

STATE OF CONNECTICUT
CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051

Phone: (860) 827-2935 Fax: (860) 827-2950

E-Mail: siting.council@ct.gov

www.ct.gov/csc

December 10, 2013

Kenneth C. Baldwin, Esq.
Robinson & Cole LLP
280 Trumbull Street
Hartford, CT 06103

RE: **EM-VER-007-131122** – Cellco Partnership d/b/a Verizon Wireless notice of intent to modify an existing telecommunications facility located at 1657 Wilbur Cross Parkway, Berlin, Connecticut.

Dear Attorney Baldwin:

The Connecticut Siting Council (Council) hereby acknowledges your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the following conditions:

- Any deviation from the proposed modification as specified in this notice and supporting materials with the Council shall render this acknowledgement invalid;
- Any material changes to this modification as proposed shall require the filing of a new notice with the Council;
- Within 45 days after completion of construction, the Council shall be notified in writing that construction has been completed;
- The validity of this action shall expire one year from the date of this letter; and
- The applicant may file a request for an extension of time beyond the one year deadline provided that such request is submitted to the Council not less than 60 days prior to the expiration.

The proposed modifications including the placement of all necessary equipment and shelters within the tower compound are to be implemented as specified here and in your notice dated November 21, 2013. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Thank you for your attention and cooperation.

Very truly yours,

Melanie A. Bachman
Acting Executive Director

MAB/CDM/jb

c: The Honorable Rachel Rochette, Mayor, Town of Berlin
Denise McNair, Town Manager, Town of Berlin
Arthur Simonian, Town Engineer, Town of Berlin
Berlin Volunteer Fire Department



KENNETH C. BALDWIN

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts

May 13, 2015

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **EM-VER-007-131122 – 1657 Wilbur Cross Parkway, Berlin, Connecticut**
EM-VER-064-131004 – 219 New Park Avenue, Hartford, Connecticut
TS-VER-115-131009 – 37 Peach Orchard Road, Prospect, Connecticut
EM-VER-118-131213 – 76 East Ridge Road, Ridgefield, Connecticut

Completion of Construction Activity

Dear Ms. Bachman:

The purpose of this letter is to notify the Siting Council that construction activity associated with the above-referenced Cellco Partnership d/b/a Verizon Wireless telecommunications facilities has been completed.

If you have any questions or need any additional information regarding these facilities please do not hesitate to contact me.

Sincerely,



Kenneth C. Baldwin

Copy to:
Tim Parks

280 Trumbull Street
Hartford, CT 06103-3597
Main (860) 275-8200
Fax (860) 275-8299
kbaldwin@rc.com
Direct (860) 275-8345

Also admitted in Massachusetts

November 21, 2013

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **Notice of Exempt Modification – Antenna Swap
1657 Wilbur Cross Parkway, Berlin, Connecticut**

Dear Ms. Bachman:

Cellco Partnership d/b/a Verizon Wireless (“Cellco”) currently maintains twelve (12) wireless telecommunications antennas at the 114-foot level on the existing 176-foot tower at the above-referenced address. The tower and underlying property are owned Berlin Volunteer Fire Department. The Council approved Cellco’s shared use of this tower in 2006. Cellco now intends to replace six (6) of its existing antennas with three (3) model BXA-70063-6BF cellular antennas and three (3) model BXA-171063-12CF AWS antennas, all at the same level. Cellco also intends to install three (3) remote radio heads (“RRHs”) behind its AWS antennas and one (1) HYBRIFLEX™ antenna cable. Included in Attachment 1 are specifications for the replacement antennas, RRHs and HYBRIFLEX™ cable.

Please accept this letter as notification pursuant to R.C.S.A. § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to Denise McNair, Town Manager for the Town of Berlin. A copy of this letter is also being sent to the Berlin Volunteer Fire Department.

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. § 16-50j-72(b)(2).



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Melanie A. Bachman
November 21, 2013
Page 2

1. The proposed modifications will not result in an increase in the height of the existing tower. Cellco's replacement antennas and RRHs will be located at the 114-foot level on the 176-foot tower.
2. The proposed modifications will not involve any change to ground-mounted equipment and, therefore, will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the modified facility will not increase radio frequency (RF) emissions at the facility to a level at or above the Federal Communications Commission (FCC) safety standard. A cumulative General Power Density table for the facility, as modified, is included behind Tab 2.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The tower and its foundation can support Cellco's proposed modifications. (*See Detailed Structural Analysis and Evaluation included in Attachment 3*).

For the foregoing reasons, Cellco respectfully submits that the proposed modifications to the above-referenced telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,



Kenneth C. Baldwin

Enclosures

Copy to:

Denise McNair, Berlin Town Manager
Berlin Volunteer Fire Department
Sandy M. Carter



ATTACHMENT 1

BXA-70063-6BF-EDIN-X

X-Pol | FET Panel | 63° | 14.5 dBd

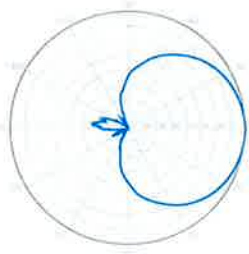
Replace "X" with desired electrical downtilt.

Antenna is also available with N connector(s). Replace "EDIN" with "N" in the model number when ordering

Electrical Characteristics	696-900 MHz		
Frequency bands	696-806 MHz	806-900 MHz	
Polarization	±45°		
Horizontal beamwidth	65°	63°	
Vertical beamwidth	13°	11°	
Gain	14.0 dBd (16.1 dBi)	14.5 dBd (16.6 dBi)	
Electrical downtilt (X)	0, 2, 3, 4, 5, 6, 8, 10		
Impedance	50Ω		
VSWR	≤1.35:1		
Upper sidelobe suppression (0°)	-18.3 dB	-18.2 dB	
Front-to-back ratio (+/-30°)	-33.4 dB	-36.3 dB	
Null fill	5% (-26.02 dB)		
Isolation between ports	< -25 dB		
Input power with EDIN connectors	500 W		
Input power with N connectors	300 W		
Lightning protection	Direct Ground		
Connector(s)	2 Ports / EDIN or N / Female / Bottom		
Mechanical Characteristics			
Dimensions Length x Width x Depth	1742 x 285 x 135 mm	68.6 x 11.2 x 5.3 in	
Depth with z-brackets	175 mm	6.9 in	
Weight without mounting brackets	8.7 kg	19.2 lbs	
Survival wind speed	> 201 km/hr > 125 mph		
Wind area	Front: 0.50 m ² Side: 0.24 m ²	Front: 5.3 ft ² Side: 2.5 ft ²	
Wind load @ 161 km/hr (100 mph)	Front: 733 N Side: 386 N	Front: 164 lbf Side: 88 lbf	
Mounting Options	Part Number	Fits Pipe Diameter	Weight
3-Point Mounting & Downtilt Bracket Kit	36210008	40-115 mm 1.57-4.5 in	6.9 kg 15.2 lbs
Concealment Configurations	For concealment configurations, order BXA-70063-6BF-EDIN-X-FP		

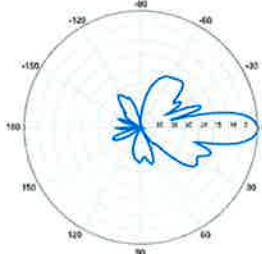


BXA-70063-6BF-EDIN-X



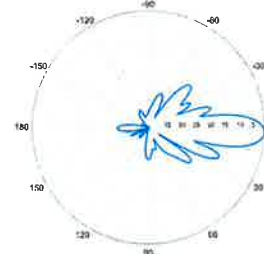
Horizontal | 750 MHz

BXA-70063-6BF-EDIN-0

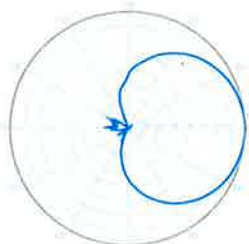


0° | Vertical | 750 MHz

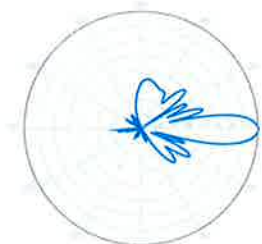
BXA-70063-6BF-EDIN-2



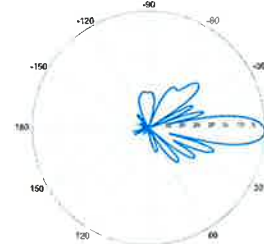
2° | Vertical | 750 MHz



Horizontal | 850 MHz



0° | Vertical | 850 MHz



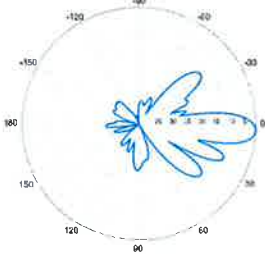
2° | Vertical | 850 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

BXA-70063-6BF-EDIN-X

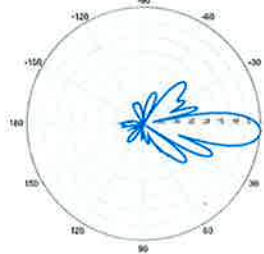
X-Pol | FET Panel | 63° | 14.5 dBd

BXA-70063-6BF-EDIN-3



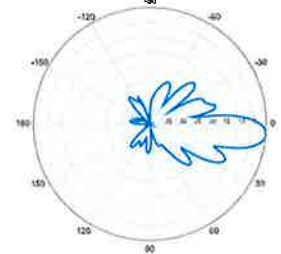
3° | Vertical | 750 MHz

BXA-70063-6BF-EDIN-4

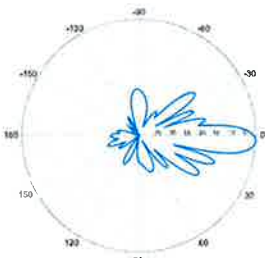


4° | Vertical | 750 MHz

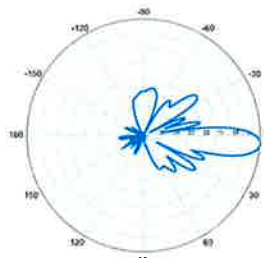
BXA-70063-6BF-EDIN-5



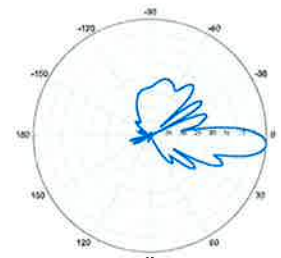
5° | Vertical | 750 MHz



3° | Vertical | 850 MHz

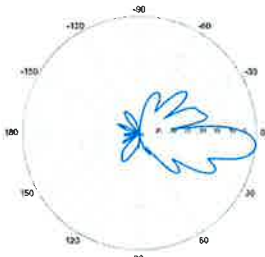


4° | Vertical | 850 MHz



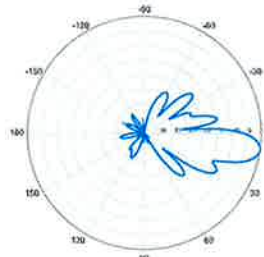
5° | Vertical | 850 MHz

BXA-70063-6BF-EDIN-6



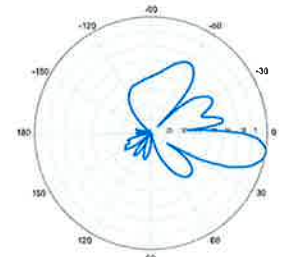
6° | Vertical | 750 MHz

BXA-70063-6BF-EDIN-8

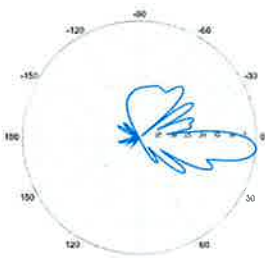


8° | Vertical | 750 MHz

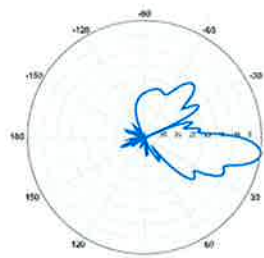
BXA-70063-6BF-EDIN-10



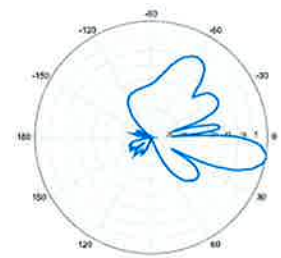
10° | Vertical | 750 MHz



6° | Vertical | 850 MHz



8° | Vertical | 850 MHz



10° | Vertical | 850 MHz

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BXA-171063-12CF-EDIN-X

X-Pol | FET Panel | 63° | 19.0 dBi

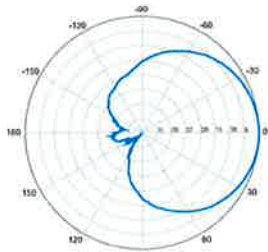
Replace "X" with desired electrical downtilt.

Antenna is also available with NE connector(s).
Replace "EDIN" with "NE" in the model number when ordering.

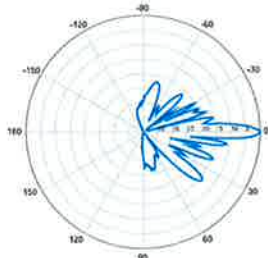
Electrical Characteristics	1710-2170 MHz			
Frequency bands	1710-1880 MHz	1850-1990 MHz	1920-2170 MHz	
Polarization	±45°	±45°	±45°	
Horizontal beamwidth	68°	65°	60°	
Vertical beamwidth	4.5°	4.5°	4.5°	
Gain	16,1 dBd / 18,2 dBi	16,5 dBd / 18,6 dBi	16,9 dBd / 19,0 dBi	
Electrical downtilt (X)	0, 2, 5			
Impedance	50Ω			
VSWR	≤1.5:1			
First upper sidelobe	< -17 dB			
Front-to-back ratio	> 30 dB			
In-band isolation	< -25 dB			
IM3 (20W carrier)	< -150 dBc			
Input power	300 W			
Lightning protection	Direct Ground			
Connector(s)	2 Ports / EDIN or NE / Female / Center (Back)			
Operating temperature	-40° to +60° C / -40° to +140° F			
Mechanical Characteristics				
Dimensions Length x Width x Depth	1842 x 154 x 105 mm		72.5 x 6.1 x 4.1 in	
Depth with z-brackets	133 mm		5.2 in	
Weight without mounting brackets	5.8 kg		12.8 lbs	
Survival wind speed	> 201 km/hr		> 125 mph	
Wind area	Front: 0.28 m ² Side: 0.19 m ²	Front: 3.1 ft ² Side: 2.1 ft ²		
Wind load @ 161 km/hr (100 mph)	Front: 460 N Side: 304 N	Front: 103 lbf Side: 68 lbf		
Mounting Options	Part Number	Fits Pipe Diameter		Weight
2-Point Mounting Bracket Kit	26799997	50-102 mm	2.0-4.0 in	2.3 kg 5 lbs
2-Point Mounting & Downtilt Bracket Kit	26799999	50-102 mm	2.0-4.0 in	3.6 kg 8 lbs
Concealment Configurations	For concealment configurations, order BXA-171063-12CF-EDIN-X-FP			



BXA-171063-12CF-EDIN-X

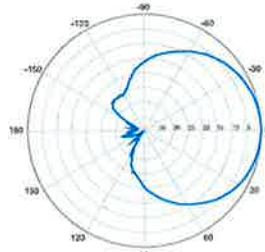


Horizontal | 1710-1880 MHz
BXA-171063-12CF-EDIN-0

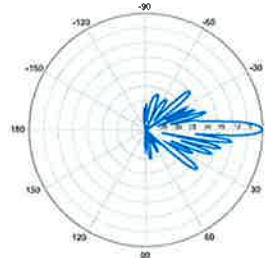


0° | Vertical | 1710-1880 MHz

BXA-171063-12CF-EDIN-X

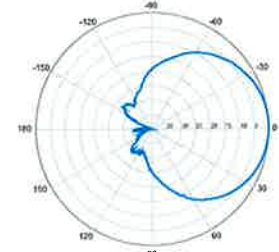


Horizontal | 1850-1990 MHz
BXA-171063-12CF-EDIN-0

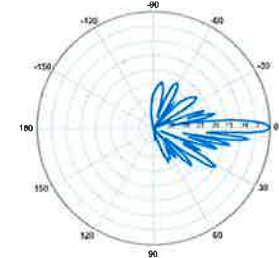


0° | Vertical | 1850-1990 MHz

BXA-171063-12CF-EDIN-X



Horizontal | 1920-2170 MHz
BXA-171063-12CF-EDIN-0



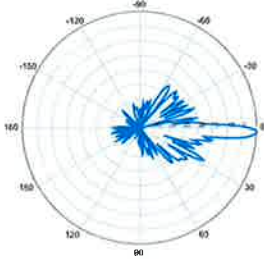
0° | Vertical | 1920-2170 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

BXA-171063-12CF-EDIN-X

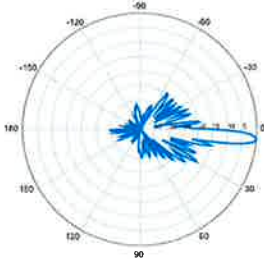
X-Pol | FET Panel | 63° | 19.0 dBi

BXA-171063-12CF-EDIN-2



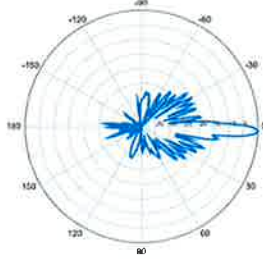
2° | Vertical | 1710-1880 MHz

BXA-171063-12CF-EDIN-5



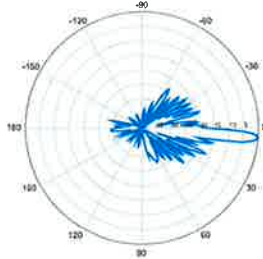
5° | Vertical | 1710-1880 MHz

BXA-171063-12CF-EDIN-2



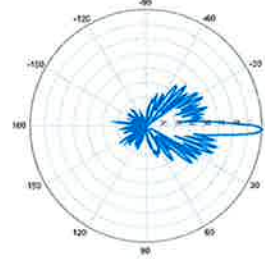
2° | Vertical | 1850-1990 MHz

BXA-171063-12CF-EDIN-5



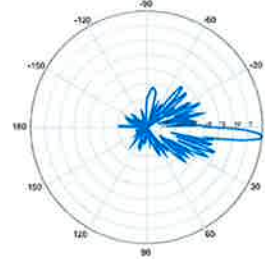
5° | Vertical | 1850-1990 MHz

BXA-171063-12CF-EDIN-2



2° | Vertical | 1920-2170 MHz

BXA-171063-12CF-EDIN-5



5° | Vertical | 1920-2170 MHz

Quoted performance parameters are provided to offer typical or range values only and may vary as a result of normal manufacturing and operational conditions. Extreme operational conditions and/or stress on structural supports is beyond our control. Such conditions may result in damage to this product. Improvements to product may be made without notice.

Alcatel-Lucent RRH2x40-07-U

REMOTE RADIO HEAD

The Alcatel-Lucent RRH2x40-07-U is a high-power, small form-factor Remote Radio Head (RRH) operating in the North American Digital Dividend / 700MHz frequency band (3GPP Band 13). The Alcatel-Lucent RRH2x40-07-U is designed with an eco-efficient approach, providing operators with the means to achieve high quality and capacity coverage with minimum site requirements.



A distributed eNodeB expands deployment options by using two components, a Base Band Unit (BBU) containing the digital assets and a separate RRH containing the radio-frequency (RF) elements. This modular design optimizes available space and allows the main components of an eNodeB to be installed separately, within the same site or several kilometres apart.

The Alcatel-Lucent RRH2x40-07-U is linked to the BBU by an optical-fiber connection carrying downlink and uplink digital radio signals along with operations, administration and maintenance (OA&M) information. The Alcatel-Lucent RRH2x40-07-U has two transmit RF paths, 40 W RF output power per transmit path, and is designed to manage up to two-way receive diversity. The device is ideally suited to support macro coverage, with multiple-input multiple-output (MIMO) 2x2 operation in up to 10 MHz of bandwidth.

The Alcatel-Lucent RRH2x40-07-U is designed to make available all the benefits of a distributed eNodeB, with excellent RF characteristics, with low

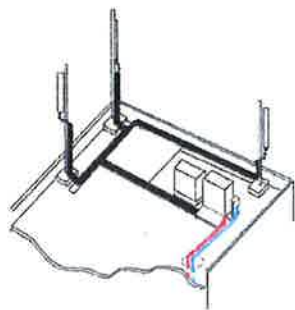
capital expenditures (CAPEX) and low operating expenditures (OPEX). The limited space available in some sites may prevent the installation of traditional single-cabinet BTS equipment or require costly cranes to be employed, leaving coverage holes. However, many of these sites can host an Alcatel-Lucent RRH2x40-07-U installation, providing more flexible site selection and improved network quality along with greatly reduced installation time and costs.

Fast, low-cost installation and deployment

The Alcatel-Lucent RRH2x40-07-U is a zero-footprint solution and operates noise-free, simplifying negotiations with site property owners and minimizing environmental impacts. Installation can easily be done by a single person because the Alcatel-Lucent RRH2x40-07-U is compact and weighs less than 23 kg (50 lb), eliminating the need for a crane to hoist the BTS cabinet to the rooftop. A site can be in operation in less than one day — a fraction of the time required for a traditional BTS.

Excellent RF performance

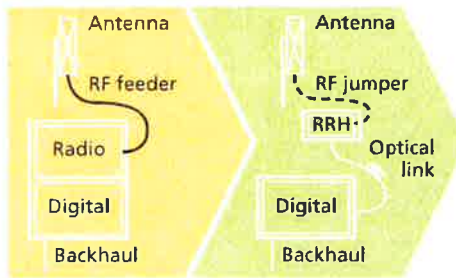
Because of its small size and weight, the Alcatel-Lucent RRH2x40-07-U can be installed close to the antenna. Operators can therefore locate the Alcatel-Lucent RRH2x40-07-U where RF engineering is deemed ideal, minimizing trade-offs between available sites and RF optimum sites. The RF feeder cost and installation costs are reduced or eliminated, and there is no need for a Tower Mounted Amplifier (TMA) because losses introduced by the RF feeder are greatly reduced. The Alcatel-Lucent RRH2x40-07-U provides more RF power while at the same time consuming less electricity.



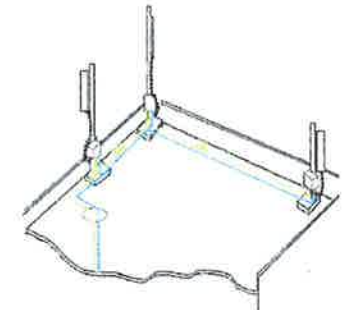
Macro

Features

- Zero-footprint deployment
- Easy installation, with a lightweight unit can be carried and set up by one person
- Optimized RF power, with flexible site selection and elimination of a TMA
- Convection-cooled (fanless), noise-free, and heaterless unit
- Best-in-class power efficiency, with significantly reduced energy consumption



RRH for space-constrained cell sites



Distributed

Benefits

- Leverages existing real estate with lower site costs
- Reduces installation costs, with fewer installation materials and simplified logistics
- Decreases power costs and minimizes environmental impacts, with the potential for eco-sustainable power options
- Improves RF performance and adds flexibility to network planning

Technical specifications

Physical dimensions

- Height: 390 mm (15.4 in.)
- Width: 380 mm (15 in.)
- Depth: 210 mm (8.2 in.)
- Weight (without mounting kit): less than 23 kg (50 lb)

Power

- Power supply: -48V

Operating environment

- Outdoor temperature range:
 - With solar load: -40°C to +50°C (-40°F to +122°F)
 - Without solar load: -40°C to +55°C (-40°F to +131°F)
- Passive convection cooling (no fans)

- Enclosure protection
 - IP65 (International Protection rating)

RF characteristics

- Frequency band: 700 MHz; 3GPP Band 13
- Bandwidth: up to 10 MHz
- RF output power at antenna port:
 - 40 W nominal RF power for each Tx port
- Rx diversity: 2-way or 4-way
- Noise figure: below 2.5 dB typical
- ALD features
 - TMA
 - Remote electrical tilt (RET) support (AISG v2.0)

Optical characteristics

Type/number of fibers

- Up to 3.12 Gb/s line bit rate
- Single-mode variant
 - One SM fiber (9/125 μm) per RRH2x, carrying UL and DL using CWDM (at 1550/1310 nm)
- Multi-mode variant
 - Two MM fibers (50/125 μm) per RRH2x: one carrying UL, the other carrying DL (at 850 nm)

Optical fiber length

- Up to 500 m (0.31 mi), using MM fiber
- Up to 20 km (12.43 mi), using SM fiber

Alarms and ports

- Six external alarms
- Two optical ports to support daisy-chaining

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HYBRIFLEX™ RRH Hybrid Feeder Cabling Solution, 1-5/8", Single-Mode Fiber

Product Description

RFS' HYBRIFLEX Remote Radio Head (RRH) hybrid feeder cabling solution combines optical fiber and DC power for RRHs in a single lightweight aluminum corrugated cable, making it the world's most innovative solution for RRH deployments.

It was developed to reduce installation complexity and costs at Cellular sites. HYBRIFLEX allows mobile operators deploying an RRH architecture to standardize the RRH installation process and eliminate the need for and cost of cable grounding. HYBRIFLEX combines optical fiber (multi-mode or single-mode) and power in a single corrugated cable. It eliminates the need for junction boxes and can connect multiple RRHs with a single feeder. Standard RFS CELLFLEX® accessories can be used with HYBRIFLEX cable. Both pre-connectorized and on-site options are available.

