Filed by:
G. Scott Shepherd, Site Development Specialist II - SBA Communications
134 Flanders Rd., Suite 125, Westborough, MA 01581
508.251.0720 x 3807 - GShepherd@sbasite.com

September 21, 2020

Melanie A. Bachman
Acting Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

RE: Notice of Exempt Modification
81 Montevideo Rd., Avon, CT
Latitude: 41.803100
Longitude: -72.801300
T-Mobile Site #: CT11284A_Anchor

Dear Ms. Bachman:

T-Mobile currently maintains three (3) 600/700/1900/2500 MHz antennas at the 136-foot level of the existing 150-foot Self-Supporting Tower at **81 Montevideo Rd., Avon, CT**. The 150-foot tower is owned by SBA 2012 TC Assets, LLC. The property is owned by Monte LLC. T-Mobile now intends to install one (1) additional 2500 MHz antenna. The new antennas would be installed at the 136-foot level of the tower.

Please note: Per the Connecticut Siting Council Website: CSC COVID 19 Guidelines. *In order to prevent the spread of Coronavirus and protect the health and safety of our members and staff, as of March 18, 2020, the Connecticut Siting Council shall convert to full remote operations until March 30, 2020. Please be advised that during this time period, all hard copy filing requirements will be waived in lieu of an electronic filing. Please also be advised that the March 26, 2020 regular meeting shall be held via teleconference. The Council's website is not equipped with an on-line filing fee receipt service. Therefore, filing fees and/or direct cost charges associated with matters received electronically during the above-mentioned time period will be directly invoiced at a later date.*

Planned Modifications:

TOWER

Remove:

- N/A

Remove and Replace:

- N/A

Install New:

- (1) Ericsson AIR 6449 B41 - antenna
- (1) Ericsson Radio 4415 B25 – RRU
- (1) 1-5/8" fiber

Existing Equipment to Remain:

- (1) Ericsson AIR32 KRD901146-1_B66 – antenna
- (1) Ericsson AIR21 KRC118023-1 – antenna
- (1) RFS-APXVAARR24_43-U-NA20 - antenna
- (1) Ericsson KRY112 144/2 - TMA
- (1) Ericsson 4449 B71+B12 – RRU
- (2) 1-5/8" coax
- (1) ½" coax for GPS (ground)
- (1) 1-5/8" fiber

Entitlements:

- (4) 1-5/8" cable
- (1) 1-1/4" hybrid cable

GROUND

Install New:

- Equipment mounted to proposed 8'x 8' x 6' concrete pad within 8' x 8' lease area in existing compound under proposed 8' x 8' weather canopy.
- Equipment inside existing 6160 equipment cabinet

Original tower was approved by Town of Avon zoning agencies, case# EM-MCM-004-070824 (enclosed), prior to statewide change in law in 2001, which brought it under the CSC's jurisdiction. Approval is for the replacement of a 150' self-support structure. The Tower was later approved by the CSC on Oct. 24, 2007, Docket# EM-MCM-004-070824 (enclosed) There were no further post construction stipulations set. Please see attached.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. §16.50j-72(b)(2). In accordance with R.C.S.A. § 16.50j-73, a copy of this letter is being sent to the Town of Avon's Director of Planning and Community Development, Hiram Peck III, and Town Manager, Brandon Robertson, as well as to the property owner, Monte LLC. (Separate notice is not being sent to tower owner, as it belongs to SBA.)

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. §16.50j-72(b)(2).

1. The proposed modifications will not result in an increase in the height of the existing structure.
2. The proposed modification will not require the extension of the site boundary.
3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
5. The proposed modification will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-referenced telecommunication facility constitute an exempt modifications under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

G. Scott Shepherd
Site Development Specialist II
SBA COMMUNICATIONS CORPORATION
134 Flanders Rd., Suite 125
Westborough, MA 01581

508.251.0720 x3804 + T
508.366.2610 + F
508.868.6000 + C
GShepherd@sbasite.com

Attachments
cc: Hiram Peck III, Director of Planning and Community Development/with attachments
Avon Town Hall, 60 West Main St., Avon, CT 06001
Brandon Robertson, Town Manager / with attachments
Avon Town Hall, 60 West Main St., Avon, CT 06001
Monte LLC / with attachments
40 Woodland St., Hartford, CT. 06105

