16DWVS-E-A20 PANEL ANTENNAS TO REPLACE EXISTING (6) PANEL ANTENNAS.

PROPOSED 8" SCH. 80 X 29'-6" LONG PIPE MAST (O.D. = 8.63")

EXISTING ONE (1) HOME RUN CABLE

EXISTING TWELVE (12) 1-5/8" ϕ COAX CABLES

PROPOSED SIX (6) 1-5/8" ϕ COAX CABLES ATTACHED TO THE MONOPOLE @ 4-FT O.C. W/ ANDREW DOUBLE CLICK-ON HANNGERS PART NO. L7CLICK AND RELATED HARDWARE

EXISTING 145' TALL CL&P STEEL POLE STRUCTURE NO. 10256

PROPOSED SIX (6) 1-5/8" DIA. COAX CABLES

EXISTING TWELVE (12) 1-5/8" DIA. COAX CABLES AND ONE (1) HOME RUN CABLE

EXISTING ONE (1) HOME RUN CABLE

EXISTING TWELVE (12) 1-5/8" ϕ COAX CABLES

PROPOSED SIX (6) 1-5/8" ϕ COAX CABLES ATTACHED TO THE MONOPOLE @ 4-FT O.C. W/ ANDREW DOUBLE CLICK-ON HANNGERS PART NO. L7CLICK AND RELATED HARDWARE

2 COAX PLAN
EL-1 SCALE: NOT TO SCALE

3 COAX PLAN
EL-1 SCALE: NOT TO SCALE

1 TOWER & MAST ELEVATION
EL-1 SCALE: NOT TO SCALE

REVISIONS		
00	5/10/10	ISSUED FOR NJ REVIEW
01	8/15/10	CONSTRUCTION

CEN TEK engineering
Centered on Solutions™
www.CentekEng.com
(203) 488-0580
(203) 488-8587 Fax
63-2 North Branford Road, Branford, CT 06405

CT11110C
CL&P 10256
8 CHIMNEY DRIVE
BETHEL, CT 06801

PROJECT NO: 10021.CO5
DRAWN BY: TJL
CHECKED BY: CFC
SCALE: AS NOTED
DATE: 4/29/10

T

TOWER AND MAST ELEVATION
EL-1
DWG. 1 OF 7

DESIGNED BY:	TAL
DRAWN BY:	CFC
CHECKED BY:	CFC
DATE:	8/13/10
SCALE:	AS SHOWN
PROJECT NO.:	10251100
DATE:	05/07/10
SCALE:	AS SHOWN
PROJECT NO.:	10251100

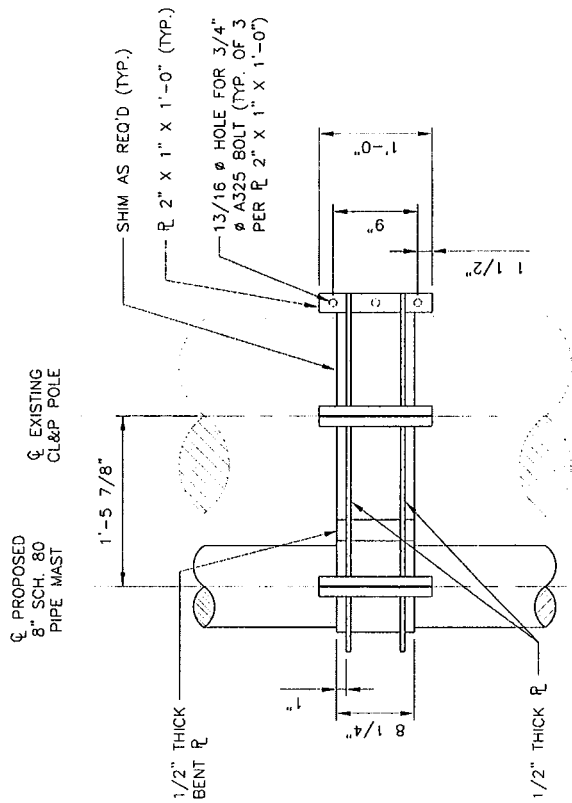


GENEX
 2201 Madison
 4275 Wilshire Blvd
 Los Angeles, CA 90048
 (310) 551-1100
 www.genex.com