Features/Benefits

- ▶ Aluminum corrugated armor with outstanding bending characteristics - minimizes installation time and enables mechanical protection and shielding
- ▶ Same accessories as 1 5/8" coaxial cable
- ▶ Outer conductor grounding - Eliminates typical grounding requirements and saves on installation costs
- ▶ Lightweight solution and compact design - Decreases tower loading
- ▶ Robust cabling - Eliminates need for expensive cable trays and ducts
- ▶ Installation of tight bundled fiber optic cable pairs directly to the RRH - Reduces CAPEX and wind load by eliminating need for interconnection
- ▶ Optical fiber and power cables housed in single corrugated cable - Saves CAPEX by standardizing RRH cable installation and reducing installation requirements
- ▶ Outdoor polyethylene jacket - Ensures long-lasting cable protection



Figure 1: HYBRIFLEX Series

Technical Specifications

Outer Conductor Armor	Corrugated Aluminum	[mm (in.)]	46.5 (1.83)
Jacket	Polyethylene, PE	[mm (in.)]	50.3 (1.98)
UV-Protection	Individual and External Jacket		Yes
Weight, Approximate			
		[kg/m (lb/ft)]	1.9 (1.30)
Minimum Bending Radius, Single Bending		[mm (in.)]	200 (8)
Minimum Bending Radius, Repeated Bending		[mm (in.)]	500 (20)
Recommended/Maximum Clamp Spacing		[m (ft)]	1.0 / 1.2 (3.25 / 4.0)
DC-Resistance			
Outer Conductor Armor		[Ω/km (Ω/1000ft)]	0.68 (0.205)
Power Cable, 8 4mm ² (8AWG)		[Ω/km (Ω/1000ft)]	2.1 (0.307)
Version			
Quantity, Fiber Count			Single-mode OM3 16 (8 pairs)
Core/Clad		[μm]	50/125
Primary Coating (Acrylate)		[μm]	245
Buffer Diameter, Nominal		[μm]	900
Secondary Protection, Jacket, Nominal		[mm (in.)]	2.0 (0.08)
Minimum Bending Radius		[mm (in.)]	104 (4.1)
Insertion Loss @ wavelength 850nm		dB/km	3.0
Insertion Loss @ wavelength 1310nm		dB/km	1.0
Standards (Meets or exceeds)			UL94-V0 UL1666 RoHS Compliant
Size (Power)			
Quantity, Wire Count (Power)		[mm (AWG)]	8 4 (8)
Size (Alarm)		[mm (AWG)]	0 8 (18)
Quantity, Wire Count (Alarm)			4 (2 pairs)
Type			UV protected
Strands			19
Primary Jacket Diameter, Nominal		[mm (in.)]	6.3 (0.27)
Standards (Meets or exceeds)			NFPA 130, ICEA S-95-638 UL Type XHHW-2, UL 44 UL-L5 Limited Smoke, UL VW-1 IEEE-383 (1974), IEEE1202/FT4 RoHS Compliant
Installation Temperature			
Installation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)
Operation Temperature		[°C (°F)]	-40 to +65 (-40 to 149)

* This data is provisional and subject to change

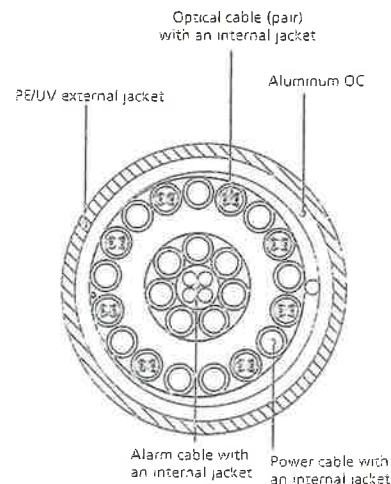


Figure 2: Construction Detail

All information contained in the present datasheet is subject to confirmation at time of ordering

ATTACHMENT 2

ATTACHMENT 3

DETAILED STRUCTURAL ANALYSIS AND EVALUATION OF AN EXISTING 176' MONOPOLE FOR PROPOSED ANTENNA ARRANGEMENT

Site: Berlin Fire Department
Address: 1657 Wilbur Cross Parkway,
Berlin, CT

prepared for



Verizon Wireless
99 East River Drive
East Hartford, Connecticut 06108

prepared by



URS CORPORATION
500 ENTERPRISE DRIVE, SUITE 3B
ROCKY HILL, CT 06067
TEL. 860-529-8882

36917417.00000
VZ5-168

November 11, 2013

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- 6. DRAWINGS AND DATA**
 - **TNX TOWER INPUT / OUTPUT SUMMARY**
 - **TNX TOWER DETAILED OUTPUT**
 - **ANCHOR BOLT AND BASE PLATE ANALYSIS**
 - **FOUNDATION ANALYSIS**

EXECUTIVE SUMMARY - continued

This analysis is based on:


- 1) The tower structure's theoretical capacity, not including any assessment of the condition of the tower.
- 2) Geotechnical investigation and report performed by Dr. Clarence Welti, P.E., P.C. dated June 11, 2002.
- 3) Tower geometry and structural member sizes utilized in the preparation of this report obtained from manufacturers original design documents for a 176' tapered steel monopole, prepared by Engineered Endeavors, Inc., EEI Job #: 11129, signed and sealed September 16, 2002.
- 4) Foundation geometry utilized in the preparation of this report obtained from manufacturers original design documents for a 176; tapered steel monopole, prepared by Engineered Endeavors, Inc., EEI Job # 11129, signed and sealed September 20, 2002.
- 5) Previous structural analysis performed by URS Corp, project number VS1-034 / 36922118, signed and sealed September 29, 2005.
- 6) Previous structural analysis performed by URS Corp, project number VZ1-005 / 36912556, signed and sealed February 21, 2006.
- 7) Previous structural analysis performed by Natcomm Consulting Engineers, Inc., project number 08007.C05, signed and sealed February 14, 2008.
- 8) Site documentation and visual verification of existing appurtenances conducted from grade by URS during July 2008.
- 9) Previous structural analysis performed by URS Corp, project number PWS-002 / 36923532, signed and sealed July 18, 2008.
- 10) Previous structural analysis performed by URS Corp, project number MXN-003 / 36924391, signed and sealed April 7, 2010.
- 11) Previous structural analysis performed by URS Corp, project number VZ5-047 / 36931194, signed and sealed June 23, 2010.
- 12) Previous structural analysis performed by Hudson Design. Group, LLC., signed and sealed April 5, 2012.
- 13) Previous structural analysis performed by Tectonic Engineering & Surveying Consultants, project number 6318.43-846, signed and sealed November 16, 2012.
- 14) Antenna and mount configuration as specified within Section 2 and 6 of this report.

This report is only valid as per the assumptions and data utilized in this report for antenna inventory, mounts and associated cables. The user of this report shall field verify the assumption of the antenna and mount configuration as well as the physical condition of the tower. Notify the engineer in writing immediately if any of the information in this report is found to be other than specified.

If you should have any questions, please call.

Sincerely,

URS Corporation


Richard A. Sambor, P.E.
Senior Structural Engineer



RAS/mcd
cc: MJE, ICA – URS, CF/Book

2. INTRODUCTION

The subject tower is located at 1657 Wilbur Cross Parkway in Berlin, CT. The structure is an existing 176' steel tapered monopole structure, designed and manufactured by Engineered Endeavors, Inc.

The inventory is summarized in the table below:

Antenna Type	Carrier	Mount	Antenna Centerline Elevation	Cable
(1) 8' Dipole Antenna (2) 8' Omni Antennas (2) 4' Omni Antennas (2) 18"x18" Panel Antenna (2) Kathrein Scala MF-900B Grid Dish	Town (existing)	Low-Profile Platform	175'	(9) 1 5/8" (within monopole)
(1) Telewave ANT150D6-9 4-Bay Dipole Antenna	Town (reserved)	See Above Mount	175'	(1) 1 5/8" (within monopole)
(3) Katherin Scala 800-10121 Panel Antennas (3) KMW AM-X-CD-16-65 Panel Antennas (6) RRU Units (1) Raycap DC6-48-60-18-8F Surge Arrestor (6) LGP21401 TMA's	AT&T (existing)	(3) 12' T-Arm Mounts	170'	(6) 1 5/8" (1) Fiber Optic (2) DC Power (within monopole)
(6) EMS DR65-19-00DPQ Panel Antennas (6) Decibel PCS 1900 TMA's	T-Mobile (existing)	(3) 12' T-Arm Mounts	160'	(12) 1 5/8" (within monopole)
(3) EMS DR65-19-00DPQ Panel Antennas (6) Decibel PCS 1900 TMA's	T-Mobile (reserved)	See Above Mount	160'	(12) 1 5/8" (within monopole)
(1) Telewave ANT120D3 2-Bay Dipole Antenna (1) Grid Dish (1) 18"x18" Panel Antenna	Town (existing)	See Above Mount	159'	(3) 1 5/8" (within monopole)
(3) Andrew VHLP2.5 Dishes (2) 3' Dishes (3) Kathrein Scala 840-10054 Panel Antennas (6) Dapa 48000 Antennas (3) Samsung RRH Units (3) RFS APXVSPP18-C-A20 Panel Antennas (3) 800 MHz RRH Units (3) 1900 RRH Units	Sprint (existing)	(1) 12' Low-Profile Platform	150'	(4) 1/2" (12) 1 5/8" (6) CAT5 (3) 1 1/4" Hybriflex (within monopole)
(1) 18"x18" Panel Antenna (1) 10' 2-Bay Dipole Antenna	Town (existing)	(1) 6' Extension Arm	129'	(2) 1 5/8" (within monopole)
(3) Katherin 742 213 Panel Antennas	Pocket Wireless (existing)	(3) Pipe Mounts	123'	(6) 1 5/8" (Andrew AVA7-50 exterior of monopole)
(3) LNX-6514DAS-T4M-750_4 Panel Antennas (1) Amphenol BXA-185060/12CF 2 Panel Antennas (2) MG D3-900T0	Verizon (existing)	Low-Profile Platform	114'	(6) 1 5/8" (outside monopole)

Antenna Type	Carrier	Mount	Antenna Centerline Elevation	Cable
(2) Amphenol BXA-70063-6BF Panel Antennas (Alpha & Beta Sectors) (1) Amphenol BXA-70080-4BF Panel Antennas (Gamma Sector) (3) Amphenol BXA-171063-12CF-EDIN-2 Panel Antennas (3) AWS RRH Units (1) Raycap DB-T1-6Z-8A-0Z Distribution Box	Verizon (Proposed)	(1) Low-Profile Platform	114'	(1) 1 5/8 Fiber Optic Cable (outside monopole) (Proposed) (12) 1 5/8" (within monopole)
(1) 20' 4-Bay Dipole antenna (1) 18"x18" Panel Antenna (1) Grid Dish	Town (existing)	6' Extension Arm	100'	(2) 1 5/8" (within monopole)
(1) GPS Antenna	Sprint (existing)	3' Standoff Mount	75'	(1) 1/2" (within monopole)
(1) VIC-100 GPS Antenna	T-Mobile (reserved)	3' Standoff Mount	60'	(1) 1/2" (within monopole)
(1) Scanner Antenna	Town (existing)	3' Standoff Mount	60'	(1) 1/2" (within monopole)
(1) GPS	AT&T (existing)	Flush Mounted Assembly	45'	(1) 1/2" (outside monopole)

Note: Base of structure established as 1.0ft above average grade.

This structural analysis of the communications tower was performed by URS Corporation (URS) for Verizon Wireless. The purpose of this analysis was to investigate the structural integrity of the existing tower with its existing and proposed antenna loads. This analysis was conducted to evaluate stress on the tower and the effect of forces to the foundation of the tower resulting from existing and proposed antenna arrangement.

3. ANALYSIS METHODOLOGY AND LOADING CONDITIONS

The structural analysis was conducted in accordance with the 2005 Connecticut State Building Code, TIA/EIA-222-F - Structural Standard for Steel Antenna Towers and Antenna Supporting Structures, and the American Institute of Steel Construction (AISC) Manual of Steel Construction - Allowable Stress Design (ASD).

The analysis was conducted using RISA Tower 5.3. Two load conditions were evaluated as shown below which were compared to allowable stresses according to AISC and TIA/EIA.

Load Condition 1 = 80 mph (fastest mile) Wind Load (without ice) + Tower Dead Load
 Load Condition 2 = 69 mph (fastest mile) Wind Load (with ice) + Ice Load + Tower Dead Load

Please note that wind pressure is a function of velocity squared. Under Load Condition 2, a 25 percent reduction in wind pressure is allowed by code to account for the unlikelihood of the full wind pressure and ice load occurring at the same time. The same results may be achieved by utilizing a lower wind pressure without taking the 25 percent reduction, as shown above.

The TIA/EIA standard permits a one-third increase in allowable stresses for towers and monopoles less than 700 feet tall. For the purposes of this analysis, in computing the load capacity the allowable stresses of the tower members were increased by one-third.

4. FINDINGS AND EVALUATION

Combined axial and bending stresses on the monopole structure were evaluated to compare with allowable stresses in accordance with AISC. The calculated stresses under the proposed loading were **BELOW** the allowable stresses (see table below). Detailed analysis and calculations for the proposed load condition are provided in section 6 of this report. Additionally, the anchor bolts, base plate and foundation were all found to be within the allowable limits.

TABLE 1: Proposed Tower Base Reactions

Base Reactions	Reactions
Axial Load (kips)	53
Shear (kips)	36
O.T. Moment (ft-kips)	4398

TABLE 2: Proposed Tower Component Stress vs. Capacity Summary

Component (Section No.)	Controlling Component / Elevation	Stress Ratio (% capacity)	Pass/Fail	Comments:
Pole Shaft (L3)	43'-86.13'	89.0 %	Pass	
Anchor Bolts	Compression	87 %	Pass	
Base Plate	Bending	86 %	Pass	
Caisson Foundation	Flexure	69 %	Pass	

5. CONCLUSIONS AND RECOMMENDATIONS

The results of the analysis indicate that the tower structure has the capacity to support the proposed loading conditions. **The tower and its foundation are considered structurally adequate with the wind load classification specified above and the proposed antenna loading.**

Limitations/Assumptions:

This report is based on the following:

1. Tower inventory as listed in this report.
2. Tower is properly installed and maintained.
3. All members are as specified in the original design documents and are in good condition.
4. All required members are in place.
5. All bolts are in place and are properly tightened.
6. Tower is in plumb condition.
7. All member protective coatings are in good condition.
8. All tower members were properly designed, detailed, fabricated, and installed and have been properly maintained since erection.
9. Foundations were properly constructed to support original design loads as specified in the original design documents.
10. All coaxial cable is installed within the monopole unless specified otherwise.

URS is not responsible for any modifications completed prior to or hereafter in which URS is not or was not directly involved. Modifications include but are not limited to:

- A. Adding antennas
- B. Removing/replacing antennas
- C. Adding coaxial cables

URS hereby states that this document represents the entire report and that it assumes no liability for any factual changes that may occur after the date of this report. All representations, recommendations, and conclusions are based upon information contained and set forth herein. If you are aware of any information which conflicts with that which is contained herein, or you are aware of any defects arising from original design, material, fabrication, or erection deficiencies, you should disregard this report and immediately contact URS. URS disclaims all liability for any representation, recommendation, or conclusion not expressly stated herein.

Ongoing and Periodic Inspection and Maintenance:

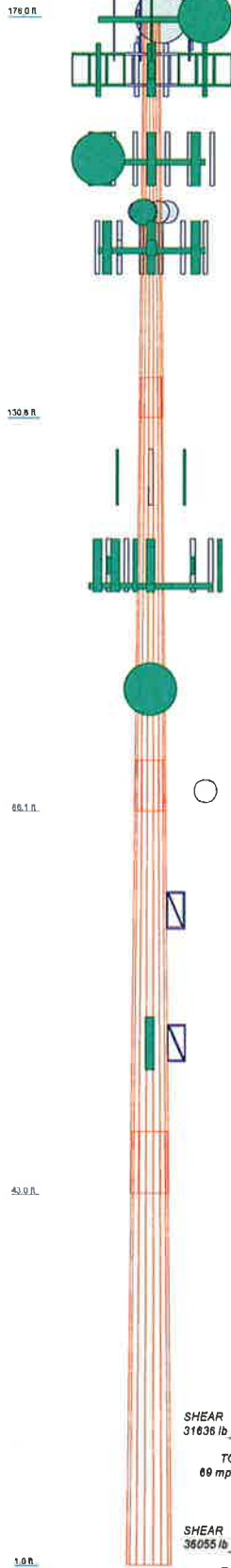
After the Contractor has successfully completed the installation and the work has been accepted, the owner will be responsible for the ongoing and periodic inspection and maintenance of the tower.

The owner shall refer to TIA/EIA-222-F for recommendations for maintenance and inspection. The frequency of the inspection and maintenance intervals is to be determined by the owner based upon actual site and environmental conditions. It is recommended that a complete and thorough inspection of the entire tower structural system be performed at least yearly and more frequently as conditions warrant. According to TIA/EIA-222-F section 14.1, Note 1: It is recommended that the structure be inspected after severe wind and/or ice storms or other extreme loading conditions.

6. DRAWINGS AND DATA

TNX TOWER INPUT/OUTPUT SUMMARY

Section	1	2	3	4
Length (ft)	48.25	48.13	48.88	48.00
Number of Stairs	18	18	18	18
Thickness (in)	0.2500	0.3125	0.3750	0.4375
Stair Length (ft)	4.50	5.75	7.00	8.25
Top Dia (in)	21.0000	30.2500	39.5000	48.7500
Bot Dia (in)	21.0000	41.8750	51.3800	60.8800
Grade			A572-55	
Height (ft)	3195.0	5921.5	8653.1	12376.6



DESIGNED APPURTENANCE LOADING

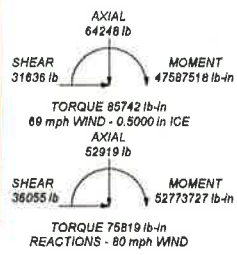
TYPE	ELEVATION	TYPE	ELEVATION
12' Low Profile Platform (Tower)	176	(2) 48000 (Sprinkler)	150
4 Bay Dipole (Tower)	176	(2) 48000 (Sprinkler)	150
(2) 2' Dia # 0mm (Tower)	176	APXVSRP16-C-A20 (Clearwire)	150
Grid Dish (Tower)	176	APXVSRP16-C-A20 (Clearwire)	150
Dipole (Tower)	176	APXVSRP16-C-A20 (Clearwire)	150
Grid Dish (Tower)	176	Andrew 800MHz RRH (Clearwire)	150
(2) 1.5' Dia 4' 0mm w/Pipe Mount (Tower)	176	Andrew 800MHz RRH (Clearwire)	150
4 Bay Dipole (reserved) (Tower)	176	Parasitic RSH 1200MHz (Clearwire)	150
12'x18'x4' Panel (Tower)	176	Parasitic RSH 1200MHz (Clearwire)	150
Valmont T-Arm (1) (ATJ/Cingular)	170	Parasitic RSH 1200MHz (Clearwire)	150
Valmont T-Arm (1) (ATJ/Cingular)	170	Parasitic RSH 1200MHz (Clearwire)	150
Valmont T-Arm (1) (ATJ/Cingular)	170	843 10054 (Clearwire)	150
800-10121 (ATJ/Cingular)	170	840 10054 (Clearwire)	150
800-10121 (ATJ/Cingular)	170	840 10054 (Clearwire)	150
800-10121 (ATJ/Cingular)	170	Remote Radio Heads U-RAS (Clearwire)	150
(2) LFG 21401 TMA (ATJ/Cingular)	170	Remote Radio Heads U-RAS (Clearwire)	150
(2) LFG 21401 TMA (ATJ/Cingular)	170	2 Bay Dipole (Tower)	130
AKX-CD-1449 (ATJ/Cingular)	170	18'x18'x4' Panel (Tower)	130
AKX-CD-1449 (ATJ/Cingular)	170	6' Side Mount Standoff (Tower)	130
AKX-CD-1449 (ATJ/Cingular)	170	742-213 (Phasel Wireless)	124
(2) RRUS-11 (ATJ/Cingular)	170	742-213 (Phasel Wireless)	124
(2) RRUS-11 (ATJ/Cingular)	170	742-213 (Phasel Wireless)	124
(2) RRUS-11 (ATJ/Cingular)	170	BXA-700548F-EDB-X (Verizon)	114
(2) RRUS-11 (ATJ/Cingular)	170	BXA-700548F-EDB-X (Verizon)	114
DCS-48-60-164F (ATJ/Cingular)	170	BXA-700548F-EDB-X (Verizon)	114
Valmont T-Arm (1) (T-Mobile)	160	BXA-700548F-EDB-X (Verizon)	114
Valmont T-Arm (1) (T-Mobile)	160	BXA-700548F-EDB-X (Verizon)	114
Valmont T-Arm (1) (T-Mobile)	160	RH_2X48-AV5 (Verizon)	114
(2) DR85-19-00DFQ (T-Mobile)	160	RH_2X48-AV5 (Verizon)	114
(2) DR85-19-00DFQ (T-Mobile)	160	RH_2X48-AV5 (Verizon)	114
(2) DR85-19-00DFQ (T-Mobile)	160	CB-T1-62-448-02 (Verizon)	114
(2) DR85-19-00DFQ (T-Mobile)	160	Rmax MG D3-900Tx (Verizon)	114
(2) DR85-19-00DFQ (T-Mobile)	160	LNK-651408-74M (Verizon)	114
(2) DR85-19-00DFQ (T-Mobile)	160	LNK-651408-74M (Verizon)	114
(2) TMA 10'x8'x3' (T-Mobile)	160	Rmax MG D3-900Tx (Verizon)	114
(2) TMA 10'x8'x3' (T-Mobile)	160	LNK-651408-74M (Verizon)	114
(2) TMA 10'x8'x3' (T-Mobile)	160	LNK-651408-74M (Verizon)	114
(2) TMA 10'x8'x3' (T-Mobile)	160	BXA-18206012CF 2 (Verizon)	114
(2) TMA 10'x8'x3' (T-Mobile)	160	Andrew 12'x4' Low Profile Platform (Verizon)	112
(2) TMA 10'x8'x3' (T-Mobile)	160	6' Side Mount Standoff (Tower)	100
2 Bay Dipole (Tower)	160	4 Bay Dipole (Tower)	100
Grid Dish (Tower)	160	Grid Dish (Tower)	100
12'x18'x4' Panel (Tower)	160	18'x18'x4' Panel (Tower)	100
WLP2-S-180 (Clearwire)	154	Side Mount Standoff (Sprinkler)	75
WLP2-S-180 (Clearwire)	154	GPI (Sprinkler)	75
WLP2-S-180 (Clearwire)	154	Scanner Antenna (Tower) (reserved)	60
FXCF-52 (Clearwire)	154	Side Mount Standoff (T-Mobile) (reserved)	60
FXCF-52 (Clearwire)	154	GPI (Sprinkler)	60
Remote Radio Heads U-RAS (Clearwire)	150	Side Mount Standoff (T-Mobile) (reserved)	60
Low Profile Platform (Sprinkler)	150	GPI (ATJ)	46
(2) 48000 (Sprinkler)	150		

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-55	55 ksi	80 ksi			

TOWER DESIGN NOTES

1. Tower is located in Hartford County, Connecticut.
2. Tower designed for a 80 mph basic wind in accordance with the TIA/EIA-222-F Standard.
3. Tower is also designed for a 69 mph basic wind with 0.50 in ice.
4. Deflections are based upon a 50 mph wind.
5. TOWER RATING: 69%



URS Corporation		176' EEI Monopole, Berlin, CT	
500 Enterprise Drive, Suite 3B		Project: Berlin Fire Department	
Rocky Hill, CT 06067		Client: Verizon Wireless	
Phone: 860-529-8882		Drawn by: Michael Dalickas	
FAX: 860-529-3891		Code: TIA/EIA-222-F	
		Date: 11/11/13	
		Scale: NTS	
		Path: \\server\projects\176\monopole\176_eei_monopole.dwg	
		Draw No: E-1	

TNX TOWER DETAILED OUTPUT

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: 860-529-8882 FAX: 860-529-3991	Job 176' EEI Monopole, Berlin, CT	Page 1 of 24
	Project Berlin Fire Department	Date 14:42:15 11/11/13
	Client Verizon Wireless	Designed by Michael Dalickas

Tower Input Data

There is a pole section.

This tower is designed using the TIA/EIA-222-F standard.

The following design criteria apply:

Tower is located in Hartford County, Connecticut.

Basic wind speed of 80 mph.

Nominal ice thickness of 0.5000 in.

Ice density of 56 pcf.

A wind speed of 69 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 50 mph.