Exhibit List

Exhibit 1	Check Copy	X To Be Invoiced by CSC per Covid 19 Guidelines
Exhibit 2	Notification Receipts	x
Exhibit 3	Property Card	x
Exhibit 4	Property Map	x
Exhibit 5	Original Zoning Approval	Town of Holland P&Z Commission 11/10/97
Exhibit 6	Construction Drawings	Chappell Engineering 9/3/20
Exhibit 7	Structural Analysis	TES dated 6/30/20
Exhibit 8	Mount Analysis	TES dated 7/8/20
Exhibit 9	EME Report	EBI Consulting 7/9/20

EXHIBIT 1

Normally, Exhibit 1 would contain a copy of the check for the filing fee.

EXHIBIT 2

After printing this label:
1. Use the 'Print' button on this page to print your label to your laser or inkjet printer.
2. Fold the printed page along the horizontal line.
3. Place label in shipping pouch and affix it to your shipment so that the barcode portion of the label can be read and scanned.

Warning: Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.
Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com.FedEx will not be responsible for any claim in excess of $100 per package, whether the result of loss, damage, delay, non-delivery,misdelivery,or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim.Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental,consequential, or special is limited to the greater of $100 or the authorized declared value. Recovery cannot exceed actual documented loss.Maximum for items of extraordinary value is $1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our ServiceGuide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

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EXHIBIT 3

Property at 00081 MONTEVIDEO ROAD — Prop ID 3180081 — Printed 14-Feb-2019 8:15 PM — Design and Layout (C) Right/Angles

Administrative Information

Owner name: MONTE LLC
Second name:
Address: 40 WOODLAND STREET
City/state: HARTFORD CT Zip: 06105

Location Information

Map: 015 Clerk map: 04 104
Lot: 3180081 Neigh.: Zone: RU2A Vol: 455 Page: 057

Assessments

Assmt category	Qty	Amount
Resident Land	2.00	140,000
Resident Dwellng	1.00	261,560
Resident Outbldg	2.00	1,020
Resident Excess	.50	2,630

Exemptions

Exempt	Cat	Amount

Last sale

Sale date: 12-Jan-1987
Sale price: 460,000
Sale valid:

Values

Mkt value :
Cost value: 578,871

Summary

Total assessments	405,210
Total exemptions	
Net assessment	405,210

Utilities

Water	Well
Sewer	Septic
Gas	None

Sales ratios

Cost/sale : 1.2584
Mkt/sale :
Assmt/sale: .8809

Land Information

Type	Use	Acres/SqFt	Rate	Total	Infl Fact	Value	70% Value
PRIM	11	2.000	400,000	400,000	.50	200,000	140,000
Primary Site		87,120					
RES	12	.500	7,500	3,750		3,750	2,625
Residual		21,780					
		2.500 acres		Total land value		203,750	142,625

Residential Dwelling Information

Subject	Code	Description
Style	02	Cape
Exterior Walls	01	Clapboards
Roof Material	01	Asphalt Shingles
Roof Type	01	Gable
Foundation	01	Poured Concrete
Interior Walls	02	Drywall
Floors	01	Hardwood
Heating System	02	Forced Hot Air
Fuel	01	Oil
Attic	99	None
Grade	40	B
Garage	23	Attached 3 car
Area Over Gar.	03	Partial
Basement	01	Full
Bsmt Fin Qual	02	Rec Room w/o air
Air Condition	01	Central Air
Interior Cond	04	Average
Exterior Cond	04	Average

Condominium

Story Height 1.5

Total Rooms	8	Garage cars	3
Bedrooms	4	Unfinished area	
Family Rooms	1	Dormer linear f	
Full Baths	3	Masonry trim sf	
Half Baths	1	Finish bsmt sz	
Addtn'l fixtures	3	Rec Room Size	1,380
Whirlpools	1	Living area	4,565
Saunas		# Living Units	
M/F stacks			
W/B stacks	1		
W/B openings	2		

Actual Year Built: 1955

Building Valuation Summary

			Area	
Dwelling	Frame	1 1/2 story w/bsmt	1,671	256,410
Basement	Full			
Heating	Yes	A/C Yes		7,770
Plumbing	3 F/B	1 H/B 3 Add'l fix. 1 Wh/p	Saunas	13,600
Attic	None	Attic size:		
Additions				186,962
Other Features		WB Stks RR		29,510
Sub-Total				494,252
Grade	B	Factor 1.2600		622,758
CDU		C&D Factor 1.00		622,758
Depreciation		40 %		373,655
		Computed cost value @ 70%		261,559

