



September 22, 2017

Melanie A. Bachman Acting Executive Director Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification for Sprint 2.5 Rework Crown Site BU: 841295

**Sprint Site ID: CT33XC515** 

719 Amity Road, Bethany, CT 06524

Latitude: 41° 26′ 33.93′′/ Longitude: -72° 59′ 32.86′′

Dear Ms. Bachman:

Sprint currently maintains three (3) antennas at the 131-foot level of the existing 150-foot monopole at 719 Amity Road in Bethany, CT. The tower is owned by Crown Castle. The property is owned by the Town of Bethany. Sprint intends to install three (3) antennas, three (3) RRHs, and one (1) hybrid cable.

This facility was approved by the Connecticut Siting Council in Docket No. 168 on June 6, 1995. This approval included the conditions that:

1. The self-supporting monopole tower shall be no taller than necessary to provide the proposed communication service and the tower shall not exceed a total height of 150 feet above ground level (AGL).

This modification complies with the aforementioned condition(s).

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to Ms. Derrylyn Gorski, First Selectman, Town of Bethany, Planning & Zoning, and Tax Collector, Town of Bethany, as the property owner.

- 1. The proposed modifications will not result in an increase in the height of the existing tower.
- 2. The proposed modifications will not require the extension of the site boundary.

- 3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
- 5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

# Sincerely,

Jeffrey Barbadora Real Estate Specialist 12 Gill Street, Suite 5800, Woburn, MA 01801 781-729-0053 Jeff.Barbadora@crowncastle.com

# Attachments:

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc:

Ms. Derrylyn Gorski, First Selectman Bethany Town Hall 40 Peck Road Bethany, CT 06524 Tax Collector
Bethany Town Hall
40 Peck Road
Bethany, CT 06524

Planning & Zoning Bethany Town Hall 40 Peck Road Bethany, CT 06524 DOCKET NO. 168 - An application of Springwich Cellular Limited Partnership for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance, and operation of a cellular telecommunications facility located on the former site of the Bethany Airport, 719 Amity Road (Route 63) in Bethany, Connecticut.

ConnecticutSitingCouncil

} July 6, 1995

# **DECISION AND ORDER**

Pursuant to the foregoing Findings of Fact, and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a cellular telecommunications tower and equipment building at the proposed site in Bethany, Connecticut, including effects on the natural environment; ecological integrity and balance, public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate either alone or cumulatively with other effects when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes § 16-50k, be issued to Springwich Cellular Limited Partnership (Springwich), for the construction, operation, and maintenance of a cellular telecommunications tower, associated equipment, and building at the proposed site located at the Bethany Airport, 719 Amity Road, Bethany, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

- 1. The self-supporting monopole tower shall be no taller than necessary to provide the proposed communications service and the tower shall not exceed a total height of 150 feet above ground level (AGL).
- 2. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D&M Plan shall be submitted to and approved by the Council prior to the commencement of facility construction and shall include detailed plans for the tower location and tower foundation; the placement of all antennas to be attached to this tower; equipment building, access road, utility line, and security fence; site clearing and tree trimming; and water drainage and erosion and sedimentation controls consistent with the Connecticut Guidelines for Soil Erosion and Sedimentation Control, as amended.
- Upon the establishment of any new State or federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.
- 4. The Certificate Holder shall provide the Council a recalculated report of electromagnetic radio frequency power density if and when circumstances in operation cause a change in power density above the levels originally calculated and provided in the application.

Decision & Order Docket No. 168 July 6, 1995 Pg. 2

- 5. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
- 6. If the facility does not initially provide, or permanently ceases to provide cellular services following completion of construction, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapplication for any continued or new use shall be made to the Council before any such use is made.
- 7. Unless otherwise approved by the Council, this Decision and Order shall be void if all construction authorized herein is not completed within three years of the effective date of this Decision and Order or within three years after all appeals to this Decision and Order have been resolved.
- 8. The Certificate Holder shall notify the Council upon completion of construction and provide the final cost to construct the facility.

Pursuant to General Statutes § 16-50p, we hereby direct that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in <u>The New Haven Register</u> and <u>Beth-Wood News</u>.

By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

The parties and intervenors to this proceeding are:

# **APPLICANT**

Springwich Cellular Limited Partnership

# INTERVENOR

Metro Mobile CTS of Hartford, Inc.

# ITS REPRESENTATIVES

Peter J. Tyrrell, Esq. Springwich Cellular Limited Partnership 227 Church Street New Haven, CT 06510

# ITS REPRESENTATIVES

Metro Mobile CTS of Hartford, Inc. 20 Alexander Drive Wallingford, CT 06492 Attn: David S. Malko, P.E., Manager Engineering & Regulatory Services

Robinson & Cole One Commercial Plaza Hartford, CT 06103-3597 Attn: Brian C.S. Freeman, Esq.

# **CERTIFICATION**

The Undersigned members of the Connecticut Siting Council (Council) hereby certify that they have heard this case, or read the record thereof, in Docket No. 168 - An application of Springwich Cellular Limited Partnership for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance, and operation of a cellular telecommunications facility located on the former site of the Bethany Airport, 719 Amity Road (Route 63) in Bethany, Connecticut, and voted as follows:

Council Members	Vote Cast
Nortune 4. Seleta	YES
Mortimer A. Gelston	
Chairman	
Commissioner Reginald J. Smith Designee: Gerald J. Heffernan	YES
Commissioner Sidney J. Holbrook Designee: Fred Riese	YES
William J. Huber	YES
	ABSENT
Gloria Dibble Pond William H. Smith	YES
Colin C. Tait	ABSTAIN
Edward S. Wilensky	ABSENT
Dana J. Wright	YES
v - V	

Dated at New Britain, Connecticut, July 6, 1995.

STATE OF CONNECTICUT }

ss. New Britain, Connecticut }

COUNTY OF HARTFORD

STATE OF CONNECTICUT } July 7, 1995

I hereby certify that the foregoing is a true and correct copy of the Findings of Fact, Opinion, and Decision and Order issued by the Connecticut Siting Council, State of Connecticut.

ATTEST:

Joel M. Rinebold Executive Director

Connecticut Siting Council

I certify that a copy of the Findings of Fact, Opinion, and Decision and Order in Docket No. 168 have been forwarded by Certified First Class Return Receipt Requested mail on July 7, 1995, to all parties and intervenors of record as listed on the attached service list, dated April 10, 1995.

ATTEST:

Gloria B. Owens

Administrative Assistant Connecticut Siting Council The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2013.



Information on the Property Records for the Municipality of Bethany was last updated on 9/20/2017.

# **Property Summary Information**

Parcel Data And Values Building Outbuildings Google Map

# **Parcel Information**

Location:	755 AMITY RD	Property Use:	Public Use	Primary Use:	Fire Station - Volunteer
Unique ID:	00016500	Map Block Lot:	117/1	Acres:	138.50
490 Acres:	0.00	Zone:	B&I	Volume / Page:	0044/0306
Developers Map / Lot:		Census:			

# **Value Information**

	Appraised Value	70% Assessed Value
Land	1,476,000	1,033,200
Buildings	1,740,164	1,218,110

	Appraised Value	70% Assessed Value
Detached Outbuildings	159,624	111,740
Total	3,375,788	2,363,050

# **Owner's Information**

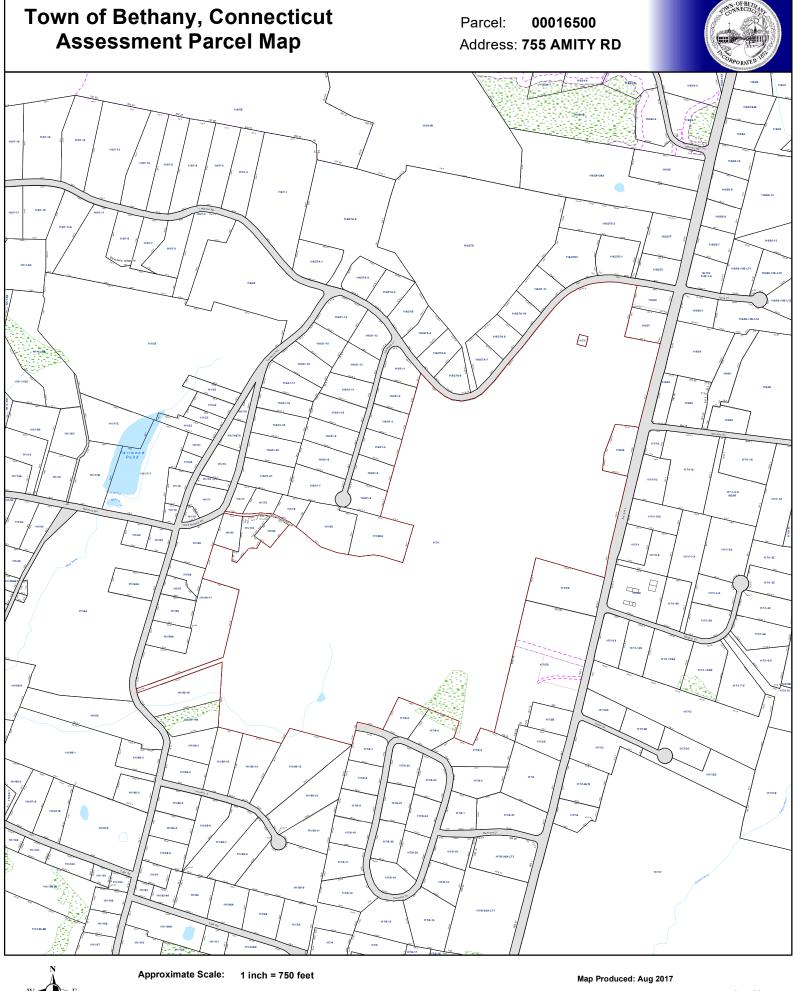
Owner's Data

BETHANY TOWN OF 40 PECK RD BETHANY CT 06524

Back To Search (JavaScript:window.history.back(1);)

Print View (PrintPage.aspx?towncode=008&uniqueid=00016500)

Information Published With Permission From The Assessor





SITE NUMBER:

CT33XC515

BETHANY-SPECTRASITE TOWER

719 AMITY ROAD BETHANY, CT 06524

# **APPROVED**

FOR CONNECTICUT

By Susan Vale at 9:01 am, Dec 05, 2014

6580 SPRINT PARKWAY **OVERLAND PARK, KANSAS 66251** 



# TECTONIC

Phone: (845) 567-6656 Fax: (845) 567-8703

# SUBMITTALS PROJECT NO: 7225.CT33XC5I5 NO DATE DESCRIPTION 0 07/08/14 PER COMMENTS 12/04/14 FOR CONSTRUCTION DO



SITE NUMBER: CT33XC515

SITE NAME: BETHANY-SPECTRASITE TOWER

SITE ADDRESS

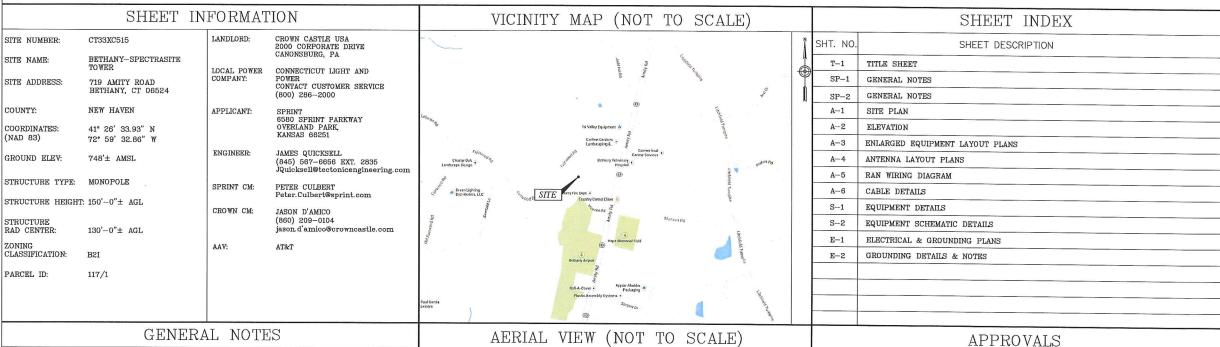
719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

TITLE SHEET

SHEET NO:

T-1



# GENERAL NOTES

- THIS IS AN UNMANNED TELECOMMUNICATION FACILITY AND NOT FOR HUMAN HABITATION: HANDICAP ACCESS REQUIREMENTS ARE NOT REQUIRED.
  FACILITY HAS NO PLUMBING OR REFRIGERANTS.
  THIS FACILITY SHALL MEET OR EXCEED ALL FAA AND FCC REGULATOR REQUIREMENTS.
- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
- DEVELOPMENT AND USE OF THIS SITE WILL CONFORM TO ALL APPLICABLE CODES
  - 2005 STATE OF CONNECTICUT BUILDING CODE.

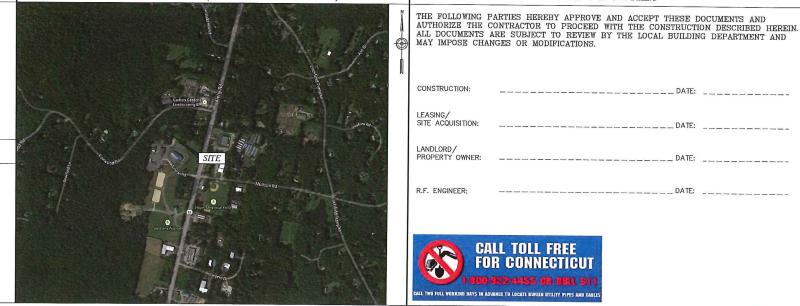
CROWN ID#: 841295

CROWN SITE NAME: BETHANY

ANSI/TIA/EIA-222-F-1996.
NATIONAL ELECTRICAL CODE, LATEST EDITION.

# PROJECT DESCRIPTION

- (1) NEW 2.5 EQUIPMENT RACK INSIDE EXIST MMBTS CABINET.
- 2. (3) NEW RFS APXVTM14-C-120 ANTENNAS.
- 3. (3) NEW TD-RRH8x20-25 RRH.
- . (1) NEW 5/8" FIBER CABLE.



### DIVISION 01000-GENERAL NOTES

- 1. THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY, MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS. AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES.
- 2. THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN . THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONTRACT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMNESSIONS OF ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF THESE CONTRACTOR FORM THESE DOCUMENTS.
- 3. THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE PROJECT OWNER'S REPRESENTATIVE OF ANY CONFLICTS, ERRORS, OR OMISSIONS PRIOR TO THE SUBMISSION OF CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK.
- 4. THE SCOPE OF WORK SHALL INCLUDE FURNISHING ALL MATERIALS, EQUIPMENT, LABOR AND ALL OTHER MATERIALS AND LABOR DEEMED NECESSARY TO COMPLETE THE WORK/PROJECT AS DESCRIBED HEREIN.
- 5. THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO I. HE CONTRACTOR SHALL WIST THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERTEY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 6. ONCE THE CONTRACTOR HAS RECEIVED AND ACCEPTED THE NOTICE TO PROCEED, CONTRACTOR WILL CONTACT THE CROWN CASTLE CONSTRUCTION MANAGER OF RECORD (NOTED ON THE FIRST PAGE ON THIS MANAGER OF RECORD (NOTED ON THE FIRST PAGE ON THIS
  CONSTRUCTION DRAWING) A MINIMUM OF 48 HOURS PRIOR TO WORK
  START. UPON ARRIVAL TO THE JOB SITE, CONTRACTOR CREW IS REQUIRED
  TO CALL 1-800-788-7011 TO NOTIFY THE CROWN CASTLE NOC WORK HAS REGUN.
- 7. THE CONTRACTOR SHALL INSTALL ALL FOUIPMENT AND MATERIALS ACCORDING TO THE MANUFACTURER'S/VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES TAKE PRECEDENCE.
- 8. THE CONTRACTOR SHALL PROVIDE A FULL SET OF CONSTRUCTION DOCUMENTS AT THE SITE UPDATED WITH THE LATEST REVISIONS AND ADDENDUMS OR CLARIFICATIONS AVAILABLE FOR THE USE BY ALL PERSONNEL INVOLVED WITH THE PROJECT.
- 9, THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL. CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER
- 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS WHICH MAY BE REQUIRED FOR THE WORK BY THE ARCHITECT/ENGINEER, THE STATE, COUNTY OR LOCAL GOVERNMENT
- 11. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS, EASEMENTS, PAVING, CURBING, ETC. DURING CONSTRUCTION. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL REPAIR ANY DAMAGE THAT MAY HAVE OCCURRED DUE TO CONSTRUCTION ON OR ABOUT THE PROPERTY.
- 12. THE CONTRACTOR SHALL KEEP THE GENERAL WORK AREA CLEAN AND HAZARD FREE DURING CONSTRUCTION AND DISPOSE OF ALL DIRT, DEBRIS, RUBBISH AND REMOVE EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY. PREMISES SHALL BE LEFT IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF ANY NATURE.
- 13. THE CONTRACTOR SHALL COMPLY WITH ALL PERTINENT SECTIONS OF THE THE CONTRACTOR SHALL COMPLY WITH ALL PERTINENT SECTIONS OF THE BASIC STATE BUILDING CODE, LATEST EDITION, AND ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJECT. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK SHALL BE RELOCATED AS DIRECTED BY THE ARCHITECT/ENGINEER. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR PIER DRILLING AROUND OR NEAR UTILITIES. THE CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT SAFETY INAINING FOR THE WORKING CREW. HIS WILL INCLUDE BUT NOT LIMITED TO A) FALL PROTECTION, B) CONFINED SPACE, C) ELECTRICAL SAFETY, D) TRENCHING AND EXCAVATION OF ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHICH INTERFERE WITH THE EXECUTION OF THE WORK SHALL BE REMOVED AND OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT THE POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK SUBJECT TO THE APPROVAL OF THE ARCHITECT/ENGINEER.
- 14. THE CONTRACTOR SHALL NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT
  DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR
  CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE LESSEE/LICENSEE REPRESENTATIVE.
- 15. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, PROPERTY LINES, ETC. ON THE JOB.
- 16. THE CONTRACTOR SHALL NOTIFY THE THE RF ENGINEER FOR ANTENNA AZIMUTH VERIFICATION (DURING ANTENNA INSTALLATION) PRIOR TO
- 17. THE CONTRACTOR SHALL SUBMIT AT THE END OF THE PROJECT A COMPLETE SET OF AS-BUILT DRAWINGS TO THE CLIENT REPRESENTATIVE.
- 18. REFER TO: CONSTRUCTION STANDARDS-SPRINT DOCUMENT EXHIBIT -STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES REV.
- 19. REFER TO: WEATHER PROOFING SPECS: EXCERPT EXH A-WIHRPRF-STD CONSTR SPECS.\_157201110421855492.DOCM.
- 20, REFER TO: COLOR CODING-SPRINT NEXTEL ANT AND LINE COLOR CODING (DRAFT) V3 09-08-11.PDF
- 21. REFER TO LATEST DOCUMENTATION REVISION.

- 18. REFER TO: CONSTRUCTION STANDARDS—SPRINT DOCUMENT EXHIBIT A—STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES REV. 4.0- 02.15,2011.DOCM.
- 19. REFER TO: WEATHER PROOFING SPECS: EXCERPT EXH A-WIHRPRF-STD CONSTR SPECS.\_157201110421855492,DOCM.
- 20. REFER TO: COLOR CODING-SPRINT NEXTEL ANT AND LINE COLOR CODING (DRAFT) V3 09-08-11.PDF
- 21. REFER TO LATEST DOCUMENTATION REVISION.

# DIVISION 03000-CONCRETE

# 1.03 APPLICABLE STANDARDS (USE LATEST EDITIONS)

- AC1-301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS. ACI-347 GUIDE TO FORM WORK FOR CONCRETE. ASTM C33- CONCRETE AGGREGATE
- ASTM C94 READY MIXED CONCRETE e. ASTM C150 PORTLAND CEMENT.
- ASTM C260 AIR—ENTRAINING ADMIXTURES FOR CONCRETE
  ASTM C309— LIQUID MEMBRANE FORMING COMPOUNDS FOR CURING CONCRETE,
- ASTM C494 CHEMICAL ADMIXTURES FOR CONCRETE
  ASTM A615— DEFORMED AND PLAIN BILLET—STEEL BARS FOR CONCRETE REINFORCEMENT J. ASTM A185- STEEL WELDED WIRE FABRIC (PLAIN) FOR CONCRETE REINFORCEMENT

### 1.04 QUALITY ASSURANCE

CONCRETE MATERIALS AND OPERATIONS SHALL BE TESTED AND INSPECTED BY THE ARCHITECT/ENGINEER AS DIRECTED BY THE CLIENT'S REPRESENTATIVE.

A. SURFACES AGAINST WHICH BACKFILL OR CONCRETE SHALL BE PLACED REQUIRE NO TREATMENT EXCEPT REPAIR OF DEFECTIVE

B. SURFACES THAT WILL BE PERMANENTLY EXPOSED SHALL PRESENT A UNIFORM FINISH PROVIDED BY THE REMOVAL OF FINS AND THE FILLING HOLES AND OTHER IRREGULARITIES WITH DRY PACK GROUT, OR BY SACKING WITH UTILITY OR ORDINARY GROUT.

C. SURFACES THAT WOULD NORMALLY BE LEVEL AND WHICH WILL BE PERMANENTLY EXPOSED TO THE WEATHER SHALL BE SLOPED FOR DRAINAGE. UNLESS ENGINEER'S DESIGN DRAWING SPECIFIES A HORIZONTAL SURFACE OR SURFACES SUCH AS STAIR TREADS, WALLS, CURBS, AND PARAPETS SHALL BE SLOPED APPROXIMATELY 1/4" PER FOOT.

D. SURFACES THAT WILL BE COVERED BY BACKFILL OR CONCRETE SHALL BE SMOOTH SCREENED.

EXPOSED SLAB SURFACES SHALL BE CONSOLIDATED, SCREENED, FLOATED, AND STEEL TROWELED. HAND OR POWER-DRIVEN EQUIPMENT MAY BE USED FOR FLOATING. FLOATING SHALL BE STATIED AS SOON AS THE SCREENED SURFACE HAS ATTAINED A STIFFNESS TO PERMIT FINISHING OPERATIONS. OPERATIONS. ALL EDGES MUST HAVE A 3/4" CHAMFER.

1.04 QUALITY ASSURANCE CONCRETE MATERIALS AND OPERATIONS SHALL BE TESTED AND INSPECTED BY THE ENGINEER.

# 3.05 PATCHING

THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY UPON REMOVAL OF THE FORMS TO OBSERVE CONCRETE SURFACE CONDITIONS. IMPERFECTIONS SHALL BE PATCHED ACCORDING TO THE ENGINEER'S

# 3.06 DEFECTIVE CONCRETE

THE CONTRACTOR SHALL NOTIFY OR REPLACE CONCRETE NOT CONFORMING TO REQUIRED LEVELS AND LINES, DETAILS, AND ELEVATIONS AS SPECIFIED IN ACI 301.

A. IMMEDIATELY AFTER PLACEMENT. THE CONTRACTOR SHALL PROTECT THE CONCRETE FROM PREMATURE DRYING, EXCESSIVELY HOT OR COLD TEMPERATURES, AND MECHANICAL INJURY. FINISHED WORK

- B. CONCRETE SHALL BE MAINTAINED WITH MINIMAL MOISTURE LOSS AT RELATIVELY CONSTANT TEMPERATURE FOR PERIOD NECESSARY FOR HYDRATION OF CEMENT AND HARDENING OF CONCRETE
- C. ALL CONCRETE SHALL BE WATER CURED PER ACCEPTABLE PRACTICES SPECIFIED BY ACI CODE (LATEST EDITION)

## DIVISION 05000 - METALS

### PART 1 -- GENERAL

## 1.01 WORK INCLUDED

- A. THE WORK CONSISTS OF THE FABRICATION AND INSTALLATION OF ALL MATERIALS TO BE FURNISHED. AND WITHOUT LIMITING THE GENERALITY THEREOF, INCLUDING ALL EQUIPMENT, LABOR AND SERVICES REQUIRED FOR ALL STRUCTURAL STEEL WORK AND ALL ITEMS INCIDENTAL AS SPECIFIED AND AS SHOWN ON THE DRAWINGS:
- STEEL FRAMING INCLUDING BEAMS, ANGLES, CHANNELS AND PLATES.
- WELDING AND BOLTING OF ATTACHMENTS.

- THE WORK SHALL CONFORM TO THE CODES AND STANDARDS OF THE FOLLOWING AGENCIES AS FURTHER CITED HEREIN:
- ASTM: AMERICAN SOCIETY FOR TESTING AND MATERIALS AS PUBLISHED IN "COMPILATION OF ASTM STANDARDS IN BUILDING CODES" OR LATEST EDITION.
- AWS: AMERICAN WELDING SOCIETY CODE OR LATEST EDITION.
  AISC: AMERICAN INSTITUTE OF STEEL CONSTRUCTION, SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" (LATEST EDITION).

### PART 2 - PRODUCTS 2.01 MATERIALS

A. STRUCTURAL STEEL: SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A36 AND A992 FOR STRUCTURAL STEEL.

ALL PROPOSED STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH AISC CODE AND ASTM SPECIFICATIONS (LATEST EDITION) ALL NEW STEEL SHALL CONFORM TO THE FOLLOWING.

- STRUCTURAL WIDE FLANGE: ASTM A992 Fv=50KSL 2. MISCELLANEOUS STEEL (PLATES), CHANNELS, ANGLES, ETC): ASTM A36 (Fy=36KSI). 3.STRUCTURAL TUBING: ASTM A500 Gr. B (Fy=46KSI).
- 4. STEEL PIPE: ASTM A53 Gr B (Fy=35KSI).

### 2.02 WELDING

- A. ALL WELDING SHALL BE DONE BY CERTIFIED WELDERS CERTIFICATION DOCUMENTS SHALL BE MADE AVAILABLE FOR ENGINEER'S AND/OR OWNER'S REVIEW IF REQUESTED.
- WELDING ELECTRODES FOR MANUAL SHIELDED METAL ARC WELDING SHALL CONFORM TO ASTM 1-233, E70 SERIES. BARE ELECTRODES AND GRANULAR FLUX USED IN THE SUBMERGED ARC PROCESS SHALL CONFORM TO AISC SPECIFICATIONS.
- C. FIELD WELDING SHALL BE DONE AS PER AWS D1.1 REQUIREMENTS VISUAL INSPECTION IS ACCEPTABLE.
- STUD WELDING SHALL BE ACCOMPLISHED BY CAPACITOR DISCHARGE (CD) WELDING TECHNIQUE USING CAPACITOR DISCHARGE STUD WELDER.
- PROVIDE STUD FASTENERS OF MATERIALS AND SIZES SHOWN ON DRAWINGS OR AS RECOMMENDED BY THE MANUFACTURER FOR STRUCTURAL LOADINGS REQUIRED
- FOLLOW MANUFACTURERS SPECIFICATIONS AND INSTRUCTIONS TO PROPERLY SELECT AND INSTALL STUD WELDS.