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.333.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Options

<ul style="list-style-type: none"> Consider Moments - Legs Consider Moments - Horizontals Consider Moments - Diagonals Use Moment Magnification √ Use Code Stress Ratios √ Use Code Safety Factors - Guys Escalate Ice Always Use Max Kz Use Special Wind Profile Include Bolts In Member Capacity Leg Bolts Are At Top Of Section Secondary Horizontal Braces Leg Use Diamond Inner Bracing (4 Sided) Add IBC .6D+W Combination 	<ul style="list-style-type: none"> Distribute Leg Loads As Uniform Assume Legs Pinned √ Assume Rigid Index Plate Use Clear Spans For Wind Area Use Clear Spans For KL/r Retension Guys To Initial Tension √ Bypass Mast Stability Checks Use Azimuth Dish Coefficients √ Project Wind Area of Appurt. Autocalc Torque Arm Areas SR Members Have Cut Ends √ Sort Capacity Reports By Component Triangulate Diamond Inner Bracing 	<ul style="list-style-type: none"> Treat Feedline Bundles As Cylinder Use ASCE 10 X-Brace Ly Rules Calculate Redundant Bracing Forces Ignore Redundant Members in FEA SR Leg Bolts Resist Compression All Leg Panels Have Same Allowable Offset Girt At Foundation √ Consider Feedline Torque Include Angle Block Shear Check <li style="padding-left: 20px;">Poles √ Include Shear-Torsion Interaction Always Use Sub-Critical Flow Use Top Mounted Sockets
--	--	---

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	176.01-130.76	45.25	4.50	18	21.0000	31.8000	0.2500	1.0000	A572-65 (65 ksi)
L2	130.76-86.13	49.13	5.75	18	30.2260	41.8200	0.3125	1.2500	A572-65 (65 ksi)
L3	86.13-43.00	48.88	7.00	18	39.8381	51.3600	0.3750	1.5000	A572-65 (65 ksi)
L4	43.00-1.00	49.00		18	48.9600	60.5000	0.4375	1.7500	A572-65 (65 ksi)

tnxTower URS Corporation 500 Enterprise Drive, Suite 3B Rocky Hill, CT 06067 Phone: 860-529-8882 FAX: 860-529-3991	Job 176' EEI Monopole, Berlin, CT	Page 2 of 24
	Project Berlin Fire Department	Date 14:42:15 11/11/13
	Client Verizon Wireless	Designed by Michael Dalickas

Tapered Pole Properties

Section	Tip Dia. in	Area in ²	J in ⁴	r in	C in	I/C in ³	J in ⁴	Iv/Q ₇ in ²	w in	w/t
L1	21.3240	16.4651	895.6507	7.3663	10.6680	83.9568	1792.4800	8.2341	3.2560	13.024
	32.2906	25.0349	3148.3461	11.2003	16.1544	194.8909	6300.8349	12.5198	5.1568	20.627
L2	31.7706	29.6704	3354.2440	10.6193	15.3548	218.4493	6712.9015	14.8380	4.7698	15.263
	42.4651	41.1703	8961.3641	14.7352	21.2446	421.8192	17934.5198	20.5890	6.8103	21.793
L3	41.8289	46.9709	9241.6271	14.0094	20.2377	456.6531	18495.4146	23.4899	6.3515	16.937
	52.1523	60.6849	19929.7987	18.0997	26.0909	763.8607	39885.8215	30.3482	8.3794	22.345
L4	51.3893	67.3795	20042.4648	17.2255	24.8717	805.8353	40111.3021	33.6962	7.8470	17.936
	61.4333	83.4043	38013.0437	21.3222	30.7340	1236.8401	76076.1060	41.7101	9.8780	22.578

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in
ft	ft ²	in						
L1				1	1	1		
176.01-130.76								
L2				1	1	1		
130.76-86.13								
L3 86.13-43.00				1	1	1		
L4 43.00-1.00				1	1	1		

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _{AA} ft/ft	Weight plf
1 5/8 (Town)	C	No	Inside Pole	176.00 - 21.00	10	No Ice 1/2" Ice	1.04 1.04
1 5/8 (AT&T/Cingular)	C	No	Inside Pole	171.00 - 9.00	6	No Ice 1/2" Ice	1.04 1.04
1 5/8 (T-Mobile)	C	No	Inside Pole	161.00 - 9.00	24	No Ice 1/2" Ice	1.04 1.04
1 5/8 (Town)	C	No	Inside Pole	161.00 - 21.00	3	No Ice 1/2" Ice	1.04 1.04
1/2 (Clearwire)	C	No	Inside Pole	151.00 - 9.00	4	No Ice 1/2" Ice	0.25 0.25
1 5/8 (Sprint/Nextel)	C	No	Inside Pole	151.00 - 9.00	12	No Ice 1/2" Ice	1.04 1.04
CATEGORY 5e (1 WIRE) (Clearwire)	C	No	Inside Pole	151.00 - 9.00	6	No Ice 1/2" Ice	0.21 0.21
1 5/8 (Town)	C	No	Inside Pole	131.00 - 21.00	2	No Ice 1/2" Ice	1.04 1.04
AVA7-50 (1-5/8 LOW DENS. FOAM) (Pocket Wireless)	C	No	CaAa (Out Of Face)	124.00 - 4.00	1	No Ice 1/2" Ice	0.72 2.23
AVA7-50 (1-5/8 LOW DENS. FOAM) (Pocket Wireless)	C	No	CaAa (Out Of Face)	124.00 - 4.00	5	No Ice 1/2" Ice	0.72 2.23
1 5/8 (Verizon)	A	No	CaAa (Out Of Face)	115.00 - 21.00	1	No Ice 1/2" Ice	1.04 2.55
1 5/8 (Verizon)	A	No	CaAa (Out Of Face)	115.00 - 21.00	5	No Ice 1/2" Ice	1.04 2.55

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Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	C _A A _A		Weight plf
						In Face ft ²	Out Face ft ²	
1 5/8 (Verizon)	C	No	Inside Pole	115.00 - 21.00	12	No Ice 1/2" Ice	0.00 0.00	1.04 1.04
1 5/8 (Town)	C	No	Inside Pole	101.00 - 21.00	2	No Ice 1/2" Ice	0.00 0.00	1.04 1.04
1/2 (Clearwire)	C	No	Inside Pole	76.00 - 9.00	1	No Ice 1/2" Ice	0.00 0.00	0.25 0.25
FB-L98B-002 (AT&T)	A	No	Inside Pole	171.00 - 10.00	1	No Ice 1/2" Ice	0.00 0.00	0.25 0.25
wr-vg122st-brda (AT&T)	A	No	Inside Pole	171.00 - 10.00	2	No Ice 1/2" Ice	0.00 0.00	0.25 0.25
1 5/8" Hybriflex (VZW)	A	No	CaAa (Out Of Face)	116.00 - 10.00	1	No Ice 1/2" Ice	0.16 0.26	0.21 1.51
1 1/4" Hybriflex Cables (Sprint)	C	No	Inside Pole	151.00 - 10.00	3	No Ice 1/2" Ice	0.00 0.00	0.42 0.42
1/2 (Sprint)	C	No	Inside Pole	76.00 - 10.00	1	No Ice 1/2" Ice	0.00 0.00	0.25 0.25
1/2 (T-Mobile)	C	No	Inside Pole	61.00 - 10.00	1	No Ice 1/2" Ice	0.00 0.00	0.25 0.25
1/2 (Town)	C	No	Inside Pole	61.00 - 10.00	1	No Ice 1/2" Ice	0.00 0.00	0.25 0.25
1/2 (AT&T)	A	No	CaAa (Out Of Face)	46.00 - 10.00	1	No Ice 1/2" Ice	0.06 0.16	0.25 0.91

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A _R	A _F	C _A A _A In Face	C _A A _A Out Face	Weight K
			ft ²	ft ²	ft ²	ft ²	
L1	176.01-130.76	A	0.000	0.000	0.000	0.000	0.03
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.90
L2	130.76-86.13	A	0.000	0.000	0.000	10.570	0.22
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	7.498	3.36
L3	86.13-43.00	A	0.000	0.000	0.000	15.723	0.31
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	8.540	3.55
L4	43.00-1.00	A	0.000	0.000	0.000	11.633	0.18
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	7.722	2.47

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R	A _F	C _A A _A In Face	C _A A _A Out Face	Weight K
				ft ²	ft ²	ft ²	ft ²	
L1	176.01-130.76	A	0.500	0.000	0.000	0.000	0.000	0.03
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1.90
L2	130.76-86.13	A	0.500	0.000	0.000	0.000	16.444	0.52
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	11.285	3.70
L3	86.13-43.00	A	0.500	0.000	0.000	0.000	24.648	0.76
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	12.853	3.94

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Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A _R ft ²	A _F ft ²	C _{AA} In Face ft ²	C _{AA} Out Face ft ²	Weight K
L4	43.00-1.00	A	0.500	0.000	0.000	0.000	20.433	0.44
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	11.622	2.82

Feed Line Center of Pressure

Section	Elevation ft	CP _X in	CP _Z in	CP _X Ice in	CP _Z Ice in
L1	176.01-130.76	0.0000	0.0000	0.0000	0.0000
L2	130.76-86.13	-0.1969	-0.2154	-0.2726	-0.3136
L3	86.13-43.00	-0.2245	-0.3481	-0.3104	-0.5095
L4	43.00-1.00	-0.2158	-0.2427	-0.3020	-0.4262

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _{AA} Front ft ²	C _{AA} Side ft ²	Weight K
12' Low Profile Platform (Town)	C	None		0.0000	176.00	No Ice 1/2" Ice	15.70 20.10	1.30 1.76
4 Bay Dipole (Town)	C	From Face	4.00 6.00 0.00	0.0000	176.00	No Ice 1/2" Ice	5.40 9.00	0.05 0.08
(2) 2" Dia 8' Omni (Town)	C	From Face	4.00 0.00 0.00	0.0000	176.00	No Ice 1/2" Ice	2.00 3.03	0.01 0.02
Grid Dish (Town)	C	From Face	4.00 -6.00 0.00	0.0000	176.00	No Ice 1/2" Ice	5.40 9.00	0.05 0.08
Dipole (Town)	B	From Face	4.00 0.00 0.00	0.0000	176.00	No Ice 1/2" Ice	5.40 9.00	0.05 0.08
Grid Dish (Town)	B	From Face	4.00 -6.00 0.00	0.0000	176.00	No Ice 1/2" Ice	5.40 9.00	0.05 0.08
(2) 1.5" Dia 4' Omni w/Pipe Mount (Town)	A	From Face	4.00 0.00 0.00	0.0000	176.00	No Ice 1/2" Ice	0.94 1.39	0.02 0.03
4 Bay Dipole (reserved) (Town)	A	From Face	4.00 6.00 0.00	0.0000	176.00	No Ice 1/2" Ice	5.40 9.00	0.05 0.08
18"x18"x4" Panel (Town)	B	From Face	4.00 0.00 1.00	0.0000	176.00	No Ice 1/2" Ice	3.15 3.39	0.02 0.03
Valmont T-Arm (1) (AT&T/Cingular)	A	None		0.0000	170.00	No Ice 1/2" Ice	10.54 14.45	0.34 0.41
Valmont T-Arm (1)	B	None		0.0000	170.00	No Ice	10.54	0.34

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Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment °	Placement ft	C _A A ₁ Front ft ²	C _A A ₁ Side ft ²	Weight K	
(AT&T/Cingular)						1/2" Ice	14.45	14.45	0.41
Valmont T-Arm (1)	C	None		0.0000	170.00	No Ice	10.54	10.54	0.34
(AT&T/Cingular)						1/2" Ice	14.45	14.45	0.41
800-10121	A	From Face	1.00	0.0000	170.00	No Ice	5.62	3.29	0.05
(AT&T/Cingular)			0.00			1/2" Ice	6.04	3.64	0.08
			0.00						
800-10121	B	From Face	1.00	0.0000	170.00	No Ice	5.62	3.29	0.05
(AT&T/Cingular)			0.00			1/2" Ice	6.04	3.64	0.08
			0.00						
800-10121	C	From Face	1.00	0.0000	170.00	No Ice	5.62	3.29	0.05
(AT&T/Cingular)			0.00			1/2" Ice	6.04	3.64	0.08
			0.00						
(2) LPG 21401 TMA	A	From Face	1.00	0.0000	170.00	No Ice	0.95	0.37	0.02
(AT&T/Cingular)			0.00			1/2" Ice	1.09	0.48	0.02
			0.00						
(2) LPG 21401 TMA	B	From Face	1.00	0.0000	170.00	No Ice	0.95	0.37	0.02
(AT&T/Cingular)			0.00			1/2" Ice	1.09	0.48	0.02
			0.00						
(2) LPG 21401 TMA	C	From Face	1.00	0.0000	170.00	No Ice	0.95	0.37	0.02
(AT&T/Cingular)			0.00			1/2" Ice	1.09	0.48	0.02
			0.00						
AM-X-CD-14-65	A	From Face	1.00	0.0000	170.00	No Ice	3.08	1.54	0.04
(AT&T/Cingular)			0.00			1/2" Ice	3.41	1.84	0.05
			0.00						
AM-X-CD-14-65	B	From Face	1.00	0.0000	170.00	No Ice	3.08	1.54	0.04
(AT&T/Cingular)			0.00			1/2" Ice	3.41	1.84	0.05
			0.00						
AM-X-CD-14-65	C	From Face	1.00	0.0000	170.00	No Ice	3.08	1.54	0.04
(AT&T/Cingular)			0.00			1/2" Ice	3.41	1.84	0.05
			0.00						
(2) RRUS-11	A	From Face	1.00	0.0000	170.00	No Ice	2.94	1.25	0.06
(AT&T/Cingular)			0.00			1/2" Ice	3.17	1.41	0.07
			0.00						
(2) RRUS-11	B	From Face	1.00	0.0000	170.00	No Ice	2.94	1.25	0.06
(AT&T/Cingular)			0.00			1/2" Ice	3.17	1.41	0.07
			0.00						
(2) RRUS-11	C	From Face	1.00	0.0000	170.00	No Ice	2.94	1.25	0.06
(AT&T/Cingular)			0.00			1/2" Ice	3.17	1.41	0.07
			0.00						
DC6-48-60-18-8F	C	None		0.0000	170.00	No Ice	1.27	1.27	0.02
(AT&T/Cingular)						1/2" Ice	1.46	1.46	0.04
Valmont T-Arm (1)	A	From Face	2.00	0.0000	160.00	No Ice	10.54	10.54	0.34
(T-Mobile)			0.00			1/2" Ice	14.45	14.45	0.41
			0.00						
Valmont T-Arm (1)	B	From Face	2.00	0.0000	160.00	No Ice	10.54	10.54	0.34
(T-Mobile)			0.00			1/2" Ice	14.45	14.45	0.41
			0.00						
Valmont T-Arm (1)	C	From Face	2.00	0.0000	160.00	No Ice	10.54	10.54	0.34
(T-Mobile)			0.00			1/2" Ice	14.45	14.45	0.41
			0.00						
(2) DR65-19-00DPQ	A	From Face	4.00	0.0000	160.00	No Ice	8.40	3.53	0.03
(T-Mobile)			0.00			1/2" Ice	8.95	3.97	0.07
			0.00						
(2) DR65-19-00DPQ	B	From Face	4.00	0.0000	160.00	No Ice	8.40	3.53	0.03
(T-Mobile)			0.00			1/2" Ice	8.95	3.97	0.07
			0.00						
(2) DR65-19-00DPQ	C	From Face	4.00	0.0000	160.00	No Ice	8.40	3.53	0.03

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A ₁ Front	C _A A ₁ Side	Weight
			Horz	Lateral					
(T-Mobile)			0.00			1/2" Ice	8.95	3.97	0.07
DR65-19-00DPQ (T-Mobile (reserved))	A	From Face	4.00		0.0000	160.00	No Ice	8.40	0.03
			0.00			1/2" Ice	8.95	3.97	0.07
			0.00						
DR65-19-00DPQ (T-Mobile (reserved))	B	From Face	4.00		0.0000	160.00	No Ice	8.40	0.03
			0.00			1/2" Ice	8.95	3.97	0.07
			0.00						
DR65-19-00DPQ (T-Mobile (reserved))	C	From Face	4.00		0.0000	160.00	No Ice	8.40	0.03
			0.00			1/2" Ice	8.95	3.97	0.07
			0.00						
(2) TMA 10"x8"x3" (T-Mobile)	A	From Face	3.00		0.0000	160.00	No Ice	0.78	0.01
			0.00			1/2" Ice	0.90	0.38	0.02
			0.00						
(2) TMA 10"x8"x3" (T-Mobile)	B	From Face	3.00		0.0000	160.00	No Ice	0.78	0.01
			0.00			1/2" Ice	0.90	0.38	0.02
			0.00						
(2) TMA 10"x8"x3" (T-Mobile)	C	From Face	3.00		0.0000	160.00	No Ice	0.78	0.01
			0.00			1/2" Ice	0.90	0.38	0.02
			0.00						
(2) TMA 10"x8"x3" (T-Mobile (reserved))	A	From Face	3.00		0.0000	160.00	No Ice	0.78	0.01
			0.00			1/2" Ice	0.90	0.38	0.02
			0.00						
(2) TMA 10"x8"x3" (T-Mobile (reserved))	B	From Face	3.00		0.0000	160.00	No Ice	0.78	0.01
			0.00			1/2" Ice	0.90	0.38	0.02
			0.00						
(2) TMA 10"x8"x3" (T-Mobile (reserved))	C	From Face	3.00		0.0000	160.00	No Ice	0.78	0.01
			0.00			1/2" Ice	0.90	0.38	0.02
			0.00						
2 Bay Dipole (Town)	C	From Face	4.00		0.0000	160.00	No Ice	0.00	0.00
			6.00			1/2" Ice	0.00	0.00	0.00
			0.00						
Grid Dish (Town)	C	From Face	4.00		0.0000	160.00	No Ice	5.40	0.05
			6.00			1/2" Ice	9.00	9.00	0.08
			0.00						
18"x18"x4" Panel (Town)	C	From Face	4.00		0.0000	160.00	No Ice	3.15	0.02
			6.00			1/2" Ice	3.39	0.85	0.03
			1.00						
840 10054 (Clearwire)	A	From Face	3.00		0.0000	150.00	No Ice	5.24	0.05
			0.00			1/2" Ice	5.61	2.64	0.08
			0.00						
840 10054 (Clearwire)	B	From Face	3.00		0.0000	150.00	No Ice	5.24	0.05
			0.00			1/2" Ice	5.61	2.64	0.08
			0.00						
840 10054 (Clearwire)	C	From Face	3.00		0.0000	150.00	No Ice	5.24	0.05
			0.00			1/2" Ice	5.61	2.64	0.08
			0.00						
Remote Radio Heads U-RAS (Clearwire)	A	From Face	3.00		0.0000	150.00	No Ice	1.80	0.03
			0.00			1/2" Ice	1.99	0.92	0.04
			0.00						
Remote Radio Heads U-RAS (Clearwire)	B	From Face	3.00		0.0000	150.00	No Ice	1.80	0.03
			0.00			1/2" Ice	1.99	0.92	0.04
			0.00						
Remote Radio Heads U-RAS (Clearwire)	C	From Face	3.00		0.0000	150.00	No Ice	1.80	0.03
			0.00			1/2" Ice	1.99	0.92	0.04
			0.00						
Low Profile Platform	C	None			0.0000	150.00	No Ice	17.30	1.50

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A _A Front	C _A A _A Side	Weight
			Horz	Lateral Vert					
			ft	ft	°	ft	ft ²	ft ²	K
(Sprint/Nextel) (2) 48000	A	From Face	3.00	0.0000		150.00	1/2" Ice 22.10	22.10	2.03
(Sprint/Nextel) (2) 48000	B	From Face	0.00 0.00	0.0000		150.00	No Ice 4.51	1.82	0.02
(Sprint/Nextel) (2) 48000	C	From Face	0.00 0.00	0.0000		150.00	1/2" Ice 4.91	2.15	0.04
APXVSP18-C-A20 (Clearwire)	A	From Face	3.00	0.0000		150.00	No Ice 8.40	5.28	0.06
APXVSP18-C-A20 (Clearwire)	B	From Face	0.00 0.00	0.0000		150.00	1/2" Ice 8.95	5.74	0.11
APXVSP18-C-A20 (Clearwire)	C	From Face	0.00 0.00	0.0000		150.00	No Ice 8.40	5.28	0.06
Andrew 800MHz RRH (Clearwire)	A	From Face	3.00	0.0000		150.00	1/2" Ice 2.36	1.97	0.06
Andrew 800MHz RRH (Clearwire)	B	From Face	0.00 0.00	0.0000		150.00	No Ice 2.36	1.97	0.06
Andrew 800MHz RRH (Clearwire)	C	From Face	0.00 0.00	0.0000		150.00	1/2" Ice 2.57	2.17	0.08
Panasonic RRH 1900MHZ (Clearwire)	A	From Face	3.00	0.0000		150.00	No Ice 2.49	3.06	0.09
Panasonic RRH 1900MHZ (Clearwire)	B	From Face	0.00 0.00	0.0000		150.00	1/2" Ice 2.71	3.30	0.12
Panasonic RRH 1900MHZ (Clearwire)	C	From Face	0.00 0.00	0.0000		150.00	No Ice 2.49	3.06	0.09
2 Bay Dipole (Town)	C	From Face	3.00	0.0000		130.00	1/2" Ice 2.71	3.30	0.12
18"x18"x4" Panel (Town)	C	From Face	6.00	0.0000		130.00	No Ice 5.40	5.40	0.05
6' Side Mount Standoff (Town)	C	From Face	0.00 0.00	0.0000		130.00	1/2" Ice 9.00	9.00	0.08
Andrew 12'-6" Low Profile Platform (Verizon)	C	None	0.00	0.0000		112.00	No Ice 3.15	0.70	0.02
BXA-185060/12CF 2 (Verizon)	A	From Face	0.00 1.00	0.0000		112.00	1/2" Ice 3.39	0.85	0.03
LNx-6514DS-T4M (Verizon)	A	From Face	3.00	0.0000		130.00	No Ice 4.97	4.97	0.07
Rymsa MG D3-900Tx (Verizon)	B	From Face	0.00 0.00	0.0000		130.00	1/2" Ice 6.12	6.12	0.13
			0.00				No Ice 14.45	14.45	1.30
							1/2" Ice 19.00	19.00	1.69
			4.00	0.0000		114.00	No Ice 3.10	2.10	0.04
			0.00				1/2" Ice 3.60	2.60	0.08
			4.00	0.0000		114.00	No Ice 8.52	9.95	0.06
			0.00				1/2" Ice 9.11	11.12	0.14
			4.00	0.0000		114.00	No Ice 5.38	5.02	0.05
			4.00				1/2" Ice 5.86	5.96	0.09

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Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _A A ₁ Front	C _A A ₁ Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft ²	ft ²	K	
Side Mount Standoff (T-Mobile (reserved))	B	From Face	1.50	0.00	0.0000	60.00	No Ice	4.97	4.97	0.07
			0.00	0.00			1/2" Ice	6.12	6.12	0.13
			0.00	0.00						
GPS (Sprint/Nextel)	B	From Face	3.00	0.00	0.0000	60.00	No Ice	1.00	1.00	0.01
			0.00	0.00			1/2" Ice	1.50	1.50	0.03
			0.00	0.00						
Scanner Antenna (Town (reserved))	C	From Face	3.00	0.00	0.0000	60.00	No Ice	1.00	1.00	0.01
			0.00	0.00			1/2" Ice	1.50	1.50	0.03
			0.00	0.00						
Side Mount Standoff (T-Mobile (reserved))	C	From Face	1.50	0.00	0.0000	60.00	No Ice	4.97	4.97	0.07
			0.00	0.00			1/2" Ice	6.12	6.12	0.13
			0.00	0.00						
GPS (AT&T) 742-213 (Pocket Wireless)	B	None			0.0000	46.00	No Ice	1.00	1.00	0.01
							1/2" Ice	1.50	1.50	0.03
742-213 (Pocket Wireless)	A	From Leg	3.00	0.00	0.0000	124.00	No Ice	5.14	2.87	0.02
			0.00	0.00			1/2" Ice	5.61	3.48	0.05
			0.00	0.00						
742-213 (Pocket Wireless)	B	From Leg	3.00	0.00	0.0000	124.00	No Ice	5.14	2.87	0.02
			0.00	0.00			1/2" Ice	5.61	3.48	0.05
			0.00	0.00						
742-213 (Pocket Wireless)	C	From Leg	3.00	0.00	0.0000	124.00	No Ice	5.14	2.87	0.02
			0.00	0.00			1/2" Ice	5.61	3.48	0.05
			0.00	0.00						

Dishes

Description	Face or Leg	Dish Type	Offset Type	Offsets:		Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter	Aperture Area	Weight	
				Horz	Vert							
			ft	ft	°	°	ft	ft	ft ²	K		
VHLP2.5-180 (Clearwire)	A	Paraboloid w/Radome	From Face	0.50	0.00	Worst		154.00	2.50	No Ice	4.90	0.07
				0.00	0.00					1/2" Ice	5.24	0.10
				0.00	0.00							
VHLP2.5-180 (Clearwire)	B	Paraboloid w/Radome	From Face	0.50	-1.00	Worst		154.00	2.50	No Ice	4.90	0.07
				0.00	0.00					1/2" Ice	5.24	0.10
				0.00	0.00							
VHLP2.5-180 (Clearwire)	B	Paraboloid w/Radome	From Face	0.50	1.00	Worst		154.00	2.50	No Ice	4.90	0.07
				0.00	0.00					1/2" Ice	5.24	0.10
				0.00	0.00							
PX3F-52 (Clearwire)	C	Paraboloid w/Shroud (HP)	From Face	0.50	1.00	Worst		154.00	3.00	No Ice	7.07	0.04
				0.00	0.00					1/2" Ice	7.47	0.08
				0.00	0.00							
PX3F-52 (Clearwire)	A	Paraboloid w/Shroud (HP)	From Face	0.50	1.00	Worst		154.00	3.00	No Ice	7.07	0.04
				0.00	0.00					1/2" Ice	7.47	0.08
				0.00	0.00							

Tower Pressures - No Ice

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$$G_H = 1.690$$

Section Elevation ft	z ft	K _Z	q _z psf	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
L1 176.01-130.76	152.08	1.547	25	99.550	A	0.000	99.550	99.550	100.00	0.000	0.000
					B	0.000	99.550		100.00	0.000	0.000
					C	0.000	99.550		100.00	0.000	0.000
L2 130.76-86.13	107.70	1.402	23	135.950	A	0.000	135.950	135.950	100.00	0.000	10.570
					B	0.000	135.950		100.00	0.000	0.000
					C	0.000	135.950		100.00	0.000	7.498
L3 86.13-43.00	64.29	1.21	20	166.326	A	0.000	166.326	166.326	100.00	0.000	15.723
					B	0.000	166.326		100.00	0.000	0.000
					C	0.000	166.326		100.00	0.000	8.540
L4 43.00-1.00	21.38	1	16	194.440	A	0.000	194.440	194.440	100.00	0.000	11.633
					B	0.000	194.440		100.00	0.000	0.000
					C	0.000	194.440		100.00	0.000	7.722

Tower Pressure - With Ice

$$G_H = 1.690$$

Section Elevation ft	z ft	K _Z	q _z psf	t _z in	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
L1 176.01-130.76	152.08	1.547	19	0.5000	103.321	A	0.000	103.321	103.321	100.00	0.000	0.000
						B	0.000	103.321		100.00	0.000	0.000
						C	0.000	103.321		100.00	0.000	0.000
L2 130.76-86.13	107.70	1.402	17	0.5000	139.669	A	0.000	139.669	139.669	100.00	0.000	16.444
						B	0.000	139.669		100.00	0.000	0.000
						C	0.000	139.669		100.00	0.000	11.285
L3 86.13-43.00	64.29	1.21	15	0.5000	169.920	A	0.000	169.920	169.920	100.00	0.000	24.648
						B	0.000	169.920		100.00	0.000	0.000
						C	0.000	169.920		100.00	0.000	12.853
L4 43.00-1.00	21.38	1	12	0.5000	197.940	A	0.000	197.940	197.940	100.00	0.000	20.433
						B	0.000	197.940		100.00	0.000	0.000
						C	0.000	197.940		100.00	0.000	11.622