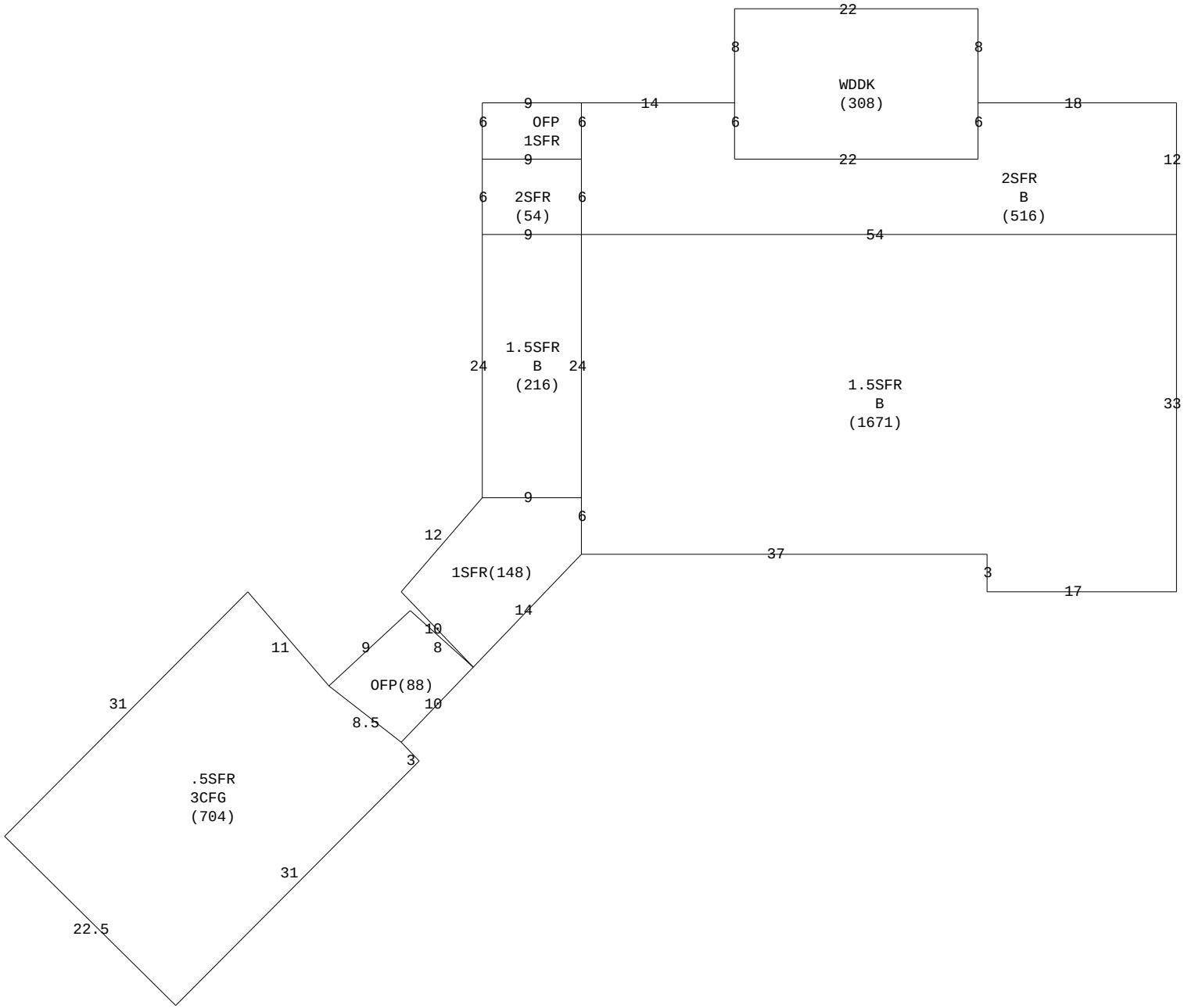
Building additions

	Category	Type		Area	Value
G	Garages	FRL	Attached frame GT 65	704	23,363
L	Living Area	FATT	Full finished attic	352	9,417
L	Living Area	AIR	Air conditioning	352	793
P	Porches, Patios, Decks	FOFF	Frame open first flo	88	3,971
L	Living Area	FRFF	Frame first floor	148	12,550
L	Living Area	AIR	Air conditioning	148	333
L	Living Area	FRFF	Frame first floor	216	18,317
L	Living Area	FRUH	Frame upper half	216	8,182
L	Living Area	BSMT	Basement addition	216	2,948
L	Living Area	AIR	Air conditioning	324	730
L	Living Area	FRFF	Frame first floor	54	4,579
L	Living Area	FRUF	Frame upper full	54	3,349