# 2.03 BOLTING

- BOLTS SHALL BE CONFORMING TO ASTM A35 HIGH STRENGTH HOT DIP GALVANIZED WITH ASTM A153 HEAVY HEX TYPE NUTS.
- BOLTS SHALL BE 3/4" (MINIMUM) CONFORMING TO ASTM A325, HOT DIP GALVANIZED, ASTM A153 NUTS SHALL BE HEAVY HEX TYPE.
- ALL CONNECTIONS SHALL BE 2 BOLTS MINIMUM.
- EXCEPT WHERE SHOWN, ALL BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS TO BE DOUBLE ANGLED CONNECTIONS WITH HIGH STRENGTH BOLTS (THREADS EXCLUDED FROM SHEAR PLANE) AND
- E. STANDARD, OVERSIZED OR HORIZONTAL SHORT SLOTTED HOLES.
- SNUG--TIGHT STRENGTH BEARING BOLTS MAY BE USED IN STANDARD HOLES CONFORMING TO ACIS, USING THE TURN OF THE NUT METHOD.
- H. FULLY-TENSIONED HIGH STRENGTH (SLIP CRITICAL) SHALL BE USED IN OVERSIZED SLOT HOLES (RESPECTIVE OF SLOT ORIENTATION).
- ALL BRACED CONNECTION, MOMENT CONNECTION AND CONNECTIONS NOTED AS "SLIP CRITICAL" SHALL BE BE SLIP CRITICAL JOINTS WITH CLASS A SURFACE CONDITIONS, UNLESS OTHERWISE NOTED.
- J. EPOXY ANCHOR ASSEMBLIES SHALL BE AS MANUFACTURED BY HILTI OR ENGINEER APPROVED EQUAL, AS FOLLOWS

# BASE MATERIAL

# ANCHOR SYSTEM

CONCRETE

HOLLOW & CROUTED CMIL OR BRICK

HILTI HIT-HY 200

# 2.04 FABRICATION

A. FABRICATION OF STEEL SHALL CONFORM TO THE AISC AND AWS

### 2.05 FINISH

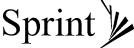
A. STRUCTURAL STEEL EXPOSED TO WEATHER SHALL BE HOT--DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. (LATEST EDITION) UNLESS OTHERWISE NOTED.

### 2.06 PROTECTION

A. UPON COMPLETION OF ERECTION, INSPECT ALL GALVANIZED STEEL AND PAINT ANY FIELD CUTS. WELDS OR GALVANIZED BREAKS WITH (2)
COATS OF ZINC-RICH COLD GALVANIZING PAINT.

### PART 3 - ERECTION

- A. PROVIDE ALL ERECTION, EQUIPMENT, BRACING, PLANKING, FIELD BOLTS, NUTS, WASHERS, DRIFT PINS, AND SIMILAR MATERIALS WHICH DO NOT FORM A PART OF THE COMPLETED CONSTRUCTION, BUT ARE NECESSARY FOR ITS PROPER ERECTION
- B. ERECT AND ANCHOR ALL STRUCTURAL STEEL IN ACCORDANCE WITH AISC REFERENCE STANDARDS. ALL WORK SHALL BE ACCURATELY SET TO ESTABLISHED SUITABLE ATTACHMENTS TO THE CONSTRUCTION OF THE BUILDING
- TEMPORARY BRACING, GUYING, AND SUPPORT SHALL BE PROVIDED TO KEEP THE STRUCTURE SET AND ALIGNED AT ALL TIMES DURING CONSTRUCTION, AND TO PREVENT DANGER TO PERSONS AND PROPERTY. CHECK ALL TEMPORARY LOADS AND STAY WITHIN SAFE CAPACITY OF ALL BUILDING COMPONENTS



6580 SPRINT PARKWAY **OVERLAND PARK, KANSAS 66251** 

# TECTONIC .

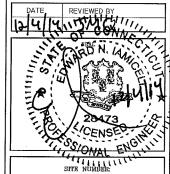
TECTONIC Engineering & Surveying Consultants P.C.

1279 Route 300 Newburgh, NY 12550 Phone: (845) 567-6656 Fax: (845) 567-8703

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# SUBMITTALS PROJECT NO: 7225.CT33XC5I5 NO DATE DESCRIPTION 0 07/08/14 PER COMMENTS I 12/04/14 FOR CONSTRUCTION



CT33XC515

SITE NAME: BETHANY-SPECTRASITE TOWER

SITE ADDRESS: 719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE: GENERAL NOTES

SHEET NO:

SP-1

# DIVISION 13000-SPECIAL CONSTRUCTION ANTENNA INSTALLATION

PART 1 - GENERAL

1.01 WORK INCLUDED

A. ANTENNAS AND HYBRIFLEX CABLES ARE FURNISHED BY CLIENT'S REPRESENTATIVE UNDER SEPARATE CONTRACT. THE CONTRACTOR SHALL ASSIST ANTENNA INSTALLATION CONTRACTOR IN TERMS OF COORDINATION AND SITE ACCESS. ERECTION SUBCONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPERTY

- INSTALL ANTENNAS AS INDICATED ON DRAWINGS AND CLIENT'S REPRESENTATIVE SPECIFICATIONS.
- INSTALL GALVANIZED STEEL ANTENNA MOUNTS AS INDICATED ON
- D. INSTALL FURNISHED GALVANIZED STEEL OR ALUMINUM WAVEGUIDE AND PROVIDE PRINTOUT OF THAT RESULT
- INSTALL HYBRIFLEX CABLES AND TERMINATIONS BETWEEN ANTENNAS AND EQUIPMENT PER MANUFACTURER'S RECOMMENDATIONS. WEATHERPROOF ALL CONNECTORS BETWEEN THE ANTENNA AND EQUIPMENT PER MANUFACTURER'S REQUIREMENTS.
- G. ANTENNA AND HYBRIFLEX CABLE GROUNDING:
- ALL EXTERIOR #6 GREEN GROUND WIRE DAISY CHAIN
  CONNECTIONS ARE TO BE WEATHER SEALED WITH ANDREWS CONNECTOR/SPLICE WEATHERPROOFING KIT TYPE 3221213 OR
- ALL HYBRIFLEX CABLE GROUNDING KITS ARE TO BE INSTALLED ON STRAIGHT RUNS OF HYBRIFLEX CABLE (NOT WITHIN BENDS). 1.02 RELATED WORK FURNISH THE FOLLOWING WORK AS SPECIFIED UNDER CONSTRUCTION DOCUMENTS, BUT COORDINATE WITH QOTHER
  - FLASHING OF OPENING INTO OUTSIDE WALLS.
- SEALING AND CAULKING ALL OPENINGS. PAINTING
- 4. CUTTING AND PATCHING.
- 1.03 REQUIREMENTS OF REGULATOR AGENCIES
- A. FURNISH U.L. LISTED EQUIPMENT WHERE SUCH LABEL IS AVAILABLE. INSTALL IN CONFORMANCE WITH U.L. STANDARDS WHERE APPLICABLE.
- INSTALL ANTENNA, ANTENNA CABLES, GROUNDING SYSTEM IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS IN EFFECT AT PROJECT LOCATION AND RECOMMENDATIONS OF STATE AND LOCAL BUILDING CODES HAVING JURISDICTION OVER SPECIFIC PORTIONS OF WORK. THIS WORK INCLUDES, BUT IS NOT LIMITED TO THE
- EIA ELECTRONIC INDUSTRIES ASSOCIATION RS-22. STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES.
- 2. FAA FEDERAL AVIATION ADMINISTRATION ADVISORY CIRCULAR AC 70/7480-IH, CONSTRUCTION MARKING AND LIGHTING.
- FCC FEDERAL COMMUNICATION COMMISSION RULES AND REGULATIONS FORM 715, OBSTRUCTION MARKING AND LIGHTING SPECIFICATION FOR ANTENNA STRUCTURES
- AISC AMERICAN INSTITUTE OF STEEL CONSTRUCTION FOR STRUCTURAL JOINTS USING ASTM 1325 OR A490 BOLTS.
- 5. NEC NATIONAL ELECTRIC CODE ON TOWER LIGHTING KITS.
- UL UNDERWRITER'S LABORATORIES APPROVED ELECTRICAL
- IN ALL CASES, PART 77 OF THE FAA RULES AND PARTS 17 AND 22 OF THE FCC RULES ARE APPLICABLE AND IN THE EVENT OF CONFLICT, SUPERSEDE ANY OTHER STANDARDS OR
- 8. LIFE SAFETY CODE NFPA, LATEST EDITION.

DIVISION 13000-EARTHWORK

PART 1 GENERAL

- WORK INCLUDED: REFER TO SURVEY AND SITE PLAN FOR WORK INCLUDED.
- 1.02 RELATED WORK
- CONSTRUCTION OF EQUIPMENT FOUNDATIONS
- INSTALLATION OF ANTENNA SYSTEM

PART 2 PRODUCTS

2.01 MATERIALS

- ROAD AND SITE MATERIALS; FILL MATERIAL SHALL BE ACCEPTABLE, SELECT FILL SHALL BE IN ACCORDANCE WITH LOCAL DEPARTMENT OF HIGHWAY AND PUBLIC TRANSPORTATION STANDARD SPECIFICATIONS.
- SOIL STERILIZER SHALL BE FPA REGISTERED OF HOURD COMPOSITION AND OF PRE-EMERGENCE DESIGN.
- SOIL STABILIZER FABRIC SHALL BE MIRAFI OR EQUAL 500X AT ACCESS ROAD AND COMPOUND.
- GRAVEL FILL; WELL GRADED, HARD, DURABLE, NATURAL SAND AND GRAVEL, FREE FROM ICE AND SNOW, ROOTS, SOD RUBBISH, AND OTHER DELETERIOUS OR ORGANIC MATTER.

MATERIAL SHALL CONFORM TO THE FOLLOWING GRADATION

GRAVEL FILL TO BE PLACED IN LIFTS OF 9" MAXIMUM THICKNESS AND 90 % DENSITY. COMPACTED TO 95

E. NO FILL OR EMBANKMENT MATERIALS SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OF EMBANKMENT

# 2.02 EQUIPMENT

- COMPACTION SHALL BE ACCOMPLISHED BY MECHANICAL MEANS. LARGER AREAS SHALL BE COMPACTED BY SHEEPS FOOT, VIBRATORY OR RUBBER TIED ROLLERS WEIGHING AT LEAST FIVE TONS. SMALLER AREAS SHALL BE COMPACTED BY POWER-DRIVER, HAND HELD TAMPERS.
- PRIOR TO OTHER EXCAVATION AND CONSTRUCTION EFFORTS GRUB ORGANIC MATERIAL TO A MINIMUM OF 6" BELOW ORIGINAL GROUND
- UNLESS OTHERWISE INSTRUCTED BY CLIENT'S REPRESENTATIVE, REMOVE TREES, BRUSH AND DEBRIS FROM THE PROPERTY TO AN AUTHORIZED DISPOSAL LOCATION.
- D. PRIOR TO PLACEMENT OF FILL OR BASE MATERIALS, ROLL THE SOIL.
- WHERE UNSTABLE SOIL CONDITIONS ARE ENCOUNTERED, LINE THE GRUBBED AREAS WITH STABILIZER MAT PRIOR TO PLACEMENT OF FILL. OR BASE MATERIAL.

# 3.03 INSTALLATION

- THE SITE AND TURNAROUND AREAS SHALL BE AT THE SUB-BASE COURSE ELEVATION PRIOR TO FORMING FOUNDATIONS. GRADE OR FILL THE SITE AND ACCESS ROAD AS REQUIRED TO PRODUCE EVEN DESCRIPTION OF THE PROPERTY OF THE STREAM O DISTRIBUTION OF SPOILS RESULTING FROM FOUNDATION EXCAVATIONS. THE RESULTING GRADE SHALL CORRESPOND WITH SAID SUB-BASE COURSE, ELEVATIONS ARE TO BE CALCULATED FORM FINISHED GRADES OR SLOPES INDICATED.
- THE ACCESS ROAD SHALL BE BROUGHT TO BASE COURSE ELEVATION PRIOR TO FOUNDATION CONSTRUCTION
- C. DO NOT CREATE DEPRESSIONS WHERE WATER MAY POND.
- THE CONTRACT INCLUDES ALL NECESSARY GRADING, BANKING. DITCHING AND COMPLETE SURFACE COURSE FOR ACCESS ROAD.
  ALL ROADS OR ROUTES UTILIZED FOR ACCESS TO PUBLIC
  THOROUGHFARE IS INCLUDED IN SCOPE OF WORK UNLESS
- WHEN IMPROVING AN EXISTING ACCESS ROAD GRADE THE EXISTING ROAD TO REMOVE ANY ORGANIC MATTER AND SMOOTH THE SURFACE BEFORE PLACING FILL OR STONE.
- PLACE FILL OR STONE IN 3" MAXIMUM LIFTS AND COMPACT BEFORE PLACING NEXT LIFT.
- THE FINISH GRADE, INCLUDING TOP SURFACE COURSE, SHALL EXTEND A MINIMUM OF 12" BEYOND THE SITE FENCE AND SHALL COVER THE AREA AS INDICATED.
- RIPRAP SHALL BE APPLIED TO THE SIDE SLOPES OF ALL FENCED AREAS, PARKING AREAS AND TO ALL OTHER SLOPES GREATER THAN 2:1.
- RIPRAP SHALL BE APPLIED TO THE SIDES OF DITCHES OR DRAINAGE SWALES AS INDICATED ON PLANS.
- RIPRAP ENTIRE DITCH FOR 6'-0" IN ALL DIRECTIONS AT CULVERT

- SEED, FERTILIZER AND STRAW COVER SHALL BE APPLIED TO ALL OTHER DISTURBED AREAS AND DITCHES, DRAINAGE, SWALES, NOT OTHERWISE RIP—RAPPED.
- UNDER NO CIRCUMSTANCES SHALL DITCHES, SWALES OR CULVERTS BE PLACED SO THEY DIRECT WATER TOWARDS, OR PERMIT STANDING WATER IMMEDIATELY ADJACENT TO SITE. IF OWNER DESIGNS OR IF DESIGN ELEVATIONS CONFLICT WITH THIS GUIDANCE ADVISE THE OWNER IMMEDIATELY.
- IF A DITCH LIES WITH SLOPE GREATER THAN TEN PERCENT, MOUND DIVERSIONARY HEADWALL IN THE DITCH AT CULVERT ENTRANCES. RIP—RAP THE UPSTREAM SIDE OF THE HEADWALL AS WELL AS THE DITCH FOR 6'-0" ABOVE THE CULVERT
- IF A DITCH LIES WITH SLOPES GREATER THAN TEN PERCENT, MOUND DIVERSIONARY HEADWALLS IN THE DITCH FOR 6'-0" ABOVE THE CULVERT ENTRANCE.
- SEED AND FERTILIZER SHALL BE APPLIED TO SURFACE CONDITIONS WHICH WILL ENCOURAGE ROOTING, RAKE AREAS TO BE SEEDED TO EVEN THE SURFACE AND TO LOOSEN THE SOIL.
- SOW SEED IN TWO DIRECTIONS IN TWICE THE QUANTITY RECOMMENDED BY THE SEED PRODUCER.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE GROWTH OF SEEDED AND LANDSCAPED AREAS BY WATERING UP TO THE POINT OF RELEASE FROM THE CONTRACT. CONTINUE TO REWORK BARE AREAS UNTIL COMPLETE COVERAGE IS OBTAINED.

### 3.04 FIELD QUALITY CONTROL

- A. COMPACTION SHALL BE D-1557 FOR SITE WORK AND 95 % MAXIMUM DENSITY UNDER SLAB AREAS. AREAS OF SETTLEMENT WILL BE EXCAVATED AND REFILLED AT CONTRACTOR'S EXPENSE. REQUIRED. USE OF EROSION CONTROL MESH OR MULCH NET SHALL BE AN ACCEPTABLE ALTERNATIVE.
- B. THE COMPACTION TEST RESULTS SHALL BE AVAILABLE PRIOR TO THE CONCRETE POUR.

- A. PROTECT SEEDED AREAS FORM EROSION BY SPREADING STRAW TO A UNIFORM LOOSE DEPTH OF 1"-2". STAKE AND TIE DOWN AS REQUIRED. USE OF EROSION CONTROL MESH OR MULCH NET SHALL BE AN ACCEPTABLE ALTERNATIVE.
- ALL TREES PLACED IN CONJUNCTION WITH A LANDSCAPE CONTRACT SHALL BE WRAPPED, TIED WITH HOSE PROTECTED WIRE AND SECURED TO STAKES EXTENDING 2'-O" INTO THE GROUND ON FOUR SIDES OF THE TREE.
- ALL EXPOSED AREAS SHALL BE PROTECTED AGAINST WASHOUTS AND SOIL EROSION, STRAW BALES SHALL BE PLACED AT THE INLET APPROACH TO ALL NEW OR EXISTING CULVERTS. REFER TO DETAILS ON DRAWINGS

SYMBOLS	ABBREVIATIONS
	GROUND WIRE
———E———E—	ELECTRIC
	TELEPHONE
CHE CHE CHE CHE	OVERHEAD WIRE
	PROPERTY LINE
_xxx	CHAIN LINK FENCE
A-1	ANTENNA MARK
(E)	EXISTING
(P)	PROPOSED DETAIL
DET #	REFERENCE
<b>♦</b>	SURFACE ELEVATION



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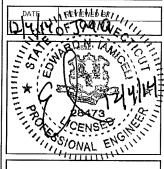
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	SU	JBMITTALS	
PRO	JECT NO:	7225.CT33XC5I5	
NO	DATE	DESCRIPTION	B)
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	12/04/14	FOR CONSTRUCTION	DO



SITE NUMBER: CT33XC515

SITE NAME: BETHANY-SPECTRASITE TOWER

SITE ADDRESS:

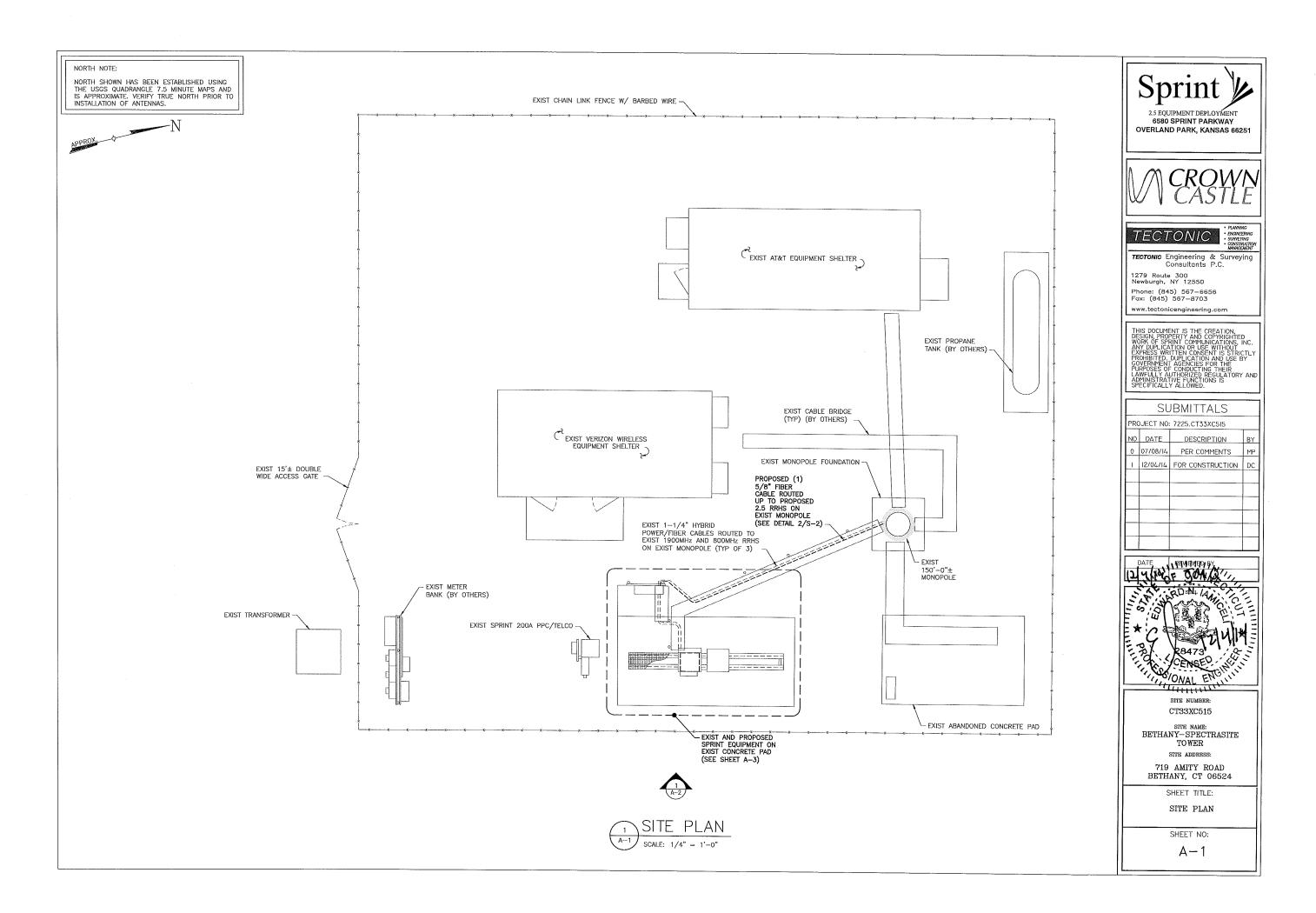
719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

GENERAL NOTES

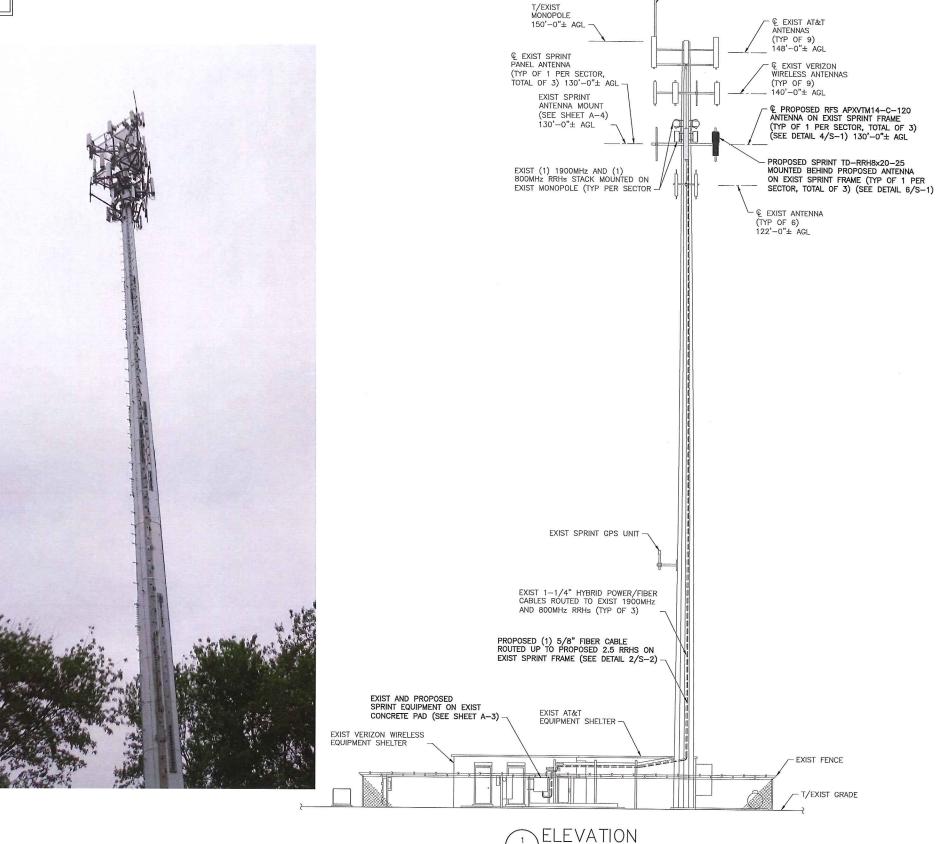
SHEET NO:

SP-2



THE PROPOSED INSTALLATIONS, EXISTING MOUNTS AND EXISTING MONOPOLE SHALL BE ANALYZED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONNECTICUT (TO BE COORDINATED BY OTHERS)

THE EXISTING MOUNT HAS BEEN
ANALYZED BY TECTONIC ENGINEERING
AND FOUND TO BE ADEQUATE TO
SUPPORT THE PROPOSED SPRINT
UPGRADE AS DETAILED IN THE
STRUCTURAL ANALYSIS EVALUATION
LETTER DATED 12/3/14.



SCALE: 1/8" = 1'-0"



- EXIST LIGHTNING ROD

2.5 EQUIPMENT DEPLOYMENT 6580 SPRINT PARKWAY OVERLAND PARK, KANSAS 66251



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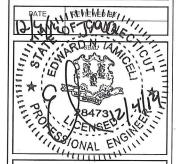
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	SL	JBMITTALS	
PRO	DJECT NO:	7225.CT33XC5I5	
NO	DATE	DESCRIPTION	BY
0	07/08/14	PER COMMENTS	MP
1	12/04/14	FOR CONSTRUCTION	DC
		20	



SITE NUMBER: CT33XC515

SITE NAME:
BETHANY-SPECTRASITE
TOWER
SITE ADDRESS:

719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

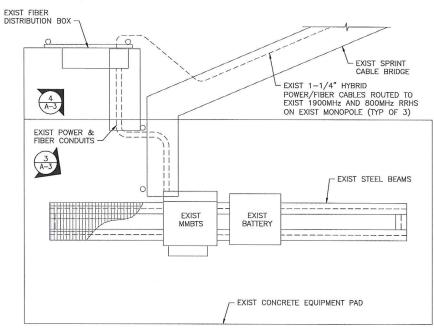
ELEVATION

SHEET NO:

NORTH NOTE:

NORTH SHOWN HAS BEEN ESTABLISHED USING THE USGS QUADRANGLE 7.5 MINUTE MAPS AND IS APPROXIMATE. VERIFY TRUE NORTH PRIOR TO INSTALLATION OF ANTENNAS.