Tower Pressure - Service

$$G_H = 1.690$$

Section Elevation ft	z ft	K _Z	q _z psf	A _G ft ²	F a c e	A _F ft ²	A _R ft ²	A _{leg} ft ²	Leg %	C _A A _A In Face ft ²	C _A A _A Out Face ft ²
L1 176.01-130.76	152.08	1.547	10	99.550	A	0.000	99.550	99.550	100.00	0.000	0.000
					B	0.000	99.550		100.00	0.000	0.000
					C	0.000	99.550		100.00	0.000	0.000
L2 130.76-86.13	107.70	1.402	9	135.950	A	0.000	135.950	135.950	100.00	0.000	10.570
					B	0.000	135.950		100.00	0.000	0.000
					C	0.000	135.950		100.00	0.000	7.498

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Section Elevation	z	K _Z	q _z	A _G	F _a c e	A _F	A _R	A _{leg}	Leg %	C _A A ₁ In Face ft ²	C _A A ₁ Out Face ft ²
ft	ft		psf	ft ²		ft ²	ft ²	ft ²			
L3 86.13-43.00	64.29	1.21	8	166.326	A	0.000	166.326	166.326	100.00	0.000	15.723
					B	0.000	166.326		100.00	0.000	0.000
					C	0.000	166.326		100.00	0.000	8.540
L4 43.00-1.00	21.38	1	6	194.440	A	0.000	194.440	194.440	100.00	0.000	11.633
					B	0.000	194.440		100.00	0.000	0.000
					C	0.000	194.440		100.00	0.000	7.722

Tower Forces - No Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F _a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.20	A	1	0.65	1	1	1	99.550	2.77	61.21	C
			B	1	0.65	1	1	1	99.550			
			C	1	0.65	1	1	1	99.550			
L2 130.76-86.13	3.58	5.92	A	1	0.65	1	1	1	135.950	4.12	92.40	C
			B	1	0.65	1	1	1	135.950			
			C	1	0.65	1	1	1	135.950			
L3 86.13-43.00	3.86	8.95	A	1	0.65	1	1	1	166.326	4.41	102.29	C
			B	1	0.65	1	1	1	166.326			
			C	1	0.65	1	1	1	166.326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194.440	4.04	96.08	C
			B	1	0.65	1	1	1	194.440			
			C	1	0.65	1	1	1	194.440			
Sum Weight:	12.01	30.64						OTM	1219.92 kip-ft	15.34		

Tower Forces - No Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F _a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.20	A	1	0.65	1	1	1	99.550	2.77	61.21	C
			B	1	0.65	1	1	1	99.550			
			C	1	0.65	1	1	1	99.550			
L2 130.76-86.13	3.58	5.92	A	1	0.65	1	1	1	135.950	4.12	92.40	C
			B	1	0.65	1	1	1	135.950			
			C	1	0.65	1	1	1	135.950			
L3 86.13-43.00	3.86	8.95	A	1	0.65	1	1	1	166.326	4.41	102.29	C
			B	1	0.65	1	1	1	166.326			
			C	1	0.65	1	1	1	166.326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194.440	4.04	96.08	C
			B	1	0.65	1	1	1	194.440			
			C	1	0.65	1	1	1	194.440			
Sum Weight:	12.01	30.64						OTM	1219.92 kip-ft	15.34		

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Tower Forces - No Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.20	A	1	0.65	1	1	1	99,550	2.77	61.21	C
			B	1	0.65	1	1	1	99,550			
			C	1	0.65	1	1	1	99,550			
L2 130.76-86.13	3.58	5.92	A	1	0.65	1	1	1	135,950	4.12	92.40	C
			B	1	0.65	1	1	1	135,950			
			C	1	0.65	1	1	1	135,950			
L3 86.13-43.00	3.86	8.95	A	1	0.65	1	1	1	166,326	4.41	102.29	C
			B	1	0.65	1	1	1	166,326			
			C	1	0.65	1	1	1	166,326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194,440	4.04	96.08	C
			B	1	0.65	1	1	1	194,440			
			C	1	0.65	1	1	1	194,440			
Sum Weight:	12.01	30.64						OTM	1219.92 kip-ft	15.34		

Tower Forces - No Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.20	A	1	0.65	1	1	1	99,550	2.77	61.21	C
			B	1	0.65	1	1	1	99,550			
			C	1	0.65	1	1	1	99,550			
L2 130.76-86.13	3.58	5.92	A	1	0.65	1	1	1	135,950	4.12	92.40	C
			B	1	0.65	1	1	1	135,950			
			C	1	0.65	1	1	1	135,950			
L3 86.13-43.00	3.86	8.95	A	1	0.65	1	1	1	166,326	4.41	102.29	C
			B	1	0.65	1	1	1	166,326			
			C	1	0.65	1	1	1	166,326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194,440	4.04	96.08	C
			B	1	0.65	1	1	1	194,440			
			C	1	0.65	1	1	1	194,440			
Sum Weight:	12.01	30.64						OTM	1219.92 kip-ft	15.34		

Tower Forces - With Ice - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.95	A	1	0.65	1	1	1	103,321	2.16	47.64	C
			B	1	0.65	1	1	1	103,321			
			C	1	0.65	1	1	1	103,321			
L2 130.76-86.13	4.22	6.94	A	1	0.65	1	1	1	139,669	3.44	77.17	C
			B	1	0.65	1	1	1	139,669			
			C	1	0.65	1	1	1	139,669			

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L3 86.13-43.00	4.70	10.20	A	1	0.65	1	1	1	169.920	3.70	85.74	C
			B	1	0.65	1	1	1	169.920			
			C	1	0.65	1	1	1	169.920			
L4 43.00-1.00	3.26	14.02	A	1	0.65	1	1	1	197.940	3.34	79.46	C
			B	1	0.65	1	1	1	197.940			
			C	1	0.65	1	1	1	197.940			
Sum Weight:	14.11	35.11						OTM	995.26 kip-ft	12.64		

Tower Forces - With Ice - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.95	A	1	0.65	1	1	1	103.321	2.16	47.64	C
			B	1	0.65	1	1	1	103.321			
			C	1	0.65	1	1	1	103.321			
L2 130.76-86.13	4.22	6.94	A	1	0.65	1	1	1	139.669	3.44	77.17	C
			B	1	0.65	1	1	1	139.669			
			C	1	0.65	1	1	1	139.669			
L3 86.13-43.00	4.70	10.20	A	1	0.65	1	1	1	169.920	3.70	85.74	C
			B	1	0.65	1	1	1	169.920			
			C	1	0.65	1	1	1	169.920			
L4 43.00-1.00	3.26	14.02	A	1	0.65	1	1	1	197.940	3.34	79.46	C
			B	1	0.65	1	1	1	197.940			
			C	1	0.65	1	1	1	197.940			
Sum Weight:	14.11	35.11						OTM	995.26 kip-ft	12.64		

Tower Forces - With Ice - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1 176.01-130.76	1.93	3.95	A	1	0.65	1	1	1	103.321	2.16	47.64	C
			B	1	0.65	1	1	1	103.321			
			C	1	0.65	1	1	1	103.321			
L2 130.76-86.13	4.22	6.94	A	1	0.65	1	1	1	139.669	3.44	77.17	C
			B	1	0.65	1	1	1	139.669			
			C	1	0.65	1	1	1	139.669			
L3 86.13-43.00	4.70	10.20	A	1	0.65	1	1	1	169.920	3.70	85.74	C
			B	1	0.65	1	1	1	169.920			
			C	1	0.65	1	1	1	169.920			
L4 43.00-1.00	3.26	14.02	A	1	0.65	1	1	1	197.940	3.34	79.46	C
			B	1	0.65	1	1	1	197.940			
			C	1	0.65	1	1	1	197.940			
Sum Weight:	14.11	35.11						OTM	995.26 kip-ft	12.64		

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Tower Forces - With Ice - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K	e						ft ²	K	plf	
L1 176.01-130.76	1.93	3.95	A	1	0.65	1	1	1	103.321	2.16	47.64	C
			B	1	0.65	1	1	1	103.321			
			C	1	0.65	1	1	1	103.321			
L2 130.76-86.13	4.22	6.94	A	1	0.65	1	1	1	139.669	3.44	77.17	C
			B	1	0.65	1	1	1	139.669			
			C	1	0.65	1	1	1	139.669			
L3 86.13-43.00	4.70	10.20	A	1	0.65	1	1	1	169.920	3.70	85.74	C
			B	1	0.65	1	1	1	169.920			
			C	1	0.65	1	1	1	169.920			
L4 43.00-1.00	3.26	14.02	A	1	0.65	1	1	1	197.940	3.34	79.46	C
			B	1	0.65	1	1	1	197.940			
			C	1	0.65	1	1	1	197.940			
Sum Weight:	14.11	35.11						OTM	995.26 kip-ft	12.64		

Tower Forces - Service - Wind Normal To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K	e						ft ²	K	plf	
L1 176.01-130.76	1.93	3.20	A	1	0.65	1	1	1	99.550	1.08	23.91	C
			B	1	0.65	1	1	1	99.550			
			C	1	0.65	1	1	1	99.550			
L2 130.76-86.13	3.58	5.92	A	1	0.65	1	1	1	135.950	1.61	36.10	C
			B	1	0.65	1	1	1	135.950			
			C	1	0.65	1	1	1	135.950			
L3 86.13-43.00	3.86	8.95	A	1	0.65	1	1	1	166.326	1.72	39.96	C
			B	1	0.65	1	1	1	166.326			
			C	1	0.65	1	1	1	166.326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194.440	1.58	37.53	C
			B	1	0.65	1	1	1	194.440			
			C	1	0.65	1	1	1	194.440			
Sum Weight:	12.01	30.64						OTM	476.53 kip-ft	5.99		

Tower Forces - Service - Wind 45 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K	e						ft ²	K	plf	
L1 176.01-130.76	1.93	3.20	A	1	0.65	1	1	1	99.550	1.08	23.91	C
			B	1	0.65	1	1	1	99.550			
			C	1	0.65	1	1	1	99.550			
L2 130.76-86.13	3.58	5.92	A	1	0.65	1	1	1	135.950	1.61	36.10	C

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
130.76-86.13			B	1	0.65	1	1	1	135.950			
			C	1	0.65	1	1	1	135.950			
L3	3.86	8.95	A	1	0.65	1	1	1	166.326	1.72	39.96	C
86.13-43.00			B	1	0.65	1	1	1	166.326			
			C	1	0.65	1	1	1	166.326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194.440	1.58	37.53	C
			B	1	0.65	1	1	1	194.440			
			C	1	0.65	1	1	1	194.440			
Sum Weight:	12.01	30.64						OTM	476.53	5.99		
									kip-ft			

Tower Forces - Service - Wind 60 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1	1.93	3.20	A	1	0.65	1	1	1	99.550	1.08	23.91	C
176.01-130.76			B	1	0.65	1	1	1	99.550			
			C	1	0.65	1	1	1	99.550			
L2	3.58	5.92	A	1	0.65	1	1	1	135.950	1.61	36.10	C
130.76-86.13			B	1	0.65	1	1	1	135.950			
			C	1	0.65	1	1	1	135.950			
L3	3.86	8.95	A	1	0.65	1	1	1	166.326	1.72	39.96	C
86.13-43.00			B	1	0.65	1	1	1	166.326			
			C	1	0.65	1	1	1	166.326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194.440	1.58	37.53	C
			B	1	0.65	1	1	1	194.440			
			C	1	0.65	1	1	1	194.440			
Sum Weight:	12.01	30.64						OTM	476.53	5.99		
									kip-ft			

Tower Forces - Service - Wind 90 To Face

Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
L1	1.93	3.20	A	1	0.65	1	1	1	99.550	1.08	23.91	C
176.01-130.76			B	1	0.65	1	1	1	99.550			
			C	1	0.65	1	1	1	99.550			
L2	3.58	5.92	A	1	0.65	1	1	1	135.950	1.61	36.10	C
130.76-86.13			B	1	0.65	1	1	1	135.950			
			C	1	0.65	1	1	1	135.950			
L3	3.86	8.95	A	1	0.65	1	1	1	166.326	1.72	39.96	C
86.13-43.00			B	1	0.65	1	1	1	166.326			
			C	1	0.65	1	1	1	166.326			
L4 43.00-1.00	2.65	12.57	A	1	0.65	1	1	1	194.440	1.58	37.53	C
			B	1	0.65	1	1	1	194.440			
			C	1	0.65	1	1	1	194.440			
Sum Weight:	12.01	30.64						OTM	476.53	5.99		

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Section Elevation	Add Weight	Self Weight	F a c e	e	C _F	R _R	D _F	D _R	A _E	F	w	Ctrl. Face
ft	K	K							ft ²	K	plf	
									kip-ft			

Force Totals

Load Case	Vertical Forces	Sum of Forces X	Sum of Forces Z	Sum of Overturning Moments, M _x	Sum of Overturning Moments, M _z	Sum of Torques
	K	K	K	kip-ft	kip-ft	kip-ft
Leg Weight	30.64					
Bracing Weight	0.00					
Total Member Self-Weight	30.64			1.09	0.50	
Total Weight	52.92			1.09	0.50	
Wind 0 deg - No Ice		0.04	-36.05	-4228.04	-6.61	0.47
Wind 30 deg - No Ice		17.99	-31.24	-3665.00	-2111.73	3.64
Wind 45 deg - No Ice		25.43	-25.52	-2994.38	-2982.96	4.90
Wind 60 deg - No Ice		31.13	-18.06	-2119.63	-3650.88	5.83
Wind 90 deg - No Ice		35.92	-0.04	-6.02	-4211.64	6.46
Wind 120 deg - No Ice		31.09	18.00	2109.50	-3643.76	5.36
Wind 135 deg - No Ice		25.38	25.47	2986.51	-2972.90	4.24
Wind 150 deg - No Ice		17.93	31.21	3660.07	-2099.41	2.83
Wind 180 deg - No Ice		-0.04	36.05	4230.22	7.62	-0.47
Wind 210 deg - No Ice		-17.99	31.24	3667.18	2112.73	-3.64
Wind 225 deg - No Ice		-25.43	25.52	2996.57	2983.97	-4.90
Wind 240 deg - No Ice		-31.13	18.06	2121.82	3651.88	-5.83
Wind 270 deg - No Ice		-35.92	0.04	8.21	4212.65	-6.46
Wind 300 deg - No Ice		-31.09	-18.00	-2107.31	3644.77	-5.36
Wind 315 deg - No Ice		-25.38	-25.47	-2984.32	2973.91	-4.24
Wind 330 deg - No Ice		-17.93	-31.21	-3657.88	2100.42	-2.83
Member Ice	4.47					
Total Weight Ice	64.25			1.45	1.93	
Wind 0 deg - Ice		0.02	-31.64	-3765.02	-2.99	0.27
Wind 30 deg - Ice		15.79	-27.41	-3262.87	-1879.25	3.90
Wind 45 deg - Ice		22.32	-22.39	-2665.32	-2655.91	5.37
Wind 60 deg - Ice		27.32	-15.84	-1886.04	-3251.44	6.48
Wind 90 deg - Ice		31.54	-0.02	-3.47	-3751.90	7.33
Wind 120 deg - Ice		27.30	15.80	1880.43	-3246.53	6.21
Wind 135 deg - Ice		22.28	22.35	2661.27	-2648.96	4.99
Wind 150 deg - Ice		15.75	27.39	3260.85	-1870.73	3.43
Wind 180 deg - Ice		-0.02	31.64	3767.92	6.84	-0.27
Wind 210 deg - Ice		-15.79	27.41	3265.76	1883.10	-3.90
Wind 225 deg - Ice		-22.32	22.39	2668.22	2659.76	-5.37
Wind 240 deg - Ice		-27.32	15.84	1888.94	3255.30	-6.48
Wind 270 deg - Ice		-31.54	0.02	6.36	3755.76	-7.33
Wind 300 deg - Ice		-27.30	-15.80	-1877.53	3250.38	-6.21
Wind 315 deg - Ice		-22.28	-22.35	-2658.37	2652.81	-4.99
Wind 330 deg - Ice		-15.75	-27.39	-3257.95	1874.59	-3.43
Total Weight	52.92			1.09	0.50	
Wind 0 deg - Service		0.01	-14.08	-1650.24	-3.14	0.18
Wind 30 deg - Service		7.03	-12.20	-1430.30	-825.45	1.42
Wind 45 deg - Service		9.93	-9.97	-1168.34	-1165.78	1.92
Wind 60 deg - Service		12.16	-7.05	-826.64	-1426.68	2.28
Wind 90 deg - Service		14.03	-0.01	-1.01	-1645.73	2.52
Wind 120 deg - Service		12.15	7.03	825.36	-1423.91	2.10
Wind 135 deg - Service		9.91	9.95	1167.95	-1161.85	1.66
Wind 150 deg - Service		7.00	12.19	1431.06	-820.64	1.10
Wind 180 deg - Service		-0.01	14.08	1653.77	2.41	-0.18

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Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Overturning Moments, M_x kip-ft	Sum of Overturning Moments, M_z kip-ft	Sum of Torques kip-ft
Wind 210 deg - Service		-7.03	12.20	1433.83	824.73	-1.42
Wind 225 deg - Service		-9.93	9.97	1171.88	1165.05	-1.92
Wind 240 deg - Service		-12.16	7.05	830.18	1425.96	-2.28
Wind 270 deg - Service		-14.03	0.01	4.55	1645.01	-2.52
Wind 300 deg - Service		-12.15	-7.03	-821.83	1423.18	-2.10
Wind 315 deg - Service		-9.91	-9.95	-1164.41	1161.12	-1.66
Wind 330 deg - Service		-7.00	-12.19	-1427.52	819.92	-1.10

Load Combinations

Comb. No.	Description
1	Dead Only
2	Dead+Wind 0 deg - No Ice
3	Dead+Wind 30 deg - No Ice
4	Dead+Wind 45 deg - No Ice
5	Dead+Wind 60 deg - No Ice
6	Dead+Wind 90 deg - No Ice
7	Dead+Wind 120 deg - No Ice
8	Dead+Wind 135 deg - No Ice
9	Dead+Wind 150 deg - No Ice
10	Dead+Wind 180 deg - No Ice
11	Dead+Wind 210 deg - No Ice
12	Dead+Wind 225 deg - No Ice
13	Dead+Wind 240 deg - No Ice
14	Dead+Wind 270 deg - No Ice
15	Dead+Wind 300 deg - No Ice
16	Dead+Wind 315 deg - No Ice
17	Dead+Wind 330 deg - No Ice
18	Dead+Ice+Temp
19	Dead+Wind 0 deg+Ice+Temp
20	Dead+Wind 30 deg+Ice+Temp
21	Dead+Wind 45 deg+Ice+Temp
22	Dead+Wind 60 deg+Ice+Temp
23	Dead+Wind 90 deg+Ice+Temp
24	Dead+Wind 120 deg+Ice+Temp
25	Dead+Wind 135 deg+Ice+Temp
26	Dead+Wind 150 deg+Ice+Temp
27	Dead+Wind 180 deg+Ice+Temp
28	Dead+Wind 210 deg+Ice+Temp
29	Dead+Wind 225 deg+Ice+Temp
30	Dead+Wind 240 deg+Ice+Temp
31	Dead+Wind 270 deg+Ice+Temp
32	Dead+Wind 300 deg+Ice+Temp
33	Dead+Wind 315 deg+Ice+Temp
34	Dead+Wind 330 deg+Ice+Temp
35	Dead+Wind 0 deg - Service
36	Dead+Wind 30 deg - Service
37	Dead+Wind 45 deg - Service
38	Dead+Wind 60 deg - Service
39	Dead+Wind 90 deg - Service
40	Dead+Wind 120 deg - Service
41	Dead+Wind 135 deg - Service
42	Dead+Wind 150 deg - Service
43	Dead+Wind 180 deg - Service
44	Dead+Wind 210 deg - Service

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Comb. No.	Description
45	Dead+Wind 225 deg - Service
46	Dead+Wind 240 deg - Service
47	Dead+Wind 270 deg - Service
48	Dead+Wind 300 deg - Service
49	Dead+Wind 315 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Force K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	176.01 - 130.76	Pole	Max Tension	10	0.00	-0.00	0.00
			Max. Compression	18	-16.05	0.49	0.37
			Max. Mx	14	-10.60	459.86	-1.94
			Max. My	2	-10.59	-1.93	460.43
			Max. Vy	14	-18.15	459.86	-1.94
			Max. Vx	10	18.20	2.19	-459.82
			Max. Torque	24			2.04
L2	130.76 - 86.13	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-30.07	0.85	-2.66
			Max. Mx	14	-21.76	1455.69	-5.51
			Max. My	10	-21.75	4.34	-1462.19
			Max. Vy	14	-27.61	1455.69	-5.51
			Max. Vx	10	27.75	4.34	-1462.19
			Max. Torque	31			7.47
L3	86.13 - 43	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-44.52	0.73	-1.91
			Max. Mx	14	-34.72	2705.08	-6.95
			Max. My	10	-34.71	5.77	-2717.26
			Max. Vy	14	-32.14	2705.08	-6.95
			Max. Vx	10	32.28	5.77	-2717.26
			Max. Torque	31			7.43
L4	43 - 1	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	18	-64.25	1.93	-1.45
			Max. Mx	14	-52.90	4377.44	-8.60
			Max. My	10	-52.90	8.00	-4395.63
			Max. Vy	14	-35.96	4377.44	-8.60
			Max. Vx	10	36.09	8.00	-4395.63
			Max. Torque	31			7.27

Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Pole	Max. Vert	19	64.25	-0.02	31.64
	Max. H _x	14	52.92	35.92	-0.04
	Max. H _z	2	52.92	-0.04	36.05
	Max. M _x	2	4393.31	-0.04	36.05
	Max. M _z	6	4376.39	-35.92	0.04
	Max. Torsion	31	7.15	31.54	-0.02
	Min. Vert	1	52.92	0.00	0.00
	Min. H _x	6	52.92	-35.92	0.04

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Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Min. H _z	10	52.92	0.04	-36.05
	Min. M _x	10	-4395.63	0.04	-36.05
	Min. M _z	14	-4377.44	35.92	-0.04
	Min. Torsion	23	-7.14	-31.54	0.02

Tower Mast Reaction Summary

Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead Only	52.92	0.00	0.00	1.09	0.50	0.00
Dead+Wind 0 deg - No Ice	52.92	0.04	-36.05	-4393.31	-6.96	0.52
Dead+Wind 30 deg - No Ice	52.92	17.99	-31.24	-3808.31	-2194.36	3.61
Dead+Wind 45 deg - No Ice	52.92	25.43	-25.52	-3111.52	-3099.65	4.84
Dead+Wind 60 deg - No Ice	52.92	31.13	-18.06	-2202.59	-3793.68	5.73
Dead+Wind 90 deg - No Ice	52.92	35.92	-0.04	-6.36	-4376.39	6.31
Dead+Wind 120 deg - No Ice	52.92	31.09	18.00	2191.93	-3786.29	5.20
Dead+Wind 135 deg - No Ice	52.92	25.38	25.47	3103.27	-3089.17	4.09
Dead+Wind 150 deg - No Ice	52.92	17.93	31.21	3803.19	-2181.48	2.70
Dead+Wind 180 deg - No Ice	52.92	-0.04	36.05	4395.63	8.00	-0.51
Dead+Wind 210 deg - No Ice	52.92	-17.99	31.24	3810.62	2195.44	-3.59
Dead+Wind 225 deg - No Ice	52.92	-25.43	25.52	3113.80	3100.73	-4.82
Dead+Wind 240 deg - No Ice	52.92	-31.13	18.06	2204.86	3794.76	-5.72
Dead+Wind 270 deg - No Ice	52.92	-35.92	0.04	8.60	4377.44	-6.32
Dead+Wind 300 deg - No Ice	52.92	-31.09	-18.00	-2189.68	3787.30	-5.22
Dead+Wind 315 deg - No Ice	52.92	-25.38	-25.47	-3100.99	3090.17	-4.11
Dead+Wind 330 deg - No Ice	52.92	-17.93	-31.21	-3800.89	2182.48	-2.72
Dead+Ice+Temp	64.25	0.00	0.00	1.45	1.93	-0.00
Dead+Wind 0 deg+Ice+Temp	64.25	0.02	-31.64	-3960.42	-3.25	0.34
Dead+Wind 30 deg+Ice+Temp	64.25	15.79	-27.41	-3432.25	-1976.88	3.87
Dead+Wind 45 deg+Ice+Temp	64.25	22.32	-22.39	-2803.72	-2793.86	5.29
Dead+Wind 60 deg+Ice+Temp	64.25	27.32	-15.84	-1984.02	-3420.31	6.36
Dead+Wind 90 deg+Ice+Temp	64.25	31.54	-0.02	-3.73	-3946.75	7.14
Dead+Wind 120 deg+Ice+Temp	64.25	27.30	15.80	1978.01	-3415.12	6.01
Dead+Wind 135 deg+Ice+Temp	64.25	22.28	22.35	2799.43	-2786.50	4.81
Dead+Wind 150 deg+Ice+Temp	64.25	15.75	27.39	3430.17	-1967.83	3.28
Dead+Wind 180 deg+Ice+Temp	64.25	-0.02	31.64	3963.60	7.28	-0.33
Dead+Wind 210 deg+Ice+Temp	64.25	-15.79	27.41	3435.40	1980.97	-3.85
Dead+Wind 225 deg+Ice+Temp	64.25	-22.32	22.39	2806.85	2797.95	-5.28
Dead+Wind 240 deg+Ice+Temp	64.25	-27.32	15.84	1987.12	3424.39	-6.35
Dead+Wind 270 deg+Ice+Temp	64.25	-31.54	0.02	6.80	3950.79	-7.15
Dead+Wind 300 deg+Ice+Temp	64.25	-27.30	-15.80	-1974.92	3419.11	-6.03
Dead+Wind 315 deg+Ice+Temp	64.25	-22.28	-22.35	-2796.31	2790.48	-4.82
Dead+Wind 330 deg+Ice+Temp	64.25	-15.75	-27.39	-3427.02	1971.82	-3.29
Dead+Wind 0 deg - Service	52.92	0.01	-14.08	-1717.73	-2.40	0.20
Dead+Wind 30 deg - Service	52.92	7.03	-12.20	-1488.91	-858.01	1.42
Dead+Wind 45 deg - Service	52.92	9.93	-9.97	-1216.35	-1212.11	1.90
Dead+Wind 60 deg - Service	52.92	12.16	-7.05	-860.82	-1483.58	2.25
Dead+Wind 90 deg - Service	52.92	14.03	-0.01	-1.76	-1711.48	2.48
Dead+Wind 120 deg - Service	52.92	12.15	7.03	858.09	-1480.66	2.05
Dead+Wind 135 deg - Service	52.92	9.91	9.95	1214.55	-1207.98	1.62
Dead+Wind 150 deg - Service	52.92	7.00	12.19	1488.33	-852.95	1.07
Dead+Wind 180 deg - Service	52.92	-0.01	14.08	1720.07	3.46	-0.20
Dead+Wind 210 deg - Service	52.92	-7.03	12.20	1491.25	859.07	-1.41
Dead+Wind 225 deg - Service	52.92	-9.93	9.97	1218.69	1213.18	-1.90
Dead+Wind 240 deg - Service	52.92	-12.16	7.05	863.16	1484.64	-2.25
Dead+Wind 270 deg - Service	52.92	-14.03	0.01	4.09	1712.54	-2.49