L	Living Area	AIR	Air conditioning	108	243
L	Living Area	FRFF	Frame first floor	54	4,579
P	Porches, Patios, Decks	FOFF	Frame open first flo	54	2,437
L	Living Area	FRFF	Frame first floor	516	43,757
L	Living Area	FRUF	Frame upper full	516	32,003
L	Living Area	BSMT	Basement addition	516	7,043
L	Living Area	AIR	Air conditioning	1,032	2,324
P	Porches, Patios, Decks	DECK	Wood deck	308	6,044
			Total additions		186,962

Outbuilding Information

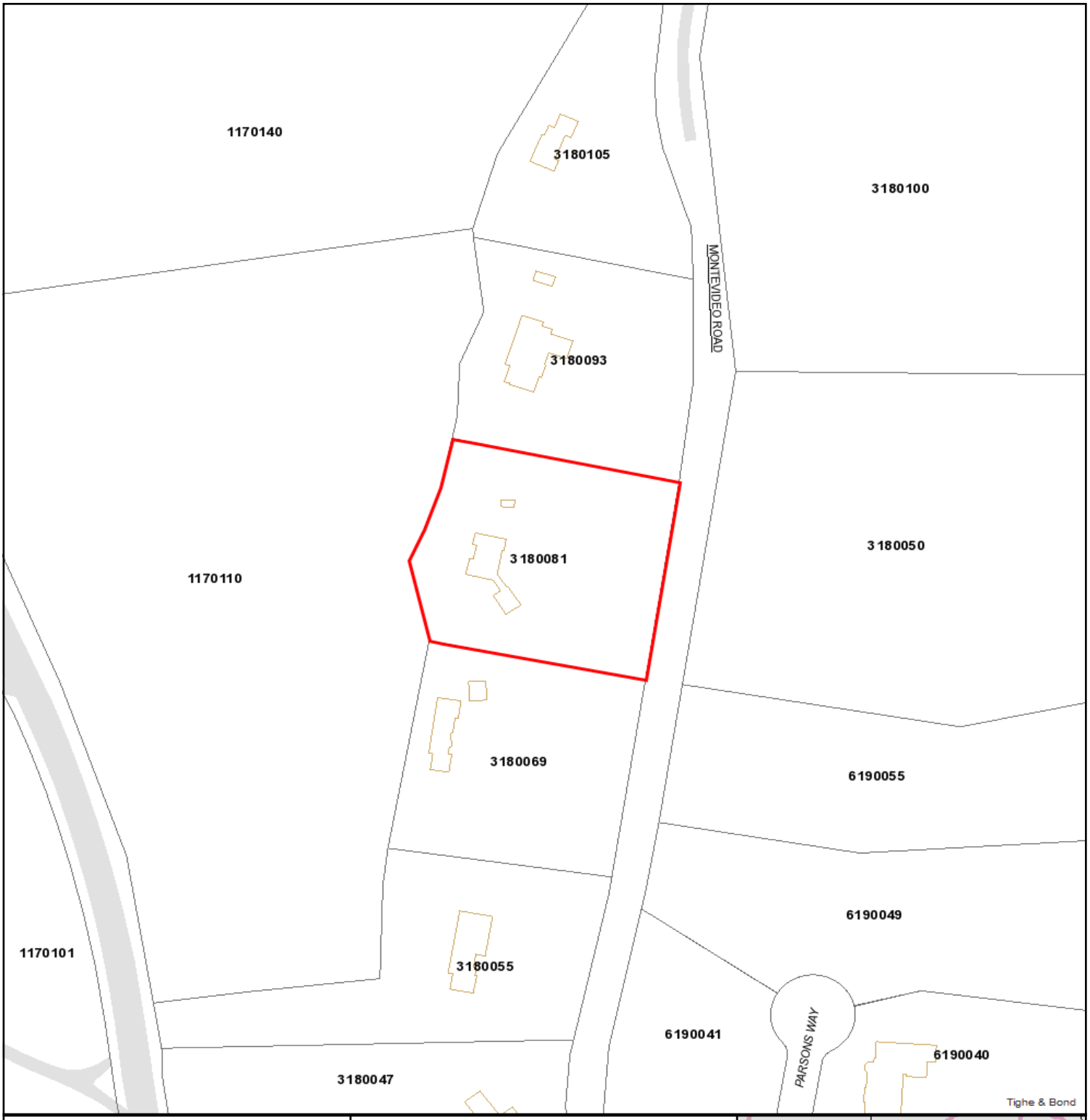
Description	Wid	Len	Area	Rate	Year	Cnd	RCN	Depr	Value
RS1 Frame	10	9	90	20.00		C	1,800	50	900
Utility Shed									
RC2 Canopy	8	9	72	15.63		C	1,125	50	560
Value at 70%			1,022			Value at 100%			1,460