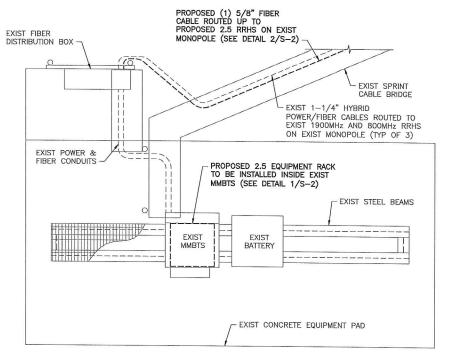


TENLARGED EQUIP. LAYOUT PLAN (EXIST)





EXIST EQUIPMENT PAD SCALE: NTS



ENLARGED EQUIP. LAYOUT PLAN (FINAL)



EXIST FIBER DISTRIBUTION BOX A-3 SCALE: NTS

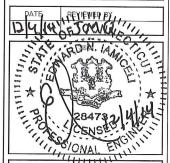


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SUBMITTALS PROJECT NO: 7225.CT33XC5I5 NO DATE DESCRIPTION 0 07/08/14 PER COMMENTS 12/04/14 FOR CONSTRUCTION DC



SITE NUMBER: CT33XC515

SITE NAME: BETHANY-SPECTRASITE TOWER

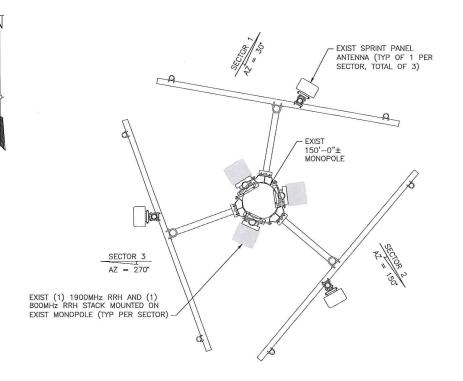
SITE ADDRESS:

719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

ENLARGED EQUIPMENT LAYOUT PLANS

SHEET NO:

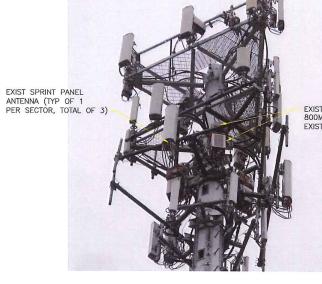


THE PROPOSED INSTALLATIONS, EXISTING MOUNTS AND EXISTING MONOPOLE SHALL BE ANALYZED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONNECTICUT (TO BE COORDINATED BY OTHERS)

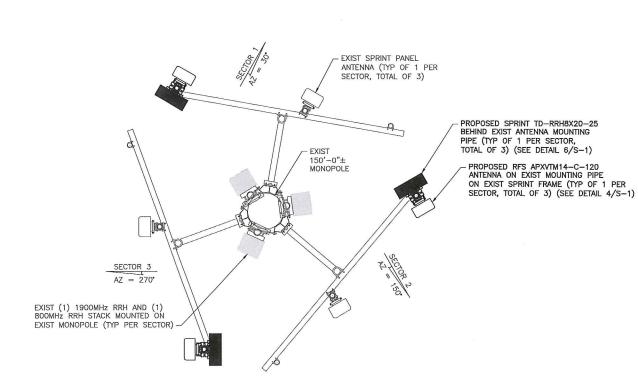
> THE EXISTING MOUNT HAS BEEN ANALYZED BY TECTONIC ENGINEERING AND FOUND TO BE ADEQUATE TO SUPPORT THE PROPOSED SPRINT UPGRADE AS DETAILED IN THE STRUCTURAL ANALYSIS EVALUATION

LETTER DATED 12/3/14.

EXIST (1) 1900MHz RRH AND (1) 800MHz RRH STACK MOUNTED ON EXIST MONOPOLE (TYP PER SECTOR)

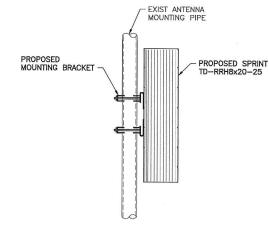


ANTENNA LAYOUT PLAN (EXIST) SCALE: 1/2" = 1'-0"



ANTENNA LAYOUT PLAN (FINAL)

SCALE: 1/2" = 1'-0"



# RRH MOUNTING DETAIL

# ANTENNA DATA

Status	Exist	Proposed
Antenna Manufacturer	RFS-CEL WAVE	RFS-CEL WAVE
<b>Antenna Model Number</b>	APXVSPP18-C-A20	APXVTM14-C-120
Number of Antennas	3	3
Antenna RAD Center	130'	130'
Antenna Azimuth	30/150/270	30/150/270
Antenna RRH Model Number	1900MHZ/800MHZ RRHs	TD-RRHx20-25
Number of RRH	6	3



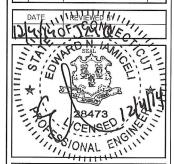
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1	12/04/14	FOR CONSTRUCTION	DO



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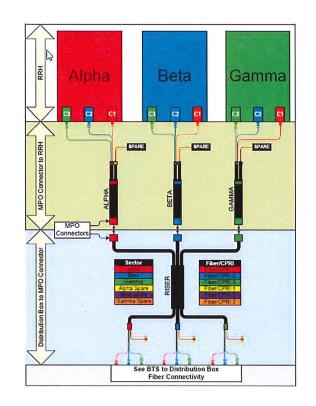
BETHANY-SPECTRASITE TOWER SITE ADDRESS:

719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

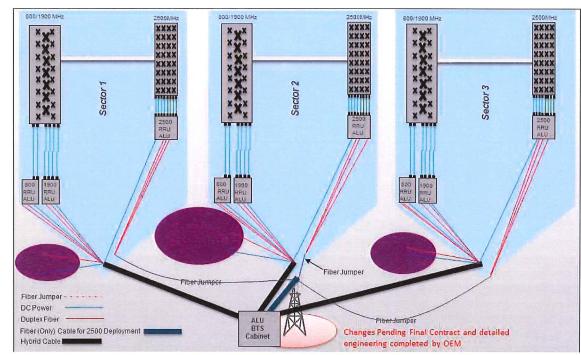
ANTENNA LAYOUT PLANS

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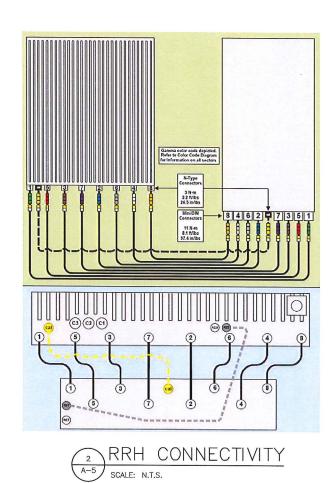


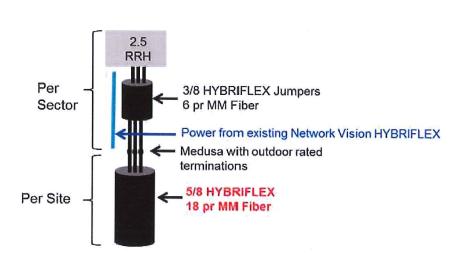
2.5 CABLE COLOR CODING

SCALE: N.T.S.















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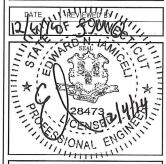
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SITE NAME: BETHANY—SPECTRASITE TOWER

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SHEET TITLE:

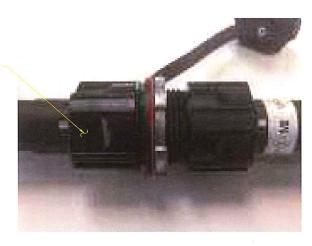
RAN WIRING DIAGRAM

SHEET NO:

IMPORTANTI! LINE UP WHITE MARKINGS ON JUMPER AND RISER IP—MPO CONNECTOR. PUSH THE WHITE MARK ON THE JUMPER CONNECTOR FLUSH AGAINST THE RED SEAL ON THE RISER CONNECTION



IMPORTANTII ROTATE THE BAYONET HOUSING CLOCKWISE UNTIL A CLICK SOUND IS HEARD TO ENSURE A GOOD CONNECTION



TRUNK-LINE TO JUMPER CONNECTION (MPO) TO BE INSTALLED PER MANUFACTURER REQUIREMENTS. SEE DETAIL. DC POWER BREAKOUT FIBER BREAKOUT BREAKOUTS TO RRH NEW 2.5 RRU -CABLE TERMINATION USE EXIST NV ENCLOSURE FURNISHED SPARE HYBRIFLEX WITH CABLE DC CONDUCTORS EXIST RRU - INSTALL (1) 1-1/4"ø HYBRID CABLE - INSTALL (1) 3/4"ø FIBER LINE

2.5 HYBRID CABLE W/FIBER & DC FEEDERS

FIBER ONLY TRUNK LINES

HYBRIFLEX RISER/JUMPER CONNECTION DETAILS

A-6 SCALE: N.T.S.



# SPECIAL NOTES: CABLE MARKINGS AT RAD CENTER AND ALL WALL/BLDG. PENETRATIONS

- $\bullet$  ALL COLOR CODE TAPE SHALL BE 3M-35 AND SHALL BE INSTALLED USING A MINIMUM OF (3) WRAPS OF TAPE.
- ALL COLOR BANDS INSTALLED AT THE TOWER TOP SHALL BE A MINIMUM OF 3" WIDE AND SHALL HAVE A MINIMUM OF 3/4" OF SPACING BETWEEN EACH COLOR.
- ALL COLOR BANDS INSTALLED AT OR NEAR THE GROUND MAY BE ONLY 3/4" WIDE. EACH TOP-JUMPER SHALL BE COLOR CORDED WITH (1) SET OF 3" WIDE BANDS.
- $\bullet$  Each main coax shall be color coded with (1) set of 3" bands near the TOP-JUMPER CONNECTION AND WITH 3/4" color bands just prior to entering the BTS or transmitter building.
- ALL BOTTOM JUMPERS SHALL BE COLOR CODED WITH (1) SET OF 3/4" BANDS ON EACH END OF THE BOTTOM JUMPER.
   ALL COLOR CODES SHALL BE INSTALLED SO AS TO ALIGN NEATLY WITH ONE ANOTHER
- FROM SIDE—TO—SIDE.

   EACH COLOR BAND SHALL HAVE A MINIMUM OF (3) WRAPS AND SHALL BE NEATLY
- TRIMMED AND SMOOTHED OUT AS TO AVOID UNRAVELING.

   X-POLE ANTENNAS SHOULD USE "XX-1" FOR THE "+45" PORT, "XX-2" FOR THE "-45"
- PORT.

  COLOR BAND #4 REFERS TO THE FREQUENCY BAND: ORANGE=850, VIOLET=1900. USED
- ON JUMPERS ONLY.

  RF FEEDLINE SHALL BE IDENTIFIED WITH A METAL TAG (STAINLESS OR BRASS) AND STAMPED WITH THE SECTOR, ANTENNA POSITION, AND CABLE NUMBER.
- ANTENNAS MUST BE IDENTIFIED, USING THE SECTOR LETTER AND ANTENNA NUMBER, WITH A BLACK MARKER PRIOR TO INSTALLATION.



6580 SPRINT PARKWAY

OVERLAND PARK, KANSAS 66251

# CASTLE

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 MANAGEMENT

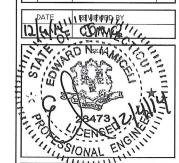
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# SUBMITTALS PROJECT NO: 7225.CT33XC5I5 NO DATE DESCRIPTION E 0 07/08/I4 PER COMMENTS I I 12/04/I4 FOR CONSTRUCTION I



SITE NUMBER: CT33XC515

SITE NAME:
BETHANY—SPECTRASITE
TOWER

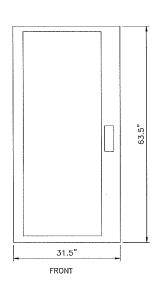
SITE ADDRESS:

719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

CABLE DETAILS

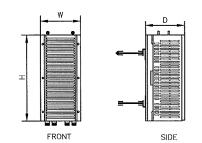
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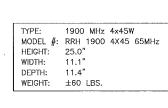


# 9927 MMBTS MODULAR CELL SPECIFICATIONS: HEIGHT: 63.5" WIDTH: 31.5" DEPTH: 38.0"

# (EXIST) MMBTS CABINET FRONT APXVSPP18-C-A20

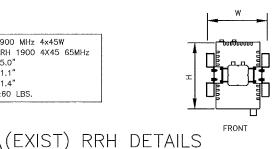
(EXIST) ANTENNA DETAILS

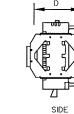


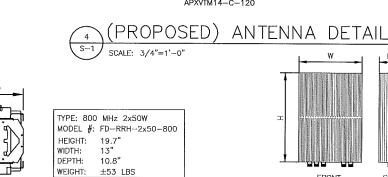


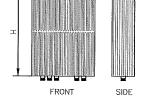
SCALE: 3/4"=1'-0"

SCALE: 1 1/2"=1'-0"



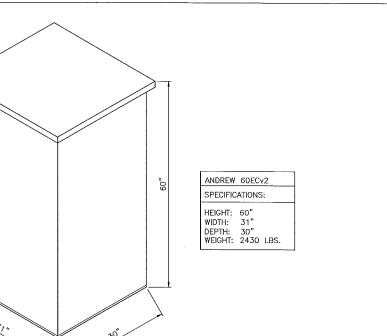




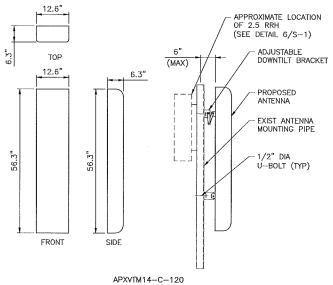


TYPE: 2.5 RRH
MODEL #: TD—RRH8x20—25
HEIGHT: 26.1"
WIDTH: 18.6" WIDTH: DEPTH: DEPTH: 6.71" WEIGHT: ±70 LBS

(PROPOSED) RRH DETAIL S-1 SCALE: N.T.S.



# (EXIST) BATTERY CABINET





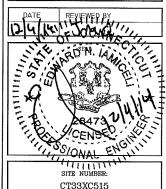
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Ι.					
	SUBMITTALS				
	PRO	JECT NO:	7225.CT33XC5I5		
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	1	12/04/14	FOR CONSTRUCTION	DC	



CT33XC515

SITE NAME: BETHANY-SPECTRASITE TOWER SITE ADDRESS:

719 AMITY ROAD BETHANY, CT 06524

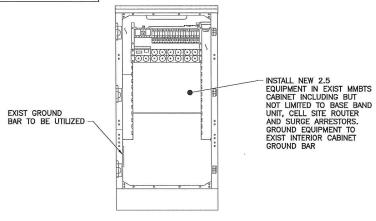
SHEET TITLE:

EQUIPMENT DETAILS

SHEET NO:

S-1

LOCATIONS SHOWN FOR INSTALLATION OF NEW
EQUIPMENT IN EXISTING CABINET ARE APPROXIMATE, ACTUAL SPACE AVAILABLE TO BE VERIFIED IN FIELD ON A SITE BY SITE BASIS.



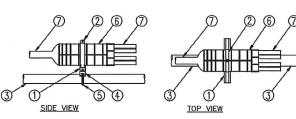
FRONT ELEVATION (CABINET INTERIOR)

# MMBTS INTERIOR DETAIL SCALE: N.T.S.

- LEGEND:
  1. P1000T-HG UNISTRUT,
  12" LONG,
  2. 6" PIPE HANGER.
  3. EXISTING SUPPORT PIPE.
  4. NEW STANDOFF BRACKET,
- ANDREW PART# 30848-4.

  NEW ROUND MEMBER
  ADAPTER SIZED FOR
  EXISTING PIPE SUPPORT.
  BREAKOUT UNIT.
  CABLE.







# RFS HYBRIFLEX RISER CABLES SCHEDULE

Fiber Only (Existing DC Power)	Hybrid cable MN: H8058-M12-050F 12x multi-mode fiber pairs, Top: Outdoor protected connectors, Bottom:LC Connectors, 5/8 cable, 50ft	50 ft
6 5	MN: HB058-M12-075F	75 ft
ber ng I	MN: HB058-M12-100F	100 ft
E its	MN:HB058-M12-125F	125 ft
ă	MN:HB058-M12-150F	150 ft
	MN:HB058-M12-175F	175 ft
	MN:HB058-M12-200F	200 ft

5	Hybrid cable MN: HB114-08U3M12-050F 3x 8 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC	50 ft
8 AWG Power	Connectors, 1 1/4 cable, 50ft MN: HB114-08U3M12-075F	75 ft
Ŋ	MN: HB114-08U3M12-100F	100 ft
ĕ	MN: HB114-08U3M12-125F	125 ft
	MN: HB114-08U3M12-150F	150 ft
	MN: HB114-08U3M12-175F	175 ft
	MN: HB114-08U3M12-200F	200 f

'G Power	Hybrid cable MN: HB114-13U3M12-225F 3x 6 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 1 1/4 cable, 225ft	225 ft
6 AWG	MN: HB114-13U3M12-250F	250 ft
9	MN: HB114-13U3M12-275F	275 ft
	MN: HB114-13U3M12-300F	300 ft

AWG Power	Hybrid cable MN: HB114-21U3M12-225F 3x 6 AWG power pairs, 12x multi-mode fiber pairs, Outdoor rated connectors & LC Connectors, 1.1/4 cable, 225ft	325 ft
4	MN: HB114-21U3M12-350F	350 ft
	MN: HB114-21U3M12-375F	375 ft

# RFS HYBRIFLEX JUMPER CABLE SCHEDULE

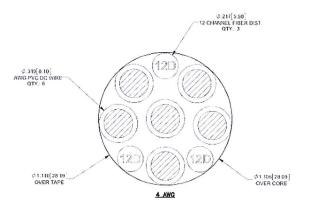
_	Hybrid Jumper cable MN: HBF012-M3-5F1	, 5 ft
Fiber Only	5 ft, 3x multi-mode fiber pairs, Outdoor & LC connectors, 1/2 cable	
ő	MN: HBF012-M3-10F1	10 ft
g	MN: HBF012-M3-15F1	15 ft
IL.	MN: HBF012-M3-20F1	20 ft
	MN: HBF012-M3-25F1	25 ft
	MN: HBF012-M3-30F1	30 ft

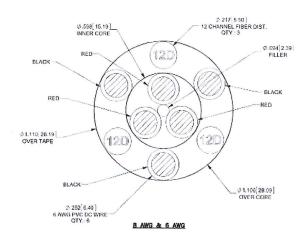
Power	Hybrid Jumper cable MN: HBF058-08U1M3-5F1 5ft, 1x 8 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	5 ft
ט	MN: HBF058-08U1M3-10F1	10 ft
8 AW	MN: HBF058-08U1M3-15F1	15 ft
∞	MN: HBF058-08U1M3-20F1	20 ft
	MN: HBF058-08U1M3-25F1	25 ft
	MN: HBF058-08U1M3-30F1	30 ft

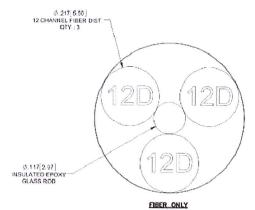
6 AWG Power	Hybrid Jumper cable MN: HBF058-13U3M3-5F1 5 ft, 1x 6 AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 5/8 cable	5 ft
9	MN: HBF058-13U1M3-10F1	10 ft
- ₹	MN: HBF058-13U1M3-15F1	15 ft
9	MN: HBF058-13U1M3-20F1	20 ft
	MN: HBF058-13U1M3-25F1	25 ft
	MN: HBF058-13U1M3-30F1	30 ft

	Hybrid Jumper cable	
	MN: HBF078-21U1M3-5F1	5 ft
ower	5ft, 1x 4AWG power pair, 3x multi-mode fiber pairs, Outdoor & LC Connectors, 7/8 cable	311
4 AWG Pov	MN: HBF078-21U1M3-10F1	10 ft
	MN: HBF078-21U1M3-15F1	15 ft
4	MN: HBF078-21U1M3-20F1	20 ft
	MN: HBF078-21U1M3-25F1	25 ft
	MN: HBF078-21U1M3-30F1	30 ft

HYBRID CABLE	DC CONDUCTO	OR SIZE GUIDELINE	
MANUF:	RFS		
<u>CABLE</u>	<u>LENGTH</u>	DC CONDUCTOR	CABLE DIAMETER
FIBER ONLY	VARIES	USE NV HYBRIFLEX	7/8"
HYBRIFLEX	<200'	8 AWG	1-1/4"
HYBRIFLEX	225-300'	6 AWG	1-1/4"
HYBRIFLEX	325-375'	4 AWG	1-1/4"













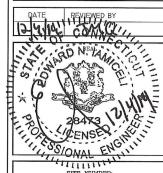
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PRO	JECT NO:	7225.CT33XC5I5	
NO	DATE	DESCRIPTION	BY
0	07/08/14	PER COMMENTS	MF
1	12/04/14	FOR CONSTRUCTION	DC



SITE NUMBER: CT33XC515

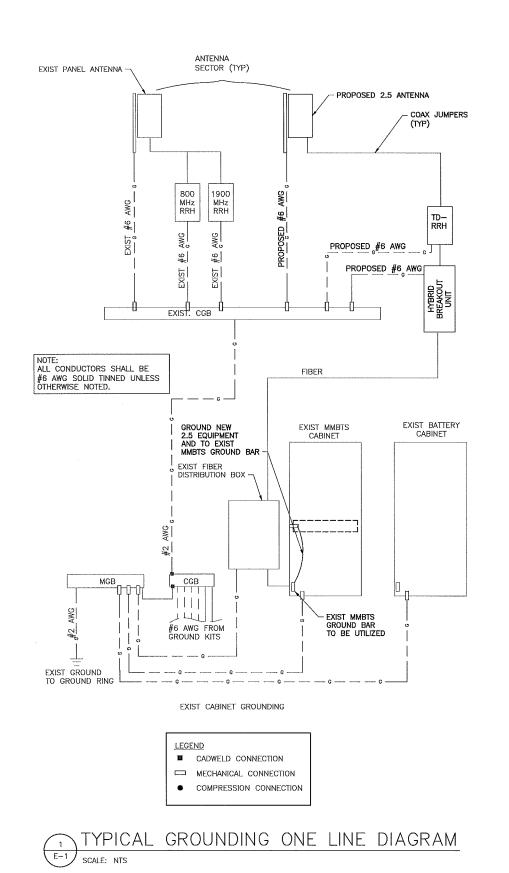
SITE NAME: BETHANY-SPECTRASITE TOWER SITE ADDRESS:

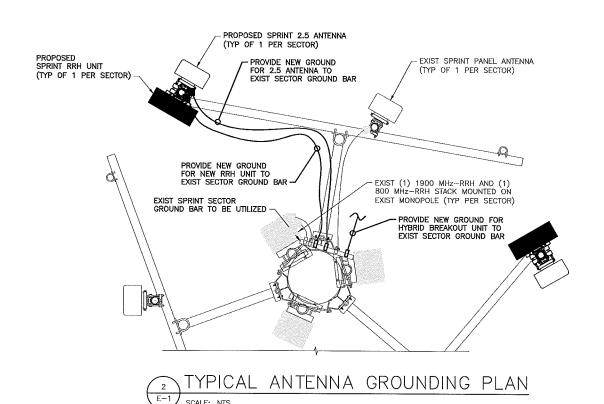
719 AMITY ROAD BETHANY, CT 06524

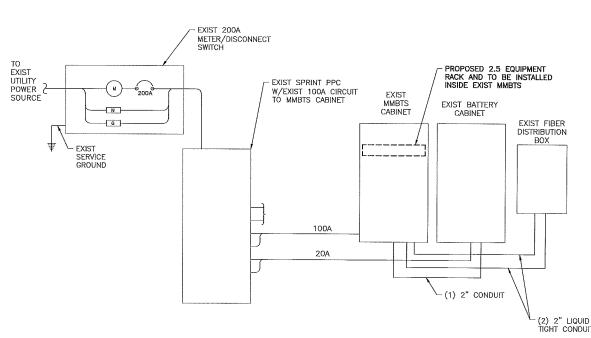
SHEET TITLE: EQUIPMENT SCHEMATIC DETAILS

SHEET NO:

S-2







TYPICAL ELECTRICAL & TELCO PLAN

SCALE: NTS



2.5 EQUIPMENT DEPLOYMENT 6580 SPRINT PARKWAY OVERLAND PARK, KANSAS 66251

# CASTLE

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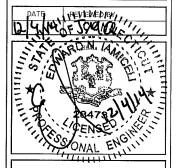
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SUBMITTALS									
PRO	PROJECT NO: 7225.CT33XC5!5								
NO	DATE	DESCRIPTION	BY						
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SITE NUMBER: CT33XC515

SITE NAME: BETHANY—SPECTRASITE TOWER

SITE ADDRESS:

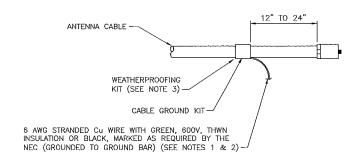
719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

ELECTRICAL & GROUNDING PLANS

SHEET NO:

E-1



### CONNECTION OF CABLE GROUND KIT TO ANTENNA CABLE

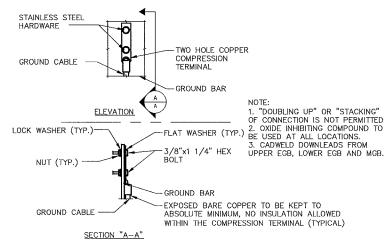
# NOTES:

DO NOT INSTALL CABLE GROUND KIT AT A BEND AND ALWAYS DIRECT GROUND WIRE DOWN TO GROUND BAR

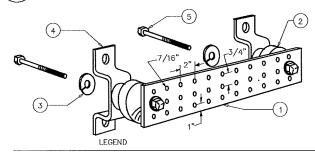
GROUNDING KIT SHALL BE TYPE AND PART NUMBER AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER.