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Load Combination	Vertical K	Shear _x K	Shear _z K	Overturning Moment, M _x kip-ft	Overturning Moment, M _z kip-ft	Torque kip-ft
Dead+Wind 300 deg - Service	52.92	-12.15	-7.03	-855.75	1481.71	-2.05
Dead+Wind 315 deg - Service	52.92	-9.91	-9.95	-1212.22	1209.04	-1.62
Dead+Wind 330 deg - Service	52.92	-7.00	-12.19	-1485.99	854.00	-1.07

Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-52.92	0.00	0.00	52.92	0.00	0.000%
2	0.04	-52.92	-36.05	-0.04	52.92	36.05	0.000%
3	17.99	-52.92	-31.24	-17.99	52.92	31.24	0.000%
4	25.43	-52.92	-25.52	-25.43	52.92	25.52	0.000%
5	31.13	-52.92	-18.06	-31.13	52.92	18.06	0.000%
6	35.92	-52.92	-0.04	-35.92	52.92	0.04	0.000%
7	31.09	-52.92	18.00	-31.09	52.92	-18.00	0.000%
8	25.38	-52.92	25.47	-25.38	52.92	-25.47	0.000%
9	17.93	-52.92	31.21	-17.93	52.92	-31.21	0.000%
10	-0.04	-52.92	36.05	0.04	52.92	-36.05	0.000%
11	-17.99	-52.92	31.24	17.99	52.92	-31.24	0.000%
12	-25.43	-52.92	25.52	25.43	52.92	-25.52	0.000%
13	-31.13	-52.92	18.06	31.13	52.92	-18.06	0.000%
14	-35.92	-52.92	0.04	35.92	52.92	-0.04	0.000%
15	-31.09	-52.92	-18.00	31.09	52.92	18.00	0.000%
16	-25.38	-52.92	-25.47	25.38	52.92	25.47	0.000%
17	-17.93	-52.92	-31.21	17.93	52.92	31.21	0.000%
18	0.00	-64.25	0.00	0.00	64.25	0.00	0.000%
19	0.02	-64.25	-31.64	-0.02	64.25	31.64	0.000%
20	15.79	-64.25	-27.41	-15.79	64.25	27.41	0.000%
21	22.32	-64.25	-22.39	-22.32	64.25	22.39	0.000%
22	27.32	-64.25	-15.84	-27.32	64.25	15.84	0.000%
23	31.54	-64.25	-0.02	-31.54	64.25	0.02	0.000%
24	27.30	-64.25	15.80	-27.30	64.25	-15.80	0.000%
25	22.28	-64.25	22.35	-22.28	64.25	-22.35	0.000%
26	15.75	-64.25	27.39	-15.75	64.25	-27.39	0.000%
27	-0.02	-64.25	31.64	0.02	64.25	-31.64	0.000%
28	-15.79	-64.25	27.41	15.79	64.25	-27.41	0.000%
29	-22.32	-64.25	22.39	22.32	64.25	-22.39	0.000%
30	-27.32	-64.25	15.84	27.32	64.25	-15.84	0.000%
31	-31.54	-64.25	0.02	31.54	64.25	-0.02	0.000%
32	-27.30	-64.25	-15.80	27.30	64.25	15.80	0.000%
33	-22.28	-64.25	-22.35	22.28	64.25	22.35	0.000%
34	-15.75	-64.25	-27.39	15.75	64.25	27.39	0.000%
35	0.01	-52.92	-14.08	-0.01	52.92	14.08	0.000%
36	7.03	-52.92	-12.20	-7.03	52.92	12.20	0.000%
37	9.93	-52.92	-9.97	-9.93	52.92	9.97	0.000%
38	12.16	-52.92	-7.05	-12.16	52.92	7.05	0.000%
39	14.03	-52.92	-0.01	-14.03	52.92	0.01	0.000%
40	12.15	-52.92	7.03	-12.15	52.92	-7.03	0.000%
41	9.91	-52.92	9.95	-9.91	52.92	-9.95	0.000%
42	7.00	-52.92	12.19	-7.00	52.92	-12.19	0.000%
43	-0.01	-52.92	14.08	0.01	52.92	-14.08	0.000%
44	-7.03	-52.92	12.20	7.03	52.92	-12.20	0.000%
45	-9.93	-52.92	9.97	9.93	52.92	-9.97	0.000%
46	-12.16	-52.92	7.05	12.16	52.92	-7.05	0.000%
47	-14.03	-52.92	0.01	14.03	52.92	-0.01	0.000%
48	-12.15	-52.92	-7.03	12.15	52.92	7.03	0.000%
49	-9.91	-52.92	-9.95	9.91	52.92	9.95	0.000%

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Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
50	-7.00	-52.92	-12.19	7.00	52.92	12.19	0.000%

Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	4	0.00000001	0.00000001
2	Yes	4	0.00000001	0.00049363
3	Yes	5	0.00000001	0.00096327
4	Yes	6	0.00000001	0.00004670
5	Yes	5	0.00000001	0.00090173
6	Yes	5	0.00000001	0.00009777
7	Yes	5	0.00000001	0.00098836
8	Yes	6	0.00000001	0.00004647
9	Yes	5	0.00000001	0.00090109
10	Yes	4	0.00000001	0.00061933
11	Yes	5	0.00000001	0.00092358
12	Yes	6	0.00000001	0.00004671
13	Yes	5	0.00000001	0.00098908
14	Yes	5	0.00000001	0.00010756
15	Yes	5	0.00000001	0.00088563
16	Yes	6	0.00000001	0.00004647
17	Yes	5	0.00000001	0.00096892
18	Yes	4	0.00000001	0.00000001
19	Yes	5	0.00000001	0.00036246
20	Yes	6	0.00000001	0.00011261
21	Yes	6	0.00000001	0.00012340
22	Yes	6	0.00000001	0.00010591
23	Yes	5	0.00000001	0.00041737
24	Yes	6	0.00000001	0.00011589
25	Yes	6	0.00000001	0.00012293
26	Yes	6	0.00000001	0.00010600
27	Yes	5	0.00000001	0.00036391
28	Yes	6	0.00000001	0.00010840
29	Yes	6	0.00000001	0.00012373
30	Yes	6	0.00000001	0.00011585
31	Yes	5	0.00000001	0.00042381
32	Yes	6	0.00000001	0.00010445
33	Yes	6	0.00000001	0.00012291
34	Yes	6	0.00000001	0.00011359
35	Yes	4	0.00000001	0.00014458
36	Yes	5	0.00000001	0.00010147
37	Yes	5	0.00000001	0.00011220
38	Yes	5	0.00000001	0.00008958
39	Yes	4	0.00000001	0.00046205
40	Yes	5	0.00000001	0.00010802
41	Yes	5	0.00000001	0.00011161
42	Yes	5	0.00000001	0.00008966
43	Yes	4	0.00000001	0.00015349
44	Yes	5	0.00000001	0.00009378
45	Yes	5	0.00000001	0.00011287
46	Yes	5	0.00000001	0.00010765
47	Yes	4	0.00000001	0.00047903
48	Yes	5	0.00000001	0.00008708
49	Yes	5	0.00000001	0.00011140
50	Yes	5	0.00000001	0.00010345

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Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	176.01 - 130.76	47.753	44	2.4414	0.0048
L2	135.26 - 86.13	27.873	44	2.0847	0.0071
L3	91.88 - 43	12.127	44	1.3196	0.0042
L4	50 - 1	3.395	44	0.6373	0.0015

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
176.00	12' Low Profile Platform	44	47.748	2.4414	0.0048	27805
170.00	Valmont T-Arm (1)	44	44.681	2.4027	0.0053	23132
160.00	Valmont T-Arm (1)	44	39.619	2.3331	0.0060	8683
154.00	VHLP2 5-180	44	36.643	2.2854	0.0064	6315
150.00	840 10054	44	34.696	2.2500	0.0066	5343
130.00	2 Bay Dipole	44	25.607	2.0093	0.0071	3401
124.00	742-213	44	23.142	1.9137	0.0069	3389
114.00	BXA-185060/12CF 2	44	19.317	1.7373	0.0062	3369
112.00	Andrew 12'-6" Low Profile Platform	44	18.595	1.7002	0.0061	3364
100.00	6' Side Mount Standoff	44	14.562	1.4725	0.0050	3339
75.00	Side Mount Standoff	44	7.830	1.0235	0.0029	3267
60.00	Side Mount Standoff	44	4.890	0.7849	0.0020	3220
46.00	GPS	44	2.901	0.5805	0.0013	3474

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	176.01 - 130.76	121.642	11	6.2298	0.0123
L2	135.26 - 86.13	71.067	11	5.3173	0.0205
L3	91.88 - 43	30.956	11	3.3682	0.0123
L4	50 - 1	8.672	11	1.6278	0.0043

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
176.00	12' Low Profile Platform	11	121.629	6.2296	0.0123	11153
170.00	Valmont T-Arm (1)	11	113.827	6.1303	0.0136	9278
160.00	Valmont T-Arm (1)	11	100.952	5.9521	0.0162	3481

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Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
154.00	VHLP2.5-180	11	93.381	5.8299	0.0176	2530
150.00	840 10054	11	88.427	5.7393	0.0185	2140
130.00	2 Bay Dipole	11	65.299	5.1252	0.0204	1357
124.00	742-213	11	59.022	4.8814	0.0198	1350
114.00	BXA-185060/12CF 2	11	49.281	4.4321	0.0180	1338
112.00	Andrew 12'-6" Low Profile Platform	11	47.441	4.3377	0.0175	1336
100.00	6' Side Mount Standoff	11	37.164	3.7576	0.0144	1322
75.00	Side Mount Standoff	11	19.995	2.6135	0.0084	1287
60.00	Side Mount Standoff	11	12.490	2.0047	0.0057	1265
46.00	GPS	11	7.411	1.4829	0.0038	1362

Compression Checks

Pole Design Data

Section No.	Elevation	Size	L	L _n	Kl/r	F _a	A	Actual P	Allow. P _a	Ratio P/P _a
	ft		ft	ft		ksi	in ²	K	K	
L1	176.01 - 130.76 (1)	TP31.8x21x0.25	45.25	0.00	0.0	39.000	24.1827	-10.59	943.13	0.011
L2	130.76 - 86.13 (2)	TP41.82x30.226x0.3125	49.13	0.00	0.0	39.000	39.8244	-21.75	1553.15	0.014
L3	86.13 - 43 (3)	TP51.36x39.8381x0.375	48.88	0.00	0.0	39.000	58.7209	-34.71	2290.12	0.015
L4	43 - 1 (4)	TP60.5x48.96x0.4375	49.00	0.00	0.0	39.000	83.4043	-52.90	3252.77	0.016

Pole Bending Design Data

Section No.	Elevation	Size	Actual M _x	Actual f _{bx}	Allow. F _{bx}	Ratio f _{bx} /F _{bx}	Actual M _y	Actual f _{by}	Allow. F _{by}	Ratio f _{by} /F _{by}
	ft		kip-ft	ksi	ksi		kip-ft	ksi	ksi	
L1	176.01 - 130.76 (1)	TP31.8x21x0.25	461.92	30.491	39.000	0.782	0.00	0.000	39.000	0.000
L2	130.76 - 86.13 (2)	TP41.82x30.226x0.3125	1464.31	44.531	39.000	1.142	0.00	0.000	39.000	0.000
L3	86.13 - 43 (3)	TP51.36x39.8381x0.375	2719.27	45.635	39.000	1.170	0.00	0.000	39.000	0.000
L4	43 - 1 (4)	TP60.5x48.96x0.4375	4397.81	42.668	39.000	1.094	0.00	0.000	39.000	0.000

Pole Shear Design Data

Section No.	Elevation	Size	Actual V	Actual f _v	Allow. F _v	Ratio f _v /F _v	Actual T	Actual f _{vt}	Allow. F _{vt}	Ratio f _{vt} /F _{vt}
	ft		K	ksi	ksi		kip-ft	ksi	ksi	
L1	176.01 - 130.76 (1)	TP31.8x21x0.25	18.22	0.754	26.000	0.058	1.68	0.054	26.000	0.002
L2	130.76 - 86.13 (2)	TP41.82x30.226x0.3125	27.75	0.697	26.000	0.054	2.75	0.041	26.000	0.002

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Section No.	Elevation ft	Size	Actual V K	Actual f _v ksi	Allow. F _v ksi	Ratio f _v F _v	Actual T kip-ft	Actual f _{vt} ksi	Allow. F _{vt} ksi	Ratio f _{vt} F _{vt}
L3	86.13 - 43 (3)	TP51.36x39.8381x0.375	32.27	0.550	26.000	0.042	3.73	0.031	26.000	0.001
L4	43 - 1 (4)	TP60.5x48.96x0.4375	36.09	0.433	26.000	0.033	3.60	0.017	26.000	0.001

Pole Interaction Design Data

Section No.	Elevation ft	Ratio P P _u	Ratio f _{bx} F _{bx}	Ratio f _{by} F _{by}	Ratio f _v F _v	Ratio f _{vt} F _{vt}	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	176.01 - 130.76 (1)	0.011	0.782	0.000	0.058	0.002	0.794	1.333	H1-3+VT ✓
L2	130.76 - 86.13 (2)	0.014	1.142	0.000	0.054	0.002	1.157	1.333	H1-3+VT ✓
L3	86.13 - 43 (3)	0.015	1.170	0.000	0.042	0.001	1.186	1.333	H1-3+VT ✓
L4	43 - 1 (4)	0.016	1.094	0.000	0.033	0.001	1.111	1.333	H1-3+VT ✓

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	SF*P _{allow} K	% Capacity	Pass Fail	
L1	176.01 - 130.76	Pole	TP31.8x21x0.25	1	-10.59	1257.19	59.6	Pass	
L2	130.76 - 86.13	Pole	TP41.82x30.226x0.3125	2	-21.75	2070.35	86.8	Pass	
L3	86.13 - 43	Pole	TP51.36x39.8381x0.375	3	-34.71	3052.73	89.0	Pass	
L4	43 - 1	Pole	TP60.5x48.96x0.4375	4	-52.90	4335.94	83.3	Pass	
							Summary		
							Pole (L3)	89.0	Pass
							RATING =	89.0	Pass

**ANCHOR BOLT AND
BASE PLATE ANALYSIS**

ANCHOR BOLT AND BASE PLATE ANALYSIS

Input Data

Tower Reactions:

Overturning Moment:	OM := 4373ft·kips	<i>user input</i>
Shear Force:	Shear := 36-kips	<i>user input</i>
Axial Force:	Axial := 53-kips	<i>user input</i>

Anchor Bolt Data:

Use ASTM A615 Grade 75		<i>user input</i>
Number of Anchor Bolts = N	N := 18	<i>user input</i>
Diameter of Bolt Circle:	D_{bc} := 70.00in	<i>user input</i>
Bolt "Column" Distance:	l := 3.0in	<i>user input</i>
Bolt Ultimate Strength:	F_u := 100-ksi	<i>user input</i>
Bolt Yield Strength:	F_y := 75-ksi	<i>user input</i>
Bolt Modulus:	E := 29000-ksi	<i>user input</i>
Anchor Bolt Diameter	D := 2.25in	<i>user input</i>
Threads per Inch:	n := 4.5	<i>user input</i>

Base Plate Data:

Use ASTM A572 Grade 60		<i>user input</i>
Plate Yield Strength:	F_{ybp} := 60-ksi	<i>user input</i>
Base Plate Thickness:	PlateThickness := 2.00-in	<i>user input</i>
Base Plate Diameter:	D_{bp} := 76.00in	<i>user input</i>
Outer Pole Diameter:	D_{pole} := 60.50in	<i>user input</i>

Geometric Layout Data:

Distance from the center of gravity of the group to bolt in question = d(i)

Radius of Bolt Circle: $R_{bc} := \frac{D_{bc}}{2}$

Distance to Bolts: $i := 1..N$

$$d_i := \begin{cases} \theta \leftarrow 2 \cdot \pi \cdot \left(\frac{i}{N}\right) \\ d \leftarrow R_{bc} \cdot \sin(\theta) \end{cases}$$

$d_1 = 11.97 \cdot \text{in}$	$d_7 = 22.50 \cdot \text{in}$
$d_2 = 22.50 \cdot \text{in}$	$d_8 = 11.97 \cdot \text{in}$
$d_3 = 30.31 \cdot \text{in}$	$d_9 = 0.00 \cdot \text{in}$
$d_4 = 34.47 \cdot \text{in}$	$d_{10} = -11.97 \cdot \text{in}$
$d_5 = 34.47 \cdot \text{in}$	$d_{11} = -22.50 \cdot \text{in}$
$d_6 = 30.31 \cdot \text{in}$	etc.

Critical Distances For Bending in Plate:

Outer Pole Radius: $R_{pole} := \frac{D_{pole}}{2}$ $R_{pole} = 30.25 \cdot \text{in}$

Moment Arms of Bolts about Neutral Axis: $MA_i := \text{if}(d_i \geq R_{pole}, d_i - R_{pole}, 0 \text{in})$

$MA_1 = 0.00 \cdot \text{in}$	$MA_7 = 0.00 \cdot \text{in}$
$MA_2 = 0.00 \cdot \text{in}$	$MA_8 = 0.00 \cdot \text{in}$
$MA_3 = 0.06 \cdot \text{in}$	$MA_9 = 0.00 \cdot \text{in}$
$MA_4 = 4.22 \cdot \text{in}$	$MA_{10} = 0.00 \cdot \text{in}$
$MA_5 = 4.22 \cdot \text{in}$	$MA_{11} = 0.00 \cdot \text{in}$
$MA_6 = 0.06 \cdot \text{in}$	etc.

Effective Width of Baseplate for Bending: $\text{EffectiveWidth} := .9 \cdot 2 \cdot \sqrt{\left(\frac{D_{bp}}{2}\right)^2 - \left(\frac{D_{pole}}{2}\right)^2}$ $\text{EffectiveWidth} = 41.40 \cdot \text{in}$

Anchor Bolt Analysis:Polar Moment of Inertia I_p :

$$I_p := \sum_i (d_i)^2 \quad I_p = 1.103 \times 10^4 \cdot \text{in}^2$$

Gross Area of Bolt:

$$A_g := \frac{\pi}{4} \cdot D^2 \quad A_g = 3.976 \cdot \text{in}^2$$

Net Area of Bolt:

$$A_n := \frac{\pi}{4} \cdot \left(D - \frac{0.9743 \cdot \text{in}}{n} \right)^2 \quad A_n = 3.248 \cdot \text{in}^2$$

Net Diameter:

$$D_n := \frac{2 \cdot \sqrt{A_n}}{\sqrt{\pi}} \quad D_n = 2.03 \cdot \text{in}$$

Radius of Gyration of Bolt:

$$r := \frac{D_n}{4} \quad r = 0.51 \cdot \text{in}$$

Section Modulus of Bolt:

$$S_x := \frac{\pi \cdot D_n^3}{32} \quad S_x = 0.826 \cdot \text{in}^3$$

Anchor Bolt Bending Stress:

Maximum Applied Bending:

$$M_x := \left(\frac{\text{Shear}}{N} \right) \cdot l \quad M_x = 0.500 \cdot \text{ft} \cdot \text{kips}$$

$$f_{bx} := \frac{M_x}{S_x} \quad f_{bx} = 7.3 \cdot \text{ksi}$$

Allowable Bending

$$F_{bx} := 1.333 \cdot 0.60 \cdot F_y \quad F_{bx} = 60.0 \cdot \text{ksi}$$

Note: 1.333 increase allowed per TIA/EIA



Job 176' EEI Monopole - Berlin, CT

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Description Anchor Bolt and Base Plate Analysis

Computed by MCD

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Date 11/11/13

Checked by

Date

Check Tensile Forces:

Maximum Tensile Force (Gross Area):

$$\text{AllowableTension} := 1.333 \cdot (0.33 \cdot A_g \cdot F_u)$$

$$\text{AllowableTension} = 174.9 \cdot \text{kips}$$

Note: 1.333 increase allowed per TIA/EIA

Maximum Tensile Force (Net Area):

$$F_{\text{net.area}} := 1.333 \cdot (0.60 \cdot A_n \cdot F_y)$$

$$F_{\text{net.area}} = 194.8 \cdot \text{kips}$$

Note: 1.333 increase allowed per TIA/EIA

Applied Tension:

$$\text{MaxTension} := \frac{\text{OM} \cdot R_{bc}}{I_p} - \frac{\text{Axial}}{N}$$

$$\text{MaxTension} = 163.6 \cdot \text{kips}$$

Check Stresses:

Note: Bolts supplied are "upset bolts." Use net area for checking per AISC.

$$\frac{\text{MaxTension}}{F_{\text{net.area}}} = 0.84$$

$$\text{Condition} := \text{if} \left(\frac{\text{MaxTension}}{F_{\text{net.area}}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition = "OK"

Check Compression & Combined Stresses (if required):

Check to see if a complete combined stress analysis is required:

Per ASCE Manual 72: "If the clearance between the base plate and concrete does not exceed two times the bolt diameter a bending stress analysis of the bolts is NOT normally required."

Set the clear space between the plate and bolt to zero and remove bending stresses if a combined stress analysis is not required:

$$l := \begin{cases} 1 & \text{if } l > 2 \cdot D_n \\ 0.00\text{in} & \text{otherwise} \end{cases} \quad l = 0.00\text{in}$$

$$f_{bx} := \begin{cases} f_{bx} & \text{if } l > 2 \cdot D_n \\ 0.0\text{ksi} & \text{otherwise} \end{cases} \quad f_{bx} = 0.0\text{ksi}$$

Allowable Compressive Force:

$$K := 0.65$$

$$C_c := \sqrt{\frac{2 \cdot \pi^2 \cdot E}{F_y}} \quad C_c = 87.36$$

$$F_a := \begin{cases} \frac{\left[1 - \frac{\left(\frac{K \cdot l}{r} \right)^2}{2 \cdot C_c^2} \right] \cdot F_y}{\frac{5}{3} + \frac{3 \cdot \left(\frac{K \cdot l}{r} \right)}{8 \cdot C_c} - \frac{\left(\frac{K \cdot l}{r} \right)^3}{8 \cdot C_c^3}} & \text{if } \frac{K \cdot l}{r} \leq C_c \\ \frac{12 \cdot \pi^2 \cdot E}{23 \cdot \left(\frac{K \cdot l}{r} \right)^2} & \text{if } \frac{K \cdot l}{r} > C_c \end{cases} \quad F_a = 45.0\text{ksi}$$

$$F_a := 1.333 \cdot F_a \quad \text{Note: 1.333 increase allowed per TIA/EIA} \quad F_a = 60.0\text{ksi}$$

Applied Compressive Force:

$$\text{MaxCompression} := \frac{OM \cdot R_{bc}}{I_p} + \frac{\text{Axial}}{N} \quad \text{MaxCompression} = 169.5\text{kips}$$

$$f_a := \frac{\text{MaxCompression}}{A_n} \quad f_a = 52.2\text{ksi}$$

Check Combined Stresses:

$$\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} = 0.87$$

$$\text{Condition} := \text{if} \left(\frac{f_a}{F_a} + \frac{f_{bx}}{F_{bx}} \leq 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition = "OK"

Base Plate Analysis:

Force from Bolt(s):

$$C_i := \frac{OM \cdot d_i}{I_p} + \frac{Axial}{N}$$

$C_1 = 59.9 \cdot \text{kips}$	$C_7 = 110.0 \cdot \text{kips}$
$C_2 = 110.0 \cdot \text{kips}$	$C_8 = 59.9 \cdot \text{kips}$
$C_3 = 147.2 \cdot \text{kips}$	$C_9 = 2.9 \cdot \text{kips}$
$C_4 = 167.0 \cdot \text{kips}$	$C_{10} = -54.0 \cdot \text{kips}$
$C_5 = 167.0 \cdot \text{kips}$	$C_{11} = -104.1 \cdot \text{kips}$
$C_6 = 147.2 \cdot \text{kips}$	etc.

Bending Stress in Plate:

$$f_{bp} := \sum_i \frac{6 \cdot C_i \cdot MA_i}{\text{EffectiveWidth} \cdot \text{PlateThickness}^2} \quad f_{bp} = 51.7 \cdot \text{ksi}$$

Check Stresses:

$$\frac{f_{bp}}{1.333 \cdot 0.75 F_{y_{bp}}} = 0.86$$

$$\text{Condition} := \text{if} \left(\frac{f_{bp}}{1.333 \cdot 0.75 F_{y_{bp}}} < 1.00, \text{"OK"}, \text{"Overstressed"} \right)$$

Condition = "OK"

FOUNDATION ANALYSIS

Job	176' EEI Monopole - Berlin, CT	Project No.	VZ5-168	Sheet	1	of	2
Description	Caisson Foundation Evaluation	Computed by	MCD	Date	11/11/13		
		Checked by		Date			

Check Foundation Depth TIA/EIA-222-F 7.2.5

Shear Force: $S := 36k$ *USER INPUT*

Overturing Moment: $M := 4373ft \cdot k$ *USER INPUT*

Foundation Diameter: $d := 7.5ft$ *USER INPUT*

Overall Length of Caisson: $L_c := 39ft$ *USER INPUT*

Depth From Top of Caisson to Grade: $L_{pag} := 4ft$ *USER INPUT*

Depth of Caisson Below Ground Level: $LD := L_c - L_{pag}$ $LD = 35.0ft$ *USER INPUT*

Depth Required:

$$LD1 := 2.0ft + \left(\frac{S \cdot ft^2}{3k \cdot d} \right) + 2ft \cdot \left(\frac{M \cdot ft}{3 \cdot k \cdot d} + \frac{S \cdot ft}{2k} + \frac{S^2 \cdot ft^3}{18k^2 \cdot d^2} \right)^{.5} \quad LD1 = 32.8ft$$

DepthCheck := if(LD1 ≤ LD, "OK", "NO GOOD") DepthCheck = "OK"

Note: Result not applicable. Actual soil is better than normal soil as defined in TIA/EIA 222 F. Refer to L-Pile analysis.