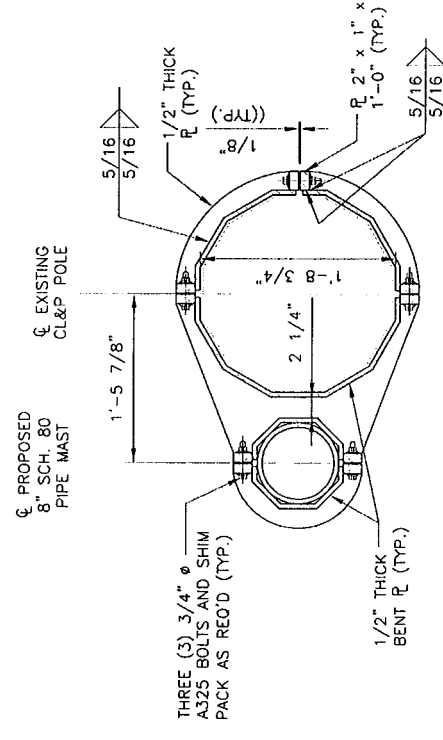
T-MOBILE
 PROJECT NO. 10251100
CT11-110C
 CL&P STRUCTURE 10256
 8 CHERRY LANE
 BETHLE, CT 06808

TOP CONNECTION DETAILS

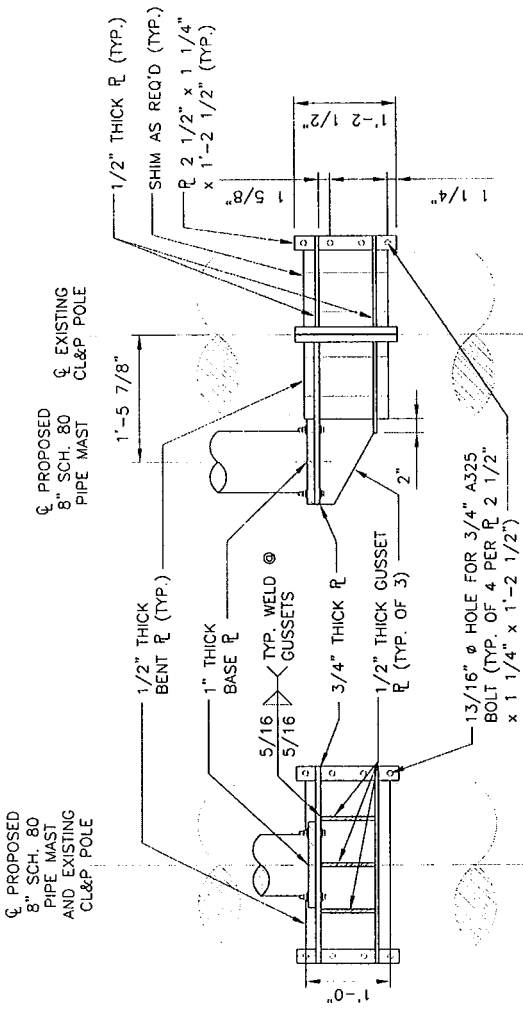
SHEET NO. **S-1**



1 TOP PCS BRACKET DETAIL
 S-1 SCALE: 1" = 1'-0"

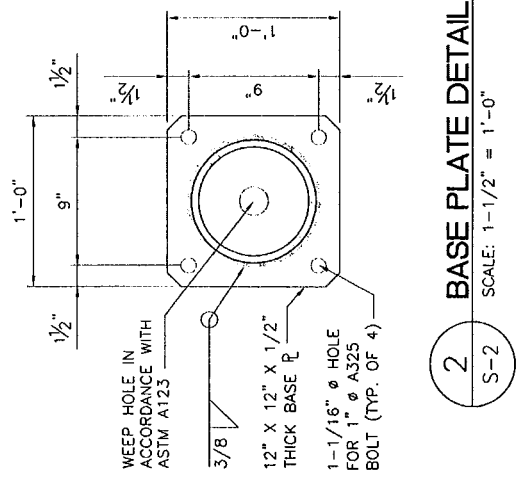


2 TOP PCS BRACKET PLAN VIEW
 S-1 SCALE: 1" = 1'-0"