WEATHER PROOFING SHALL BE (TYPE AND PART NUMBER) AS SUPPLIED OR RECOMMENDED BY CABLE MANUFACTURER AND APPROVED BY CONTRACTOR,

# CABLE GROUNDING KIT DETAIL



# GROUNDING BAR CONN. DETAIL SCALE: NTS



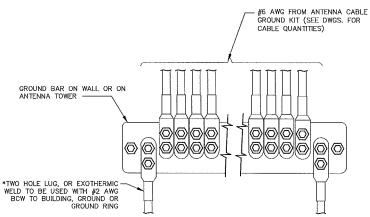
- 1- COPPER TINNED GROUND BAR. 1/4"X 4"X 20". OR OTHER LENGTH AS REQUIRED
- HOLE CENTERS TO MATCH NEMA DOUBLE LUG CONFIGURATION 2- INSULATORS, NEWTON INSTRUMENT CAT. NO. 3061-4 OR EQUAL
- 5/8" LOCKWASHERS OR EQUAL
- 4- WALL MOUNTING BRACKET, NEWTON INSTRUMENT CO. CAT NO. A-6056 OR EQUAL
- 5- 5/8-11 X 1" H.H.C.S.BOLTS

SCALE: NTS

ALL BOLTS, NUTS, WASHERS AND LOCK WASHERS SHALL BE 18-8

# E-2

GROUNDING BAR DETAIL



- GROUND BARS AT THE BOTTOM OF TOWERS/MONOPOLES SHALL ONLY USE EXOTHERMIC WELDS.
- ATTACH "DO NOT DISCONNECT" LABELS TO GROUND BARS, CAN USE BRASS TAG "DO NOT DISCONNECT" AT EACH HYBRID GROUND POINT OR BACK-A-LITE PLATE LABEL ON GROUND BAR.
- CONNECT SEQUENCE- BOLT/WASHER/NO-OX/GROUND BAR/NO-OX/WASHER/LOCK-WASHER/NUT. THIS IS REPEATED FOR EACH

# \ANTENNA GROUND BAR DETAIL

# **GROUNDING NOTES:**

- 1. GROUNDING SHALL BE IN ACCORDANCE WITH NEC ARTICLE 250-GROUNDING AND BONDING.
- 2. ALL GROUND WIRES SHALL BE #2 AWG UNLESS NOTED OTHERWISE.
- 3. ALL GROUNDING WIRES SHALL PROVIDE A STRAIGHT, DOWNWARD PATH TO GROUND WITH GRADUAL BENDS AS REQUIRED. GROUND WIRES SHALL NOT BE LOOPED OR SHARPLY BENT.
- 4. EACH EQUIPMENT CABINET SHALL BE CONNECTED TO THE MASTER ISOLATION GROUND BAR (MGB) WITH #2 AWG INSULATED STRANDED COPPER WIRE, EQUIPMENT CABINETS WALL HAVE (2)
- 5. PROVIDE DEDICATED #2 AWG COPPER GROUND WIRE FROM EACH ANTENNA MOUNTING PIPE TO ASSOCIATED CIGBE.
- 6. THE CONTRACTOR SHALL VERIFY THAT THE EXISTING GROUND BARS HAVE ENOUGH SPACE/HOLES FOR ADDITIONAL TWO HOLE LUGS.
- 7. ALL CONDUITS SHALL BE RIGID GALVANIZED STEEL AND SHALL BE PROVIDED WITH
- 8. PROVIDE GROUND CONNECTIONS FOR ALL METALLIC STRUCTURES, ENCLOSURES, RACEWAYS AND OTHER CONDUCTIVE ITEMS ASSOCIATED WITH THE INSTALLATION OF CARRIER'S EQUIPMENT
- 9. WHEN CABLE LENGTH IS OVER 20' THE MANUFACTURERS GROUND KIT MUST BE INSTALLED PER THE MANUFACTURERS SPECIFICATIONS.
- REFER TO "ANTI-THEFT UPDATE TO SPRINT GROUNDING 082412,PDF" FOR GUIDELINE TO SUSPECTED OR ACTUAL THEFT OF GROUNDING.
- 11. HOME RUN GROUNDS ARE NOT APPROVED BY CROWN CASTLE CONSTRUCTION STANDARDS AND THAT ANTENNA BUSS BARS SHOULD BE INSTALLED DIRECTLY TO TOWER STEEL WITHOUT INSULATORS OR DOWN CONDUCTORS

# PROTECTIVE GROUNDING SYSTEM GENERAL NOTES:

- 1. AT ALL TERMINATIONS AT EQUIPMENT ENCLOSURES, PANEL, AND FRAMES OF EQUIPMENT AND WHERE EXPOSED FOR GROUNDING. CONDUCTOR TERMINATION SHALL BE PERFORMED UTILIZING TWO HOLE BOLTED TONGUE COMPRESSION TYPE LUGS WITH STAINLESS STEEL SELF-TAPPING SCREWS
- 2. ALL CLAMPS AND SUPPORTS USED TO SUPPORT THE GROUNDING SYSTEM CONDUCTORS AND PVC CONDUITS SHALL BE PVC TYPE (NON CONDUCTIVE). DO NOT USE METAL BRACKETS OR SUPPORTS WHICH WOULD FORM A COMPLETE RING AROUND ANY GROUNDING CONDUCTOR
- 3. ALL GROUNDING CONNECTIONS SHALL BE COATED WITH A COPPER SHIELD ANTI-CORROSIVE AGENT SUCH AS T&B KOPR SHIELD. VERIFY PRODUCT WITH PROJECT MANAGER
- 4. ALL BOLTS, WASHERS, AND NUTS USED ON GROUNDING CONNECTIONS SHALL BE STAINLESS STEEL.
- 5. INSTALL GROUND BUSHING ON ALL METALLIC CONDUITS AND BOND TO THE EQUIPMENT GROUND
- 6. GROUND ANTENNA BASES, FRAMES, CABLE RACKS, AND OTHER METALLIC COMPONENTS WITH #2 INSULATED TINNED STRANDED COPPER GROUNDING CONDUCTORS AND CONNECT TO INSULATED SURFACE MOUNTED GROUND BARS. CONNECTION DETAILS SHALL FOLLOW MANUFACTURER'S SPECIFICATIONS FOR GROUNDING.
- 7. GROUND HYBRID CABLE SHIELD AT BOTH ENDS USING MANUFACTURER'S GUIDELINES.

# ELECTRICAL AND GROUNDING NOTES

- 1. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC) AS WELL AS APPLICABLE STATE AND LOCAL CODES.
- 2. ALL ELECTRICAL ITEMS SHALL BE U.L. APPROVED OR LISTED AND PROCURED PER
- ELECTRICAL AND TELCO WIRING OUTSIDE A BUILDING AND EXPOSED TO WEATHER SHALL BE IN WATER TIGHT GALVANIZED RIGID STEEL CONDUITS OR SCHEDULE 80 PVC (AS PERMITTED BY CODE) AND WHERE REQUIRED IN LIQUID TIGHT FLEXIBLE METAL OR NONMETALLIC CONDUITS.
- 4. BURIED CONDUIT SHALL BE SCHEDULE 40 PVC.
- 5. ELECTRICAL WIRING SHALL BE COPPER WITH TYPE XHHW, THWN, OR THNN
- 6. RUN TELCO CONDUIT OR CABLE BETWEEN TELEPHONE UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE TELCO CABINET AND BTS CABINET AS INDICATED ON THIS DRAWING PROVIDE FULL LENGTH PULL ROPE IN INSTALLED TELCO CONDUIT. PROVIDE GREENLEE CONDUIT MEASURING TAPE AT EACH END.
- 7. WHERE CONDUIT BETWEEN BTS AND PROJECT OWNER CELL SITE PPC AND BETWEEN BTS AND PROJECT OWNER CELL SITE TELCO SERVICE CABINET ARE UNDERGROUND USE PVC. SCHEDULE 40 CONDUIT. ABOVE THE GROUND PORTION OF THESE
- 8. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.
- 9. GROUNDING SHALL COMPLY WITH NEC ART. 250.
- 10. GROUND HYBRID CABLE SHIELDS AT 3 LOCATIONS USING MANUFACTURER'S HYBRID CABLE GROUNDING KITS SUPPLIED BY PROJECT OWNER.
- 11. USE #2 COPPER STRANDED WIRE WITH GREEN COLOR INSULATION FOR ABOVE GRADE GROUNDING (UNLESS OTHERWISE SPECIFIED) AND #2 SOLID TINNED BARE COPPER WIRE FOR BELOW GRADE GROUNDING AS INDICATED ON THE DRAWING
- 12. ALL GROUND CONNECTIONS TO BE BURNDY HYGROUND COMPRESSION TYPE CONNECTORS OR CADWELD EXOTHERMIC WELD. DO NOT ALLOW BARE COPPER WIRE TO BE IN CONTACT WITH GALVANIZED STEEL.
- 13. ROUTE GROUNDING CONDUCTORS ALONG THE SHORTEST AND STRAIGHTEST PATH POSSIBLE, EXCEPT AS OTHERWISE INDICATED. GROUNDING LEADS SHOULD NEVER BE BENT AT RIGHT ANGLE. ALWAYS MAKE AT LEAST 12" RADIUS BENDS. #2 WIRE CAN BE BENT AT 6" RADIUS WHEN NECESSARY, BOND ANY METAL OBJECTS WITHIN 6 FEET OF PROJECT OWNER EQUIPMENT OR CABINET TO MASTER GROUND BAR OR
- 14. CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO HOLE COMPRESSION TYPE COPPER LUGS. APPLY OXIDE INHIBITING COMPOUND TO ALL LOCATIONS
- 15. APPLY OXIDE INHIBITING COMPOUND TO ALL COMPRESSION TYPE GROUND
- 16. BOND ANTENNA MOUNTING BRACKETS, HYBRID CABLE GROUND KITS, AND RRHS TO EGB PLACED NEAR THE ANTENNA LOCATION.
- 17. BOND ANTENNA EGB'S AND MGB TO GROUND RING
- 18. CONTRACTOR SHALL TEST COMPLETED GROUND SYSTEM AND RECORD RESULT FOR PROJECT CLOSE-OUT DOCUMENTATION, 5 OHMS MINIMUM RESISTANCE REQUIRED.
- 19. CONTRACTOR SHALL CONDUCT ANTENNA HYBRID CARLES GPS COAX AND RRH RETURN-LOSS AND DISTANCE- TO-FAULT MEASUREMENTS (SWEEP TESTS) AND RECORD RESULTS FOR PROJECT CLOSE OUT.
- 20. CONTRACTOR SHALL CHECK CAPACITY OF EXISTING SERVICE & PANEL ON SITE TO DETERMINE IF CAPACITY EXISTS TO ACCOMMODATE THE ADDED LOAD OF THIS PROJECT. ADVISE ENGINEER OF ANY DISCREPANCY.
- 21. LOCATION OF ALL OUTLET, BOXES, ETC, AND THE TYPE OF CONNECTION (PLUG OR DIRECT) SHALL BE CONFIRMED WITH THE OWNER'S REPRESENTATIVE PRIOR TO
- 22. ELECTRICAL CHARACTERISTICS OF ALL EQUIPMENT (NEW AND EXISTING) SHALL BE FIELD VERIFIED WITH THE OWNERS REPRESENTATIVE AND EQUIPMENT SUPPLIER PRIOR TO ROUGH—IN OF CONDUIT AND WIRE. ALL EQUIPMENT SHALL BE PROPERLY CONNECTED ACCORDING TO THE NAMEPLATE DATA FURNISHED ON THE EQUIPMENT.



**OVERLAND PARK, KANSAS 66251** 



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NO	DATE	DESCRIPTION	BY
0	07/08/14	PER COMMENTS	MF
1	12/04/14	FOR CONSTRUCTION	DO
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SITE NUMBER: CT33XC515

BETHANY-SPECTRASITE TOWER

SITE ADDRESS:

719 AMITY ROAD BETHANY, CT 06524

SHEET TITLE:

GROUNDING DETAILS & NOTES

SHEET NO:

E-2



Date: August 10, 2017

Charles McGuirt Crown Castle 3530 Toringdon Way Suite 300 Charlotte, NC 28277 704-405-6607

Paul J. Ford and Company 250 East Broad Street, Suite 600 Columbus, Ohio 43215 nmiller@pjfweb.com 614-221-6679

Subject:

Structural Analysis Report

Carrier Designation:

**Sprint PCS Co-Locate** Carrier Site Number:

Carrier Site Name:

CT33XC515 CT33XC515

Crown Castle Designation:

Crown Castle BU Number: Crown Castle Site Name:

841295 **BETHANY** 

Crown Castle JDE Job Number: Crown Castle Work Order Number: **Crown Castle Application Number:** 

447377 1440119 396924 Rev. 4

Engineering Firm Designation:

Paul J. Ford and Company Project Number: 37517-0759.003.7805

Site Data:

719 AMITY ROAD, BETHANY, New Haven County, CT

Latitude 41° 26′ 33.93″, Longitude -72° 59′ 32.86″

151 Foot - Monopole Tower

Dear Charles McGuirt.

Paul J. Ford and Company is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1067159, in accordance with application 396924, revision 4.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment

Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

**Sufficient Capacity** 

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 125 mph converted to a nominal 3-second gust wind speed of 97 mph per Section 1609.3 and Appendix N as required for use in the ANSI/TIA-222-G-2005 Standard, "Structural Standard for Antenna Supporting Structures and Antennas", with ANSI/TIA-222-G-1-2007 and ANSI/TIA-222-G-2-2009 Addenda per Exception #5 of Section 1609.1.1. Risk Category II, Exposure Category C and Topographic Category 1 were used in this analysis.

We at Paul J. Ford and Company appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any otherwise please give us a call.

Respectfully submitted by:

Nathan C. Miller Structural Designer 525

Wathen C. Willer

tnxTower Report - version 7.0.5.1

AUG 11 2017



Date: August 10, 2017

Charles McGuirt Crown Castle 3530 Toringdon Way Suite 300 Charlotte, NC 28277 704-405-6607 Paul J. Ford and Company 250 East Broad Street, Suite 600 Columbus, Ohio 43215 nmiller@pjfweb.com 614-221-6679

Subject: Structural Analysis Report

Carrier Designation: Sprint PCS Co-Locate

Carrier Site Number: CT33XC515
Carrier Site Name: CT33XC515

Crown Castle BU Number: 841295

Crown Castle Site Name:BETHANYCrown Castle JDE Job Number:447377Crown Castle Work Order Number:1440119Crown Castle Application Number:396924 Rev. 4

Engineering Firm Designation: Paul J. Ford and Company Project Number: 37517-0759.003.7805

Site Data: 719 AMITY ROAD, BETHANY, New Haven County, CT

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# 1) INTRODUCTION

This tower is a 151 ft Monopole tower mapped by FDH in March of 2016.

# 2) ANALYSIS CRITERIA

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 125 mph converted to a nominal 3-second gust wind speed of 97 mph per Section 1609.3 and Appendix N as required for use in the ANSI/TIA-222-G-2005 Standard, "Structural Standard for Antenna Supporting Structures and Antennas", with ANSI/TIA-222-G-1-2007 and ANSI/TIA-222-G-2-2009 Addenda per Exception #5 of Section 1609.1.1. Risk Category II, Exposure Category C and Topographic Category 1 were used in this analysis.

**Table 1 - Proposed Antenna and Cable Information** 

Mounting Level (ft)	Elevetion	Number of Antennas	Antenna Manufacturer		Number of Feed Lines	Feed Line Size (in)	Note
		3	alcatel lucent	TD-RRH8x20-25			
130.0	130.0	3	rfs celwave	APXVTM14-C-120 w/ Mount Pipe	1	1-1/4	

Table 2 - Existing and Reserved Antenna and Cable Information

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note								
	160.0	1	dbspectra	DS1F03F36D-N	2	7/8	2								
		6	adc	CG-1900DD-FULL-DIN											
		6	communication components inc.	DTMABP7819VG12A											
		6	ericsson	RRUS-11											
		3	kathrein	800 10121 w/ Mount Pipe		2/0									
148.0	149.0	3	kathrein	860 10025	2 2	3/8 5/8	1								
					6	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe	12	1-5/8						
			12	powerwave technologies	LGP21901										
		1	raycap	DC6-48-60-18-8F	_										
		1	tower mounts	Platform Mount [LP 602-1]											
		3	alcatel lucent	RRH2x60-700											
		3	alcatel lucent	RRH4X45-AWS4 B66	1	1-5/8	2								
140.0	140.0	6 andrew	andrew	SBNHH-1D65B w/ Mount Pipe		1-5/6									
		3	decibel	DB854DG65ESX w/ Mount Pipe	12	1-5/8	1								
		1	rfs celwave	DB-T1-6Z-8AB-0Z			2								
		1	tower mounts	Platform Mount [LP 303-1]			1								

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
	132.0	1	tower mounts	Pipe Mount [PM 601-3]			
132.0		3	alcatel lucent	TME-1900MHz RRH			
	131.0	3	alcatel lucent	800 EXTERNAL NOTCH FILTER			1
		3	alcatel lucent	800MHZ RRH			
	133.0	1	pctel	GPS-TMG-HR-26NCM			
130.0	130.0	3	rfs celwave	APXVSPP18-C-A20 w/ Mount Pipe	1 3	1/2 1-5/8	1
		1	tower mounts	T-Arm Mount [TA 602-3]			
	122.0	1	tower mounts	T-Arm Mount [TA 702-3]			
122.0	400.0	3	ericsson	ERICSSON AIR 21 B2A B4P w/ Mount Pipe	13	1-5/8	1
	120.0	3	ericsson	ERICSSON AIR 21 B4A B2P w/ Mount Pipe			

Notes:

- 1) 2) Existing Equipment
- Reserved Equipment

**Table 3 - Design Antenna and Cable Information** 

Mounting Level (ft)	<b>-</b> 1	Number of Antennas	Antenna Manufacturer	Number of Feed Lines	

# 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided** 

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	FDH, 15BBNL1600, 2/18/2016	6133952	CCISITES
4-POST-MODIFICATION INSPECTION	B+T, 83154.004, 8/3/2012	5135928	CCISITES
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	FDH, 16BBMT1500, 2/17/2016	6133920	CCISITES
4-TOWER MAPPING	FDH, 16BBMW1500, 3/11/2016	6133951	CCISITES
4-TOWER REINFORCEMENT DESIGN/DRAWINGS/DATA	B+T, 84427.0002, 7/19/2012	4945157	CCISITES

# 3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

# 3.2) Assumptions

- Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. Paul J. Ford and Company should be notified to determine the effect on the structural integrity of the tower.

# 4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
L1	151 - 125.25	Pole	TP22.4079x17.59x0.2188	1	-8.19	1093.26	59.2	Pass
L2	125.25 - 118.5	Pole	TP23.6708x22.4079x0.3502	2	-10.13	1260.17	71.4	Pass
L3	118.5 - 117.75	Pole	TP23.8112x23.6708x0.6321	3	-10.30	2198.20	43.2	Pass
L4	117.75 - 97.5	Pole	TP27.6x23.8112x0.4541	4	-13.58	1813.76	85.5	Pass
L5	97.5 - 95	Pole	TP27.6303x26.0525x0.546	5	-15.48	2231.87	80.4	Pass
L6	95 - 92.5	Pole	TP28.0981x27.6303x0.7826	6	-16.29	3226.76	59.3	Pass
L7	92.5 - 87.75	Pole	TP28.9869x28.0981x0.5331	7	-17.47	2291.92	89.1	Pass
L8	87.75 - 84	Pole	TP29.6885x28.9869x0.8388	8	-18.82	3658.26	60.5	Pass
L9	84 - 63.25	Pole	TP33.571x29.6885x0.5695	9	-24.87	2893.18	98.3	Pass
L10	63.25 - 56.75	Pole	TP34.7872x33.571x0.5589	10	-26.89	2950.13	103.2	Pass <sup>2</sup>
L11	56.75 - 47.5	Pole	TP36.518x34.7872x0.6274	11	-28.55	3594.19	89.1	Pass
L12	47.5 - 34.25	Pole	TP38.372x34.4211x0.6672	12	-36.56	4118.71	92.4	Pass
L13	34.25 - 26.75	Pole	TP39.7752x38.372x0.6551	13	-39.57	4200.93	95.8	Pass
L14	26.75 - 16.75	Pole	TP41.6461x39.7752x0.6406	14	-43.69	4313.28	99.9	Pass
L15	16.75 - 14.25	Pole	TP42.1139x41.6461x0.7269	15	-44.85	4737.72	92.8	Pass
L16	14.25 - 0	Pole	TP44.78x42.1139x0.6927	16	-51.48	4866.66	98.3	Pass
							Summary	
						Pole (L10)	103.2	Pass <sup>2</sup>
						Rating =	103.2	Pass <sup>2</sup>

Table 6 - Tower Component Stresses vs. Capacity - LC7

Notes	Component	` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` ` `		Pass / Fail
1	Anchor Rods	0	83.7	Pass
1	Base Plate	0	87.8	Pass
1	Base Foundation Structural Steel	0	57.6	Pass
1	Base Foundation Soil Interaction	0	4.8	Pass

Structure Rating (max from all components) =	103.2% <sup>2</sup>
--	---------------------

Notes:

# 4.1) Recommendations

The monopole and its foundation have sufficient capacity to carry the proposed loading configuration. No modifications are required at this time.

See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

<sup>2)</sup> Capacities up to 105% are considered acceptable based on analysis methods used.

# APPENDIX A TNXTOWER OUTPUT

# **Tower Input Data**

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

- 1) Tower is located in New Haven County, Connecticut.
- ASCE 7-10 Wind Data is used (wind speeds converted to nominal values).
- 3) Basic wind speed of 97.00 mph.
- 4) Structure Class II.
- 5) Exposure Category C.
- 6) Topographic Category 1.
- 7) Crest Height 0.00 ft.
- 8) Nominal ice thickness of 0.7500 in.
- 9) Ice thickness is considered to increase with height.
- 10) Ice density of 56.00 pcf.
- A wind speed of 50.00 mph is used in combination with ice.
- 12) Temperature drop of 50.00 °F.
- Deflections calculated using a wind speed of 60.00 mph.
- 14) A non-linear (P-delta) analysis was used.
- 15) Pressures are calculated at each section.
- Stress ratio used in pole design is 1.
- Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

# **Options**

Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification

- √ Use Code Stress Ratios
- √ Use Code Safety Factors Guys Escalate Ice
   Always Use Max Kz
   Use Special Wind Profile

Include Bolts In Member Capacity

Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) SR Members Have Cut Ends SR Members Are Concentric Distribute Leg Loads As Uniform Assume Legs Pinned

- √ Assume Rigid Index Plate
- √ Use Clear Spans For Wind Area
  Use Clear Spans For KL/r
  Retension Guys To Initial Tension
- √ Bypass Mast Stability Checks
- √ Use Azimuth Dish Coefficients
- √ Project Wind Area of Appurt.

Autocalc Torque Arm Areas

Add IBC .6D+W Combination Sort Capacity Reports By Component Triangulate Diamond Inner Bracing Treat Feed Line Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation

√ Consider Feed Line Torque Include Angle Block Shear Check Use TIA-222-G Bracing Resist. Exemption
Use TIA-222-G Tension Splice

Poles

 Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets

Exemption

# **Tapered Pole Section Geometry**

Section	Elevation ft	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
		ft	ft	Sides	in	in	in	in	
L1	151.00-125.25	25.75	0.00	12	17.5900	22.4079	0.2188	0.8752	A572-65 (65 ksi)
L2	125.25-118.50	6.75	0.00	12	22.4079	23.6708	0.3502	1.4007	Reinf 42.26 ksi (42 ksi)
L3	118.50-117.75	0.75	0.00	12	23.6708	23.8112	0.6321	2.5283	Reinf 41.09 ksi (41 ksi)
L4	117.75-97.50	20.25	3.42	12	23.8112	27.6000	0.4541	1.8163	Reinf 41.27 ksi (41 ksi)
L5	97.50-95.00	5.92	0.00	12	26.0525	27.6303	0.5460	2.1841	Reinf 41.33 ksi (41 ksi)
L6	95.00-92.50	2.50	0.00	12	27.6303	28.0981	0.7826	3.1302	Reinf 41.34 ksi (41 ksi)
L7	92.50-87.75	4.75	0.00	12	28.0981	28.9869	0.5331	2.1324	Reinf 41.38 ksi (41 ksi)
L8	87.75-84.00	3.75	0.00	12	28.9869	29.6885	0.8388	3.3552	Reinf 41.40 ksi (41 ksi)
L9	84.00-63.25	20.75	0.00	12	29.6885	33.5710	0.5695	2.2779	Reinf 42.16 ksi

tnxTower Report - version 7.0.5.1

Section	Elevation ft	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	"	ft	ft	Sides	in	in	in	in	
									(42 ksi)
L10	63.25-56.75	6.50	0.00	12	33.5710	34.7872	0.5589	2.2358	Reinf 42.23 ksi (42 ksi)
L11	56.75-47.50	9.25	4.50	12	34.7872	36.5180	0.6274	2.5098	Reinf 44.76 ksi (45 ksi)
L12	47.50-34.25	17.75	0.00	12	34.4211	38.3720	0.6672	2.6686	Reinf 44.84 ksi (45 ksi)
L13	34.25-26.75	7.50	0.00	12	38.3720	39.7752	0.6551	2.6205	Reinf 44.89 ksi (45 ksi)
L14	26.75-16.75	10.00	0.00	12	39.7752	41.6461	0.6406	2.5623	Reinf 44.97 ksi (45 ksi)
L15	16.75-14.25	2.50	0.00	12	41.6461	42.1139	0.7269	2.9075	Reinf 43.13 ksi (43 ksi)
L16	14.25-0.00	14.25		12	42.1139	44.7800	0.6927	2.7709	Reinf 43.64 ksi (44 ksi)

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	ole Pro

Section	Tip Dia.	Area	1	r	С	I/C	J	It/Q	W	w/t
Section	in	in <sup>2</sup>	in⁴	in	in	in <sup>3</sup>	in⁴	in <sup>2</sup>	in	VV/ L
L1	18.2105	12.2386	471.3881	6.2189	9.1116	51.7348	955.1601	6.0235	4.1277	18.865
	23.1984	15.6330	982.4449	7.9437	11.6073	84.6403	1990.6997	7.6941	5.4189	24.767
L2	23.1984	24.8718	1544.5964	7.8967	11.6073	133.0712	3129.7711	12.2412	5.0668	14.469
	24.5059	26.2959	1825.3900	8.3488	12.2615	148.8717	3698.7351	12.9420	5.4053	15.436
L3	24.5059	46.8900	3176.7634	8.2479	12.2615	259.0845	6436.9839	23.0778	4.6498	7.357
	24.6511	47.1756	3235.1670	8.2981	12.3342	262.2927	6555.3255	23.2184	4.6874	7.416
L4	24.6511	34.1505	2378.0522	8.3618	12.3342	192.8017	4818.5786	16.8079	5.1645	11.374
	28.5736	39.6902	3733.1789	9.7182	14.2968	261.1199	7564.4326	19.5343	6.1799	13.61
L5	27.9149	44.8460	3724.0108	9.1313	13.4952	275.9506	7545.8555	22.0718	5.5187	10.107
	28.6050	47.6200	4458.7250	9.6962	14.3125	311.5265	9034.5858	23.4371	5.9416	10.881
L6	28.6050	67.6521	6224.2014	9.6115	14.3125	434.8785	12611.919	33.2963	5.3077	6.782
	20,0002	60 0200	GEEE 0407	0.7700	14 5540	450 2024	5 13282.700	22 0765	E 4220	6.943
	29.0893	68.8309	6555.2437	9.7790	14.5548	450.3831	13282.700	33.8765	5.4330	6.943
L7	29.0893	47.3167	4588.9869	9.8683	14.5548	315.2899	9298.5317	23.2878	6.1016	11.446
_,	30.0094	48.8423	5047.3312	10.1864	15.0152	336.1483	10227.261	24.0387	6.3398	11.893
					.0.0.02	00000	5		0.0000	
L8	30.0094	76.0270	7688.6548	10.0770	15.0152	512.0583	15579.299	37.4182	5.5205	6.581
							1			
	30.7358	77.9221	8278.0830	10.3282	15.3787	538.2840	16773.640	38.3509	5.7085	6.805
							5			
L9	30.7358	53.3954	5778.8880	10.4246	15.3787	375.7733	11709.593	26.2796	6.4303	11.292
							8			
	34.7553	60.5147	8412.3257	11.8146	17.3898	483.7508	17045.652	29.7835	7.4709	13.119
					4= 0000	.==	6	00 0 10 1		40.44=
L10	34.7553	59.4147	8264.6806	11.8183	17.3898	475.2604	16746.483	29.2421	7.4991	13.417
	20 04 4 4	04 0000	0040 4004	40.0507	40.0400	E44 0004	5 18666.399	20.2404	7 0050	14
	36.0144	61.6036	9212.1924	12.2537	18.0198	511.2264	4	30.3194	7.8250	14
L11	36.0144	69.0150	10279.215	12.2292	18.0198	570.4404	20828.478	33.9671	7.6414	12.179
LII	30.0144	09.0100	8	12.2232	10.0190	370.4404	0	33.307 1	7.0414	12.179
	37.8062	72.5117	11922.154	12.8488	18.9163	630.2574	24157.516	35.6881	8.1053	12.918
	0000_		0		.0.0.00		3		0000	
L12	36.6723	72.5121	10544.995	12.0839	17.8301	591.4139	21367.019	35.6883	7.4369	11.147
			6				9			
	39.7256	80.9995	14698.118	13.4983	19.8767	739.4656	29782.373	39.8655	8.4957	12.734
			6				1			
L13	39.7256	79.5644	14446.906	13.5026	19.8767	726.8271	29273.349	39.1592	8.5279	13.017
			5				3			
	41.1783	82.5245	16120.091	14.0050	20.6035	782.3942	32663.676	40.6160	8.9040	13.591
			9				5			
L14	41.1783	80.7217	15779.669	14.0102	20.6035	765.8716	31973.887	39.7288	8.9430	13.961
	40 4450	0.4.5000	2	44.0000	04 5707	044 4700	9	44.0004	0.4444	44 744
	43.1153	84.5808	18152.792	14.6800	21.5727	841.4706	36782.478	41.6281	9.4444	14.744
L15	12 1152	05 7724	3 20468.320	14.6491	21 5727	948.8066	7 41474.365	47.1363	0.2122	10 675
L15	43.1153	95.7724	20468.320	14.0491	21.5727	940.0000	6	41.1303	9.2132	12.675
			U				U			