Property at 00081 MONTEVIDEO ROAD | Prop ID 3180081 Card 01 | Scale: 1" = 13.73 feet



EASEMENT AGREEMENT MONTE TO MC TOWERS INC $100,000 V 708 P 268 3-31-17
AMENDMNT TO EASEMNT MONTE TO MCM ACQUISITION 710/534 6-19-17 & 711/422

EXHIBIT 4



81 MONTEVIDEO ROAD

7/30/2020 6:44:12

1"=200'

Property Information

GISPin	3180081
Location	81 MONTEVIDEO ROAD
Sale Price	460000

The information depicted on this map is for planning purposes only. It is not adequate for legal boundary definition, regulatory interpretation, or parcel-level analyses.

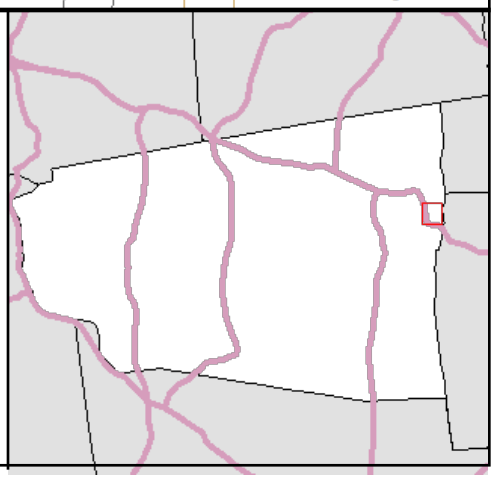


EXHIBIT 5

SITE NAME: Avon (Montevideo) SITE ID: CT22071-A

Transaction: Message Center Management, Inc. (MCM) Jihan/Sam Toth

ZONING/PERMITTING COMPLETION FORM

CT-06

Address: 81 Montevideo Road, Avon, CT 06001

Jurisdiction: Connecticut Siting Council – Zoning (currently) Zoning District: RU2A
Town of Avon - Permitting

Zoning Approval Type: Exempt Modification Case #: EM-MCM-004-070824

Approval Date: 10/16/2007 Approved Height: 150

Conditions of Approval:

	Yes
Removal Bond	☐
Site Plan Submittal	☐
Fall Zone	☐
Periodic Inspections	☐
Periodic Reporting	☐
Approval Renewal	☐
Additional Conditions	☐

The original tower was approved by Town of Avon zoning agencies prior to a statewide change in the law in 2001 which subsequently brought it under the Connecticut Siting Council's jurisdiction.
Approval is for replacement of a 150 ft. guyed tower with a 150 ft. self-support structure. A routine & periodic inspection of the existing tower in January of 2007 showed the existing tower doesn't meet current structural standards. Therefore, tower needed to be replaced to meet current & future structural safety standards.