Moment Capacity:

Bending Moment: $M_u := 4684.29ft \cdot k$ *USER INPUT-FROM LPILE*

Moment Capacity: $M_n := 8817.64ft \cdot k$ *USER INPUT-FROM LPILE*

Factor of Safety: $FS := \frac{M_n}{M_u}$ $FS = 1.9$

Factor of Safety Required $FS_{reqd} := 1.3$ FOSCheck := if(FS ≥ FS_{reqd}, "OK", "NO GOOD") FOSCheck = "OK"

Factor of Safety Ratio: $FS_{ratio} := \left(\frac{FS_{reqd}}{FS} \right) = 0.69$

Axial Capacity:

Applied Axial Load: $A1 := 53k$ *USER INPUT*

Concrete Weight: $A2 := .150 \frac{k}{ft^3} \cdot LD \cdot \pi \frac{d^2}{4}$ $A2 = 231.9 \cdot k$

Total Axial Load: $AT := A1 + A2$ $AT = 284.9 \cdot k$

Number of Rebar: $n := 30$ *USER INPUT*

Area of Rebar: $Ar := 1.56in^2$ *USER INPUT* #11

Rebar Yield Strength: $fy := 60ksi$ *USER INPUT*

Area of Concrete: $Ag := \pi \cdot \frac{d^2}{4}$ $Ag = 44.2 \cdot ft^{2.0}$

Concrete Comp Strength: $fc := 4ksi$ *USER INPUT*

Axial Capacity: $Po := n \cdot Ar \cdot fy + (Ag - n \cdot Ar) \cdot 0.85 \cdot fc$ $Po = 24278.7 \cdot k$

AxialCheck := if(AT ≤ Po, "OK", "NO GOOD") $AxialCheck = "OK"$

Caisson Analysis.lp6o

Pile Plus for Windows, Version 2012-06.031
Analysis of Individual Piles and Drilled Shafts
Subjected to Lateral Loading Using the p-y Method
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URS Corporation
Rocky Hill, CT

Serial Number of Security Device: 138585063
Company Name Stored in Security Device: URS

Files Used for Analysis

Path to file locations: Y:\Back-up VZ5-168\MathCAD\Foundation\
Name of input data file: Caisson Analysis.lp6d
Name of output report file: Caisson Analysis.lp6o
Name of plot output file: Caisson Analysis.lp6p
Name of runtime message file: Caisson Analysis.lp6r

Date and Time of Analysis

Date: November 11, 2013 Time: 14:51:24

Problem Title

176' EEI Monopole - Berlin Fire Department

Job Number:

Client:

Engineer:

Description:

Program Options

Engineering units are US Customary Units: pounds, inches, feet

Basic Program Options:

This analysis computes pile response to lateral loading and will compute nonlinear moment-curvature and nominal moment capacity for section types with nonlinear properties.

Computation Options:

- Analysis does not use p-y multipliers (individual pile or shaft only)
- Analysis assumes no shear resistance at pile tip
- Analysis for fixed-length pile or shaft only
- No computation of foundation stiffness matrix values
- Report pile response for full length of pile
- Analysis assumes no loading by soil movements acting on pile
- No p-y curves to be computed and reported for user-specified depths

Solution Control Parameters:

- Number of pile increments = 100
- Maximum number of iterations allowed = 200
- Deflection tolerance for convergence = 1.0000E-05 in
- Maximum allowable deflection = 100.0000 in

Pile Response Output Options:

Caisson Analysis.lp60

- Values of pile-head deflection, bending moment, shear force, and soil reaction are printed for full length of pile.
- Printing Increment (nodal spacing of output points) = 1

Pile Structural Properties and Geometry

Total Number of Sections = 1
Total Pile Length = 39.00 ft
Depth of ground surface below top of pile = 4.00 ft

Pile dimensions used for p-y curve computations defined using 2 points. p-y curves are computed using values of pile diameter interpolated over the length of the pile.

Point	Depth X ft	Pile Diameter in
1	0.00000	90.0000000
2	39.000000	90.0000000

Input Structural Properties:

Pile Section No. 1:

Section Type = Drilled Shaft with Permanent Casing
Section Length = 39.00000000 ft
Section Diameter = 90.00000000 in

Ground Slope and Pile Batter Angles

Ground Slope Angle = 0.000 degrees
= 0.000 radians
Pile Batter Angle = 0.000 degrees
= 0.000 radians

Soil and Rock Layering Information

The soil profile is modelled using 4 layers

Layer 1 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 4.00000 ft
Distance from top of pile to bottom of layer = 5.00000 ft
Effective unit weight at top of layer = 61.99890 pcf
Effective unit weight at bottom of layer = 61.99890 pcf
Friction angle at top of layer = 30.00000 deg.
Friction angle at bottom of layer = 30.00000 deg.
Subgrade k at top of layer = 1.00000 pci
Subgrade k at bottom of layer = 1.00000 pci

Layer 2 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 5.00000 ft
Distance from top of pile to bottom of layer = 11.00000 ft
Effective unit weight at top of layer = 61.99890 pcf
Effective unit weight at bottom of layer = 61.99890 pcf
Friction angle at top of layer = 32.00000 deg.
Friction angle at bottom of layer = 32.00000 deg.
Subgrade k at top of layer = 25.00000 pci
Subgrade k at bottom of layer = 25.00000 pci

Layer 3 is sand, p-y criteria by Reese et al., 1974

Caisson Analysis.lp60

Distance from top of pile to top of layer = 11.00000 ft
 Distance from top of pile to bottom of layer = 16.00000 ft
 Effective unit weight at top of layer = 61.99890 pcf
 Effective unit weight at bottom of layer = 61.99890 pcf
 Friction angle at top of layer = 34.00000 deg.
 Friction angle at bottom of layer = 34.00000 deg.
 Subgrade k at top of layer = 35.00000 pci
 Subgrade k at bottom of layer = 35.00000 pci

Layer 4 is sand, p-y criteria by Reese et al., 1974

Distance from top of pile to top of layer = 16.00000 ft
 Distance from top of pile to bottom of layer = 39.00000 ft
 Effective unit weight at top of layer = 61.99890 pcf
 Effective unit weight at bottom of layer = 61.99890 pcf
 Friction angle at top of layer = 36.00000 deg.
 Friction angle at bottom of layer = 36.00000 deg.
 Subgrade k at top of layer = 120.00000 pci
 Subgrade k at bottom of layer = 120.00000 pci

(Depth of lowest soil layer extends 0.00 ft below pile tip)

 Summary of Soil Properties

Uniaxial In-situ Layer	RQD % In-situ	Layer Soil Type	Strain Elastic Factor	Layer Depth	Effective Rock Unit Wt.	Undrained Mass Cohesion	Angle of Friction		
								Test Property	or Subgrade (p-y Curve Criteria)
Num. psi	Test	or Subgrade (p-y Curve Criteria)	J Factor	ft	pci	psi	psf	deg.	Test Type
1	Sand (Reese, et al.)			4.000	1.000	61.999	--	30.000	--
--	--	--	--	5.000	1.000	61.999	--	30.000	--
2	Sand (Reese, et al.)			5.000	25.000	61.999	--	32.000	--
--	--	--	--	11.000	25.000	61.999	--	32.000	--
3	Sand (Reese, et al.)			11.000	35.000	61.999	--	34.000	--
--	--	--	--	16.000	35.000	61.999	--	34.000	--
4	Sand (Reese, et al.)			16.000	120.000	61.999	--	36.000	--
--	--	--	--	39.000	120.000	61.999	--	36.000	--

 Loading Type

Static loading criteria were used when computing p-y curves for all analyses.

 Pile-head Loading and Pile-head Fixity Conditions

Number of loads specified = 2

Load No. Length	Load Type	Condition		Caisson Analysis.lp60		Axial Thrust Force, lbs	Compute Top y vs. Pile
		1		Condition 2			
1	1	V =	31636. lbs	M =	47587518. in-lbs	64248.	False
2	1	V =	36055. lbs	M =	52773727. in-lbs	52919.	False

V = perpendicular shear force applied to pile head
M = bending moment applied to pile head
y = lateral deflection relative to pile axis
S = pile slope relative to original pile batter angle
R = rotational stiffness applied to pile head
Axial thrust is assumed to be acting axially for all pile batter angles.

Computations of Nominal Moment Capacity and Nonlinear Bending Stiffness

Axial thrust force values were determined from pile-head loading conditions

Number of Pile Sections Analyzed = 1

Pile Section No. 1:

Dimensions and Properties of Drilled Shaft with Permanent Casing:

Length of Section	=	39.00000000	ft
Outer Diameter of Casing	=	90.00000000	in
Concrete Cover Thickness Inside Casing	=	3.29532744	in
Casing Wall Thickness	=	0.00000000	in
Moment of Inertia of Steel Casing	=	0.00000000	in ⁴
Yield Stress of Casing	=	60.00000000	ksi
Elastic Modulus of Casing	=	29000.	ksi
Number of Reinforcing Bars	=	30	bars
Area of Single Reinforcing Bar	=	1.56000000	sq. in.
Edge-to-Edge Bar Spacing	=	7.16126554	in
Rebar Offset	=	0.00000000	in
Yield Stress of Reinforcing Bars	=	60.00000000	ksi
Elastic Modulus of Reinforcing Bars	=	29000.	ksi
Area of Concrete	=	6314.92512352	sq. in.
Cross-sectional Area of Steel Casing	=	0.00000000	sq. in.
Area of All Steel (Casing and Bars)	=	46.80000000	sq. in.
Area Ratio of All Steel	=	0.74	percent

Axial Structural Capacities:

Nom. Axial Structural Capacity = 0.85 Fc AC + Fy As	=	18911.060	kips
Tensile Load for Cracking of Concrete	=	-2443.521	kips
Nominal Axial Tensile Capacity	=	-2808.000	kips

Reinforcing Bar Dimensions and Positions Used in Computations:

Bar Number	Bar Diam. inches	Bar Area sq. in.	X inches	Y inches
1	1.41000	1.56000	40.99967	0.00000
2	1.41000	1.56000	40.10373	8.52431
3	1.41000	1.56000	37.45506	16.67607
4	1.41000	1.56000	33.16943	24.09900
5	1.41000	1.56000	27.43414	30.46869
6	1.41000	1.56000	20.49984	35.50676
7	1.41000	1.56000	12.66960	38.99301
8	1.41000	1.56000	4.28563	40.77507
9	1.41000	1.56000	-4.28563	40.77507
10	1.41000	1.56000	-12.66960	38.99301
11	1.41000	1.56000	-20.49984	35.50676
12	1.41000	1.56000	-27.43414	30.46869
13	1.41000	1.56000	-33.16943	24.09900
14	1.41000	1.56000	-37.45506	16.67607
15	1.41000	1.56000	-40.10373	8.52431
16	1.41000	1.56000	-40.99967	0.00000
17	1.41000	1.56000	-40.10373	-8.52431
18	1.41000	1.56000	-37.45506	-16.67607
19	1.41000	1.56000	-33.16943	-24.09900
20	1.41000	1.56000	-27.43414	-30.46869
21	1.41000	1.56000	-20.49984	-35.50676

Caisson Analysis.lp6o				
22	1.41000	1.56000	-12.66960	-38.99301
23	1.41000	1.56000	-4.28563	-40.77507
24	1.41000	1.56000	4.28563	-40.77507
25	1.41000	1.56000	12.66960	-38.99301
26	1.41000	1.56000	20.49984	-35.50676
27	1.41000	1.56000	27.43414	-30.46869
28	1.41000	1.56000	33.16943	-24.09900
29	1.41000	1.56000	37.45506	-16.67607
30	1.41000	1.56000	40.10373	-8.52431

Concrete Properties:

Compressive Strength of Concrete	=	3.00000000	ksi
Modulus of Elasticity of Concrete	=	3122.01857778	ksi
Modulus of Rupture of Concrete	=	-0.41079191	ksi
Compression Strain at Peak Stress	=	0.00163356	
Tensile Strain at Fracture of Concrete	=	-0.00011596	
Maximum Coarse Aggregate Size	=	0.00000000	in

Number of Axial Thrust Force Values Determined from Pile-head Loadings = 2

Number	Axial Thrust Force kips
1	52.919
2	64.248

Definitions of Run Messages and Notes:

- C = concrete in section has cracked in tension.
- Y = stress in reinforcing steel has reached yield stress.
- T = ACI 318-08 criteria for tension-controlled section met, tensile strain in reinforcement exceeds 0.005 while simultaneously compressive strain in concrete more than than 0.003. See ACI 318-08, Section 10.3.4.
- Z = depth of tensile zone in concrete section is less than 10 percent of section depth.

Bending Stiffness (EI) = Computed Bending Moment / Curvature.
 Position of neutral axis is measured from edge of compression side of pile.
 Compressive stresses and strains are positive in sign.
 Tensile stresses and strains are negative in sign.

Axial Thrust Force = 52.919 kips

Bending Concrete Curvature Stress rad/in. ksi	Max Steel	Bending Moment Stress in-kip ksi	Max Casing Run Stiffness Msg kip-in2	Depth to N Axis in	Max Comp Strain in/in	Max Tens Strain in/in	Max Stress ksi
0.000000313		3997.0397906	12790527330.	51.9214107	0.0000162	-0.0000119	
0.0587881		0.4664597	0.4664597				
0.000000625		7973.3866365	12757418618.	48.4726220	0.0000303	-0.0000260	
0.1092281		0.8704100	0.8704100				
0.000000938		11928.	12723650089.	47.3231126	0.0000444	-0.0000400	
0.1592321		1.2743628	1.2743628				
0.000001250		15862.	12689715062.	46.7484059	0.0000584	-0.0000541	
0.2088001		1.6783172	1.6783172				
0.000001563		19775.	12655713274.	46.4036177	0.0000725	-0.0000681	
0.2579320		2.0822733	2.0822733				
0.000001875		23666.	12621678090.	46.1737887	0.0000866	-0.0000822	
0.3066278		2.4862310	2.4862310				
0.000002188		27535.	12587623797.	46.0096501	0.0001006	-0.0000962	
0.3548876		2.8901903	2.8901903				
0.000002500		31384.	12553557560.	45.8865682	0.0001147	-0.0001103	
0.4027112		3.2941512	3.2941512				
0.000002813		31384.	11158717831.	23.8909754	0.0000672	-0.0001859	
0.2372638		-5.3553142	-5.3553142 C				
0.000003125		31384.	10042846048.	23.5916035	0.0000737	-0.0002075	
0.2597401		-5.9774797	-5.9774797 C				
0.000003438		31384.	9129860044.	23.3464127	0.0000803	-0.0002291	
0.2821215		-6.5996701	-6.5996701 C				
0.000003750		31384.	8369038373.	23.1431961	0.0000868	-0.0002507	
0.3044257		-7.2217399	-7.2217399 C				
0.000004063		31384.	7725266191.	22.9722708	0.0000933	-0.0002723	
0.3266525		-7.8436887	-7.8436887 C				

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0.000004375	31384.	7173461463.	22.8211620	0.0000998	-0.0002939
0.3487177	-8.4662213	-8.4662213	C		
0.000004688	31384.	6695230699.	22.6902522	0.0001064	-0.0003155
0.3706923	-9.0887470	-9.0887470	C		
0.000005000	31384.	6276778780.	22.5765718	0.0001129	-0.0003371
0.3925903	-9.7111471	-9.7111471	C		
0.000005313	31384.	5907556499.	22.4770831	0.0001194	-0.0003587
0.4144116	-10.3334212	-10.3334212	C		
0.000005625	31384.	5579358916.	22.3894234	0.0001259	-0.0003803
0.4361559	-10.9555690	-10.9555690	C		
0.000005938	31384.	5285708446.	22.3117277	0.0001325	-0.0004019
0.4578232	-11.5775900	-11.5775900	C		
0.000006250	31384.	5021423024.	22.2425036	0.0001390	-0.0004235
0.4794133	-12.1994837	-12.1994837	C		
0.000006563	31384.	4782307642.	22.1805432	0.0001456	-0.0004451
0.5009262	-12.8212497	-12.8212497	C		
0.000006875	31384.	4564930022.	22.1248584	0.0001521	-0.0004666
0.5223615	-13.4428876	-13.4428876	C		
0.000007188	31384.	4366454803.	22.0746326	0.0001587	-0.0004882
0.5437193	-14.0643969	-14.0643969	C		
0.000007500	31384.	4184519187.	22.0282048	0.0001652	-0.0005098
0.5649750	-14.6859904	-14.6859904	C		
0.000007813	31384.	4017138419.	21.9850180	0.0001718	-0.0005314
0.5861262	-15.3076912	-15.3076912	C		
0.000008125	31384.	3862633095.	21.9457257	0.0001783	-0.0005529
0.6072004	-15.9292571	-15.9292571	C		
0.000008438	31384.	3719572610.	21.9098973	0.0001849	-0.0005745
0.6281974	-16.5506876	-16.5506876	C		
0.000008750	31384.	3586730731.	21.8771635	0.0001914	-0.0005961
0.6491171	-17.1719822	-17.1719822	C		
0.000009063	31384.	3463050361.	21.8472062	0.0001980	-0.0006176
0.6699594	-17.7931405	-17.7931405	C		
0.000009375	31384.	3347615349.	21.8197495	0.0002046	-0.0006392
0.6907240	-18.4141618	-18.4141618	C		
0.000009688	31384.	3239627757.	21.7945534	0.0002111	-0.0006607
0.7114110	-19.0350458	-19.0350458	C		
0.0000100	31384.	3138389390.	21.7714076	0.0002177	-0.0006823
0.7320200	-19.6557918	-19.6557918	C		
0.0000103	31384.	3043286681.	21.7501275	0.0002243	-0.0007038
0.7525510	-20.2763994	-20.2763994	C		
0.0000106	31384.	2953778249.	21.7305501	0.0002309	-0.0007254
0.7730038	-20.8968683	-20.8968683	C		
0.0000109	31384.	2869384585.	21.7125313	0.0002375	-0.0007469
0.7933783	-21.5171974	-21.5171974	C		
0.0000113	31384.	2789679458.	21.6959427	0.0002441	-0.0007684
0.8136743	-22.1373864	-22.1373864	C		
0.0000116	31384.	2714282716.	21.6806699	0.0002507	-0.0007899
0.8338917	-22.7574349	-22.7574349	C		
0.0000119	31384.	2642854223.	21.6666107	0.0002573	-0.0008115
0.8540302	-23.3773423	-23.3773423	C		
0.0000122	31384.	2575088730.	21.6536732	0.0002639	-0.0008330
0.8740898	-23.9971081	-23.9971081	C		
0.0000128	31384.	2449474646.	21.6308410	0.0002771	-0.0008760
0.9139716	-25.2362125	-25.2362125	C		
0.0000134	32471.	2416430286.	21.6116034	0.0002904	-0.0009190
0.9535356	-26.4747439	-26.4747439	C		
0.0000141	33905.	2411002898.	21.5954922	0.0003037	-0.0009619
0.9927807	-27.7126981	-27.7126981	C		
0.0000147	35337.	2405934289.	21.5821188	0.0003170	-0.0010049
1.0317055	-28.9500696	-28.9500696	C		
0.0000153	36768.	2401179718.	21.5711584	0.0003303	-0.0010478
1.0703089	-30.1868545	-30.1868545	C		
0.0000159	38197.	2396701586.	21.5623373	0.0003436	-0.0010907
1.1085894	-31.4230480	-31.4230480	C		
0.0000166	39625.	2392467961.	21.5554230	0.0003570	-0.0011336
1.1465457	-32.6586453	-32.6586453	C		
0.0000172	41052.	2388451545.	21.5502172	0.0003704	-0.0011765
1.1841765	-33.8936418	-33.8936418	C		
0.0000178	42476.	2384628866.	21.5465494	0.0003838	-0.0012193
1.2214805	-35.1280324	-35.1280324	C		
0.0000184	43899.	2380979624.	21.5442726	0.0003972	-0.0012622
1.2584562	-36.3618123	-36.3618123	C		
0.0000191	45321.	2377486175.	21.5432589	0.0004107	-0.0013050
1.2951023	-37.5949765	-37.5949765	C		
0.0000197	46741.	2374133108.	21.5433971	0.0004241	-0.0013477
1.3314174	-38.8275198	-38.8275198	C		
0.0000203	48159.	2370906899.	21.5445894	0.0004376	-0.0013905
1.3674000	-40.0594371	-40.0594371	C		
0.0000209	49576.	2367795633.	21.5467503	0.0004511	-0.0014332
1.4030488	-41.2907232	-41.2907232	C		
0.0000216	50991.	2364788769.	21.5498039	0.0004647	-0.0014760
1.4383622	-42.5213726	-42.5213726	C		
0.0000222	52404.	2361876943.	21.5536833	0.0004782	-0.0015187
1.4733388	-43.7513800	-43.7513800	C		

Caisson Analysis.lp60

0.0000228	53816.	2359051815.	21.5583288	0.0004918	-0.0015613
1.5079771	-44.9807400	-44.9807400 C			
0.0000234	55226.	2356305922.	21.5636874	0.0005054	-0.0016040
1.5422755	-46.2094469	-46.2094469 C			
0.0000241	56634.	2353632572.	21.5697116	0.0005190	-0.0016466
1.5762326	-47.4374951	-47.4374951 C			
0.0000247	58041.	2351025742.	21.5763590	0.0005327	-0.0016892
1.6098469	-48.6648787	-48.6648787 C			
0.0000253	59446.	2348479999.	21.5835913	0.0005463	-0.0017318
1.6431166	-49.8915921	-49.8915921 C			
0.0000259	60849.	2345990422.	21.5913741	0.0005600	-0.0017743
1.6760403	-51.1176291	-51.1176291 C			
0.0000266	62251.	2343552550.	21.5996764	0.0005737	-0.0018169
1.7086164	-52.3429839	-52.3429839 C			
0.0000272	63650.	2341162322.	21.6084702	0.0005875	-0.0018594
1.7408432	-53.5676503	-53.5676503 C			
0.0000278	65048.	2338816038.	21.6177300	0.0006012	-0.0019019
1.7727190	-54.7916220	-54.7916220 C			
0.0000284	66445.	2336510311.	21.6274328	0.0006150	-0.0019443
1.8042422	-56.0148927	-56.0148927 C			
0.0000291	67839.	2334242043.	21.6375576	0.0006288	-0.0019868
1.8354112	-57.2374559	-57.2374559 C			
0.0000297	69231.	2332008388.	21.6480854	0.0006427	-0.0020292
1.8662241	-58.4593053	-58.4593053 C			
0.0000303	70622.	2329806725.	21.6589988	0.0006565	-0.0020716
1.8966792	-59.6804340	-59.6804340 C			
0.0000309	72011.	2327634759.	21.6702821	0.0006704	-0.0021140
1.9267749	-60.0000000	-60.0000000 CY			
0.0000316	73398.	2325490033.	21.6819206	0.0006843	-0.0021563
1.9565092	-60.0000000	-60.0000000 CY			
0.0000322	74780.	2323256174.	21.6935368	0.0006983	-0.0021986
1.9858554	-60.0000000	-60.0000000 CY			
0.0000328	75977.	2315493455.	21.6874543	0.0007116	-0.0022415
2.0135907	-60.0000000	-60.0000000 CY			
0.0000334	77037.	2303897688.	21.6687136	0.0007245	-0.0022848
2.0400466	-60.0000000	-60.0000000 CY			
0.0000341	77972.	2289090193.	21.6391303	0.0007371	-0.0023285
2.0653379	-60.0000000	-60.0000000 CY			
0.0000347	78889.	2274286113.	21.6094474	0.0007496	-0.0023723
2.0902097	-60.0000000	-60.0000000 CY			
0.0000353	79733.	2257914650.	21.5740018	0.0007618	-0.0024163
2.1142660	-60.0000000	-60.0000000 CY			
0.0000359	80456.	2238771404.	21.5283746	0.0007737	-0.0024607
2.1371922	-60.0000000	-60.0000000 CY			
0.0000366	81163.	2219836663.	21.4832042	0.0007855	-0.0025051
2.1597394	-60.0000000	-60.0000000 CY			
0.0000372	81869.	2201514327.	21.4399752	0.0007973	-0.0025496
2.1820182	-60.0000000	-60.0000000 CY			
0.0000397	84253.	2122915906.	21.2417021	0.0008430	-0.0027288
2.2653315	-60.0000000	-60.0000000 CY			
0.0000422	86316.	2046011261.	21.0370516	0.0008875	-0.0029094
2.3420047	-60.0000000	-60.0000000 CY			
0.0000447	87943.	1967964290.	20.8103251	0.0009300	-0.0030919
2.4111878	-60.0000000	-60.0000000 CY			
0.0000472	89375.	1894029432.	20.5926237	0.0009717	-0.0032752
2.4754425	-60.0000000	-60.0000000 CY			
0.0000497	90782.	1827069035.	20.3990473	0.0010136	-0.0034583
2.5361451	-60.0000000	-60.0000000 CY			
0.0000522	91816.	1759347519.	20.1837717	0.0010533	-0.0036435
2.5902702	-60.0000000	-60.0000000 CY			
0.0000547	92746.	1695933983.	19.9721972	0.0010922	-0.0038296
2.6399273	-60.0000000	-60.0000000 CY			
0.0000572	93671.	1637960138.	19.7821046	0.0011313	-0.0040156
2.6865805	-60.0000000	-60.0000000 CY			
0.0000597	94585.	1584675272.	19.6103686	0.0011705	-0.0042014
2.7301581	-60.0000000	-60.0000000 CY			
0.0000622	95326.	1532881471.	19.4348539	0.0012086	-0.0043883
2.7693629	-60.0000000	-60.0000000 CY			
0.0000647	95897.	1482465435.	19.2493915	0.0012452	-0.0045767
2.8040666	-60.0000000	-60.0000000 CY			
0.0000672	96456.	1435618673.	19.0737049	0.0012815	-0.0047654
2.8357099	-60.0000000	-60.0000000 CY			
0.0000697	97010.	1392070188.	18.9128591	0.0013180	-0.0049539
2.8646898	-60.0000000	-60.0000000 CY			
0.0000722	97560.	1351476112.	18.7653505	0.0013546	-0.0051423
2.8909709	-60.0000000	-60.0000000 CY			
0.0000747	98105.	1313538503.	18.6298785	0.0013914	-0.0053305
2.9145171	-60.0000000	-60.0000000 CY			
0.0000772	98630.	1277800752.	18.5031893	0.0014282	-0.0055187
2.9352028	-60.0000000	-60.0000000 CY			
0.0000797	99028.	1242702068.	18.3635763	0.0014633	-0.0057085
2.9522599	-60.0000000	-60.0000000 CY			
0.0000822	99335.	1208640454.	18.2178357	0.0014973	-0.0058996
2.9662502	-60.0000000	-60.0000000 CY			