Section	Tip Dia.	Area	1	r	С	I/C	J	It/Q	W	w/t
	in	in <sup>2</sup>	in⁴	in	in	in <sup>3</sup>	in⁴	in <sup>2</sup>	in	
	43.5995	96.8672	21178.281 8	14.8165	21.8150	970.8133	42912.940 6	47.6751	9.3385	12.848
L16	43.5995	92.3936	20233.558 8	14.8288	21.8150	927.5071	40998.675 6	45.4733	9.4300	13.613
	46.3597	98.3407	24397.506 3	15.7832	23.1960	1051.7962	49435.962 3	48.4003	10.1445	14.644

Tower Elevation ft	Gusset Area (per face) ft²	Gusset Thickness in	Gusset Grade Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Stitch Bolt Spacing
L1 151.00-			1	1	1			
125.25								
L2 125.25-			1	1	1			
118.50								
L3 118.50-			1	1	1			
117.75								
L4 117.75-			1	1	1			
97.50								
L5 97.50-			1	1	1			
95.00								
L6 95.00-			1	1	1			
92.50								
L7 92.50-			1	1	1			
87.75								
L8 87.75-			1	1	1			
84.00								
L9 84.00-			1	1	1			
63.25								
L10 63.25-			1	1	1			
56.75								
L11 56.75-			1	1	1			
47.50								
L12 47.50-			1	1	1			
34.25								
L13 34.25-			1	1	1			
26.75								
L14 26.75-			1	1	1			
16.75								
L15 16.75-			1	1	1			
14.25								
L16 14.25-			1	1	1			
0.00								

# Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face	Allow	Component	Placement	Total	Number	Clear	Width or	Perimete	Weight
	or	Shield	Type	ft	Number	Per Row	Spacing	Diamete	r	plf
	Leg						in	r	in	
								in		
***										
***										

# Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number		C <sub>A</sub> A <sub>A</sub> ft²/ft	Weight plf
1 1/4" Flat	С	No	CaAa (Out Of	38.70 - 0.00	2	No Ice	0.21	0.00
Reinforcement			Face)			1/2" Ice	0.32	0.00
	_					1" Ice	0.43	0.00
1 1/4" Flat	С	No	CaAa (Out Of	90.50 - 38.70	1	No Ice	0.21	0.00
Reinforcement			Face)			1/2" Ice	0.32	0.00
			,			1" Ice	0.43	0.00
1 1/4" Flat	С	No	CaAa (Out Of	67.50 - 52.50	1	No Ice	0.21	0.00

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number		C <sub>A</sub> A <sub>A</sub> ft²/ft	Weight plf
Reinforcement	Log		Face)			1/2" Ice	0.32	0.00
			/			1" Ice	0.43	0.00
1" Flat Reinforcement	С	No	CaAa (Out Of	120.50 - 90.50	1	No Ice	0.17	0.00
1 Tat Remioreement	0	110	Face)	120.00 00.00	•	1/2" Ice	0.28	0.00
			i ace)			1" Ice	0.39	0.00
1" Flat Dainfaraamant	_	No	CoAo (Out Of	07.00 02.00	4			
1" Flat Reinforcement	С	No	CaAa (Out Of	97.00 - 82.00	1	No Ice	0.17	0.00
			Face)			1/2" Ice	0.28	0.00
***						1" Ice	0.39	0.00
	_		0 4 (0 10)	100 50 110 50			0.00	0.00
Aero MP3-03	С	No	CaAa (Out Of	126.50 - 116.50	1	No Ice	0.26	0.00
			Face)			1/2" Ice	0.37	0.00
						1" Ice	0.48	0.00
***								
***								
***								
LDF2-50(3/8)	С	No	Inside Pole	148.00 - 0.00	2	No Ice	0.00	0.08
` ,						1/2" Ice	0.00	0.08
						1" Ice	0.00	0.08
LDF7-50A(1-5/8)	С	No	Inside Pole	148.00 - 0.00	12	No Ice	0.00	0.82
	•					1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
9776(5/8)	С	No	Inside Pole	148.00 - 0.00	2	No Ice	0.00	0.02
9770(3/6)	C	INO	IIISIUE FUIE	140.00 - 0.00	2	1/2" Ice	0.00	0.28
						1" Ice		
040004 004(7/0)	0	NI-	In alida Dala	440.00 0.00	0		0.00	0.28
810921-001(7/8)	С	No	Inside Pole	148.00 - 0.00	2	No Ice	0.00	0.40
						1/2" Ice	0.00	0.40
***						1" Ice	0.00	0.40
	_							
LDF7-50A(1-5/8)	С	No	Inside Pole	140.00 - 0.00	12	No Ice	0.00	0.82
						1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
HB158-1-08U8-	С	No	Inside Pole	140.00 - 0.00	1	No Ice	0.00	1.30
S8J18(1-5/8)						1/2" Ice	0.00	1.30
						1" Ice	0.00	1.30
***								
LDF4-50A(1/2)	С	No	Inside Pole	130.00 - 0.00	1	No Ice	0.00	0.15
						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
LDF7-50A(1-5/8)	С	No	Inside Pole	130.00 - 0.00	3	No Ice	0.00	0.82
	-				-	1/2" Ice	0.00	0.82
						1" Ice	0.00	0.82
HB114-21U3M12-	С	No	Inside Pole	130.00 - 0.00	1	No Ice	0.00	1.22
XXXF(1-1/4)	5	140	moide i die	100.00 - 0.00	'	1/2" Ice	0.00	1.22
//// (1-1/ <del>4</del> )						1" Ice	0.00	1.22
***						1 100	0.00	1.22
MI E Usbrid	С	No	Inside Pole	122.00 - 0.00	1	No Ice	0.00	1.07
MLE Hybrid	C	INO	IIISIUE POIE	122.00 - 0.00	ı			
9Power/18Fiber RL						1/2" Ice	0.00	1.07
2(1-5/8)	_		0 4 /0 / 5	100.00 0.00	_	1" Ice	0.00	1.07
AL7-50(1-5/8)	С	No	CaAa (Out Of	122.00 - 0.00	2	No Ice	0.20	0.52
			Face)			1/2" Ice	0.30	2.02
						1" Ice	0.40	4.14
AL7-50(1-5/8)	С	No	CaAa (Out Of	122.00 - 0.00	10	No Ice	0.00	0.52
			Face)			1/2" Ice	0.00	2.02
						1" Ice	0.00	4.14

# Feed Line/Linear Appurtenances Section Areas

Tower Sectio n	Tower Elevation ft	Face	A <sub>R</sub> ft²	A <sub>F</sub>	$C_AA_A$ In Face ${\it ft}^2$	C <sub>A</sub> A <sub>A</sub> Out Face ft²	Weight K
L1	151.00-125.25	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	0.000	0.328	0.44
L2	125.25-118.50	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	0.000	3.477	0.20
L3	118.50-117.75	Α	0.000	0.000	0.000	0.000	0.00

Tower	Tower	Face	A <sub>R</sub> ft <sup>2</sup>	$A_F$	$C_AA_A$	$C_A A_A$	Weight
Sectio	Elevation		ft <sup>2</sup>	2	In Face	Out Face	K
n	ft			ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	0.000	0.616	0.03
L4	117.75-97.50	Α	0.000	0.000	0.000	0.000	0.00
		B C	0.000	0.000	0.000	0.000	0.00
			0.000	0.000	0.000	11.641	0.68
L5	97.50-95.00	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	0.000	1.730	0.08
L6	95.00-92.50	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	1.813	0.08
L7	92.50-87.75	Α	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
	07.75.04.00	С	0.000	0.000	0.000	3.560	0.16
L8	87.75-84.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
1.0	04.00.00.05	C	0.000	0.000	0.000	2.876	0.13
L9	84.00-63.25	A	0.000	0.000	0.000	0.000	0.00
		B C	0.000	0.000	0.000	0.000	0.00
1.40	62 2E E6 7E		0.000	0.000	0.000	13.675	0.70
L10	63.25-56.75	A B	0.000	0.000	0.000	0.000	0.00
		С	0.000	0.000	0.000	0.000 5.256	0.00 0.22
1.44	56.75-47.50		0.000 0.000	0.000 0.000	0.000 0.000	0.000	0.22
L11	30.73-47.30	A B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	6.438	0.00
L12	47.50-34.25	A	0.000	0.000	0.000	0.000	0.00
LIZ	47.30-34.23	В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	8.881	0.45
L13	34.25-26.75	A	0.000	0.000	0.000	0.000	0.00
LIJ	34.23-20.73	В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	6.065	0.25
L14	26.75-16.75	A	0.000	0.000	0.000	0.000	0.00
LIT	20.75-10.75	В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	8.087	0.34
L15	16.75-14.25	Ä	0.000	0.000	0.000	0.000	0.00
	.0.70 11.20	В	0.000	0.000	0.000	0.000	0.00
		Č	0.000	0.000	0.000	2.022	0.08
L16	14.25-0.00	Ä	0.000	0.000	0.000	0.000	0.00
	1 1.20 0.00	В	0.000	0.000	0.000	0.000	0.00
		Č	0.000	0.000	0.000	11.523	0.48

# Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_AA_A$	Weight
Sectio	Elevation	or	Thickness	ft <sup>2</sup>	_	In Face	Out Face	K
n	ft	Leg	in		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	
L1	151.00-125.25	Α	1.730	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	0.809	0.44
L2	125.25-118.50	Α	1.709	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	9.194	0.54
L3	118.50-117.75	Α	1.704	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	1.695	0.10
L4	117.75-97.50	Α	1.688	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	33.377	2.57
L5	97.50-95.00	Α	1.669	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	5.106	0.32
L6	95.00-92.50	Α	1.665	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	5.329	0.31
L7	92.50-87.75	Α	1.659	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00

Tower	Tower	Face	Ice	A <sub>R</sub> ft <sup>2</sup>	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Sectio	Elevation	or	Thickness	ft²		In Face	Out Face	K
n	ft	Leg	in		ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	
		С		0.000	0.000	0.000	10.212	0.59
L8	87.75-84.00	Α	1.651	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	8.103	0.47
L9	84.00-63.25	Α	1.625	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	36.911	2.54
L10	63.25-56.75	Α	1.592	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	13.997	0.78
L11	56.75-47.50	Α	1.570	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	16.958	1.10
L12	47.50-34.25	Α	1.532	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	23.378	1.57
L13	34.25-26.75	Α	1.488	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	15.490	0.84
L14	26.75-16.75	Α	1.438	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	20.234	1.09
L15	16.75-14.25	Α	1.391	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	4.958	0.26
L16	14.25-0.00	Α	1.285	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		С		0.000	0.000	0.000	26.992	1.39

# **Feed Line Center of Pressure**

Section	Elevation	CP <sub>X</sub>	CPz	$CP_X$	CPz
	ft	in	in	Ice	Ice
				in	in
L1	151.00-125.25	-0.0177	0.0102	-0.0371	0.0214
L2	125.25-118.50	-0.5169	0.2985	-0.9418	0.5438
L3	118.50-117.75	-0.7355	0.4246	-1.2648	0.7303
L4	117.75-97.50	-0.5717	0.3301	-1.1041	0.6375
L5	97.50-95.00	-0.6714	0.3876	-1.2906	0.7451
L6	95.00-92.50	-0.6992	0.4037	-1.3358	0.7712
L7	92.50-87.75	-0.7211	0.4163	-1.3589	0.7846
L8	87.75-84.00	-0.7386	0.4264	-1.3816	0.7977
L9	84.00-63.25	-0.6673	0.3853	-1.2767	0.7371
L10	63.25-56.75	-0.7963	0.4597	-1.4844	0.8570
L11	56.75-47.50	-0.7109	0.4105	-1.3659	0.7886
L12	47.50-34.25	-0.6967	0.4022	-1.3567	0.7833
L13	34.25-26.75	-0.8184	0.4725	-1.5368	0.8873
L14	26.75-16.75	-0.8248	0.4762	-1.5440	0.8914
L15	16.75-14.25	-0.8291	0.4787	-1.5427	0.8907
L16	14.25-0.00	-0.8346	0.4819	-1.5211	0.8782

# **Shielding Factor Ka**

Tower	Feed Line	Description	Feed Line	Ka	K <sub>a</sub>
Section	Record No.	-	Segment	No Ice	Ice
			Ēlev.		

Discrete 7	Tower	Loads
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Description	Face	Offset	Offsets:	Azimuth	Placement		$C_AA_A$	$C_AA_A$	Weight
	or Leg	Туре	Horz Lateral	Adjustmen t	ft		Front ft²	Side ft²	K
	Leg		Vert	۰			"	"	
			ft						
			ft						
(2) 111 )/ 22 12 27			ft						
(2) AM-X-CD-16-65-00T-	Α	From Leg	4.00	0.00	148.00	No Ice 1/2"	8.26	6.30	0.07
RET w/ Mount Pipe			0.00 1.00			lce	8.82 9.35	7.48 8.37	0.14 0.21
			1.00			1" Ice	3.33	0.57	0.21
(2) AM-X-CD-16-65-00T-	В	From Leg	4.00	0.00	148.00	No Ice	8.26	6.30	0.07
RET w/ Mount Pipe		-	0.00			1/2"	8.82	7.48	0.14
			1.00			Ice	9.35	8.37	0.21
(2) AM × CD 16 65 00T	С	From Log	4.00	0.00	149.00	1" Ice No Ice	0.26	6 20	0.07
(2) AM-X-CD-16-65-00T- RET w/ Mount Pipe	C	From Leg	4.00 0.00	0.00	148.00	1/2"	8.26 8.82	6.30 7.48	0.07 0.14
TCT W/ Would't Tipe			1.00			Ice	9.35	8.37	0.14
						1" Ice	0.00	0.0.	0.2.
800 10121 w/ Mount Pipe	Α	From Leg	4.00	0.00	148.00	No Ice	5.74	4.95	0.07
			0.00			1/2"	6.34	6.02	0.12
			1.00			Ice	6.86	6.81	0.18
800 10121 w/ Mount Pipe	В	From Leg	4.00	0.00	148.00	1" Ice No Ice	5.74	4.95	0.07
800 10121 W/ Mount Pipe	Ь	Fiolii Leg	0.00	0.00	146.00	1/2"	6.34	6.02	0.07
			1.00			Ice	6.86	6.81	0.18
						1" Ice			
800 10121 w/ Mount Pipe	С	From Leg	4.00	0.00	148.00	No Ice	5.74	4.95	0.07
			0.00			1/2"	6.34	6.02	0.12
			1.00			Ice	6.86	6.81	0.18
(2) CG-1900DD-FULL-DIN	Α	From Leg	4.00	0.00	148.00	1" Ice No Ice	1.10	0.29	0.01
(2) CG-1900DD-1 OEE-DIN	^	i ioni Leg	0.00	0.00	140.00	1/2"	1.23	0.29	0.01
			1.00			lce	1.37	0.46	0.03
						1" Ice			
(2) CG-1900DD-FULL-DIN	В	From Leg	4.00	0.00	148.00	No Ice	1.10	0.29	0.01
			0.00			1/2"	1.23	0.37	0.02
			1.00			Ice 1" Ice	1.37	0.46	0.03
(2) CG-1900DD-FULL-DIN	С	From Leg	4.00	0.00	148.00	No Ice	1.10	0.29	0.01
(2) 33 13332 1 322 2	•		0.00	0.00		1/2"	1.23	0.37	0.02
			1.00			Ice	1.37	0.46	0.03
	_					1" Ice			
(4) LGP21901	Α	From Leg	4.00	0.00	148.00	No Ice	0.23	0.16	0.01
			0.00 1.00			1/2" Ice	0.29 0.36	0.21 0.28	0.01 0.01
			1.00			1" Ice	0.50	0.20	0.01
(4) LGP21901	В	From Leg	4.00	0.00	148.00	No Ice	0.23	0.16	0.01
` ,		Ü	0.00			1/2"	0.29	0.21	0.01
			1.00			Ice	0.36	0.28	0.01
(4) L CD24004	_	Г I	4.00	0.00	440.00	1" Ice	0.00	0.40	0.04
(4) LGP21901	С	From Leg	4.00 0.00	0.00	148.00	No Ice 1/2"	0.23 0.29	0.16 0.21	0.01 0.01
			1.00			Ice	0.25	0.21	0.01
						1" Ice	0.00	0.20	0.0.
(2) DTMABP7819VG12A	Α	From Leg	4.00	0.00	148.00	No Ice	0.98	0.34	0.02
		-	0.00			1/2"	1.10	0.42	0.03
			1.00			Ice	1.23	0.51	0.04
(2) DTMABP7819VG12A	В	From Leg	4.00	0.00	148.00	1" Ice No Ice	0.98	0.34	0.02
(2) DININDI 1013VO1ZA	D	i ioni Leg	0.00	0.00	140.00	1/2"	1.10	0.42	0.02
			1.00			Ice	1.23	0.51	0.04
						1" Ice			
(2) DTMABP7819VG12A	С	From Leg	4.00	0.00	148.00	No Ice	0.98	0.34	0.02
			0.00			1/2"	1.10	0.42	0.03
			1.00			Ice 1" Ice	1.23	0.51	0.04
860 10025	Α	From Leg	4.00	0.00	148.00	No Ice	0.14	0.12	0.00
			0.00	0.00		1/2"	0.19	0.17	0.00
			<del>-</del>				-		<del>-</del>

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement ft		C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K
			ft ft ft						
			1.00			Ice 1" Ice	0.25	0.23	0.01
860 10025	В	From Leg	4.00	0.00	148.00	No Ice	0.14	0.12	0.00
			0.00 1.00			1/2" Ice 1" Ice	0.19 0.25	0.17 0.23	0.00 0.01
860 10025	С	From Leg	4.00	0.00	148.00	No Ice	0.14	0.12	0.00
			0.00 1.00			1/2" Ice 1" Ice	0.19 0.25	0.17 0.23	0.00 0.01
(2) RRUS-11	Α	From Leg	4.00	0.00	148.00	No Ice	2.79	1.19	0.05
			0.00 1.00			1/2" Ice	3.00 3.21	1.34 1.50	0.07 0.09
			1.00			1" Ice	3.21	1.50	0.09
(2) RRUS-11	В	From Leg	4.00	0.00	148.00	No Ice	2.79	1.19	0.05
			0.00 1.00			1/2" Ice 1" Ice	3.00 3.21	1.34 1.50	0.07 0.09
(2) RRUS-11	С	From Leg	4.00	0.00	148.00	No Ice	2.79	1.19	0.05
			0.00			1/2"	3.00	1.34	0.07
DC6-48-60-18-8F	Α	From Leg	1.00 4.00	0.00	148.00	Ice 1" Ice No Ice	3.21 0.92	1.50 0.92	0.09 0.02
D00-40-00-10-01		1 Tolli Log	0.00	0.00	140.00	1/2"	1.46	1.46	0.02
			1.00			Ice 1" Ice	1.64	1.64	0.06
DS1F03F36D-N	Α	From Leg	4.00 0.00	0.00	148.00	No Ice 1/2"	6.69 8.95	6.69 8.95	0.06 0.11
			12.00			Ice 1" Ice	11.23	11.23	0.17
Platform Mount [LP 602-1]	С	None		0.00	148.00	No Ice	32.03	32.03	1.34
						1/2" Ice 1" Ice	38.71 45.39	38.71 45.39	1.80 2.26
***									
DB854DG65ESX w/ Mount	Α	From Leg	4.00 0.00	0.00	140.00	No Ice 1/2"	5.55 5.94	4.10 4.73	0.04 0.08
Pipe			0.00			Ice 1" Ice	6.34	5.36	0.08
DB854DG65ESX w/ Mount	В	From Leg	4.00	0.00	140.00	No Ice	5.55	4.10	0.04
Pipe		1 Tolli Log	0.00	0.00	140.00	1/2"	5.94	4.73	0.08
	_		0.00			Ice 1" Ice	6.34	5.36	0.14
DB854DG65ESX w/ Mount Pipe	С	From Leg	4.00 0.00	0.00	140.00	No Ice 1/2"	5.55 5.94	4.10 4.73	0.04 0.08
гіре			0.00			Ice 1" Ice	6.34	5.36	0.14
(2) SBNHH-1D65B w/	Α	From Leg	4.00	0.00	140.00	No Ice	8.42	7.42	0.08
Mount Pipe			0.00 0.00			1/2" Ice 1" Ice	8.96 9.48	8.45 9.35	0.15 0.23
(2) SBNHH-1D65B w/	В	From Leg	4.00	0.00	140.00	No Ice	8.42	7.42	0.08
Mount Pipe			0.00 0.00			1/2" Ice 1" Ice	8.96 9.48	8.45 9.35	0.15 0.23
(2) SBNHH-1D65B w/	С	From Leg	4.00	0.00	140.00	No Ice	8.42	7.42	0.08
Mount Pipe			0.00 0.00			1/2" Ice	8.96 9.48	8.45 9.35	0.15 0.23
RRH4X45-AWS4 B66	Α	From Leg	4.00	0.00	140.00	1" Ice No Ice	2.66	1.59	0.06
TAM HAMOTAWOT BOO	^	. Tom Log	0.00 0.00	0.00	170.00	1/2" Ice	2.88 3.10	1.77 1.96	0.08 0.11
DDHAVAE AMEA DEC	D	Erom Loc	4.00	0.00	140.00	1" Ice	2 66	1 50	0.06
RRH4X45-AWS4 B66	В	From Leg	4.00 0.00	0.00	140.00	No Ice 1/2"	2.66 2.88	1.59 1.77	0.06 0.08

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustmen	Placement ft		C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight K
	Leg		Lateral Vert ft ft ft	t °			ft <sup>2</sup>	ft <sup>2</sup>	
			0.00			Ice 1" Ice	3.10	1.96	0.11
RRH4X45-AWS4 B66	С	From Leg	4.00 0.00 0.00	0.00	140.00	No Ice 1/2" Ice 1" Ice	2.66 2.88 3.10	1.59 1.77 1.96	0.06 0.08 0.11
RRH2x60-700	Α	From Leg	4.00 0.00 0.00	0.00	140.00	No Ice 1/2" Ice 1" Ice	3.50 3.76 4.03	1.82 2.05 2.29	0.06 0.08 0.11
RRH2x60-700	В	From Leg	4.00 0.00 0.00	0.00	140.00	No Ice 1/2" Ice 1" Ice	3.50 3.76 4.03	1.82 2.05 2.29	0.06 0.08 0.11
RRH2x60-700	С	From Leg	4.00 0.00 0.00	0.00	140.00	No Ice 1/2" Ice 1" Ice	3.50 3.76 4.03	1.82 2.05 2.29	0.06 0.08 0.11
DB-T1-6Z-8AB-0Z	Α	From Leg	4.00 0.00 0.00	0.00	140.00	No Ice 1/2" Ice 1" Ice	4.80 5.07 5.35	2.00 2.19 2.39	0.04 0.08 0.12
2.375" OD x 6' Mount Pipe	Α	None		0.00	140.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.03 0.04 0.05
2.375" OD x 6' Mount Pipe	В	None		0.00	140.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.03 0.04 0.05
2.375" OD x 6' Mount Pipe	С	None		0.00	140.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.03 0.04 0.05
Platform Mount [LP 303-1]	С	None		0.00	140.00	No Ice 1/2" Ice 1" Ice	14.66 18.87 23.08	14.66 18.87 23.08	1.25 1.48 1.71
1900MHz RRH	Α	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	2.49 2.70 2.91	3.26 3.48 3.72	0.04 0.08 0.11
1900MHz RRH	В	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	2.49 2.70 2.91	3.26 3.48 3.72	0.04 0.08 0.11
1900MHz RRH	С	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	2.49 2.70 2.91	3.26 3.48 3.72	0.04 0.08 0.11
800 EXTERNAL NOTCH FILTER	Α	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	0.66 0.76 0.87	0.32 0.40 0.48	0.01 0.02 0.02
800 EXTERNAL NOTCH FILTER	В	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	0.66 0.76 0.87	0.32 0.40 0.48	0.01 0.02 0.02
800 EXTERNAL NOTCH FILTER	С	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice	0.66 0.76 0.87	0.32 0.40 0.48	0.01 0.02 0.02
800MHZ RRH	Α	From Leg	4.00 0.00	0.00	132.00	1" Ice No Ice 1/2"	2.13 2.32	1.77 1.95	0.05 0.07

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustmen t	Placement ft		C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K
			ft ft ft						
			-1.00			Ice 1" Ice	2.51	2.13	0.10
800MHZ RRH	В	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	2.13 2.32 2.51	1.77 1.95 2.13	0.05 0.07 0.10
800MHZ RRH	С	From Leg	4.00 0.00 -1.00	0.00	132.00	No Ice 1/2" Ice 1" Ice	2.13 2.32 2.51	1.77 1.95 2.13	0.05 0.07 0.10
Pipe Mount [PM 601-3]	С	None		0.00	132.00	No Ice 1/2" Ice 1" Ice	4.39 5.48 6.57	4.39 5.48 6.57	0.20 0.24 0.28
APXVSPP18-C-A20 w/ Mount Pipe	Α	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice	8.26 8.82 9.35	6.95 8.13 9.02	0.08 0.15 0.23
APXVSPP18-C-A20 w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.00	130.00	1" Ice No Ice 1/2" Ice 1" Ice	8.26 8.82 9.35	6.95 8.13 9.02	0.08 0.15 0.23
APXVSPP18-C-A20 w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	8.26 8.82 9.35	6.95 8.13 9.02	0.08 0.15 0.23
GPS-TMG-HR-26NCM	Α	From Leg	4.00 0.00 3.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	0.13 0.18 0.24	0.13 0.18 0.24	0.00 0.00 0.01
APXVTM14-C-120 w/ Mount Pipe	Α	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	6.58 7.03 7.47	4.96 5.75 6.47	0.08 0.13 0.19
APXVTM14-C-120 w/ Mount Pipe	В	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	6.58 7.03 7.47	4.96 5.75 6.47	0.08 0.13 0.19
APXVTM14-C-120 w/ Mount Pipe	С	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	6.58 7.03 7.47	4.96 5.75 6.47	0.08 0.13 0.19
TD-RRH8x20-25	Α	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
TD-RRH8x20-25	В	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
TD-RRH8x20-25	С	From Leg	4.00 0.00 0.00	0.00	130.00	No Ice 1/2" Ice 1" Ice	4.05 4.30 4.56	1.53 1.71 1.90	0.07 0.10 0.13
2.375" OD x 6' Mount Pipe	Α	None		0.00	130.00	No Ice 1/2" Ice 1" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.03 0.04 0.05
2.375" OD x 6' Mount Pipe	В	None		0.00	130.00	No Ice 1/2" Ice	1.43 1.92 2.29	1.43 1.92 2.29	0.03 0.04 0.05
2.375" OD x 6' Mount Pipe	С	None		0.00	130.00	1" Ice No Ice 1/2"	1.43 1.92	1.43 1.92	0.03 0.04