Planning/Zoning:

Phone: Email:

Bldg./Code Enforcement: Susan Gatcomb

Phone: 860-409-4316 Email: sgatcomb@avonct.gov

Submitted by: Batches Estes Date: 3/27/2017
Zoning Compliance

TO BE COMPLETED BY CORPORATE

	Yes	No	N/A	
Zoning Approval Attached (required)	☒	☐	☐	
Building Permit Attached (required)	☒	☐	☐	Date Recd
08-0916				3/24/2008
Certificate of Occupancy or Compliance (CO) attached (required)	☒	☐	☐	11/3/2008

Zoning Manager Approval: A. Masuda Date 3/28/2017
Ashley Masuda

STATE OF CONNECTICUT

CONNECTICUT SITING COUNCIL

Ten Franklin Square, New Britain, CT 06051
Phone: (860) 827-2935 Fax: (860) 827-2950
E-Mail: siting.council@ct.gov
Internet: ct.gov/csc

Daniel F. Caruso
Chairman

October 24, 2007

Christopher B. Fisher, Esq.
Cuddy & Feder LLP
445 Hamilton Avenue, 14th Floor
White Plains, NY 10601

RE: **EM-MCM-004-070824** – Message Center Management, Inc. notice of intent to modify an existing telecommunications facility located at 81 Montevideo Road, Avon, Connecticut.

Dear Attorney Fisher:

At a public meeting held on October 16, 2007, the Connecticut Siting Council (Council) acknowledged your notice to modify this existing telecommunications facility, pursuant to Section 16-50j-73 of the Regulations of Connecticut State Agencies with the condition that Message Center Management, Inc. will flag any trees slated for removal in the field and note the same on the construction drawings to be submitted as part of any building permit application.

The proposed modifications are to be implemented as specified here and in your notice dated August 23, 2007 and additional information dated September 28, 2007, including the placement of all necessary equipment and shelters within the tower compound. The modifications are in compliance with the exception criteria in Section 16-50j-72 (b) of the Regulations of Connecticut State Agencies as changes to an existing facility site that would not increase tower height, extend the boundaries of the tower site, increase noise levels at the tower site boundary by six decibels, and increase the total radio frequencies electromagnetic radiation power density measured at the tower site boundary to or above the standard adopted by the State Department of Environmental Protection pursuant to General Statutes § 22a-162. This facility has also been carefully modeled to ensure that radio frequency emissions are conservatively below State and federal standards applicable to the frequencies now used on this tower.

This decision is under the exclusive jurisdiction of the Council. Please be advised that the validity of this action shall expire one year from the date of this letter. Any additional change to this facility will require explicit notice to this agency pursuant to Regulations of Connecticut State Agencies Section 16-50j-73. Such notice shall include all relevant information regarding the proposed change with cumulative worst-case modeling of radio frequency exposure at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin 65. Any deviation from this format may result in the Council implementing enforcement proceedings pursuant to General Statutes § 16-50u including, without limitation, imposition of expenses resulting from such failure and of civil penalties in an amount not less than one thousand dollars per day for each day of construction or operation in material violation.

Thank you for your attention and cooperation.

Very truly yours,

David F. Caruso FOC

Daniel F. Caruso
Chairman

DFC/MP/cm

c: The Honorable Richard W. Hines, Chairman Town Council, Town of Avon
Steven V. Kushner, Town Planner, Town of Avon
Hans Fiedler, Message Center Management, Inc.

G:\EM\MCM\Avon\dc101607.DOC

CUDDY & FEDER LLP

August 23, 2007

BY FEDEX - Priority Overnight
Hon. Daniel F. Caruso, Chairman
and Members of the Connecticut Siting Council
10 Franklin Square
New Britain, Connecticut 06051

Re: Message Center Management, Inc.
Notice of Exempt Modification
Replacement Tower
81 Montevideo Road, Avon, Connecticut

Dear Chairman and Members of the Council:

On behalf of Message Center Management, Inc. ("MCM"), enclosed please find its notice of exempt modification with respect to the above referenced matter together with a check in the amount of $500, the filing fee. We respectfully request that this matter be placed on the next Council agenda for acknowledgment. In the interim, should your of the Council's staff have any questions regarding this matter, please do not hesitate to contact us. Thank you.