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0.0000847	99639.	1176552004.	18.0824171	0.0015314	-0.0060905
2.9778666	-60.0000000	-60.0000000 CY			
0.0000872	99940.	1146266194.	17.9564605	0.0015656	-0.0062813
2.9870772	-60.0000000	-60.0000000 CY			
0.0000897	100238.	1117631493.	17.8392031	0.0016000	-0.0064719
2.9938497	-60.0000000	-60.0000000 CY			
0.0000922	100532.	1090512783.	17.7299661	0.0016345	-0.0066624
2.9981504	-60.0000000	-60.0000000 CY			
0.0000947	100822.	1064789194.	17.6281435	0.0016692	-0.0068527
2.9999449	-60.0000000	-60.0000000 CY			
0.0000972	101107.	1040329749.	17.5317238	0.0017039	-0.0070430
2.9958309	-60.0000000	-60.0000000 CY			
0.0000997	101382.	1016995842.	17.4342973	0.0017380	-0.0072339
2.9989777	-60.0000000	-60.0000000 CY			
0.0001022	101653.	994773958.	17.3431409	0.0017723	-0.0074246
2.9998462	-60.0000000	-60.0000000 CY			
0.0001047	101910.	973472894.	17.2564043	0.0018065	-0.0076153
2.9955979	-60.0000000	-60.0000000 CY			
0.0001072	102158.	953073021.	17.1737208	0.0018408	-0.0078061
2.9987063	-60.0000000	-60.0000000 CY			
0.0001097	102316.	932797289.	17.0807305	0.0018735	-0.0079983
2.9999478	-60.0000000	-60.0000000 CY			
0.0001122	102465.	913333994.	16.9921092	0.0019063	-0.0081906
2.9957789	-60.0000000	-60.0000000 CY			
0.0001147	102602.	894621646.	16.9067727	0.0019390	-0.0083829
2.9964960	-60.0000000	-60.0000000 CY			
0.0001172	102738.	876694720.	16.8260025	0.0019718	-0.0085751
2.9989264	-60.0000000	-60.0000000 CY			
0.0001197	102872.	859503741.	16.7495257	0.0020047	-0.0087672
2.9999613	-60.0000000	-60.0000000 CY			
0.0001222	103003.	842994476.	16.6775044	0.0020378	-0.0089591
2.9959842	-60.0000000	-60.0000000 CY			
0.0001247	103129.	827099027.	16.6042296	0.0020703	-0.0091515
2.9947705	-60.0000000	-60.0000000 CY			
0.0001272	103250.	811797109.	16.5313078	0.0021026	-0.0093443
2.9976116	-60.0000000	-60.0000000 CY			
0.0001297	103371.	797077043.	16.4619086	0.0021349	-0.0095370
2.9993584	60.0000000	60.0000000 CY			
0.0001322	103490.	782905670.	16.3958422	0.0021673	-0.0097295
2.9999971	60.0000000	60.0000000 CY			
0.0001347	103608.	769244266.	16.3334208	0.0021999	-0.0099220
2.9956661	60.0000000	60.0000000 CY			
0.0001372	103724.	756074240.	16.2738836	0.0022326	-0.0101143
2.9927936	60.0000000	60.0000000 CY			
0.0001522	104402.	686011175.	15.9698031	0.0024304	-0.0112665
2.9909608	60.0000000	60.0000000 CY			
0.0001672	104974.	627883540.	15.7172433	0.0026277	-0.0124191
2.9966514	60.0000000	60.0000000 CY			
0.0001822	105266.	577789423.	15.4367926	0.0028124	-0.0135845
2.9970835	60.0000000	60.0000000 CY			
0.0001972	105460.	534821061.	15.2332204	0.0030038	-0.0147431
2.9918521	60.0000000	60.0000000 CY			
0.0002122	105628.	497804656.	15.0745855	0.0031986	-0.0158982
2.9961980	60.0000000	60.0000000 CY			
0.0002272	105778.	465599591.	14.9482587	0.0033961	-0.0170508
2.9949080	60.0000000	60.0000000 CY			
0.0002422	105864.	437113978.	14.8714082	0.0036017	-0.0181952
2.9881673	60.0000000	60.0000000 CY			
0.0002572	105938.	411909117.	14.8000935	0.0038064	-0.0193405
2.9992334	60.0000000	60.0000000 CY			

Axial Thrust Force = 64.248 kips

Bending Concrete Curvature Stress rad/in. ksi	Bending Max Steel Moment Stress in-kip ksi	Bending Max Casing Run Stiffness Msg kip-in2	Depth to N Axis in	Max Comp Strain in/in	Max Tens Strain in/in	Max Stress ksi
0.00000313	3996.7444896	12789582367.	53.4033265	0.0000167	-0.0000114	
0.0604720	0.4798895	0.4798895				
0.000000625	7973.0859893	12756937583.	49.2160926	0.0000308	-0.0000255	
0.1109032	0.8838854	0.8838854				
0.000000938	11928.	12723326520.	47.8204713	0.0000448	-0.0000395	
0.1608984	1.2878847	1.2878847				
0.000001250	15862.	12689470515.	47.1227224	0.0000589	-0.0000536	
0.2104575	1.6918862	1.6918862				
0.000001563	19774.	12655516198.	46.7041175	0.0000730	-0.0000676	
0.2595805	2.0958897	2.0958897				
0.000001875	23665.	12621512662.	46.4250838	0.0000870	-0.0000817	
0.3082674	2.4998952	2.4998952				

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0.000002188	27535.	12587480993.	46.2258053	0.0001011	-0.0000958
0.3565181	2.9039027	2.9039027			
0.000002500	31384.	12553431717.	46.0763732	0.0001152	-0.0001098
0.4043327	3.3079120	3.3079120			
0.000002813	31384.	11158605970.	24.5298965	0.0000690	-0.0001841
0.2435940	-5.3032022	-5.3032022 C			
0.000003125	31384.	10042745373.	24.1779896	0.0000756	-0.0002057
0.2661689	-5.9243384	-5.9243384 C			
0.000003438	31384.	9129768521.	23.8912528	0.0000821	-0.0002272
0.2886652	-6.5453564	-6.5453564 C			
0.000003750	31384.	8368954478.	23.6448187	0.0000887	-0.0002488
0.3109710	-7.1671885	-7.1671885 C			
0.000004063	31384.	7725188749.	23.4358058	0.0000952	-0.0002704
0.3331777	-7.7890785	-7.7890785 C			
0.000004375	31384.	7173389552.	23.2576106	0.0001018	-0.0002920
0.3553068	-8.4108469	-8.4108469 C			
0.000004688	31384.	6695163582.	23.1040724	0.0001083	-0.0003136
0.3773582	-9.0324933	-9.0324933 C			
0.000005000	31384.	6276715858.	22.9705709	0.0001149	-0.0003351
0.3993317	-9.6540172	-9.6540172 C			
0.000005313	31384.	5907497278.	22.8492062	0.0001214	-0.0003567
0.4211481	-10.2760910	-10.2760910 C			
0.000005625	31384.	5579302985.	22.7412760	0.0001279	-0.0003783
0.4428717	-10.8981731	-10.8981731 C			
0.000005938	31384.	5285655460.	22.6454449	0.0001345	-0.0003999
0.4645183	-11.5201281	-11.5201281 C			
0.000006250	31384.	5021372687.	22.5599006	0.0001410	-0.0004215
0.4860876	-12.1419555	-12.1419555 C			
0.000006563	31384.	4782259702.	22.4831758	0.0001475	-0.0004431
0.5075795	-12.7636550	-12.7636550 C			
0.000006875	31384.	4564884261.	22.4140700	0.0001541	-0.0004647
0.5289940	-13.3852260	-13.3852260 C			
0.000007188	31384.	4366411032.	22.3515917	0.0001607	-0.0004862
0.5503308	-14.0066682	-14.0066682 C			
0.000007500	31384.	4184477239.	22.2949145	0.0001672	-0.0005078
0.5715898	-14.6279811	-14.6279811 C			
0.000007813	31384.	4017098149.	22.2433443	0.0001738	-0.0005293
0.5927709	-15.2491642	-15.2491642 C			
0.000008125	31384.	3862594374.	22.1962936	0.0001803	-0.0005509
0.6138739	-15.8702170	-15.8702170 C			
0.000008438	31384.	3719535323.	22.1532624	0.0001869	-0.0005725
0.6348987	-16.4911392	-16.4911392 C			
0.000008750	31384.	3586694776.	22.1138216	0.0001935	-0.0005940
0.6558451	-17.1119302	-17.1119302 C			
0.000009063	31384.	3463015646.	22.0776016	0.0002001	-0.0006155
0.6767131	-17.7325897	-17.7325897 C			
0.000009375	31384.	3347581791.	22.0440105	0.0002067	-0.0006371
0.6974942	-18.3531909	-18.3531909 C			
0.000009688	31384.	3239595282.	22.0118516	0.0002132	-0.0006586
0.7181592	-18.9739985	-18.9739985 C			
0.0000100	31384.	3138357929.	21.9821794	0.0002198	-0.0006802
0.7387462	-19.5946679	-19.5946679 C			
0.0000103	31384.	3043256174.	21.9547695	0.0002264	-0.0007017
0.7592551	-20.2151986	-20.2151986 C			
0.0000106	31384.	2953748639.	21.9294241	0.0002330	-0.0007232
0.7796857	-20.8355899	-20.8355899 C			
0.0000109	31384.	2869355821.	21.9059678	0.0002396	-0.0007448
0.8000380	-21.4558414	-21.4558414 C			
0.0000113	31384.	2789651493.	21.8842449	0.0002462	-0.0007663
0.8203116	-22.0759526	-22.0759526 C			
0.0000116	31384.	2714255506.	21.8641164	0.0002528	-0.0007878
0.8405066	-22.6959228	-22.6959228 C			
0.0000119	31384.	2642827730.	21.8454581	0.0002594	-0.0008093
0.8606227	-23.3157516	-23.3157516 C			
0.0000122	31384.	2575062916.	21.8281582	0.0002660	-0.0008308
0.8806598	-23.9354387	-23.9354387 C			
0.0000128	31384.	2449450091.	21.7972426	0.0002793	-0.0008738
0.9204964	-25.1743839	-25.1743839 C			
0.0000134	32740.	2436464114.	21.7706772	0.0002925	-0.0009168
0.9600150	-26.4127548	-26.4127548 C			
0.0000141	34174.	2430120642.	21.7478930	0.0003058	-0.0009598
0.9992143	-27.6505467	-27.6505467 C			
0.0000147	35606.	2424213582.	21.7284181	0.0003191	-0.0010027
1.0380932	-28.8877550	-28.8877550 C			
0.0000153	37036.	2418688821.	21.7118576	0.0003325	-0.0010457
1.0766502	-30.1243751	-30.1243751 C			
0.0000159	38465.	2413500630.	21.6978788	0.0003458	-0.0010886
1.1148841	-31.3604026	-31.3604026 C			
0.0000166	39893.	2408610472.	21.6861993	0.0003592	-0.0011314
1.1527936	-32.5958320	-32.5958320 C			
0.0000172	41318.	2403985095.	21.6765780	0.0003726	-0.0011743
1.1903773	-33.8306589	-33.8306589 C			
0.0000178	42743.	2399596014.	21.6688077	0.0003860	-0.0012171
1.2276338	-35.0648785	-35.0648785 C			

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0.0000184	44166.	2395418601.	21.6627094	0.0003994	-0.0012600
1.2645618	-36.2984857	-36.2984857 C			
0.0000191	45587.	2391431449.	21.6581278	0.0004129	-0.0013028
1.3011599	-37.5314756	-37.5314756 C			
0.0000197	47006.	2387615861.	21.6549274	0.0004263	-0.0013455
1.3374267	-38.7638430	-38.7638430 C			
0.0000203	48424.	2383955433.	21.6529896	0.0004398	-0.0013883
1.3733607	-39.9955827	-39.9955827 C			
0.0000209	49840.	2380435712.	21.6522099	0.0004533	-0.0014310
1.4089605	-41.2266894	-41.2266894 C			
0.0000216	51255.	2377043915.	21.6524962	0.0004669	-0.0014737
1.4442246	-42.4571578	-42.4571578 C			
0.0000222	52668.	2373768690.	21.6537669	0.0004804	-0.0015164
1.4791516	-43.6869824	-43.6869824 C			
0.0000228	54079.	2370599921.	21.6559494	0.0004940	-0.0015591
1.5137399	-44.9161578	-44.9161578 C			
0.0000234	55489.	2367528565.	21.6589790	0.0005076	-0.0016017
1.5479881	-46.1446783	-46.1446783 C			
0.0000241	56897.	2364546510.	21.6627979	0.0005213	-0.0016444
1.5818946	-47.3725382	-47.3725382 C			
0.0000247	58303.	2361646459.	21.6673542	0.0005349	-0.0016870
1.6154579	-48.5997318	-48.5997318 C			
0.0000253	59708.	2358821828.	21.6726012	0.0005486	-0.0017295
1.6486764	-49.8262531	-49.8262531 C			
0.0000259	61110.	2356066660.	21.6784970	0.0005623	-0.0017721
1.6815485	-51.0520963	-51.0520963 C			
0.0000266	62512.	2353375551.	21.6850036	0.0005760	-0.0018146
1.7140725	-52.2772552	-52.2772552 C			
0.0000272	63911.	2350743587.	21.6920868	0.0005898	-0.0018571
1.7462469	-53.5017236	-53.5017236 C			
0.0000278	65308.	2348166289.	21.6997154	0.0006035	-0.0018996
1.7780700	-54.7254954	-54.7254954 C			
0.0000284	66704.	2345639563.	21.7078612	0.0006173	-0.0019421
1.8095401	-55.9485641	-55.9485641 C			
0.0000291	68098.	2343159661.	21.7164985	0.0006311	-0.0019845
1.8406555	-57.1709234	-57.1709234 C			
0.0000297	69490.	2340723143.	21.7256039	0.0006450	-0.0020269
1.8714145	-58.3925665	-58.3925665 C			
0.0000303	70881.	2338326845.	21.7351560	0.0006588	-0.0020693
1.9018153	-59.6134868	-59.6134868 C			
0.0000309	72269.	2335967851.	21.7451354	0.0006727	-0.0021116
1.9318562	-60.0000000	-60.0000000 CY			
0.0000316	73656.	2333643470.	21.7555242	0.0006867	-0.0021540
1.9615354	-60.0000000	-60.0000000 CY			
0.0000322	75037.	2331246262.	21.7659712	0.0007006	-0.0021963
1.9908281	-60.0000000	-60.0000000 CY			
0.0000328	76248.	2323737767.	21.7600823	0.0007140	-0.0022391
2.0186035	-60.0000000	-60.0000000 CY			
0.0000334	77317.	2312276489.	21.7411832	0.0007270	-0.0022824
2.0450747	-60.0000000	-60.0000000 CY			
0.0000341	78255.	2297404505.	21.7107899	0.0007395	-0.0023261
2.0703354	-60.0000000	-60.0000000 CY			
0.0000347	79173.	2282477784.	21.6801244	0.0007520	-0.0023698
2.0951618	-60.0000000	-60.0000000 CY			
0.0000353	80027.	2266243314.	21.6446386	0.0007643	-0.0024138
2.1192372	-60.0000000	-60.0000000 CY			
0.0000359	80752.	2247006216.	21.5981522	0.0007762	-0.0024582
2.1421246	-60.0000000	-60.0000000 CY			
0.0000366	81458.	2227922923.	21.5519406	0.0007880	-0.0025026
2.1646175	-60.0000000	-60.0000000 CY			
0.0000372	82164.	2209456979.	21.5077067	0.0007998	-0.0025471
2.1868418	-60.0000000	-60.0000000 CY			
0.0000397	84561.	2130664263.	21.3070039	0.0008456	-0.0027263
2.2700346	-60.0000000	-60.0000000 CY			
0.0000422	86625.	2053328709.	21.1026543	0.0008903	-0.0029066
2.3467569	-60.0000000	-60.0000000 CY			
0.0000447	88264.	1975147158.	20.8741212	0.0009328	-0.0030891
2.4158176	-60.0000000	-60.0000000 CY			
0.0000472	89695.	1900813375.	20.6534574	0.0009746	-0.0032723
2.4798421	-60.0000000	-60.0000000 CY			
0.0000497	91103.	1833528010.	20.4574169	0.0010165	-0.0034554
2.5403244	-60.0000000	-60.0000000 CY			
0.0000522	92150.	1765745884.	20.2447368	0.0010565	-0.0036404
2.5945775	-60.0000000	-60.0000000 CY			
0.0000547	93080.	1702025800.	20.0307370	0.0010954	-0.0038264
2.6439895	-60.0000000	-60.0000000 CY			
0.0000572	94003.	1643771972.	19.8384418	0.0011345	-0.0040124
2.6903937	-60.0000000	-60.0000000 CY			
0.0000597	94918.	1590250447.	19.6648462	0.0011737	-0.0041981
2.7337281	-60.0000000	-60.0000000 CY			
0.0000622	95667.	1538358532.	19.4885029	0.0012119	-0.0043849
2.7727480	-60.0000000	-60.0000000 CY			
0.0000647	96243.	1487806912.	19.3055115	0.0012488	-0.0045730
2.8074594	-60.0000000	-60.0000000 CY			

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0.0000672	96800.	1440750883.	19.1280481	0.0012852	-0.0047617	
2.8388333	-60.0000000	-60.0000000	CY			
0.0000697	97354.	1397008043.	18.9655621	0.0013217	-0.0049502	
2.8675395	-60.0000000	-60.0000000	CY			
0.0000722	97903.	1356232896.	18.8165363	0.0013583	-0.0051386	
2.8935428	-60.0000000	-60.0000000	CY			
0.0000747	98448.	1318126156.	18.6796580	0.0013951	-0.0053267	
2.9168067	-60.0000000	-60.0000000	CY			
0.0000772	98977.	1282298237.	18.5523928	0.0014320	-0.0055149	
2.9372366	-60.0000000	-60.0000000	CY			
0.0000797	99384.	1247173503.	18.4166035	0.0014676	-0.0057043	
2.9541977	-60.0000000	-60.0000000	CY			
0.0000822	99691.	1212967915.	18.2695301	0.0015015	-0.0058953	
2.9678851	-60.0000000	-60.0000000	CY			
0.0000847	99994.	1180743851.	18.1328657	0.0015356	-0.0060862	
2.9791939	-60.0000000	-60.0000000	CY			
0.0000872	100294.	1150330073.	18.0057435	0.0015699	-0.0062770	
2.9880923	-60.0000000	-60.0000000	CY			
0.0000897	100591.	1121574406.	17.8873942	0.0016043	-0.0064676	
2.9945475	-60.0000000	-60.0000000	CY			
0.0000922	100885.	1094341156.	17.7771333	0.0016388	-0.0066580	
2.9985257	-60.0000000	-60.0000000	CY			
0.0000947	101174.	1068508939.	17.6743499	0.0016735	-0.0068483	
2.9999922	-60.0000000	-60.0000000	CY			
0.0000972	101460.	1043960774.	17.5787291	0.0017084	-0.0070384	
2.9964329	-60.0000000	-60.0000000	CY			
0.0000997	101737.	1020554791.	17.4832787	0.0017429	-0.0072290	
2.9992820	-60.0000000	-60.0000000	CY			
0.0001022	102007.	998236643.	17.3913340	0.0017772	-0.0074197	
2.9988232	-60.0000000	-60.0000000	CY			
0.0001047	102265.	976861179.	17.3040100	0.0018115	-0.0076104	
2.9962712	-60.0000000	-60.0000000	CY			
0.0001072	102512.	956376399.	17.2205137	0.0018458	-0.0078011	
2.9990606	-60.0000000	-60.0000000	CY			
0.0001097	102678.	936093132.	17.1281349	0.0018787	-0.0079931	
2.9999971	-60.0000000	-60.0000000	CY			
0.0001122	102826.	916559315.	17.0390009	0.0019116	-0.0081853	
2.9946862	-60.0000000	-60.0000000	CY			
0.0001147	102963.	897772392.	16.9528575	0.0019443	-0.0083776	
2.9971280	-60.0000000	-60.0000000	CY			
0.0001172	103099.	879774012.	16.8713209	0.0019771	-0.0085698	
2.9992637	-60.0000000	-60.0000000	CY			
0.0001197	103232.	862514505.	16.7941157	0.0020100	-0.0087618	
2.9999997	-60.0000000	-60.0000000	CY			
0.0001222	103363.	845937360.	16.7215019	0.0020432	-0.0089537	
2.9948675	-60.0000000	-60.0000000	CY			
0.0001247	103492.	830015021.	16.6525711	0.0020764	-0.0091455	
2.9956540	-60.0000000	-60.0000000	CY			
0.0001272	103614.	814654702.	16.5792420	0.0021087	-0.0093382	
2.9982016	-60.0000000	-60.0000000	CY			
0.0001297	103734.	799876291.	16.5091503	0.0021410	-0.0095308	
2.9996453	60.0000000	60.0000000	CY			
0.0001322	103853.	785646764.	16.4425433	0.0021735	-0.0097234	
2.9990485	60.0000000	60.0000000	CY			
0.0001347	103969.	771931171.	16.3794964	0.0022061	-0.0099158	
2.9943771	60.0000000	60.0000000	CY			
0.0001372	104085.	758709582.	16.3193195	0.0022388	-0.0101081	
2.9938720	60.0000000	60.0000000	CY			
0.0001522	104761.	688370686.	16.0120295	0.0024368	-0.0112600	
2.9914859	60.0000000	60.0000000	CY			
0.0001672	105343.	630087744.	15.7631929	0.0026354	-0.0124115	
2.9950557	60.0000000	60.0000000	CY			
0.0001822	105632.	579798484.	15.4842680	0.0028210	-0.0135758	
2.9979900	60.0000000	60.0000000	CY			
0.0001972	105824.	536668812.	15.2782554	0.0030127	-0.0147342	
2.9900075	60.0000000	60.0000000	CY			
0.0002122	105989.	499507643.	15.1181708	0.0032079	-0.0158890	
2.9973111	60.0000000	60.0000000	CY			
0.0002272	106134.	467166126.	14.9920716	0.0034060	-0.0170409	
2.9928405	60.0000000	60.0000000	CY			
0.0002422	106217.	438574683.	14.9139964	0.0036120	-0.0181849	
2.9904270	60.0000000	60.0000000	CY			
0.0002572	106291.	413282219.	14.8522929	0.0038198	-0.0193271	
2.9998190	60.0000000	60.0000000	CY			

 Summary of Results for Nominal (Unfactored) Moment Capacity for Section 1

Moment values interpolated at maximum compressive strain = 0.003
 or maximum developed moment if pile fails at smaller strains.

Load	Axial Thrust	Nominal Mom. Cap.	Max. Comp.
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No.	kips	Caisson Analysis, 1p60 in-kip	Strain
1	52.919	105456.176	0.00300000
2	64.248	105811.654	0.00300000

Note note that the values of moment capacity in the table above are not factored by a strength reduction factor (ϕ -factor).

In ACI 318-08, the value of the strength reduction factor depends on whether the transverse reinforcing steel bars are spirals or tied hoops.

The above values should be multiplied by the appropriate strength reduction factor to compute ultimate moment capacity according to ACI 318-08, Section 9.3.2.2 or the value required by the design standard being followed.