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft	Azimuth Adjustmen t	Placement ft		C <sub>A</sub> A <sub>A</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Side ft <sup>2</sup>	Weight K
			-			Ice 1" Ice	2.29	2.29	0.05
T-Arm Mount [TA 602-3]	С	None		0.00	130.00	No Ice 1/2" Ice 1" Ice	11.59 15.44 19.29	11.59 15.44 19.29	0.77 0.99 1.21
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	Α	From Leg	4.00 0.00 -2.00	0.00	122.00	No Ice 1/2" Ice 1" Ice	6.32 6.76 7.20	5.63 6.42 7.12	0.11 0.17 0.23
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	В	From Leg	4.00 0.00 -2.00	0.00	122.00	No Ice 1/2" Ice 1" Ice	6.32 6.76 7.20	5.63 6.42 7.12	0.11 0.17 0.23
ERICSSON AIR 21 B4A B2P w/ Mount Pipe	С	From Leg	4.00 0.00 -2.00	0.00	122.00	No Ice 1/2" Ice 1" Ice	6.32 6.76 7.20	5.63 6.42 7.12	0.11 0.17 0.23
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	Α	From Leg	4.00 0.00 -2.00	0.00	122.00	No Ice 1/2" Ice 1" Ice	6.33 6.78 7.21	5.64 6.43 7.13	0.11 0.17 0.23
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	В	From Leg	4.00 0.00 -2.00	0.00	122.00	No Ice 1/2" Ice 1" Ice	6.33 6.78 7.21	5.64 6.43 7.13	0.11 0.17 0.23
ERICSSON AIR 21 B2A B4P w/ Mount Pipe	С	From Leg	4.00 0.00 -2.00	0.00	122.00	No Ice 1/2" Ice 1" Ice	6.33 6.78 7.21	5.64 6.43 7.13	0.11 0.17 0.23
T-Arm Mount [TA 702-3]	С	None		0.00	122.00	No Ice 1/2" Ice 1" Ice	5.64 6.55 7.46	5.64 6.55 7.46	0.34 0.43 0.52

# **Tower Pressures - No Ice**

 $G_H = 1.100$ 

Section	Z	Kz	$q_z$	$A_{g}$	F	$A_F$ $ft^2$	$A_R$ $ft^2$	$A_{leg}$ $ft^2$	Leg	$C_A A_A$	$C_A A_A$
Elevation	ft		psf	ft <sup>2</sup>	а	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	%	In	Out
ft					С					Façe	Façe
					е					ft <sup>2</sup>	ft <sup>2</sup>
L1 151.00-	137.61	1.354	30.97	44.428	Α	0.000	44.428	44.428	100.00	0.000	0.000
125.25					В	0.000	44.428		100.00	0.000	0.000
					С	0.000	44.428		100.00	0.000	0.328
L2 125.25-	121.84	1.319	30.19	13.417	Α	0.000	13.417	13.417	100.00	0.000	0.000
118.50					В	0.000	13.417		100.00	0.000	0.000
					С	0.000	13.417		100.00	0.000	3.477
L3 118.50-	118.12	1.311	29.99	1.536	Α	0.000	1.536	1.536	100.00	0.000	0.000
117.75					В	0.000	1.536		100.00	0.000	0.000
					С	0.000	1.536		100.00	0.000	0.616
L4 117.75-	107.38	1.285	29.40	44.908	Α	0.000	44.908	44.908	100.00	0.000	0.000
97.50					В	0.000	44.908		100.00	0.000	0.000
					С	0.000	44.908		100.00	0.000	11.641
L5 97.50-	96.24	1.255	28.73	5.887	Α	0.000	5.887	5.887	100.00	0.000	0.000
95.00					В	0.000	5.887		100.00	0.000	0.000
					С	0.000	5.887		100.00	0.000	1.730
L6 95.00-	93.75	1.249	28.57	6.010	Α	0.000	6.010	6.010	100.00	0.000	0.000
92.50					В	0.000	6.010		100.00	0.000	0.000
					С	0.000	6.010		100.00	0.000	1.813
L7 92.50-	90.11	1.238	28.33	11.697	Α	0.000	11.697	11.697	100.00	0.000	0.000
87.75					В	0.000	11.697		100.00	0.000	0.000

Section	Z	Kz	qz	AĢ	F	$A_F$ $ft^2$	$A_R$ $ft^2$	A <sub>leg</sub> ft²	Leg	$C_A A_A$	$C_A A_A$
Elevation	ft		psf	ft <sup>2</sup>	а	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>	%	In	Out
ft					С					Face	Face
					е					ft <sup>2</sup>	ft <sup>2</sup>
					С	0.000	11.697		100.00	0.000	3.560
L8 87.75-	85.87	1.226	28.05	9.491	Α	0.000	9.491	9.491	100.00	0.000	0.000
84.00					В	0.000	9.491		100.00	0.000	0.000
					С	0.000	9.491		100.00	0.000	2.876
L9 84.00-	73.41	1.186	27.14	56.623	Α	0.000	56.623	56.623	100.00	0.000	0.000
63.25					В	0.000	56.623		100.00	0.000	0.000
					С	0.000	56.623		100.00	0.000	13.675
L10 63.25-	59.98	1.136	26.01	19.167	Α	0.000	19.167	19.167	100.00	0.000	0.000
56.75					В	0.000	19.167		100.00	0.000	0.000
					С	0.000	19.167		100.00	0.000	5.256
L11 56.75-	52.09	1.103	25.24	28.452	Α	0.000	28.452	28.452	100.00	0.000	0.000
47.50					В	0.000	28.452		100.00	0.000	0.000
					С	0.000	28.452		100.00	0.000	6.438
L12 47.50-	40.79	1.048	23.98	42.178	Α	0.000	42.178	42.178	100.00	0.000	0.000
34.25					В	0.000	42.178		100.00	0.000	0.000
					С	0.000	42.178		100.00	0.000	8.881
L13 34.25-	30.48	0.986	22.55	25.282	Α	0.000	25.282	25.282	100.00	0.000	0.000
26.75					В	0.000	25.282		100.00	0.000	0.000
					С	0.000	25.282		100.00	0.000	6.065
L14 26.75-	21.71	0.918	21.00	35.122	Α	0.000	35.122	35.122	100.00	0.000	0.000
16.75					В	0.000	35.122		100.00	0.000	0.000
					С	0.000	35.122		100.00	0.000	8.087
L15 16.75-	15.50	0.855	19.56	9.033	Α	0.000	9.033	9.033	100.00	0.000	0.000
14.25					В	0.000	9.033		100.00	0.000	0.000
					С	0.000	9.033		100.00	0.000	2.022
L16 14.25-	7.05	0.85	19.45	53.413	Α	0.000	53.413	53.413	100.00	0.000	0.000
0.00					В	0.000	53.413		100.00	0.000	0.000
					С	0.000	53.413		100.00	0.000	11.523

## **Tower Pressure - With Ice**

 $G_H = 1.100$ 

The color of the	Section	Z	Κz	qz	$t_Z$	A <sub>G</sub> ft²	F	$A_F$ $ft^2$	$A_R$ $ft^2$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Elevation	ft		psf	in	ft <sup>2</sup>	а	ft <sup>2</sup>	ft²	ft <sup>2</sup>	%	In	Out
L1 151.00	ft						С						
125.25							е					ft <sup>2</sup>	
L2 125.25-		137.61	1.354	8.23	1.7302	51.854				51.854			
L2 125.25- 118.50         121.84 1.85         1.319         8.02         1.7093         15.340 8         A         0.000 15.340 0         15.340 100.00         100.00 100.00         0.000 0.000         0.000 9.194           L3 118.50- 117.75         118.12 1.311         1.311 7.97         7.97         1.7040 8         1.749 8         A         0.000 1.749 0         1.749 100.00         1.000 0.000         0.000 0.000         0.000 0	125.25												
118.50							С						
L3 118.50- 118.12		121.84	1.319	8.02	1.7093	15.340				15.340			
L3 118.50- 118.12	118.50						_						
117.75							С						
L4 117.75- 107.38	L3 118.50-	118.12	1.311	7.97	1.7040	1.749	Α	0.000	1.749	1.749	100.00		
L4 117.75- 97.50	117.75						В	0.000	1.749		100.00	0.000	0.000
Property							С	0.000	1.749		100.00	0.000	1.695
L5 97.50-95.00 96.24 1.255 7.63 1.6695 6.591 A 0.000 6.591 6.591 100.00 0.000 0.000 0.000 0.000	L4 117.75-	107.38	1.285	7.81	1.6878	50.605	Α	0.000	50.605	50.605	100.00	0.000	0.000
L5 97.50-95.00	97.50						В	0.000	50.605		100.00	0.000	0.000
L6 95.00-92.50							С	0.000	50.605		100.00	0.000	33.377
L6 95.00-92.50	L5 97.50-95.00	96.24	1.255	7.63	1.6695	6.591	Α	0.000	6.591	6.591	100.00	0.000	0.000
L6 95.00-92.50							В	0.000	6.591		100.00	0.000	0.000
L7 92.50-87.75 90.11 1.238 7.53 1.6585 13.010 A 0.000 6.704 100.00 0.000 0.000 0.000 0.000 13.010 100.00 0.000 0.000 0.000 0.000  0.000 0.							С	0.000	6.591		100.00	0.000	
L7 92.50-87.75 90.11 1.238 7.53 1.6585 13.010 A 0.000 13.010 13.010 100.00 0.000 0.000 0.000 10.000	L6 95.00-92.50	93.75	1.249	7.59	1.6651	6.704	Α	0.000	6.704	6.704	100.00	0.000	0.000
L7 92.50-87.75 90.11 1.238 7.53 1.6585 13.010 A 0.000 13.010 13.010 100.00 0.000 0.000 0.000 13.010 100.00 0.000 0.000 0.000 0.000 13.010 100.00 0.000 0.000 0.000 10.212 0.000 10.523 10.523 100.00 0.000 0							В	0.000	6.704		100.00	0.000	0.000
L8 87.75-84.00							С	0.000	6.704		100.00	0.000	5.329
L9 84.00-63.25 73.41 1.186 7.21 1.6249 62.242 A 0.000 62.242	L7 92.50-87.75	90.11	1.238	7.53	1.6585	13.010	Α	0.000	13.010	13.010	100.00	0.000	0.000
L9 84.00-63.25 73.41 1.186 7.21 1.6249 62.242 A 0.000 62.242 62.242 100.00 0.0							В	0.000	13.010		100.00	0.000	0.000
L9 84.00-63.25 73.41 1.186 7.21 1.6249 62.242 A 0.000 62.242 62.242 100.00 0.0							С	0.000	13.010		100.00	0.000	10.212
L9 84.00-63.25	L8 87.75-84.00	85.87	1.226	7.45	1.6505	10.523	Α	0.000	10.523	10.523	100.00	0.000	0.000
L9 84.00-63.25							В	0.000	10.523		100.00	0.000	0.000
L10 63.25- 56.75 59.98 1.136 6.91 1.5924 20.892 A 0.000 20.892 20.892 100.00 0.000 0							С	0.000	10.523		100.00	0.000	8.103
L10 63.25- 56.75 59.98 1.136 6.91 1.5924 20.892 A 0.000 20.892 20.892 100.00 0.000 36.911 B 0.000 20.892 20.892 100.00 0.000 0.000	L9 84.00-63.25	73.41	1.186	7.21	1.6249	62.242	Α	0.000	62.242	62.242	100.00	0.000	0.000
L10 63.25- 59.98 1.136 6.91 1.5924 20.892 A 0.000 20.892 20.892 100.00 0.000 0.000 56.75 B 0.000 20.892								0.000			100.00		
56.75 B 0.000 20.892 100.00 0.000 0.000							С	0.000	62.242		100.00	0.000	36.911
56.75 B 0.000 20.892 100.00 0.000 0.000	L10 63.25-	59.98	1.136	6.91	1.5924	20.892	Α	0.000	20.892	20.892	100.00	0.000	0.000
							В	0.000	20.892		100.00	0.000	0.000
1							С	0.000	20.892		100.00	0.000	13.997

Section	Z	Kz	qz	$t_Z$	$A_G$ $ft^2$	F	$A_{\mathcal{E}}$	$A_R$ $ft^2$	$A_{leg}$	Leg	$C_AA_A$	$C_AA_A$
Elevation	ft		psf	in	ft <sup>2</sup>	а	ft <sup>2</sup>	ft²	ft <sup>2</sup>	%	In	Out
ft						С					Face	Face
						е					ft <sup>2</sup>	ft <sup>2</sup>
L11 56.75-	52.09	1.103	6.71	1.5700	30.872	Α	0.000	30.872	30.872	100.00	0.000	0.000
47.50						В	0.000	30.872		100.00	0.000	0.000
						С	0.000	30.872		100.00	0.000	16.958
L12 47.50-	40.79	1.048	6.37	1.5321	45.645	Α	0.000	45.645	45.645	100.00	0.000	0.000
34.25						В	0.000	45.645		100.00	0.000	0.000
						С	0.000	45.645		100.00	0.000	23.378
L13 34.25-	30.48	0.986	5.99	1.4881	27.143	Α	0.000	27.143	27.143	100.00	0.000	0.000
26.75						В	0.000	27.143		100.00	0.000	0.000
						С	0.000	27.143		100.00	0.000	15.490
L14 26.75-	21.71	0.918	5.58	1.4385	37.520	Α	0.000	37.520	37.520	100.00	0.000	0.000
16.75						В	0.000	37.520		100.00	0.000	0.000
						С	0.000	37.520		100.00	0.000	20.234
L15 16.75-	15.50	0.855	5.20	1.3908	9.612	Α	0.000	9.612	9.612	100.00	0.000	0.000
14.25						В	0.000	9.612		100.00	0.000	0.000
						С	0.000	9.612		100.00	0.000	4.958
L16 14.25-0.00	7.05	0.85	5.17	1.2855	56.466	Α	0.000	56.466	56.466	100.00	0.000	0.000
						В	0.000	56.466		100.00	0.000	0.000
						С	0.000	56.466		100.00	0.000	26.992

# **Tower Pressure - Service**

 $G_H = 1.100$ 

Section	-	V	~	Λ	F	Λ	Λ I	Λ	100	C 4	C 1
Elevation	z ft	Kz	q <sub>z</sub> psf	A <sub>G</sub> ft²	г	A <sub>F</sub> ft <sup>2</sup>	$A_R$ $ft^2$	A <sub>leg</sub> ft²	Leg %	C₄A₄ In	$C_AA_A$ Out
ft	n		μδι	π	C	11	11	π	/0	Face	Face
TL .					e					ft <sup>2</sup>	ft <sup>2</sup>
L1 151.00-	137.61	1.354	10.60	44.428	A	0.000	44.428	44.428	100.00	0.000	0.000
125.25	137.01	1.554	10.00	44.420	В	0.000	44.428	44.420	100.00	0.000	0.000
120.20					C	0.000	44.428		100.00	0.000	0.328
L2 125.25-	121.84	1.319	10.34	13.417	A	0.000	13.417	13.417	100.00	0.000	0.000
118.50	121.04	1.010	10.04	10.417	В	0.000	13.417	10.417	100.00	0.000	0.000
110.00					C	0.000	13.417		100.00	0.000	3.477
L3 118.50-	118.12	1.311	10.27	1.536	Ä	0.000	1.536	1.536	100.00	0.000	0.000
117.75					В	0.000	1.536		100.00	0.000	0.000
					C	0.000	1.536		100.00	0.000	0.616
L4 117.75-	107.38	1.285	10.06	44.908	Α	0.000	44.908	44.908	100.00	0.000	0.000
97.50					В	0.000	44.908		100.00	0.000	0.000
					С	0.000	44.908		100.00	0.000	11.641
L5 97.50-	96.24	1.255	9.83	5.887	Α	0.000	5.887	5.887	100.00	0.000	0.000
95.00					В	0.000	5.887		100.00	0.000	0.000
					С	0.000	5.887		100.00	0.000	1.730
L6 95.00-	93.75	1.249	9.78	6.010	Α	0.000	6.010	6.010	100.00	0.000	0.000
92.50					В	0.000	6.010		100.00	0.000	0.000
					С	0.000	6.010		100.00	0.000	1.813
L7 92.50-	90.11	1.238	9.70	11.697	Α	0.000	11.697	11.697	100.00	0.000	0.000
87.75					В	0.000	11.697		100.00	0.000	0.000
					С	0.000	11.697		100.00	0.000	3.560
L8 87.75-	85.87	1.226	9.60	9.491	Α	0.000	9.491	9.491	100.00	0.000	0.000
84.00					В	0.000	9.491		100.00	0.000	0.000
100100					С	0.000	9.491		100.00	0.000	2.876
L9 84.00-	73.41	1.186	9.29	56.623	Α	0.000	56.623	56.623	100.00	0.000	0.000
63.25					В	0.000	56.623		100.00	0.000	0.000
140.00.05	FO 00	4 400	8.90	40.407	C	0.000	56.623	40.407	100.00	0.000	13.675
L10 63.25- 56.75	59.98	1.136	8.90	19.167	A B	0.000 0.000	19.167 19.167	19.167	100.00 100.00	0.000	0.000
50.75					С	0.000	19.167		100.00	0.000 0.000	0.000 5.256
L11 56.75-	52.09	1.103	8.64	28.452	A	0.000	28.452	28.452	100.00	0.000	0.000
47.50	52.09	1.103	0.04	20.432	В	0.000	28.452	20.432	100.00	0.000	0.000
47.50					С	0.000	28.452		100.00	0.000	6.438
L12 47.50-	40.79	1.048	8.21	42.178	A	0.000	42.178	42.178	100.00	0.000	0.438
34.25	+0.79	1.040	0.21	74.170	В	0.000	42.178	72.110	100.00	0.000	0.000
54.25					C	0.000	42.178		100.00	0.000	8.881
L13 34.25-	30.48	0.986	7.72	25.282	A	0.000	25.282	25.282	100.00	0.000	0.000
26.75	30.∓0	0.000	2	20.202	В	0.000	25.282	20.202	100.00	0.000	0.000
					C	0.000	25.282		100.00	0.000	6.065
L14 26.75-	21.71	0.918	7.19	35.122	Ä	0.000	35.122	35.122	100.00	0.000	0.000
		3.0.0		JU		. 5.556	I	JJ		0.000	0.000

Section Elevation ft	z ft	Kz	q <sub>z</sub> psf	A <sub>G</sub> ft²	F a c	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft²	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C₄A₄ In Façe	C₄A₄ Out Façe
					e					ft <sup>2</sup>	ft <sup>2</sup>
16.75					В	0.000	35.122		100.00	0.000	0.000
					С	0.000	35.122		100.00	0.000	8.087
L15 16.75-	15.50	0.855	6.70	9.033	Α	0.000	9.033	9.033	100.00	0.000	0.000
14.25					В	0.000	9.033		100.00	0.000	0.000
					С	0.000	9.033		100.00	0.000	2.022
L16 14.25-	7.05	0.85	6.66	53.413	Α	0.000	53.413	53.413	100.00	0.000	0.000
0.00					В	0.000	53.413		100.00	0.000	0.000
					С	0.000	53.413		100.00	0.000	11.523

## **Load Combinations**

Comb.	Description
No.	
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20 21	1.2 Dead+1.6 Wind 270 deg - No Ice
22	0.9 Dead+1.6 Wind 270 deg - No Ice
23	1.2 Dead+1.6 Wind 300 deg - No Ice
24	0.9 Dead+1.6 Wind 300 deg - No Ice 1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

## **Maximum Member Forces**

Sectio	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
n	ft	Туре		Load	K	Moment	Moment
No.		• •		Comb.		kip-ft	kip-ft
L1	151 - 125.25	Pole	Max Tension	21	0.00	-0.00	-0.00
			Max. Compression	26	-24.51	0.14	3.03
			Max. Mx	20	-8.22	285.13	0.46
			Max. My	2	-8.19	0.04	287.69
			Max. Vy	20	-18.23	285.13	0.46
			Max. Vx	2	-18.36	0.04	287.69
			Max. Torque	9	10.00	0.01	2.40
L2	125.25 -	Pole	Max Tension	1	0.00	0.00	0.00
LZ	118.5	1 010	Wax Tension	•	0.00	0.00	0.00
	110.5		Max. Compression	26	-29.03	0.51	2.87
			Max. Mx	20	-10.15	415.59	0.48
			Max. My	20	-10.13	0.07	418.98
			,	20	-10.13 -21.26	415.59	0.48
			Max. Vy				
			Max. Vx	2	-21.39	0.07	418.98
	440.5	Б.	Max. Torque	9	0.00	0.00	2.40
L3	118.5 -	Pole	Max Tension	1	0.00	0.00	0.00
	117.75						
			Max. Compression	26	-29.32	0.59	2.84
			Max. Mx	20	-10.33	431.59	0.49
			Max. My	2	-10.30	0.07	435.07
			Max. Vy	20	-21.40	431.59	0.49
			Max. Vx	2	-21.53	0.07	435.07
			Max. Torque	9			2.31
L4	117.75 -	Pole	Max Tension	1	0.00	0.00	0.00
	97.5						
			Max. Compression	26	-35.05	2.35	1.93
			Max. Mx	20	-13.60	815.94	0.48
			Max. My	2	-13.58	0.19	821.39
			Max. Vy	20	-24.30	815.94	0.48
			Max. Vx	2	-24.43	0.19	821.39
			Max. Torque	9	21.10	0.10	2.29
L5	97.5 - 95	Pole	Max Tension	1	0.00	0.00	0.00
LO	57.5 - 55	1 010	Max. Compression	26	-38.06	2.99	1.59
			Max. Mx	20	-15.51	963.22	0.48
						0.24	969.38
			Max. My	2 20	-15.48		
			Max. Vy		-25.46	963.22	0.48
			Max. Vx	2	-25.59	0.24	969.38
1.0	05 00 5	D-I-	Max. Torque	7	0.00	0.00	2.08
L6	95 - 92.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-39.24	3.27	1.45
			Max. Mx	20	-16.32	1027.48	0.47
			Max. My	2	-16.29	0.26	1033.92
			Max. Vy	20	-25.95	1027.48	0.47
			Max. Vx	2	-26.08	0.26	1033.92
			Max. Torque	7			2.08
L7	92.5 - 87.75	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-41.10	3.81	1.16
			Max. Mx	20	-17.49	1152.82	0.47
			Max. My	2	-17.47	0.29	1159.83
			Max. Vy	20	-26.84	1152.82	0.47
			Max. Vx	2	-26.97	0.29	1159.83
			Max. Torque	7			2.07
L8	87.75 - 84	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-43.01	4.24	0.93
			Max. Mx	20	-18.84	1254.86	0.46
			Max. My	2	-18.82	0.32	1262.31
			Max. Vy	20	-27.59	1254.86	0.46
			Max. Vx	2	-27.72	0.32	1262.31
			Max. Torque	7		3.02	2.07
L9	84 - 63.25	Pole	Max Tension	1	0.00	0.00	0.00
LJ	UT - UU.ZU	1 016	Max. Compression	26	-51.92	6.76	-0.44
			Max. Mx	20	-24.88	1866.31	0.40
			Max. My	20	-24.86 -24.87	0.50	1876.16
			-	20		1866.31	0.40
			Max. Vy Max. Vx		-31.39	0.50	
				2	-31.52	0.50	1876.16
			Max. Torque	5			2.34

Sectio n	Elevation ft	Component Type	Condition	Gov. Load	Axial K	Major Axis Moment	Minor Axis Moment
<i>No.</i> L10	63.25 -	Pole	Max Tension	Comb. 1	0.00	<i>kip-ft</i> 0.00	0.00
	56.75		Max. Compression	26	-54.81	7.58	-0.89
			Max. Mx	20	-26.91	2074.21	0.37
			Max. My	20	-26.89	0.55	2084.80
			•	20	-32.60	2074.21	0.37
			Max. Vy Max. Vx				
			Max. Vx Max. Torque	2 5	-32.73	0.55	2084.80 2.51
L11	EG 7E 17 E	Pole	Max Tension	1	0.00	0.00	0.00
LII	56.75 - 47.5	Fole	Max. Compression	26	-57.11	8.19	-1.23
			Max. Mx	20	-28.56	2231.05	0.35
			Max. My	2	-28.55	0.60	2242.18
			Max. Vy	20	-33.46	2231.05	0.35
			Max. Vx	2	-33.59	0.60	2242.18
1.40	47.5 04.05	D-I-	Max. Torque	5	0.00	0.00	2.62
L12	47.5 - 34.25	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-67.85	10.50	-2.52
			Max. Mx	20	-36.57	2854.21	0.27
			Max. My	2	-36.56	0.77	2867.34
			Max. Vy	20	-36.65	2854.21	0.27
			Max. Vx	2	-36.78	0.77	2867.34
			Max. Torque	15			-3.15
L13	34.25 - 26.75	Pole	Max Tension	1	0.00	0.00	0.00
	_00		Max. Compression	26	-71.80	11.44	-3.06
			Max. Mx	20	-39.58	3133.67	0.23
			Max. My	2	-39.57	0.85	3147.63
			Max. Vy	20	-37.90	3133.67	0.23
			Max. Vx	2	-38.02	0.85	3147.63
			Max. Torque	15			-3.48
L14	26.75 - 16.75	Pole	Max Tension	1	0.00	0.00	0.00
	10.70		Max. Compression	26	-77.09	12.65	-3.76
			Max. Mx	20	-43.70	3520.12	0.17
			Max. My	2	-43.69	0.95	3535.16
			Max. Vy	20	-39.42	3520.12	0.17
			Max. Vx	2	-39.54	0.95	3535.16
			Max. Torque	15	-00.04	0.55	-3.92
L15	16.75 -	Pole	Max Tension	1	0.00	0.00	0.00
	14.25		Max. Compression	26	-78.54	12.94	-3.93
			Max. Mx	20	-44.86	3619.09	0.15
			Max. My	2	-44.85	0.98	3634.40
			Max. Vy	20	-39.78	3619.09	0.15
			Max. Vx	2	-39.91	0.98	3634.40
			Max. Torque	_ 15			-4.03
L16	14.25 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	26	-86.60	14.53	-4.86
			Max. Mx	20	-51.48	4200.26	0.06
			Max. My	2	-51.48	1.13	4217.08
			Max. Vy	20	-41.81	4200.26	0.06
							3.00
			Max. Vx	2	-41.93	1.13	4217.08