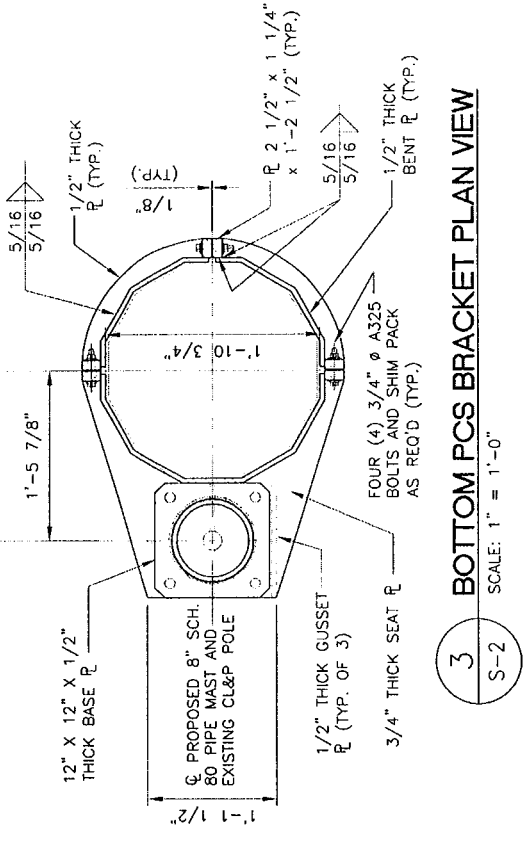


1 BOTTOM PCS BRACKET DETAIL
 SCALE: 3/4" = 1'-0"

NOTE:
 1. CL&P POLE TAPER = 0.2511"/FT (V.I.F.)



2 BASE PLATE DETAIL
 SCALE: 1-1/2" = 1'-0"



3 BOTTOM PCS BRACKET PLAN VIEW
 SCALE: 1" = 1'-0"

DESIGNED BY:	DATE:	PROJECT:	NO.:
DRAWN BY:	DATE:	DESCRIPTION:	DATE:
CHECKED BY:	DATE:	ISSUED FOR:	NO.:
DATE:	DATE:	ISSUED FOR:	NO.:

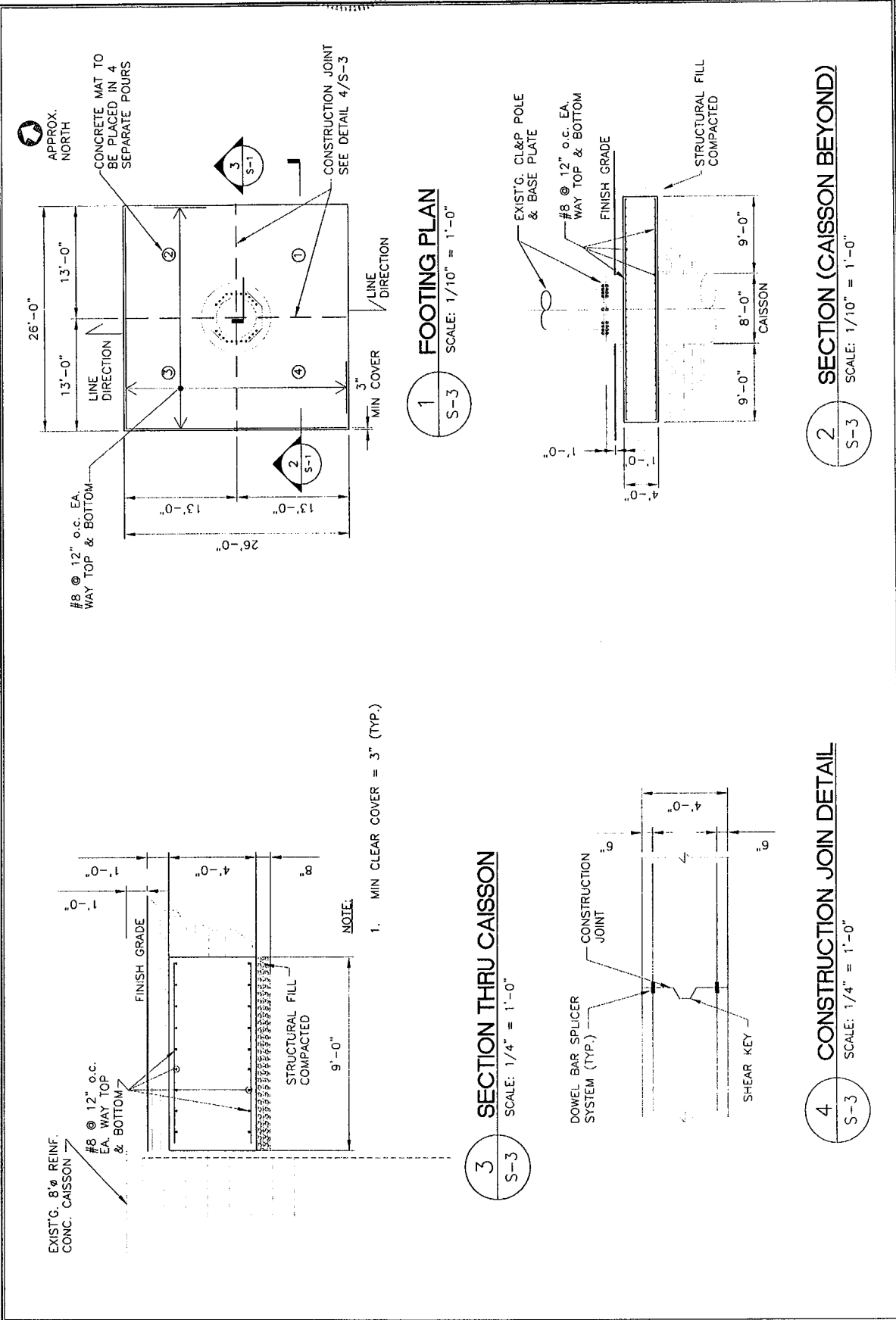
REGISTERED PROFESSIONAL ENGINEER
STATE OF CONNECTICUT
No. 10256
Charles A. Gagnier

1
S-3

2
S-3

3
S-3

4
S-3



DESIGN BASIS:

1. GOVERNING CODE: 2003 INTERNATIONAL BUILDING CODE AS MODIFIED BY THE 2005 CT STATE BUILDING CODE.
2. DESIGN CRITERIA
 WIND LOAD: (PCS MAST)
 BASIC WIND SPEED (V) = 85 MPH (FASTEST MILE); BASED ON TA/EIA-222F
 AND NU MAST DESIGN CRITERIA EXCEPTION 1.
 WIND LOAD: (UTILITY POLE & FOUNDATION)
 BASIC WIND SPEED (V) = 110 MPH (3--SECOND GUST) BASED ON NESC
 C2-2007, SECTION 25 RULE 250C.

GENERAL NOTES:

1. ALL CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE GOVERNING BUILDING CODE.
2. BEFORE BEGINNING THE WORK, THE CONTRACTOR IS RESPONSIBLE FOR MAKING SUCH INVESTIGATIONS CONCERNING PHYSICAL CONDITIONS (SURFACE AND SUBSURFACE) AT OR CONTIGUOUS TO THE SITE WHICH MAY AFFECT PERFORMANCE AND COST OF THE WORK.
3. AS THE WORK PROGRESSES, THE CONTRACTOR SHALL NOTIFY THE OWNER OF ANY CONDITIONS WHICH ARE IN CONFLICT OR OTHERWISE NOT CONSISTENT WITH THE CONSTRUCTION DOCUMENTS, AND SHALL NOT PROCEED WITH SUCH WORK UNTIL THE CONFLICT IS SATISFACTORILY RESOLVED.
4. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER FOUNDATION REMEDIATION WORK IS COMPLETE. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE AND TO ENSURE THE SAFETY OF THE STRUCTURE AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, TEMPORARY BRACING, GUYS OR TIEDOWNS, WHICH MIGHT BE NECESSARY.
5. THE CONTRACTOR SHALL LIMIT THE DURATION OF THE FOUNDATION REINFORCEMENT WORK. THE EXISTING CAISSON WITHIN THE SHOWN LIMITS IS STABLE FOR WIND SPEEDS LESS THAN 50MPH WITHOUT ICE LOADING. IF HIGHER WIND SPEED OR ICE EVENT IS EXPECTED, THE EXCAVATION AREA SHALL BE FILLED WITH COMPACT FILL MATERIAL.
6. ALL DIMENSIONS, ELEVATIONS, AND OTHER REFERENCES TO EXISTING STRUCTURES, SURFACE, AND SUBSURFACE CONDITIONS ARE APPROXIMATE. NO GUARANTEE IS MADE FOR THE ACCURACY OR COMPLETENESS OF THE INFORMATION SHOWN. THE CONTRACTOR SHALL VERIFY AND COORDINATE ALL DIMENSIONS, ELEVATIONS, ANGLES WITH EXISTING CONDITIONS AND WITH ARCHITECTURAL AND SITE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
7. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.
8. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING AND MAINTAINING ADEQUATE SHORING, BRACING, AND BARRICADES AS MAY BE REQUIRED FOR THE PROTECTION OF EXISTING PROPERTY, CONSTRUCTION WORKERS, AND FOR PUBLIC SAFETY.
9. SHOP DRAWINGS, CONCRETE MIX DESIGNS, TEST REPORTS, AND OTHER SUBMITTALS PERTAINING TO STRUCTURAL WORK SHALL BE FORWARDED TO THE OWNER FOR REVIEW BEFORE FABRICATION AND/OR INSTALLATION IS MADE. SHOP DRAWINGS SHALL INCLUDE ERECTION DRAWINGS AND COMPLETE DETAILS OF CONNECTIONS AS WELL AS MANUFACTURER'S SPECIFICATION DATA WHERE APPROPRIATE. SHOP DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE BEING SUBMITTED FOR REVIEW.
10. THE CONTRACTOR IS SOLELY RESPONSIBLE TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING STRUCTURES AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, UNDERPINNING, ETC. THAT MAY BE NECESSARY. MAINTAIN EXISTING SITE OPERATIONS, COORDINATE WORK WITH NORTHEAST UTILITIES.
11. DRAWINGS INDICATE THE MINIMUM STANDARDS, BUT IF ANY WORK SHOULD BE INDICATED TO BE SUBSTANDARD TO ANY ORDINANCES, LAWS, CODES, RULES, OR REGULATIONS BEARING ON THE WORK, THE CONTRACTOR SHALL INCLUDE IN HIS WORK AND SHALL EXECUTE THE WORK CORRECTLY IN ACCORDANCE WITH SUCH ORDINANCES, LAWS, CODES, RULES OR REGULATIONS WITH NO INCREASE IN COSTS.
12. ALL DAMAGE CAUSED TO ANY EXISTING STRUCTURE SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR WILL BE HELD LIABLE FOR ALL REPAIRS REQUIRED FOR EXISTING STRUCTURES IF DAMAGED DURING CONSTRUCTION ACTIVITIES.
13. NO DRILLING WELDING OR TAPING ON CL&P OWNED EQUIPMENT

DESIGNED BY: EA	DATE: 07/10/10
DRAWN BY: JZ	SCALE: AS SHOWN
CHECKED BY: JZ	SHEET NO. 0001.005
PROJECT: T-MOBILE	PROJECT NO. 10256
CL&P STRUCTURE 10256	CT11-110C
1 CHANEY DRIVE	NEWTON, CT 06458