 Computed Values of Pile Loading and Deflection
 for Lateral Loading for Load Case Number 1

Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 31636.000 lbs
 Applied moment at pile head = 47587518.000 in-lbs
 Axial thrust load on pile head = 64248.000 lbs

Depth Distrib. X Lat. Load inches lb/inch	Deflect. y inches	Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness lb-in ²	Soil Res. p lb/in	Soil Spr. Es*h lb/inch
0.000	0.000	47587518.	31636.	-0.006602	0.000	2.386E+12	0.000	
4.680	0.000	47737546.	31636.	-0.006508	0.000	2.386E+12	0.000	
9.360	0.000	47887545.	31636.	-0.006415	0.000	2.386E+12	0.000	
14.040	0.000	48037516.	31636.	-0.006321	0.000	2.385E+12	0.000	
18.720	0.000	48187459.	31636.	-0.006226	0.000	2.385E+12	0.000	
23.400	0.000	48337373.	31636.	-0.006131	0.000	2.384E+12	0.000	
28.080	0.000	48487259.	31636.	-0.006036	0.000	2.384E+12	0.000	
32.760	0.000	48637116.	31636.	-0.005941	0.000	2.383E+12	0.000	
37.440	0.000	48786945.	31636.	-0.005845	0.000	2.383E+12	0.000	
42.120	0.000	48936744.	31636.	-0.005749	0.000	2.383E+12	0.000	
46.800	0.000	49086515.	31636.	-0.005653	0.000	2.382E+12	0.000	
51.480	0.000	49236257.	31630.	-0.005557	0.000	2.382E+12	-2.6694	
56.160	0.000	49385911.	31609.	-0.005460	0.000	2.382E+12	-6.0490	
60.840	0.000	49535404.	31137.	-0.005362	0.000	2.381E+12	-195.9843	
65.520	0.000	49680574.	30041.	-0.005265	0.000	2.381E+12	-272.1646	
70.200	0.000	49819755.	28607.	-0.005167	0.000	2.380E+12	-340.5544	
74.880	0.000	49951447.	26866.	-0.005069	0.000	2.380E+12	-403.5196	
79.560	0.000	50074271.	24843.	-0.004971	0.000	2.380E+12	-461.2223	
84.240	0.000	50186964.	22561.	-0.004872	0.000	2.380E+12	-513.8246	
88.920	0.000	50288373.	20045.	-0.004773	0.000	2.379E+12	-561.4895	
93.600	0.000	50377454.	17317.	-0.004674	0.000	2.379E+12	-604.3802	
98.280	0.000	50453269.	14399.	-0.004575	0.000	2.379E+12	-642.6601	
102.960	0.000	50514977.	11312.	-0.004476	0.000	2.379E+12	-676.4931	
107.640	0.000	50561839.	8076.7275	-0.004376	0.000	2.379E+12	-706.0430	

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6771.0730	0.000						
112.320	0.4678	50593207.	4712.9379	-0.004277	0.000	2.379E+12	-731.4739
7318.6330	0.000						
117.000	0.4480	50608524.	1239.3868	-0.004177	0.000	2.379E+12	-752.9496
7866.1930	0.000						
121.680	0.4287	50607320.	-2325.7987	-0.004078	0.000	2.379E+12	-770.6339
8413.7530	0.000						
126.360	0.4098	50589207.	-5965.2578	-0.003978	0.000	2.379E+12	-784.6905
8961.3130	0.000						
131.040	0.3914	50553878.	-9662.3942	-0.003879	0.000	2.379E+12	-795.2824
9508.8730	0.000						
135.720	0.3735	50501100.	-14008.	-0.003779	0.000	2.379E+12	-1061.7359
13304.	0.000						
140.400	0.3560	50425037.	-18997.	-0.003680	0.000	2.379E+12	-1070.4377
14070.	0.000						
145.080	0.3391	50325500.	-24017.	-0.003581	0.000	2.379E+12	-1074.8931
14837.	0.000						
149.760	0.3225	50202390.	-29049.	-0.003482	0.000	2.380E+12	-1075.3272
15604.	0.000						
154.440	0.3065	50055698.	-34073.	-0.003383	0.000	2.380E+12	-1071.9633
16370.	0.000						
159.120	0.2909	49885498.	-39074.	-0.003285	0.000	2.380E+12	-1065.0234
17137.	0.000						
163.800	0.2757	49691942.	-44034.	-0.003187	0.000	2.381E+12	-1054.7275
17903.	0.000						
168.480	0.2610	49475255.	-48939.	-0.003090	0.000	2.381E+12	-1041.2937
18670.	0.000						
173.160	0.2468	49235733.	-53774.	-0.002993	0.000	2.382E+12	-1024.9380
19436.	0.000						
177.840	0.2330	48973732.	-58526.	-0.002896	0.000	2.383E+12	-1005.8739
20203.	0.000						
182.520	0.2197	48689672.	-63183.	-0.002801	0.000	2.383E+12	-984.3127
20970.	0.000						
187.200	0.2068	48384024.	-67734.	-0.002705	0.000	2.384E+12	-960.4628
21736.	0.000						
191.880	0.1944	48057311.	-72168.	-0.002611	0.000	2.385E+12	-934.5298
22503.	0.000						
196.560	0.1824	47710102.	-79951.	-0.002517	0.000	2.386E+12	-2391.5745
61376.	0.000						
201.240	0.1708	47310483.	-91203.	-0.002423	0.000	2.387E+12	-2416.9400
66224.	0.000						
205.920	0.1597	46857899.	-102574.	-0.002331	0.000	2.388E+12	-2442.5471
71589.	0.000						
210.600	0.1490	46351790.	-114050.	-0.002240	0.000	2.389E+12	-2461.4999
77323.	0.000						
215.280	0.1387	45791741.	-125598.	-0.002150	0.000	2.391E+12	-2473.6418
83458.	0.000						
219.960	0.1289	45177487.	-136997.	-0.002061	0.000	2.393E+12	-2397.5830
87075.	0.000						
224.640	0.1194	44510693.	-147963.	-0.001973	0.000	2.394E+12	-2289.0718
89703.	0.000						
229.320	0.1104	43793736.	-158416.	-0.001887	0.000	2.396E+12	-2177.9936
92332.	0.000						
234.000	0.1018	43029051.	-168345.	-0.001802	0.000	2.399E+12	-2064.8946
94960.	0.000						
238.680	0.0935	42219114.	-177740.	-0.001719	0.000	2.401E+12	-1950.2955
97588.	0.000						
243.360	0.0857	41366437.	-186597.	-0.001637	0.000	2.404E+12	-1834.6915
100216.	0.000						
248.040	0.0782	40473551.	-194912.	-0.001558	0.000	2.407E+12	-1718.5515
102845.	0.000						
252.720	0.0711	39543001.	-202682.	-0.001480	0.000	2.410E+12	-1602.3171
105473.	0.000						
257.400	0.0644	38577333.	-209910.	-0.001404	0.000	2.413E+12	-1486.4028
108101.	0.000						
262.080	0.0580	37579087.	-216597.	-0.001330	0.000	2.417E+12	-1371.1949
110730.	0.000						
266.760	0.0519	36550787.	-222747.	-0.001259	0.000	2.421E+12	-1257.0517
113358.	0.000						
271.440	0.0462	35494932.	-228366.	-0.001189	0.000	2.425E+12	-1144.3030
115986.	0.000						
276.120	0.0408	34413995.	-233462.	-0.001122	0.000	2.429E+12	-1033.2499
118614.	0.000						
280.800	0.0357	33310407.	-238042.	-0.001057	0.000	2.434E+12	-924.1651
121243.	0.000						
285.480	0.0309	32186558.	-242117.	-0.000994	0.000	2.442E+12	-817.2923
123871.	0.000						
290.160	0.0264	31044790.	-245697.	-0.000957	0.000	1.222E+13	-712.8364
126499.	0.000						
294.840	0.0219	29887405.	-248781.	-0.000945	0.000	1.257E+13	-604.8544
129128.	0.000						
299.520	0.0175	28716769.	-251351.	-0.000934	0.000	1.258E+13	-493.3404
131756.	0.000						
304.200	0.0132	27535325.	-253390.	-0.000924	0.000	1.259E+13	-378.3223

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134384.	0.000						
308.880	0.008875	26345592.	-254884.	-0.000914	0.000	1.260E+13	-259.8229
137012.	0.000						
313.560	0.004620	25150165.	-255814.	-0.000904	0.000	1.261E+13	-137.8601
139641.	0.000						
318.240	0.000409	23951715.	-256166.	-0.000895	0.000	1.262E+13	-12.4468
142269.	0.000						
322.920	-0.003760	22752991.	-255923.	-0.000887	0.000	1.263E+13	116.4090
144897.	0.000						
327.600	-0.007890	21556813.	-255068.	-0.000878	0.000	1.264E+13	248.7038
147526.	0.000						
332.280	-0.0120	20366080.	-253587.	-0.000871	0.000	1.265E+13	384.4387
150154.	0.000						
336.960	-0.0160	19183765.	-251462.	-0.000863	0.000	1.266E+13	523.6190
152782.	0.000						
341.640	-0.0201	18012917.	-248678.	-0.000857	0.000	1.267E+13	666.2543
155410.	0.000						
346.320	-0.0241	16856659.	-245218.	-0.000850	0.000	1.268E+13	812.3578
158039.	0.000						
351.000	-0.0280	15718192.	-241066.	-0.000844	0.000	1.269E+13	961.9464
160667.	0.000						
355.680	-0.0320	14600792.	-236206.	-0.000838	0.000	1.270E+13	1115.0406
163295.	0.000						
360.360	-0.0359	13507812.	-230621.	-0.000833	0.000	1.271E+13	1271.6635
165924.	0.000						
365.040	-0.0398	12442683.	-224294.	-0.000829	0.000	1.272E+13	1431.8413
168552.	0.000						
369.720	-0.0436	11408914.	-217210.	-0.000824	0.000	1.273E+13	1595.6026
171180.	0.000						
374.400	-0.0475	10410091.	-209351.	-0.000820	0.000	1.273E+13	1762.9777
173808.	0.000						
379.080	-0.0513	9449880.	-200700.	-0.000816	0.000	1.274E+13	1933.9988
176437.	0.000						
383.760	-0.0551	8532028.	-191488.	-0.000813	0.000	1.275E+13	2002.8361
170075.	0.000						
388.440	-0.0589	7658041.	-182051.	-0.000810	0.000	1.276E+13	2030.1632
161281.	0.000						
393.120	-0.0627	6828519.	-172496.	-0.000808	0.000	1.276E+13	2052.8822
153239.	0.000						
397.800	-0.0665	6043960.	-162846.	-0.000805	0.000	1.277E+13	2071.1754
145828.	0.000						
402.480	-0.0702	5304763.	-153120.	-0.000803	0.000	1.277E+13	2085.1935
138949.	0.000						
407.160	-0.0740	4611237.	-143338.	-0.000801	0.000	1.278E+13	2095.0640
132523.	0.000						
411.840	-0.0777	3963597.	-133520.	-0.000800	0.000	1.279E+13	2100.8957
126488.	0.000						
416.520	-0.0815	3361971.	-123683.	-0.000798	0.000	1.279E+13	2102.7841
120791.	0.000						
421.200	-0.0852	2806401.	-113847.	-0.000797	0.000	1.279E+13	2100.8139
115390.	0.000						
425.880	-0.0889	2296843.	-103988.	-0.000796	0.000	1.279E+13	2112.6007
111172.	0.000						
430.560	-0.0927	1833556.	-93918.	-0.000796	0.000	1.279E+13	2190.4474
110635.	0.000						
435.240	-0.0964	1418246.	-83484.	-0.000795	0.000	1.279E+13	2268.7937
110167.	0.000						
439.920	-0.1001	1052626.	-72681.	-0.000795	0.000	1.279E+13	2347.6575
109761.	0.000						
444.600	-0.1038	738427.	-61508.	-0.000794	0.000	1.279E+13	2427.0531
109410.	0.000						
449.280	-0.1075	477385.	-49963.	-0.000794	0.000	1.279E+13	2506.9921
109108.	0.000						
453.960	-0.1112	271252.	-38042.	-0.000794	0.000	1.279E+13	2587.4829
108850.	0.000						
458.640	-0.1150	121792.	-25743.	-0.000794	0.000	1.279E+13	2668.5316
108632.	0.000						
463.320	-0.1187	30779.	-13063.	-0.000794	0.000	1.279E+13	2750.1416
108450.	0.000						
468.000	-0.1224	0.000	0.000	-0.000794	0.000	1.279E+13	2832.3139
54150.	0.000						

* This analysis makes computations of pile response using nonlinear moment-curvature relationships.

The above values of total stress are computed for combined axial and bending stress in elastic sections and do not equal actual stresses in concrete and steel in the range of nonlinear bending.

Output Verification: Computed forces and moments are within specified convergence limits.

Output Summary for Load Case No. 1:

Caisson Analysis.lp60
 Pile-head deflection = 1.0801992 inches
 Computed slope at pile head = -0.0066020 radians
 Maximum bending moment = 50608524. inch-lbs
 Maximum shear force = -256166. lbs
 Depth of maximum bending moment = 117.0000000 inches below pile head
 Depth of maximum shear force = 318.2400000 inches below pile head
 Number of iterations = 48
 Number of zero deflection points = 1

 Computed Values of Pile Loading and Deflection
 for Lateral Loading for Load Case Number 2

Pile-head conditions are Shear and Moment (Loading Type 1)

Shear force at pile head = 36055.000 lbs
 Applied moment at pile head = 52773727.000 in-lbs
 Axial thrust load on pile head = 52919.000 lbs

Depth Distrib. X Lat. Load inches lb/inch	Deflect. y inches	Bending Moment in-lbs	Shear Force lbs	Slope S radians	Total Stress psi*	Bending Stiffness lb-in^2	Soil Res. p lb/in	Soil Spr. Es*h lb/inch
0.000	0.000	52773727.	36055.	-0.007510	0.000	2.361E+12	0.000	
4.680	1.2051	52944311.	36055.	-0.007405	0.000	2.361E+12	0.000	
9.360	1.1707	53114870.	36055.	-0.007300	0.000	2.361E+12	0.000	
14.040	1.1368	53285402.	36055.	-0.007195	0.000	2.360E+12	0.000	
18.720	1.1034	53455908.	36055.	-0.007089	0.000	2.360E+12	0.000	
23.400	1.0705	53626388.	36055.	-0.006983	0.000	2.359E+12	0.000	
28.080	1.0380	53796842.	36055.	-0.006876	0.000	2.359E+12	0.000	
32.760	1.0061	53967269.	36055.	-0.006769	0.000	2.359E+12	0.000	
37.440	0.9747	54137669.	36055.	-0.006662	0.000	2.358E+12	0.000	
42.120	0.9437	54308043.	36055.	-0.006554	0.000	2.358E+12	0.000	
46.800	0.9133	54478391.	36055.	-0.006446	0.000	2.358E+12	0.000	
51.480	0.8834	54648711.	36048.	-0.006338	0.000	2.357E+12	-3.0742	
56.160	0.8540	54818937.	36024.	-0.006229	0.000	2.357E+12	-6.9686	
60.840	0.8251	54988984.	35532.	-0.006120	0.000	2.357E+12	-203.4551	
65.520	0.7967	55154548.	34372.	-0.006011	0.000	2.356E+12	-292.2137	
70.200	0.7688	55313684.	32795.	-0.005901	0.000	2.356E+12	-381.9208	
74.880	0.7415	55464428.	30812.	-0.005791	0.000	2.356E+12	-465.5039	
79.560	0.7146	55604949.	28477.	-0.005681	0.000	2.356E+12	-532.2618	
84.240	0.6883	55733785.	25843.	-0.005570	0.000	2.355E+12	-593.1836	
88.920	0.6625	55849601.	22938.	-0.005459	0.000	2.355E+12	-648.4519	
93.600	0.6372	55951187.	19787.	-0.005348	0.000	2.355E+12	-698.2500	
98.280	0.6124	56037453.	16415.	-0.005237	0.000	2.355E+12	-742.7613	
102.960	0.5882	56107422.	12846.	-0.005126	0.000	2.355E+12	-782.1696	
107.640	0.5645	56160232.	9105.0142	-0.005014	0.000	2.355E+12	-816.6588	
112.320	0.5413	56195128.	5213.4260	-0.004902	0.000	2.354E+12	-846.4130	
117.000	0.5186	56211458.	1193.2380	-0.004791	0.000	2.354E+12	-871.6160	
121.680	0.4964	56208670.	-2934.6805	-0.004679	0.000	2.354E+12	-892.4517	
126.360	0.000							

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126.360	0.4748	56186307.	-7150.3198	-0.004567	0.000	2.354E+12	-909.1035
8961.3130	0.000						
131.040	0.4537	56144005.	-11435.	-0.004456	0.000	2.355E+12	-921.7546
9508.8730	0.000						
135.720	0.4331	56081487.	-16472.	-0.004344	0.000	2.355E+12	-1231.0890
13304.	0.000						
140.400	0.4130	55991977.	-22258.	-0.004233	0.000	2.355E+12	-1241.6884
14070.	0.000						
145.080	0.3935	55875244.	-28083.	-0.004121	0.000	2.355E+12	-1247.3642
14837.	0.000						
149.760	0.3744	55731163.	-33923.	-0.004011	0.000	2.355E+12	-1248.3685
15604.	0.000						
154.440	0.3559	55559713.	-39757.	-0.003900	0.000	2.356E+12	-1244.9519
16370.	0.000						
159.120	0.3379	55360967.	-45566.	-0.003790	0.000	2.356E+12	-1237.3628
17137.	0.000						
163.800	0.3204	55135093.	-51330.	-0.003680	0.000	2.356E+12	-1225.8477
17903.	0.000						
168.480	0.3035	54882343.	-57031.	-0.003571	0.000	2.357E+12	-1210.6508
18670.	0.000						
173.160	0.2870	54603050.	-62653.	-0.003462	0.000	2.358E+12	-1192.0137
19436.	0.000						
177.840	0.2711	54297622.	-68181.	-0.003354	0.000	2.358E+12	-1170.1752
20203.	0.000						
182.520	0.2556	53966538.	-73599.	-0.003247	0.000	2.359E+12	-1145.3715
20970.	0.000						
187.200	0.2407	53610341.	-78895.	-0.003140	0.000	2.359E+12	-1117.8354
21736.	0.000						
191.880	0.2262	53229634.	-84056.	-0.003034	0.000	2.360E+12	-1087.7965
22503.	0.000						
196.560	0.2123	52825076.	-92466.	-0.002929	0.000	2.361E+12	-2505.9855
55248.	0.000						
201.240	0.1988	52365604.	-104257.	-0.002825	0.000	2.362E+12	-2533.1437
59628.	0.000						
205.920	0.1858	51850625.	-116177.	-0.002721	0.000	2.363E+12	-2560.5155
64481.	0.000						
210.600	0.1733	51279540.	-128207.	-0.002619	0.000	2.364E+12	-2580.8361
69678.	0.000						
215.280	0.1613	50651902.	-140316.	-0.002518	0.000	2.365E+12	-2593.9121
75249.	0.000						
219.960	0.1498	49967427.	-152469.	-0.002419	0.000	2.367E+12	-2599.5637
81230.	0.000						
224.640	0.1387	49225991.	-164630.	-0.002321	0.000	2.369E+12	-2597.6231
87660.	0.000						
229.320	0.1280	48427636.	-176620.	-0.002225	0.000	2.370E+12	-2526.2612
92332.	0.000						
234.000	0.1179	47573927.	-188128.	-0.002130	0.000	2.372E+12	-2391.4728
94960.	0.000						
238.680	0.1081	46667815.	-198999.	-0.002037	0.000	2.374E+12	-2254.4016
97588.	0.000						
243.360	0.0988	45712304.	-209225.	-0.001946	0.000	2.377E+12	-2115.6002
100216.	0.000						
248.040	0.0899	44710433.	-218798.	-0.001857	0.000	2.379E+12	-1975.5917
102845.	0.000						
252.720	0.0814	43665271.	-227715.	-0.001770	0.000	2.382E+12	-1834.8683
105473.	0.000						
257.400	0.0733	42579899.	-235972.	-0.001685	0.000	2.384E+12	-1693.8910
108101.	0.000						
262.080	0.0656	41457407.	-243570.	-0.001603	0.000	2.387E+12	-1553.0895
110730.	0.000						
266.760	0.0583	40300878.	-250510.	-0.001523	0.000	2.391E+12	-1412.8614
113358.	0.000						
271.440	0.0514	39113384.	-256797.	-0.001445	0.000	2.394E+12	-1273.5723
115986.	0.000						
276.120	0.0448	37897977.	-262434.	-0.001370	0.000	2.398E+12	-1135.5557
118614.	0.000						
280.800	0.0386	36657681.	-267429.	-0.001297	0.000	2.402E+12	-999.1126
121243.	0.000						
285.480	0.0327	35395483.	-271790.	-0.001227	0.000	2.406E+12	-864.5119
123871.	0.000						
290.160	0.0271	34114334.	-275526.	-0.001159	0.000	2.410E+12	-731.9905
126499.	0.000						
294.840	0.0218	32817136.	-278647.	-0.001095	0.000	2.415E+12	-601.7536
129128.	0.000						
299.520	0.0168	31506743.	-281164.	-0.001052	0.000	6.862E+12	-473.9747
131756.	0.000						
304.200	0.0120	30185963.	-283077.	-0.001036	0.000	1.256E+13	-343.4968
134384.	0.000						
308.880	0.007142	28857657.	-284370.	-0.001025	0.000	1.257E+13	-209.0859
137012.	0.000						
313.560	0.002372	27524768.	-285025.	-0.001014	0.000	1.259E+13	-70.7603
139641.	0.000						
318.240	-0.002351	26190327.	-285023.	-0.001004	0.000	1.260E+13	71.4674
142269.	0.000						

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322.920	-0.007028	24857450.	-284347.	-0.000995	0.000	1.261E+13	217.5897
144897.	0.000						
327.600	-0.0117	23529335.	-282977.	-0.000986	0.000	1.262E+13	367.6042
147526.	0.000						
332.280	-0.0163	22209270.	-280897.	-0.000977	0.000	1.263E+13	521.5133
150154.	0.000						
336.960	-0.0208	20900625.	-278087.	-0.000969	0.000	1.264E+13	679.3242
152782.	0.000						
341.640	-0.0253	19606858.	-274529.	-0.000962	0.000	1.266E+13	841.0484
155410.	0.000						
346.320	-0.0298	18331509.	-270205.	-0.000955	0.000	1.267E+13	1006.7016
158039.	0.000						
351.000	-0.0343	17078208.	-265097.	-0.000948	0.000	1.268E+13	1176.3035
160667.	0.000						
355.680	-0.0387	15850669.	-259186.	-0.000942	0.000	1.269E+13	1349.8769
163295.	0.000						
360.360	-0.0431	14652694.	-252453.	-0.000937	0.000	1.270E+13	1527.4484
165924.	0.000						
365.040	-0.0475	13488173.	-244880.	-0.000931	0.000	1.271E+13	1709.0470
168552.	0.000						
369.720	-0.0518	12361082.	-236447.	-0.000927	0.000	1.272E+13	1894.7041
171180.	0.000						
374.400	-0.0561	11275490.	-227136.	-0.000922	0.000	1.273E+13	2084.4534
173808.	0.000						
379.080	-0.0604	10235550.	-217283.	-0.000918	0.000	1.273E+13	2126.0599
164645.	0.000						
383.760	-0.0647	9242176.	-207252.	-0.000915	0.000	1.274E+13	2160.5453
156228.	0.000						
388.440	-0.0690	8296122.	-197072.	-0.000912	0.000	1.275E+13	2190.2257
148566.	0.000						
393.120	-0.0733	7398038.	-186763.	-0.000909	0.000	1.276E+13	2215.2963
141530.	0.000						
397.800	-0.0775	6548474.	-176347.	-0.000906	0.000	1.276E+13	2235.9162
135021.	0.000						
402.480	-0.0817	5747881.	-165845.	-0.000904	0.000	1.277E+13	2252.2163
128959.	0.000						
407.160	-0.0860	4996617.	-155276.	-0.000902	0.000	1.278E+13	2264.3062
123278.	0.000						
411.840	-0.0902	4294945.	-144660.	-0.000900	0.000	1.279E+13	2272.2790
117928.	0.000						
416.520	-0.0944	3643042.	-134017.	-0.000899	0.000	1.279E+13	2276.2153
112864.	0.000						
421.200	-0.0986	3040993.	-123364.	-0.000897	0.000	1.279E+13	2276.1862
108051.	0.000						
425.880	-0.1028	2488798.	-112678.	-0.000896	0.000	1.279E+13	2290.6569
104298.	0.000						
430.560	-0.1070	1986774.	-101761.	-0.000896	0.000	1.279E+13	2374.5115
103878.	0.000						
435.240	-0.1112	1536757.	-90451.	-0.000895	0.000	1.279E+13	2458.9742
103518.	0.000						
439.920	-0.1154	1140597.	-78744.	-0.000895	0.000	1.279E+13	2544.0626
103213.	0.000						
444.600	-0.1195	800158.	-66637.	-0.000894	0.000	1.279E+13	2629.7913
102955.	0.000						
449.280	-0.1237	517317.	-54127.	-0.000894	0.000	1.279E+13	2716.1717
102741.	0.000						
453.960	-0.1279	293968.	-41212.	-0.000894	0.000	1.279E+13	2803.2125
102566.	0.000						
458.640	-0.1321	132015.	-27888.	-0.000894	0.000	1.279E+13	2890.9198
102426.	0.000						
463.320	-0.1363	33381.	-14151.	-0.000894	0.000	1.279E+13	2979.2973
102317.	0.000						
468.000	-0.1405	0.000	0.000	-0.000894	0.000	1.279E+13	3068.3462
51119.	0.000						

* This analysis makes computations of pile response using nonlinear moment-curvature relationships.

The above values of total stress are computed for combined axial and bending stress in elastic sections and do not equal actual stresses in concrete and steel in the range of nonlinear bending.

Output Verification: Computed forces and moments are within specified convergence limits.

Output Summary for Load Case No. 2:

Pile-head deflection	=	1.2400329	inches
Computed slope at pile head	=	-0.0075100	radians
Maximum bending moment	=	56211458.	inch-lbs
Maximum shear force	=	-285025.	lbs
Depth of maximum bending moment	=	117.0000000	inches below pile head
Depth of maximum shear force	=	313.5600000	inches below pile head
Number of iterations	=	31	

Number of zero deflection points = Caisson Analysis.lp60
1

Summary of Pile Response(s)

Definitions of Pile-head Loading Conditions:

Load Type 1: Load 1 = Shear, lbs, and Load 2 = Moment, in-lbs
 Load Type 2: Load 1 = Shear, lbs, and Load 2 = Slope, radians
 Load Type 3: Load 1 = Shear, lbs, and Load 2 = Rotational Stiffness, in-lbs/radian
 Load Type 4: Load 1 = Top Deflection, inches, and Load 2 = Moment, in-lbs
 Load Type 5: Load 1 = Top Deflection, inches, and Load 2 = Slope, radians

Load Case No.	Load Type No.	Pile-head Condition 1 Pile-head V(lbs) or Rotation y(inches) radians	Pile-head Condition 2 in-lb, rad., or in-lb/rad.	Axial Loading lbs	Pile-head Deflection inches	Maximum Moment in-lbs	lbs
1	1	V = 31636.	M = 47587518.	64248.	1.08019920	50608524.	
-256166.		-0.00660195					
2	1	V = 36055.	M = 52773727.	52919.	1.24003285	56211458.	
-285025.		-0.00750996					

The analysis ended normally.