## **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, X	Horizontal, Z
		Load	K	K	K
		Comb.			
Pole	Max. Vert	26	86.60	-0.00	-0.00
	Max. H <sub>x</sub>	20	51.49	41.79	0.00
	Max. H <sub>z</sub>	3	38.62	0.00	41.91
	Max. M <sub>x</sub>	2	4217.08	0.00	41.91
	$Max. M_z$	8	4197.96	-41.79	0.00
	Max. Torsion	3	4.64	0.00	41.91
	Min. Vert	21	38.62	41.79	0.00

Location	Condition	Gov. Load	Vertical K	Horizontal, X K	Horizontal, Z K
		Comb.			
	Min. H <sub>x</sub>	8	51.49	-41.79	0.00
	Min. H <sub>z</sub>	15	38.62	0.00	-41.91
	Min. M <sub>x</sub>	14	-4216.93	0.00	-41.91
	Min. $M_z$	20	-4200.26	41.79	0.00
	Min. Torsion	15	-4.64	0.00	-41.91

# **Tower Mast Reaction Summary**

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub>	Overturning Moment, Mz	Torque kip-ft
				kip-ft	kip-ft	
Dead Only	42.91	0.00	0.00	-0.04	0.93	-0.00
1.2 Dead+1.6 Wind 0 deg -	51.49	-0.00	-41.91	-4217.08	1.13	-4.64
No Ice						
0.9 Dead+1.6 Wind 0 deg -	38.62	-0.00	-41.91	-4170.06	0.84	-4.64
No Ice						
1.2 Dead+1.6 Wind 30 deg -	51.49	20.90	-36.30	-3652.36	-2098.52	-3.86
No Ice						
0.9 Dead+1.6 Wind 30 deg -	38.62	20.90	-36.30	-3611.55	-2075.39	-3.87
No Ice						
1.2 Dead+1.6 Wind 60 deg -	51.49	36.19	-20.96	-2108.74	-3635.61	-2.05
No Ice						
0.9 Dead+1.6 Wind 60 deg -	38.62	36.19	-20.96	-2085.17	-3595.32	-2.06
No Ice						
1.2 Dead+1.6 Wind 90 deg -	51.49	41.79	-0.00	-0.06	-4197.96	0.31
No Ice						
0.9 Dead+1.6 Wind 90 deg -	38.62	41.79	-0.00	-0.04	-4151.29	0.30
No Ice		22.12		0400.04		
1.2 Dead+1.6 Wind 120 deg	51.49	36.19	20.96	2108.61	-3635.60	2.60
- No Ice		22.12				
0.9 Dead+1.6 Wind 120 deg	38.62	36.19	20.96	2085.09	-3595.32	2.59
- No Ice	E4 40	00.00	00.00	2052.22	0000 54	4.40
1.2 Dead+1.6 Wind 150 deg	51.49	20.90	36.30	3652.22	-2098.51	4.18
- No Ice	00.00	00.00	00.00	2011 12	0075.00	4.40
0.9 Dead+1.6 Wind 150 deg	38.62	20.90	36.30	3611.46	-2075.38	4.18
- No Ice	E4 40	0.00	44.04	4040.00	4.40	4.04
1.2 Dead+1.6 Wind 180 deg	51.49	-0.00	41.91	4216.93	1.13	4.64
- No Ice	20.00	0.00	44.04	4400.07	0.04	4.04
0.9 Dead+1.6 Wind 180 deg	38.62	-0.00	41.91	4169.97	0.84	4.64
- No Ice	F4 40	20.00	20.20	2052.24	2400.70	2.00
1.2 Dead+1.6 Wind 210 deg	51.49	-20.90	36.30	3652.24	2100.79	3.86
- No Ice	20.62	20.00	26.20	2611 40	2077 07	3.87
0.9 Dead+1.6 Wind 210 deg	38.62	-20.90	36.30	3611.48	2077.07	3.07
- No Ice	51.49	-36.19	20.96	2108.63	3637.90	2.05
1.2 Dead+1.6 Wind 240 deg	31.49	-30.19	20.90	2100.03	3037.90	2.03
- No Ice	38.62	-36.19	20.96	2085.10	3597.02	2.06
0.9 Dead+1.6 Wind 240 deg - No Ice	30.02	-30.19	20.90	2000.10	3397.02	2.00
1.2 Dead+1.6 Wind 270 deg	51.49	-41.79	-0.00	-0.06	4200.26	-0.31
- No Ice	31.49	-41.79	-0.00	-0.00	4200.20	-0.31
0.9 Dead+1.6 Wind 270 deg	38.62	-41.79	-0.00	-0.04	4153.01	-0.30
- No Ice	30.02	-41.73	-0.00	-0.04	4133.01	-0.50
1.2 Dead+1.6 Wind 300 deg	51.49	-36.19	-20.96	-2108.76	3637.91	-2.59
- No Ice	31.49	-30.19	-20.90	-2100.70	3037.91	-2.59
0.9 Dead+1.6 Wind 300 deg	38.62	-36.19	-20.96	-2085.18	3597.03	-2.58
9	30.02	-30.19	-20.90	-2000.10	3397.03	-2.56
- No Ice 1.2 Dead+1.6 Wind 330 deg	51.49	-20.90	-36.30	-3652.38	2100.80	-4.18
- No Ice	31.48	-20.90	-30.30	-3032.30	2100.00	-4.10
0.9 Dead+1.6 Wind 330 deg	38.62	-20.90	-36.30	-3611.57	2077.07	-4.17
- No Ice	30.02	-20.90	-30.30	-3011.37	2011.01	<del>-4</del> .17
1.2 Dead+1.0 Ice+1.0 Temp	86.60	0.00	0.00	4.86	14.53	-0.00
1.2 Dead+1.0 ICe+1.0 Temp 1.2 Dead+1.0 Wind 0	86.60	0.00	-9.91	-1079.92	14.53 14.72	-0.00 -2.05
deg+1.0 lce+1.0 Temp	00.00	0.00	-5.51	-1013.32	17.12	-2.03
1.2 Dead+1.0 Wind 30	86.60	4.94	-8.58	-934.58	-525.94	-1.58
deg+1.0 lce+1.0 Temp	00.00	7.34	-0.50	-204.30	-525.94	-1.50
1.2 Dead+1.0 Wind 60	86.60	8.56	-4.96	-537.51	-921.73	-0.69
Dodd - 1.0 Willa 00	00.00	0.00	-4.50	-507.51	-021.70	-0.03

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear₂ K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M₂ kip-ft	Torque kip-ft
deg+1.0 Ice+1.0 Temp					•	
1.2 Dead+1.0 Wind 90	86.60	9.89	0.00	4.89	-1066.60	0.39
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 120	86.60	8.56	4.96	547.30	-921.73	1.36
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 150	86.60	4.94	8.58	944.36	-525.94	1.97
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 180	86.60	0.00	9.91	1089.70	14.72	2.05
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 210	86.60	-4.94	8.58	944.36	555.38	1.58
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 240	86.60	-8.56	4.96	547.30	951.17	0.69
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 270	86.60	-9.89	0.00	4.89	1096.04	-0.39
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 300	86.60	-8.56	-4.96	-537.51	951.17	-1.37
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 330	86.60	-4.94	-8.58	-934.58	555.38	-1.97
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	42.91	0.00	-8.97	-897.53	0.96	-0.00
Dead+Wind 30 deg - Service	42.91	4.47	-7.76	-777.30	-445.86	-0.26
Dead+Wind 60 deg - Service	42.91	7.74	-4.48	-448.86	-773.05	-0.45
Dead+Wind 90 deg - Service	42.91	8.94	0.00	-0.07	-892.68	-0.52
Dead+Wind 120 deg -	42.91	7.74	4.48	448.67	-772.95	-0.45
Service						
Dead+Wind 150 deg -	42.91	4.47	7.77	777.26	-445.91	-0.26
Service						
Dead+Wind 180 deg -	42.91	0.00	8.97	897.40	0.96	0.00
Service						
Dead+Wind 210 deg -	42.91	-4.47	7.77	777.26	447.83	0.26
Service						
Dead+Wind 240 deg -	42.91	-7.74	4.48	448.67	774.87	0.45
Service						
Dead+Wind 270 deg -	42.91	-8.94	0.00	-0.07	894.60	0.52
Service			2.00	2.0.	2200	3.02
Dead+Wind 300 deg -	42.91	-7.74	-4.48	-448.86	774.97	0.45
Service						20
Dead+Wind 330 deg -	42.91	-4.47	-7.76	-777.30	447.78	0.26
Service			0	50		5.20

# **Solution Summary**

	Sun	n of Applied Force	es		Sum of Reactio	ns	
Load	PX	' PY	PZ	PX	PY	PZ	% Error
Comb.	K	K	K	K	K	K	
1	0.00	-42.91	0.00	-0.00	42.91	-0.00	0.000%
2	0.00	-51.49	-41.91	0.00	51.49	41.91	0.003%
3	0.00	-38.62	-41.91	0.00	38.62	41.91	0.003%
4	20.90	-51.49	-36.30	-20.90	51.49	36.30	0.000%
5	20.90	-38.62	-36.30	-20.90	38.62	36.30	0.000%
6	36.19	-51.49	-20.96	-36.19	51.49	20.96	0.000%
7	36.19	-38.62	-20.96	-36.19	38.62	20.96	0.000%
8	41.79	-51.49	0.00	-41.79	51.49	0.00	0.003%
9	41.79	-38.62	0.00	-41.79	38.62	0.00	0.006%
10	36.19	-51.49	20.96	-36.19	51.49	-20.96	0.000%
11	36.19	-38.62	20.96	-36.19	38.62	-20.96	0.000%
12	20.90	-51.49	36.30	-20.90	51.49	-36.30	0.000%
13	20.90	-38.62	36.30	-20.90	38.62	-36.30	0.000%
14	0.00	-51.49	41.91	0.00	51.49	-41.91	0.003%
15	0.00	-38.62	41.91	0.00	38.62	-41.91	0.003%
16	-20.90	-51.49	36.30	20.90	51.49	-36.30	0.000%
17	-20.90	-38.62	36.30	20.90	38.62	-36.30	0.000%
18	-36.19	-51.49	20.96	36.19	51.49	-20.96	0.000%
19	-36.19	-38.62	20.96	36.19	38.62	-20.96	0.000%
20	-41.79	-51.49	0.00	41.79	51.49	0.00	0.003%
21	-41.79	-38.62	0.00	41.79	38.62	0.00	0.006%
22	-36.19	-51.49	-20.96	36.19	51.49	20.96	0.000%

	Sun	n of Applied Force			Sum of Reaction	ns	
Load	PX	PY	PZ	PX	PY	PZ	% Erro
Comb.	K	K	K	K	K	K	
23	-36.19	-38.62	-20.96	36.19	38.62	20.96	0.000%
24	-20.90	-51.49	-36.30	20.90	51.49	36.30	0.000%
25	-20.90	-38.62	-36.30	20.90	38.62	36.30	0.000%
26	0.00	-86.60	0.00	-0.00	86.60	-0.00	0.001%
27	0.00	-86.60	-9.91	-0.00	86.60	9.91	0.001%
28	4.94	-86.60	-8.58	-4.94	86.60	8.58	0.001%
29	8.56	-86.60	-4.96	-8.56	86.60	4.96	0.001%
30	9.89	-86.60	0.00	-9.89	86.60	-0.00	0.001%
31	8.56	-86.60	4.96	-8.56	86.60	-4.96	0.001%
32	4.94	-86.60	8.58	-4.94	86.60	-8.58	0.001%
33	0.00	-86.60	9.91	-0.00	86.60	-9.91	0.001%
34	-4.94	-86.60	8.58	4.94	86.60	-8.58	0.001%
35	-8.56	-86.60	4.96	8.56	86.60	-4.96	0.001%
36	-9.89	-86.60	0.00	9.89	86.60	-0.00	0.001%
37	-8.56	-86.60	-4.96	8.56	86.60	4.96	0.001%
38	-4.94	-86.60	-8.58	4.94	86.60	8.58	0.001%
39	0.00	-42.91	-8.97	-0.00	42.91	8.97	0.004%
40	4.47	-42.91	-7.77	-4.47	42.91	7.76	0.004%
41	7.74	-42.91	-4.48	-7.74	42.91	4.48	0.002%
42	8.94	-42.91	0.00	-8.94	42.91	-0.00	0.004%
43	7.74	-42.91	4.48	-7.74	42.91	-4.48	0.004%
44	4.47	-42.91	7.77	-4.47	42.91	-7.77	0.002%
45	0.00	-42.91	8.97	-0.00	42.91	-8.97	0.004%
46	-4.47	-42.91	7.77	4.47	42.91	-7.77	0.002%
47	-7.74	-42.91	4.48	7.74	42.91	-4.48	0.004%
48	-8.94	-42.91	0.00	8.94	42.91	-0.00	0.004%
49	-7.74	-42.91	-4.48	7.74	42.91	4.48	0.002%
50	-4.47	-42.91	-7.77	4.47	42.91	7.76	0.004%

# **Non-Linear Convergence Results**

Load	Converged?	Number	Displacement	Force
Combination		of Cycles	Tolerance	Tolerance
1	Yes	6	0.0000001	0.0000001
2	Yes	17	0.00003615	0.00013815
3	Yes	17	0.00002382	0.00010663
4	Yes	22	0.0000001	0.00009542
5	Yes	21	0.0000001	0.00013915
6	Yes	22	0.0000001	0.00009942
7	Yes	21	0.0000001	0.00014528
8	Yes	17	0.00003619	0.00009326
9	Yes	16	0.00004946	0.00014421
10	Yes	22	0.0000001	0.00009728
11	Yes	21	0.0000001	0.00014204
12	Yes	22	0.0000001	0.00009646
13	Yes	21	0.0000001	0.00014077
14	Yes	17	0.00003616	0.00013811
15	Yes	17	0.00002382	0.00010661
16	Yes	22	0.0000001	0.00009988
17	Yes	21	0.0000001	0.00014593
18	Yes	22	0.0000001	0.00009558
19	Yes	21	0.0000001	0.00013943
20	Yes	17	0.00003619	0.00009329
21	Yes	16	0.00004945	0.00014426
22	Yes	22	0.0000001	0.00009760
23	Yes	21	0.0000001	0.00014247
24	Yes	22	0.0000001	0.00009873
25	Yes	21	0.0000001	0.00014412
26	Yes	12	0.0000001	0.00004356
27	Yes	19	0.0000001	0.00008857
28	Yes	19	0.0000001	0.00012228
29	Yes	19	0.0000001	0.00012595
30	Yes	19	0.0000001	0.00008619
31	Yes	19	0.0000001	0.00012483
32	Yes	19	0.0000001	0.00012271
33	Yes	19	0.0000001	0.00008849
34	Yes	19	0.0000001	0.00013106

35	Yes	19	0.0000001	0.00012655
36	Yes	19	0.0000001	0.00008841
37	Yes	19	0.0000001	0.00012741
38	Yes	19	0.0000001	0.00013031
39	Yes	15	0.00012710	0.00004473
40	Yes	15	0.00012689	0.00014911
41	Yes	16	0.0000001	0.00009526
42	Yes	15	0.00012707	0.00005730
43	Yes	15	0.00012685	0.00014235
44	Yes	16	0.0000001	0.00009069
45	Yes	15	0.00012705	0.00004463
46	Yes	16	0.0000001	0.00009110
47	Yes	15	0.00012685	0.00014293
48	Yes	15	0.00012707	0.00005741
49	Yes	16	0.0000001	0.00009566
50	Yes	15	0.00012688	0.00014995

# **Maximum Tower Deflections - Service Wind**

Section No.	Elevation ft	Horz. Deflection	Gov. Load	Tilt	Twist
		in	Comb.		
L1	151 - 125.25	28.03	39	1.71	0.01
L2	125.25 - 118.5	19.18	39	1.49	0.00
L3	118.5 - 117.75	17.13	39	1.41	0.00
L4	117.75 - 97.5	16.91	39	1.40	0.00
L5	100.917 - 95	12.34	39	1.18	0.00
L6	95 - 92.5	10.90	39	1.13	0.00
L7	92.5 - 87.75	10.32	39	1.10	0.00
L8	87.75 - 84	9.25	39	1.04	0.00
L9	84 - 63.25	8.45	39	1.00	0.00
L10	63.25 - 56.75	4.70	39	0.72	0.00
L11	56.75 - 47.5	3.77	39	0.63	0.00
L12	52 - 34.25	3.17	39	0.58	0.00
L13	34.25 - 26.75	1.35	39	0.39	0.00
L14	26.75 - 16.75	0.81	39	0.30	0.00
L15	16.75 - 14.25	0.31	39	0.18	0.00
L16	14.25 - 0	0.23	39	0.15	0.00

## **Critical Deflections and Radius of Curvature - Service Wind**

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist	Radius of Curvature ft
148.00	(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	39	26.95	1.69	0.01	18054
140.00	DB854DG65ESX w/ Mount Pipe	39	24.12	1.64	0.01	8206
132.00	1900MHz RRH	39	21.37	1.57	0.00	4750
130.00	APXVSPP18-C-A20 w/ Mount Pipe	39	20.71	1.55	0.00	4298
122.00	ERICSSON AIR 21 B4A B2P w/ Mount Pipe	39	18.17	1.45	0.00	4171

## **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.	ft	Deflection	Load	۰	۰
		in	Comb.		
L1	151 - 125.25	131.40	2	8.00	0.04
L2	125.25 - 118.5	90.02	2	7.01	0.02
L3	118.5 - 117.75	80.41	2	6.61	0.01
L4	117.75 - 97.5	79.38	2	6.58	0.01
L5	100.917 - 95	57.94	2	5.56	0.01
L6	95 - 92.5	51.20	2	5.29	0.01
L7	92.5 - 87.75	48.46	2	5.18	0.01
L8	87.75 - 84	43.47	2	4.87	0.01

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L9	84 - 63.25	39.71	2	4.71	0.01
L10	63.25 - 56.75	22.08	2	3.40	0.00
L11	56.75 - 47.5	17.74	2	2.98	0.00
L12	52 - 34.25	14.91	2	2.71	0.00
L13	34.25 - 26.75	6.33	2	1.82	0.00
L14	26.75 - 16.75	3.80	2	1.40	0.00
L15	16.75 - 14.25	1.47	2	0.84	0.00
L16	14.25 - 0	1.06	2	0.71	0.00

## Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist	Radius of Curvature ft
148.00	(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	2	126.38	7.92	0.03	4037
140.00	DB854DG65ESX w/ Mount Pipe	2	113.11	7.69	0.03	1833
132.00	1900MHz RRH	2	100.28	7.38	0.02	1059
130.00	APXVSPP18-C-A20 w/ Mount Pipe	2	97.18	7.28	0.02	957
122.00	ERICSSON AIR 21 B4A B2P w/ Mount Pipe	2	85.32	6.80	0.01	921

## **Compression Checks**

**Pole Design Data** 

Section	Elevation	Size	L	Lu	KI/r	<i>A</i>	$P_u$	$\phi P_n$	Ratio
No.	ft		ft	ft		in <sup>2</sup>	K	K	Pu
						4-000			$\phi P_n$
L1	151 - 125.25 (1)	TP22.4079x17.59x0.2188	25.75	0.00	0.0	15.633 0	-8.19	1093.26	0.007
L2	125.25 - 118.5 (2)	TP23.6708x22.4079x0.35 02	6.75	0.00	0.0	26.295 9	-10.13	1260.17	0.008
L3	118.5 - ´ 117.75 (3)	TP23.8112x23.6708x0.63	0.75	0.00	0.0	47.175 6	-10.30	2198.20	0.005
L4	117.75 - 97.5 (4)	TP27.6x23.8112x0.4541	20.25	0.00	0.0	38.755 4	-13.58	1813.76	0.007
L5	97.5 - 95 (5)	TP27.6303x26.0525x0.54	5.92	0.00	0.0	47.620 0	-15.48	2231.87	0.007
L6	95 - 92.5 (6)	TP28.0981x27.6303x0.78	2.50	0.00	0.0	68.830 9	-16.29	3226.76	0.005
L7	92.5 - 87.75 (7)	TP28.9869x28.0981x0.53	4.75	0.00	0.0	48.842 3	-17.47	2291.92	0.008
L8	87.75 - 84 (8)	TP29.6885x28.9869x0.83	3.75	0.00	0.0	77.922 1	-18.82	3658.26	0.005
L9	84 - 63.25 (9)	TP33.571x29.6885x0.569	20.75	0.00	0.0	60.514 7	-24.87	2893.18	0.009
L10	63.25 - 56.75 (10)	TP34.7872x33.571x0.558 9 4.8.2 (1.03 CR) - 10	6.50	0.00	0.0	61.603 6	-26.89	2950.13	0.009
L11	56.75 - 47.5 (11)	TP36.518x34.7872x0.627	9.25	0.00	0.0	70.810 6	-28.55	3594.19	0.008
L12	47.5 - 34.25 (12)	TP38.372x34.4211x0.667	17.75	0.00	0.0	80.999 5	-36.56	4118.71	0.009
L13	34.25 - 26.75 (13)	TP39.7752x38.372x0.655	7.50	0.00	0.0	82.524 5	-39.57	4200.93	0.009
L14	26.75 - 16.75 (14)	TP41.6461x39.7752x0.64	10.00	0.00	0.0	84.580 8	-43.69	4313.28	0.010
L15	16.75 - 14.25 (15)	TP42.1139x41.6461x0.72	2.50	0.00	0.0	96.867	-44.85	4737.72	0.009
L16	14.25 - 0 (16)	TP44.78x42.1139x0.6927	14.25	0.00	0.0	98.340	-51.48	4866.66	0.011

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	KI/r	A in²	P <sub>u</sub> K	φ <b>P</b> <sub>n</sub> <b>K</b>	Ratio P <sub>u</sub>
									φ <b>P</b> <sub>n</sub>

# Pole Bending Design Data

Section	Elevation	Size	M <sub>ux</sub>	$\phi M_{nx}$	Ratio	M <sub>uy</sub>	$\phi M_{ny}$	Ratio
No.	ft		kip-ft	kip-ft	M <sub>ux</sub>	kip-ft	kip-ft	Muy
	151 105 05				φ <i>M</i> <sub>nx</sub>		100.00	φ <i>M</i> <sub>ny</sub>
L1	151 - 125.25 (1)	TP22.4079x17.59x0.2188	287.69	493.26	0.583	0.00	493.26	0.000
L2	125.25 - 118.5 (2)	TP23.6708x22.4079x0.35 02	418.98	594.53	0.705	0.00	594.53	0.000
L3	118.5 - 117.75 (3)	TP23.8112x23.6708x0.63 21	435.07	1018.48	0.427	0.00	1018.48	0.000
L4	117.75 - 97.5 (4)	TP27.6x23.8112x0.4541	821.39	970.58	0.846	0.00	970.58	0.000
L5	97.5 - 95 (5)	TP27.6303x26.0525x0.54 6	969.38	1216.72	0.797	0.00	1216.72	0.000
L6	95 - 92.5 (6)	TP28.0981x27.6303x0.78 26	1033.93	1759.48	0.588	0.00	1759.48	0.000
L7	92.5 - 87.75 (7)	TP28.9869x28.0981x0.53 31	1159.83	1314.47	0.882	0.00	1314.47	0.000
L8	87.75 - 84 (8)	TP29.6885x28.9869x0.83 88	1262.31	2105.93	0.599	0.00	2105.93	0.000
L9	84 - 63.25 (9)	TP33.571x29.6885x0.569 5	1876.16	1927.33	0.973	0.00	1927.33	0.000
L10	63.25 - 56.75 (10)	TP34.7872x33.571x0.558	2084.80	2040.17	1.022	0.00	2040.17	0.000
L11	56.75 - 47.5 (11)	TP36.518x34.7872x0.627	2242.18	2541.21	0.882	0.00	2541.21	0.000
L12	47.5 - 34.25 (12)	TP38.372x34.4211x0.667	2867.34	3133.40	0.915	0.00	3133.40	0.000
L13	34.25 - 26.75 (13)	TP39.7752x38.372x0.655	3147.63	3319.00	0.948	0.00	3319.00	0.000
L14	26.75 - 16.75 (14)	TP41.6461x39.7752x0.64 06	3535.16	3575.97	0.989	0.00	3575.97	0.000
L15	16.75 - 14.25 (15)	TP42.1139x41.6461x0.72	3634.40	3956.82	0.919	0.00	3956.82	0.000
L16	14.25 - 0 (16)	TP44.78x42.1139x0.6927	4217.07	4337.58	0.972	0.00	4337.58	0.000

## Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V <sub>u</sub>	φV <sub>n</sub> Κ	Ratio V <sub>u</sub>	Actual T <sub>u</sub>	φΤ <sub>n</sub> kip-ft	Ratio T <sub>u</sub>
			ĸ	•	$\overline{\phi V_n}$	kip-ft	,	$\phi T_n$
L1	151 - 125.25 (1)	TP22.4079x17.59x0.2188	18.36	546.63	0.034	0.01	1000.18	0.000
L2	125.25 - 118.5 (2)	TP23.6708x22.4079x0.35 02	21.39	630.09	0.034	0.17	1205.52	0.000
L3	118.5`- <sup>′</sup> 117.75 (3)	TP23.8112x23.6708x0.63 21	21.53	1099.10	0.020	0.19	2065.17	0.000
L4	117.75 - 97.5 (4)	TP27.6x23.8112x0.4541	24.43	906.88	0.027	0.65	1968.04	0.000
L5	97.5 - 95 (5)	TP27.6303x26.0525x0.54 6	25.59	1115.93	0.023	0.83	2467.13	0.000
L6	95 - 92.5 (6)	TP28.0981x27.6303x0.78 26	26.08	1613.38	0.016	0.92	3567.68	0.000
L7	92.5 - 87.75 (7)	TP28.9869x28.0981x0.53	26.97	1145.96	0.024	1.10	2665.35	0.000
L8	87.75 - 84 (8)	TP29.6885x28.9869x0.83 88	27.72	1829.13	0.015	1.25	4270.17	0.000
L9	84 - 63.25 (9)	TP33.571x29.6885x0.569	31.52	1446.59	0.022	1.99	3908.01	0.001
L10	63.25 - 56.75 (10)	TP34.7872x33.571x0.558	32.73	1475.06	0.022	2.28	4136.82	0.001