STRUCTURAL NOTES

SHEET NO. **N-1**

DATE: 07/10/10

SCALE: AS SHOWN

SHEET NO. 0001.005

DESIGNED BY: _____ DRAWN BY: _____ CHECKED BY: _____	DATE: 08/10/10 SCALE: AS SHOWN JOB NO. 10031.005	T-MOBILE PROJECTS CONSULTANTS INCORPORATED CL&P STRUCTURE 10256 CT11-1100			STRUCTURAL NOTES N-3
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13. INSPECTION AND TESTING OF CONCRETE WORK SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY, APPROVED AND PAID BY THE OWNER. THE INSPECTOR SHALL OBSERVE THE CONDITION OF SOILS AND FORMWORK BEFORE FOOTINGS ARE PLACED, SIZE, SPACING AND LOCATION OF REINFORCEMENT, AND PLACEMENT OF CONCRETE.
14. THE TESTING COMPANY SHALL ALSO OBTAIN A MINIMUM OF THREE (3) COMPRESSIVE STRENGTH TEST SPECIMENS FOR EACH CONCRETE MIX DESIGN, ONE SPECIMEN TESTED AT 7 DAYS, ONE AT 28 DAYS, AND ONE HELD IN RESERVE FOR FUTURE TESTING, IF NEEDED.
15. FOUR COPIES OF ALL INSPECTION TEST REPORTS SHALL BE SUBMITTED TO THE OWNER WITHIN TEN (10) WORKING DAYS OF THE DATE OF INSPECTION.

FOUNDATION CONSTRUCTION:

1. FOOTINGS SHALL BE PLACED ON SUITABLE, COMPACTED SOIL HAVING ADEQUATE BEARING CAPACITY AND FREE OF ORGANIC CONTENT, CLAY, OR OTHER UNSUITABLE MATERIAL.
2. IF UNSUITABLE SOIL IS ENCOUNTERED, NOTIFY OWNER IMMEDIATELY.
3. FOOTING SUBGRADE SHALL BE INSPECTED BY OWNER OR OWNER'S REPRESENTATIVE. GIVE 48 HOURS NOTICE.
4. ALL SOIL SURROUNDING AND UNDER ALL FOOTINGS SHALL BE KEPT REASONABLY DRY AND PROTECTED FROM FREEZING AND FROST ACTION DURING THE COURSE OF CONSTRUCTION.
5. WHERE GROUNDWATER IS ENCOUNTERED, DEWATERING SHALL BE ACCOMPLISHED CONTINUOUSLY AND COMPLETELY DURING FOUNDATION CONSTRUCTION. PROVIDE CRUSHED STONE AS REQUIRED TO STABILIZE FOOTING SUBGRADE.

SQUARE MESH SIEVES	PERCENT PASSING BY WEIGHT
PASS 5"	100
PASS 3 1/2"	90-100
PASS 1 1/2"	55-95
PASS 3/4"	25-60
PASS 1/4"	15-45
PASS #10	5-25
PASS #40	0-10
PASS #100	0-5
PASS #200	

7. PER NEC REQUIREMENTS, THE REBAR IN FOUNDATION AND FOOTING SHALL BE BONDED TO GROUND RING WITH A #2 AWG SOLID CONDUCTOR USING LISTED AND APPROVED METHODS.
8. PROVIDE CONTINUOUS GRID RINGS OF 3 NO. 8 STRANDED COPPERWELD BURIED 18" BELOW GRADE AND 2'-0" FROM FOUNDATION.

CONCRETE CONSTRUCTION:

1. CONCRETE CONSTRUCTION SHALL CONFORM TO THE FOLLOWING STANDARDS:
 ACI 211 - STANDARD PRACTICE FOR SELECTING PROPORTIONS FOR NORMAL AND HEAVYWEIGHT CONCRETE.
 ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS.
 ACI 302 - GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION
 ACI 304 - RECOMMENDED PRACTICE FOR MEASURING, MIXING, TRANSPORTING, AND PLACING CONCRETE.
 ACI 306.1 - STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING
 ACI 318 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
 CONCRETE SHALL BE AIR ENTRAINED AND SHALL DEVELOP COMPRESSIVE STRENGTH IN 28 DAYS AS FOLLOWS:
 ALL CONCRETE 3,500 PSI
3. REINFORCING STEEL SHALL BE 60,000 PSI YIELD STRENGTH.
4. ALL DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS, UNLESS OTHERWISE NOTED, MUST FOLLOW THE LATEST ACI CODE AND LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES.
5. CONCRETE COVER OVER REINFORCING SHALL BE 3 INCHES.
6. NO STEEL WIRE, METAL FORM TIES, OR ANY OTHER METAL SHALL REMAIN WITHIN THE REQUIRED COVER OF ANY CONCRETE SURFACE.
7. ALL REINFORCEMENT SHALL BE CONTINUOUS. SPLICES WILL NOT BE ALLOWED.
8. NO TACK WELDING OF REINFORCING WILL BE PERMITTED.
9. NO CALCIUM CHLORIDE OR ADMIXTURES CONTAINING MORE THAN 1 % CHLORIDE BY WEIGHT OF ADMIXTURE SHALL BE USED IN THE CONCRETE.
10. TOP OF FOOTING SURFACES SHALL RECEIVE A UNIFORM FLOAT FINISH. CURE FOOTING SURFACE WITH SONNEBORN KURE-N-SEAL WB OR APPROVED EQUAL, APPLIED AS RECOMMENDED BY MANUFACTURER.
11. PREPARATION OF SURFACES WHERE NEW CONCRETE WILL INTERFACE WITH EXISTING CAISSON: THE PERIMETER OF THE EXISTING CAISSON SHALL BE THOROUGHLY CLEANED OF ALL DIRT AND DELETERIOUS MATERIALS PRIOR TO APPLICATION OF BONDING AGENT. CONTRACTOR SHALL NOTIFY NORTHEAST UTILITIES 24 HOURS IN ADVANCE OF CLEANING.
 SIKADUR 32, HI-MOD OR ENGINEER APPROVED EQUAL SHALL BE APPLIED, IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS, TO ALL INTERFACING SURFACES BEFORE CONCRETE IS PLACED.
 CAULK JOINT BETWEEN EXISTING CONCRETE PIER AND NEW CONCRETE WITH SIKAFLEX 1-A BY SIKA CORP. OR ENGINEER APPROVED EQUAL.
 SUBMIT MANUFACTURER'S PRODUCT SPECIFICATION DATA AND INSTALLATION INSTRUCTIONS FOR REVIEW AND APPROVAL BY OWNER.
12. NEW CONCRETE FOOTING SHALL BE ALLOWED TO CURE AT LEAST 14 DAYS BEFORE WIRELESS ANTENNA MOUNT, ANTENNAS, AND CABLES ARE INSTALLED.

Connecticut Market



Worst Case Power Density

Site: CT11110C
Site Address: 8 Chimney Drive CL&P P# 321 L#1770
Town: Bethel
Tower Height: 145 ft.
Tower Style: Utility Lattice Tower

GSM Data		UMTS Data	
Base Station TX output	20 W	Base Station TX output	40 W
Number of channels	8	Number of channels	2
Antenna Model	APX16DWV-16DWV	Antenna Model	APX16DWV-16DWV
Cable Size	1 5/8 in.	Cable Size	1 5/8 in.
Cable Length	185 ft.	Cable Length	180 ft.
Antenna Height	162.0 ft.	Antenna Height	154.0 ft.
Ground Reflection	1.6	Ground Reflection	1.6
Frequency	1945.0 MHz	Frequency	2.1 GHz
Jumper & Connector loss	4.50 dB	Jumper & Connector loss	1.50 dB
Antenna Gain	18.0 dBi	Antenna Gain	18.0 dBi
Cable Loss per foot	0.0116 dB	Cable Loss per foot	0.0116 dB
Total Cable Loss	2.1460 dB	Total Cable Loss	2.0880 dB
Total Attenuation	6.6460 dB	Total Attenuation	3.5880 dB
Total EIRP per Channel (In Watts)	54.36 dBm 273.17 W	Total EIRP per Channel (In Watts)	60.43 dBm 1104.74 W
Total EIRP per Sector (In Watts)	63.40 dBm 2185.34 W	Total EIRP per Sector (In Watts)	63.44 dBm 2209.48 W
nsg	11.3540	nsg	14.4120
Power Density (S) = 0.019701 mW/cm ²		Power Density (S) = 0.022130 mW/cm ²	
T-Mobile Worst Case % MPE =		4.1831%	

Equation Used :

$$S = \frac{(1000)(grf)^2 (Power) 10^{(nsg/10)}}{4\pi (R)^2}$$

Office of Engineering and Technology (OET) Bulletin 65, Edition 97-01, August 1997

Co-Location Total

Carrier	% of Standard
Verizon	
Cingular	
Sprint	
AT&T Wireless	
Pocket	
MetroPCS	
Nextel	
Other Antenna Systems	
Total Excluding T-Mobile	0.0000 %
T-Mobile	4.1831
Total % MPE for Site	4.1831%