Section No.	Elevation ft	Size	Actual V <sub>u</sub>	φ <i>V<sub>n</sub> K</i>	Ratio V <sub>u</sub>	Actual T <sub>u</sub>	φΤ <sub>n</sub> kip-ft	Ratio T <sub>u</sub>
710.			K	K	$\frac{V_0}{\phi V_n}$	kip-ft	КΙΡ-Ι	$\frac{T_u}{\phi T_n}$
L11	56.75 - 47.5 (11)	TP36.518x34.7872x0.627	33.59	1797.10	0.019	2.47	5152.78	0.000
L12	47.5 - 34.25 (12)	TP38.372x34.4211x0.667	36.78	2059.35	0.018	3.14	6353.55	0.000
L13	34.25 - 26.75 (13)	TP39.7752x38.372x0.655	38.02	2100.47	0.018	3.48	6729.89	0.001
L14	26.75 - 16.75 (14)	TP41.6461x39.7752x0.64 06	39.54	2156.64	0.018	3.92	7250.95	0.001
L15	16.75 - 14.25 (15)	TP42.1139x41.6461x0.72 69	39.91	2368.86	0.017	4.02	8023.21	0.001
L16	14.25 - 0 (16)	TP44.78x42.1139x0.6927	41.93	2433.33	0.017	4.64	8795.25	0.001

# **Pole Interaction Design Data**

Section No.	Elevation ft	Ratio P <sub>u</sub>	Ratio M <sub>ux</sub>	Ratio M <sub>uy</sub>	Ratio V <sub>u</sub>	Ratio T <sub>u</sub>	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	151 - 125.25 (1)	φ <i>P<sub>n</sub></i> 0.007	φ <i>M<sub>nx</sub></i> 0.583	φ <i>M<sub>ny</sub></i> 0.000	φ <i>V<sub>n</sub></i> 0.034	φ <i>T</i> <sub>n</sub> 0.000	0.592	1.000	4.8.2
L2	125.25 - 118.5 (2)	0.008	0.705	0.000	0.034	0.000	0.714	1.000	4.8.2 🗸
L3	118.5 - 117.75 (3)	0.005	0.427	0.000	0.020	0.000	0.432	1.000	4.8.2 🗸
L4	117.75 - 97.5 (4)	0.007	0.846	0.000	0.027	0.000	0.855	1.000	4.8.2 🗸
L5	97.5 - 95 (5)	0.007	0.797	0.000	0.023	0.000	0.804	1.000	4.8.2 🗸
L6	95 - 92.5 (6)	0.005	0.588	0.000	0.016	0.000	0.593	1.000	4.8.2 🗸
L7	92.5 - 87.75 (7)	0.008	0.882	0.000	0.024	0.000	0.891	1.000	4.8.2 🗸
L8	87.75 - 84 (8)	0.005	0.599	0.000	0.015	0.000	0.605	1.000	4.8.2
L9	84 - 63.25 (9)	0.009	0.973	0.000	0.022	0.001	0.983	1.000	4.8.2
L10	63.25 - 56.75 (10)	0.009	1.022	0.000	0.022	0.001	1.032	1.000	4.8.2
L11	56.75 - 47.5 (11)	0.008	0.882	0.000	0.019	0.000	0.891	1.000	4.8.2 🖊
L12	47.5 - 34.25 (12)	0.009	0.915	0.000	0.018	0.000	0.924	1.000	4.8.2 🖊
L13	34.25 - 26.75 (13)	0.009	0.948	0.000	0.018	0.001	0.958	1.000	4.8.2 🗸
L14	26.75 - 16.75 (14)	0.010	0.989	0.000	0.018	0.001	0.999	1.000	4.8.2
L15	16.75 - 14.25 (15)	0.009	0.919	0.000	0.017	0.001	0.928	1.000	4.8.2
L16	14.25 - 0 (16)	0.011	0.972	0.000	0.017	0.001	0.983	1.000	4.8.2 🖊

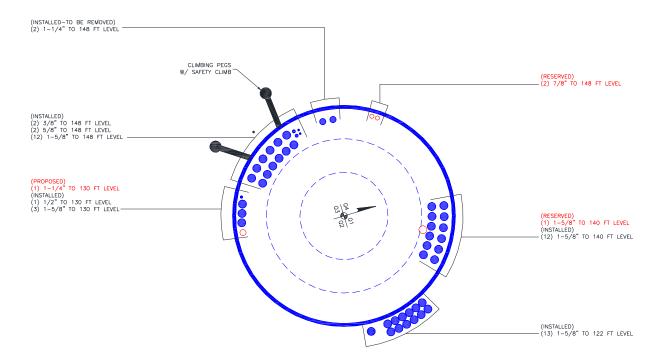
# **Section Capacity Table**

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP <sub>allow</sub> K	% Capacity	Pass Fail
L1	151 - 125.25	Pole	TP22.4079x17.59x0.2188	1	-8.19	1093.26	59.2	Pass
L2	125.25 - 118.5	Pole	TP23.6708x22.4079x0.3502	2	-10.13	1260.17	71.4	Pass
L3	118.5 - 117.75	Pole	TP23.8112x23.6708x0.6321	3	-10.30	2198.20	43.2	Pass
L4	117.75 - 97.5	Pole	TP27.6x23.8112x0.4541	4	-13.58	1813.76	85.5	Pass

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	øP <sub>allow</sub> K	% Capacity	Pass Fail
L5	97.5 - 95	Pole	TP27.6303x26.0525x0.546	5	-15.48	2231.87	80.4	Pass
L6	95 - 92.5	Pole	TP28.0981x27.6303x0.7826	6	-16.29	3226.76	59.3	Pass
L7	92.5 - 87.75	Pole	TP28.9869x28.0981x0.5331	7	-17.47	2291.92	89.1	Pass
L8	87.75 - 84	Pole	TP29.6885x28.9869x0.8388	8	-18.82	3658.26	60.5	Pass
L9	84 - 63.25	Pole	TP33.571x29.6885x0.5695	9	-24.87	2893.18	98.3	Pass
L10	63.25 - 56.75	Pole	TP34.7872x33.571x0.5589	10	-26.89	2950.13	103.2	Pass*
L11	56.75 - 47.5	Pole	TP36.518x34.7872x0.6274	11	-28.55	3594.19	89.1	Pass
L12	47.5 - 34.25	Pole	TP38.372x34.4211x0.6672	12	-36.56	4118.71	92.4	Pass
L13	34.25 - 26.75	Pole	TP39.7752x38.372x0.6551	13	-39.57	4200.93	95.8	Pass
L14	26.75 - 16.75	Pole	TP41.6461x39.7752x0.6406	14	-43.69	4313.28	99.9	Pass
L15	16.75 - 14.25	Pole	TP42.1139x41.6461x0.7269	15	-44.85	4737.72	92.8	Pass
L16	14.25 - 0	Pole	TP44.78x42.1139x0.6927	16	-51.48	4866.66	98.3	Pass
							Summary	
						Pole (L10)	103.2	Pass*
						RATING =	103.2	Pass*

<sup>\*</sup>See Note 2 of Table 6.

# APPENDIX B BASE LEVEL DRAWING



# APPENDIX C ADDITIONAL CALCULATIONS

#### 22.4079 25.75 7 12 A572-65 125.3 ft ķ 123.6708 23.67022.4079 12 9.0 Reinf 41.09Resinf 42.26 118.5 ft 0.75 က 23.81 0.4541 20.25 3.42 12 š k Bie inf 4Re B 674 auf 174 ks 374 ceinf 41.27 5.92 97.5 ft 628009525 12 95.0 ft .5330,7826 2.50 12 92.5 ft 986**9**8.09**87** 4.75 8.0 .688288 87.8 ft 83880 3.75 0.1 12 84.0 ft Reinf 41.40 33.5710 29.6885 20.75 0.4 12 ķ Reinf 42.16 63.3 ft 6.50 12 44.76 ksiReinf 42.23 ksi 56.8 ft 34.7872 36.5180 0.6274 9.25 4.50 7 12 2.2 47.5 ft Reinf 38.3720 0.6672 12 12 4.6 .84 ksi 44.89 ksi Reinf 44. 34.3 ft 38.3720 39.7752 7.50 12 2.1 26.8 ft 39.7752 41.6461 k43.13 Reinf 44.97 Reinf 10.00 4 12 2.8 16.8 ft .6461 7269 15 2.50 12 9.0 14.3 ft 42.1139 44.7800 48604F 0.6927 14.25 16 12 4.6 Reinf 0.0 ft 30.3 Number of Sides Socket Length Thickness (in) Bot Dia (in) Top Dia (in) Weight (K) Length ( Grade

#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
(2) AM-X-CD-16-65-00T-RET w/ Mount	148	2.375" OD x 6' Mount Pipe	140
Pipe		2.375" OD x 6' Mount Pipe	140
(2) AM-X-CD-16-65-00T-RET w/ Mount	148	Platform Mount [LP 303-1]	140
Pipe		1900MHz RRH	132
(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	148	1900MHz RRH	132
800 10121 w/ Mount Pipe	148	1900MHz RRH	132
800 10121 w/ Mount Pipe	148	800 EXTERNAL NOTCH FILTER	132
800 10121 w/ Mount Pipe	148	800 EXTERNAL NOTCH FILTER	132
(2) CG-1900DD-FULL-DIN	148	800 EXTERNAL NOTCH FILTER	132
(2) CG-1900DD-FULL-DIN	148	800MHZ RRH	132
(2) CG-1900DD-FULL-DIN	148	800MHZ RRH	132
(4) LGP21901	148	800MHZ RRH	132
(4) LGP21901	148	Pipe Mount [PM 601-3]	132
(4) LGP21901	148	APXVSPP18-C-A20 w/ Mount Pipe	130
(2) DTMABP7819VG12A	148	APXVSPP18-C-A20 w/ Mount Pipe	130
(2) DTMABP7819VG12A	148	APXVSPP18-C-A20 w/ Mount Pipe	130
(2) DTMABP7819VG12A	148	GPS-TMG-HR-26NCM	130
860 10025	148	APXVTM14-C-120 w/ Mount Pipe	130
860 10025	148	APXVTM14-C-120 w/ Mount Pipe	130
860 10025	148	APXVTM14-C-120 w/ Mount Pipe	130
(2) RRUS-11	148	TD-RRH8x20-25	130
(2) RRUS-11	148	TD-RRH8x20-25	130
(2) RRUS-11	148	TD-RRH8x20-25	130
DC6-48-60-18-8F	148	2.375" OD x 6' Mount Pipe	130
DS1F03F36D-N	148	2.375" OD x 6' Mount Pipe	130
Platform Mount [LP 602-1]	148	2.375" OD x 6' Mount Pipe	130
DB854DG65ESX w/ Mount Pipe	140	T-Arm Mount [TA 602-3]	130
DB854DG65ESX w/ Mount Pipe	140	ERICSSON AIR 21 B4A B2P w/ Mount	122
DB854DG65ESX w/ Mount Pipe	140	Pipe	
(2) SBNHH-1D65B w/ Mount Pipe	140	ERICSSON AIR 21 B4A B2P w/ Mount	122
(2) SBNHH-1D65B w/ Mount Pipe	140	Pipe	122
(2) SBNHH-1D65B w/ Mount Pipe	140	ERICSSON AIR 21 B4A B2P w/ Mount Pipe	122
RRH4X45-AWS4 B66	140	ERICSSON AIR 21 B2A B4P w/ Mount	122
RRH4X45-AWS4 B66	140	Pipe	·
RRH4X45-AWS4 B66	140	ERICSSON AIR 21 B2A B4P w/ Mount	122
RRH2x60-700	140	Pipe	
RRH2x60-700	140	ERICSSON AIR 21 B2A B4P w/ Mount	122
RRH2x60-700	140	Pipe	
DB-T1-6Z-8AB-0Z	140	T-Arm Mount [TA 702-3]	122
2.375" OD x 6' Mount Pipe	140	-	

**MATERIAL STRENGTH** 

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-65	65 ksi	80 ksi	Reinf 42.16 ksi	42 ksi	53 ksi
Reinf 42.26 ksi	42 ksi	53 ksi	Reinf 42.23 ksi	42 ksi	53 ksi
Reinf 41.09 ksi	41 ksi	52 ksi	Reinf 44.76 ksi	45 ksi	56 ksi
Reinf 41.27 ksi	41 ksi	52 ksi	Reinf 44.84 ksi	45 ksi	57 ksi
Reinf 41.33 ksi	41 ksi	52 ksi	Reinf 44.89 ksi	45 ksi	57 ksi
Reinf 41.34 ksi	41 ksi	52 ksi	Reinf 44.97 ksi	45 ksi	57 ksi
Reinf 41.38 ksi	41 ksi	52 ksi	Reinf 43.13 ksi	43 ksi	54 ksi
Reinf 41.40 ksi	41 ksi	52 ksi	Reinf 43.64 ksi	44 ksi	55 ksi

#### **TOWER DESIGN NOTES**

- 1. Tower is located in New Haven County, Connecticut.
- 2. Tower designed for Exposure C to the TIA-222-G Standard.
- Tower designed for a 97.00 mph basic wind in accordance with the TIA-222-G Standard.
- Tower is also designed for a 50.00 mph basic wind with 0.75 in ice. Ice is considered to increase in thickness with height.
- MOME5. Deflections are based upon a 60.00 mph wind.
- 1097 ki<sub>l</sub>6. Topographic Category 1 with Crest Height of 0.00 ft

TORQUE 2 kip-ft 50.00 mph WIND - 0.7500 in ICE AXIAL

**ALL REACTIONS** 

ARE FACTORED

AXIAL

87 K

SHEAR

10 K

51 K MOMENT SHEAR 42 K 4217 kip-ft

TORQUE 5 kip-ft REACTIONS - 97.00 mph WIND

#### Tower Structure Class II.

8. TOWER RATING: 103.2%

Paul J. Ford and Company 250 East Broad Street, Suite 600 Columbus. Ohio 43215 Phone: 614-221-6679

FAX:

lob: 151 Ft. Monopole / Bethany, CT							
Project: PJF 37517-07559.003	/ BU 841295						
Client: Crown Castle	Drawn by: nmiller	App'd:					
Code: TIA-222-G	Date: 08/10/17	Scale: NTS					
Path:		Dwg No. E-					



v4.4 - Effective 7-12-13

Date: 8/10/2017

PJF Project: 37517-0759.003.7805 Client Ref. # BU 841295

Site Name: Bethany Description: 151' MP Owner: Crown Castle Engineer: NCM

## Asymmetric Anchor Rod Analysis

Moment = 4217 Axial = 51.0 kips 42.0 Shear = kips Anchor Qty = 18

TIA Ref. G ASIF = N/A Max Ratio = 105.0%

Location = η = Threads = Base Plate 0.50 N/A

for BP, Rev. G Sect. 4.9.9 for FP, Rev. G

Item	Nominal Anchor Dia, in	Spec	Fy, ksi	Fu, ksi	Location, degrees	Anchor Circle, in	Area Override, in <sup>2</sup>	Area, in <sup>2</sup>	Max Net Compressio n, kips	Max Net Tension, kips	Load for Capacity Calc, kips	Capacity Override, kips	Capacity, kips	Capacity Ratio
1	2.250	#18J A615 Gr 75	75	100	0.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
2	2.250	#18J A615 Gr 75	75	100	30.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
3	2.250	#18J A615 Gr 75	75	100	60.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
4	2.250	#18J A615 Gr 75	75	100	90.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
5	2.250	#18J A615 Gr 75	75	100	120.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
6	2.250	#18J A615 Gr 75	75	100	150.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
7	2.250	#18J A615 Gr 75	75	100	180.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
8	2.250	#18J A615 Gr 75	75	100	210.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
9	2.250	#18J A615 Gr 75	75	100	240.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
10	2.250	#18J A615 Gr 75	75	100	270.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
11	2.250	#18J A615 Gr 75	75	100	300.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
12	2.250	#18J A615 Gr 75	75	100	330.0	52.75	0.00	3.98	213.09	207.50	217.69	0.00	260.00	83.7%
13	2.250	Williams R71	127.7	150	45.0	52.75	0.00	4.14	221.87	216.05	226.66	0.00	489.60	46.3%
14	2.250	Williams R71	127.7	150	105.0	52.75	0.00	4.14	221.87	216.05	226.66	0.00	489.60	46.3%
15	2.250	Williams R71	127.7	150	165.0	52.75	0.00	4.14	221.87	216.05	226.66	0.00	489.60	46.3%
16	2.250	Williams R71	127.7	150	225.0	52.75	0.00	4.14	221.87	216.05	226.66	0.00	489.60	46.3%
17	2.250	Williams R71	127.7	150	285.0	52.75	0.00	4.14	221.87	216.05	226.66	0.00	489.60	46.3%
18	2.250	Williams R71	127.7	150	345.0	52.75	0.00	4.14	221.87	216.05	226.66	0.00	489.60	46.3%

## Stiffened or Unstiffened, Ungrouted, Circular Base Plate - Any Rod Material

**TIA Rev G** <u>Assumption:</u> Clear space between bottom of leveling nut and top of concrete **not** exceeding (1)\*(Rod Diameter)

Site Data	•	
BU#:		
Site Name:		
App #:		
Pole M	anufacturer:	Other

Anchor Rod Data						
Qty:	24					
Diam:	2.25	in				
Rod Material:	A615-J					
Strength (Fu):	100	ksi				
Yield (Fy):	75	ksi				
Bolt Circle:	52.75	in				

Plate Data						
Diam:	58.75	in				
Thick:	3	in				
Grade:	50	ksi				
Single-Rod B-eff:	6.00	in				

Stiffener Data (Welding at both sides)						
Config:	0	*				
Weld Type:						
Groove Depth:		in **				
Groove Angle:		degrees				
Fillet H. Weld:		< Disregard				
Fillet V. Weld:		in				
Width:		in				
Height:		in				
Thick:		in				
Notch:		in				
Grade:		ksi				
Weld str.:		ksi				

Pole Data						
Diam:	44.78	in				
Thick:	0.375	in				
Grade:	65	ksi				
# of Sides:	12	"0" IF Round				
Fu	80	ksi				
Reinf. Fillet Weld	0	"0" if None				

Reactions						
Mu:	5829.7981	ft-kips				
Axial, Pu:	51	kips				
Shear, Vu:	42	kips				
Eta Factor, η	0.5	TIA G (Fig. 4-4)				

Reactions and anchor quantity adjusted to account for post installed anchors.

If No stiffeners, Criteria:	AISC LRFD	<-Only Applcable to Unstiffened Cases

Anchor Rod Results		Rigid
Max Rod (Cu+ Vu/ή):	226.7 Kips	AISC LRFD
Allowable Axial, Φ*Fu*Anet:	260.0 Kips	φ*Tn

See Asymmetric Anchor Rod Analysis for Anchor Capacity Usage.

Base Plate Results	Flexural Check
Base Plate Stress:	39.5 ksi
Allowable Plate Stress:	45.0 ksi
Base Plate Stress Ratio:	87.8% Pass

Rigid
AISC LRFD
φ*Fy
Y.L. Length:
27.88

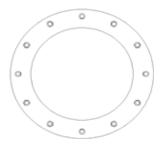
#### <u>n/a</u>

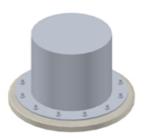
#### **Stiffener Results**

Horizontal Weld: n/a
Vertical Weld: n/a
Plate Flex+Shear, fb/Fb+(fv/Fv)^2: n/a
Plate Tension+Shear, ft/Ft+(fv/Fv)^2: n/a
Plate Comp. (AISC Bracket): n/a

#### **Pole Results**

Pole Punching Shear Check: n/a





<sup>\* 0 =</sup> none, 1 = every bolt, 2 = every 2 bolts, 3 = 2 per bolt

<sup>\*\*</sup> Note: for complete joint penetration groove welds the groove depth must be exactly 1/2 the stiffener thickness for calculation purposes

PJF Job No. **37517-0759.003** 

Project Name: **BU 841295** 

NCM Engineer:

page 1

### **Factored Foundation Loads:**

Factored Axial Load (+Comp, -Ten) = Factored Horiz. Load at Top of Pier = Factored OTM at Top of Pier =

LOI	LUZ	
51	38.25	kips
42	42	kips
4217	4217	kips

## **LRFD Resistance and Load Factors:**

Φ
0.75
0.75

0.75

Dead	Load	l Factors

Dodd Loc	ad I doloro
1.2	0.9
1.2	0.9

## Concrete Weight = **Soil Properties:**

Soil Bearing =

Soil Weight =

Depth to Water Table =
Uplift Cone from

3.3	ft
Тор	of footing

Layer	Soil	Cohesion	Friction	Ult	Depth
Thk	Density		Angle	Bearing	
ft	pcf	ksf	degrees	ksf	ft
2	110	0	30		2.00
4	115	0	30		6.00
-	•	. •	• •		0.00
6	130	0	30	30	12.00

#### Dimensions:

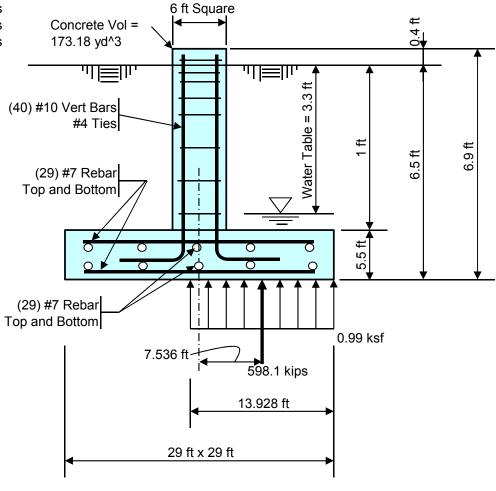
Pier Shape =	Square	
Pier Width =	6	ft Square
Pier Height above Grade =	0.4	ft
Depth to Bottom of Footing =	6.5	ft
Footing Thickness =	5.5	ft
Footing Width, B =	29	ft
Footing Length, L =	29	ft

#### Concrete:

Concrete Strength =	3	ksi
Rebar Strength =	60	ksi

### **Summary Results:**

-	Required		Available	
Maximum Net Soil Bearing =	1.089	ksf	22.500	ksf
Uplift =	0.0	kips	469.3	kips
Punching Shear Stress =	0.017	ksi	0.164	ksi
Bending Shear Stress =	191.7	kips	1677.9	kips
Bending Moment =	1822	k-ft	4549.2	k-ft
Conc Pier Reinforcing Steel =	4275.8	k-ft	7428.7	k-ft



Total Pad Reinf Stl =	34.80	in^2 < 41.34 in^2 = Min Stl
Total Pier Reinf Stl =	50.80	in^2 >= 25.92 in^2 = Min Stl, OK
Footing Thickness =	5.50	ft >= 2.12 ft = Min Ftg Thk, OK

Stress Ratio =	4.8%	in Soil Bearing
Stress Ratio =	0.0%	in Uplift
Stress Ratio =	10.6%	in Punching Shear
Stress Ratio =	11.4%	in Bending Shear
Stress Ratio =	40.1%	in Bending Moment
Stress Ratio =	57.6%	in Pier Rebar



## RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

## **SPRINT Existing Facility**

Site ID: CT33XC515

Bethany-Spectrasite Tower 719 Amity Road Bethany, CT 06524

**September 10, 2017** 

EBI Project Number: 6217003918

Site Compliance Summary				
Compliance Status:	COMPLIANT			
Site total MPE% of				
FCC general	9.33 %			
population 9.55 %				
allowable limit:				



September 10, 2017

SPRINT Attn: RF Engineering Manager 1 International Boulevard, Suite 800 Mahwah, NJ 07495

Emissions Analysis for Site: CT33XC515 – Bethany-Spectrasite Tower

EBI Consulting was directed to analyze the proposed SPRINT facility located at **719 Amity Road**, **Bethany**, **CT**, for the purpose of determining whether the emissions from the Proposed SPRINT Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm2). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 850 MHz Band is approximately 567  $\mu$ W/cm². The general population exposure limit for the 1900 MHz (PCS) and 2500 MHz (BRS) bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

#### **CALCULATIONS**

Calculations were done for the proposed SPRINT Wireless antenna facility located at **719 Amity Road**, **Bethany**, **CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since SPRINT is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 1 CDMA channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 2) 2 LTE channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 3) 5 CDMA channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 16 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 5) 8 LTE channels (2500 MHz (BRS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the RFS APXVSPP18-C-A20 and the RFS APXVTM14-C-120 for transmission in the 850 MHz, 1900 MHz (PCS) and 2500 MHz (BRS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed antennas are **130 feet** above ground level (AGL) for **Sector A**, **130 feet** above ground level (AGL) for **Sector B** and **130 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general population threshold limits.



#### **SPRINT Site Inventory and Power Data by Antenna**

Sector:	A	Sector:	В	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	RFS APXVSPP18-C-A20	Make / Model:	RFS APXVSPP18-C-A20	Make / Model:	RFS APXVSPP18-C-A20
Gain:	13.4 / 15.9 dBd	Gain:	13.4 / 15.9 dBd	Gain:	13.4 / 15.9 dBd
Height (AGL):	130 feet	Height (AGL):	130 feet	Height (AGL):	130 feet
Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)	Frequency Bands	850 MHz / 1900 MHz (PCS)
Channel Count	10	Channel Count	10	Channel Count	10
Total TX Power(W):	220 Watts	Total TX Power(W):	220 Watts	Total TX Power(W):	220 Watts
ERP (W):	7,537.38	ERP (W):	7,537.38	ERP (W):	7,537.38
Antenna A1 MPE%	2.00 %	Antenna B1 MPE%	2.00 %	Antenna C1 MPE%	2.00 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVTM14-C-120	Make / Model:	RFS APXVTM14-C-120	Make / Model:	RFS APXVTM14-C-120
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	130 feet	Height (AGL):	130 feet	Height (AGL):	130 feet
Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)	Frequency Bands	2500 MHz (BRS)
Channel Count	8	Channel Count	8	Channel Count	8
Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts
ERP (W):	6,224.72	ERP (W):	6,224.72	ERP (W):	6,224.72
Antenna A2 MPE%	1.45 %	Antenna B2 MPE%	1.45 %	Antenna C2 MPE%	1.45 %

Site Composite MPE%			
Carrier	MPE%		
SPRINT – Max per sector	3.45 %		
Beth Fire Dept	0.04 %		
Beth Hwy Dept	0.07 %		
AT&T	1.86 %		
Verizon Wireless	3.88 %		
T-Mobile	0.03 %		
Site Total MPE %:	9.33 %		

SPRINT Sector A Total:	3.45 %
SPRINT Sector B Total:	3.45 %
SPRINT Sector C Total:	3.45 %
Site Total:	9.33 %

SPRINT _ Max Values per Frequency Band / Technology Per Sector	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
Sprint 850 MHz CDMA	1	437.55	130	1.02	850 MHz	567	0.18%
Sprint 850 MHz LTE	2	437.55	130	2.05	850 MHz	567	0.36%
Sprint 1900 MHz (PCS) CDMA	5	622.47	130	7.28	1900 MHz (PCS)	1000	0.73%
Sprint 1900 MHz (PCS) LTE	2	1,556.18	130	7.28	1900 MHz (PCS)	1000	0.73%
Sprint 2500 MHz (BRS) LTE	8	778.09	130	14.55	2500 MHz (BRS)	1000	1.45%
						Total:	3.45%



## **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the SPRINT facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

SPRINT Sector	Power Density Value (%)		
Sector A:	3.45 %		
Sector B:	3.45 %		
Sector C:	3.45 %		
SPRINT Maximum	3.45 %		
Total (per sector):			
Site Total:	9.33 %		
Site Compliance Status:	COMPLIANT		

The anticipated composite MPE value for this site assuming all carriers present is **9.33** % of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.