Very truly yours,

Christopher B. Fisher

Enclosures
cc: Phillip K. Schenck, Avon Town Manager
Steven Kushner, Town Planner
Maria Scotti, MCM
Hans Fiedler, MCM

CONNECTICUT SITING COUNCIL

NOTICE OF EXEMPT MODIFICATION BY MESSAGE CENTER MANAGEMENT, INC. ("MCM") REGARDING A REPLACEMENT TOWER AT ITS EXISTING FACILITY IN AVON CONNECTICUT

Pursuant to Connecticut General Statutes § 16-50g et. seq., and Section 16-50j-72(b)(3) of the Regulations of Connecticut State Agencies adopted pursuant thereto, Message Center Management, Inc. ("MCM") hereby notifies the Connecticut Siting Council of its intent to modify an existing facility located at 81 Montevideo Road, Avon, Connecticut (the "Avon Facility") by replacing the tower to the same height.

The Existing Avon Tower Facility

The Avon Facility consists of a one hundred fifty (150') foot guyed tower (the "Tower"), building and other improvements located on sizeable parcel of property in the Town of Avon. The Avon Facility is principally used by Omnipoint Communications Inc. ("T-Mobile") and Sprint Spectrum, L.P. ("Sprint") that provide "cellular" services to the public as that term is defined and used in Section 16-50i(a)(6) of the Connecticut General Statutes. The Avon Facility was originally approved by Town of Avon zoning agencies prior to a statewide change in the law in 2001 which subsequently brought it under the Siting Council's jurisdiction.

Reasons for the proposed Tower Replacement

MCM's consulting engineers completed a routine and periodic inspection of the existing Tower in January of 2007, a copy of which is enclosed. As noted therein, the existing tower does not meet current structural standards. As such a recommendation was made to replace the tower along with other interim measures which MCM has since completed. At this time, MCM is proposing to replace the tower to meet current and future structural safety standards.

Replacement Tower

As shown on the enclosed plans prepared by URS Corporation, including a survey, site plan, and tower elevation, MCM proposes replacing the existing 150' guyed tower with a self-support lattice tower maintaining the same height and antenna locations (the "Replacement Tower"). The Replacement Tower will be located immediately adjacent to the existing Tower approximately 15' to the east and will be designed to taper to a uniform tower face for a significant portion of its height above grade. No other antenna or equipment modifications are proposed by MCM.

MCM's Tower Replacement Constitutes An Exempt Modification

The proposed replacement of the existing Tower constitutes an exempt modification of an existing facility as defined in Connecticut General Statutes Section 16-50i(d) and Council regulations promulgated pursuant thereto. Specifically 16-50j-72(b)(3) of the Council's regulations provides that, among other exempt modifications, included is a:

> Replacement of an existing CATV tower or telecommunications tower and associated equipment with a tower that is no taller than the tower to be replaced....

Here the Replacement Tower is the same height as the existing Tower and as such the modification is exempt from the requirement for any further processing or approvals.

MCM's Discussions with Town Planning Officials

We note that representatives of MCM have discussed this project with Town of Avon Planning Officials regarding the structural issues and different tower replacement options. As part of those discussions MCM noted for the Town that a replacement guyed tower would require substantial tree clearing at the Avon Facility site. In lieu of same, MCM has elected to replace the tower with a self support structure which will continue to accommodate users of the site and have a minimal impact on tree clearing. For reference purposes, we have also included photosimulations of the existing Tower and proposed Replacement Tower.

Conclusion

MCM requests that the Connecticut Siting Council acknowledge that its proposed Replacement Tower at the Avon Facility meets the Council's exemption criteria specified in Section 16-72(b)(3) of its regulations. We note that no other land use or zoning approvals are required for the project pursuant to Section 16-50x of the Connecticut General Statutes. As such, upon receipt of the Council's acknowledgment, MCM intends to procure signed/sealed tower drawings from the manufacturer of the Replacement Tower and file for a building permit from the Town of Avon.

Respectfully Submitted.

Christopher B. Fisher, Esq.
On behalf of MCM

cc: Phillip K. Schenck, Avon Town Manager
Steven Kushner, Town Planner
Maria Scotti, MCM
Hans Fiedler, MCM

TECTONIC
Practical Solutions, Exceptional Service

CORPORATE OFFICE:
Mountainville, NY (800) 829-6531

TECTONIC Engineering & Surveying Consultants P.C.
955 Little Britain Road
New Windsor, NY 12553

(845) 567-6656 FAX: (845) 567-8703
www.tectonicengineering.com

Virginia King
Message Center Management
40 Woodland Street
Hartford, CT 06105

January 30, 2007

RE: W.O. 3997.07
AVON TOWER UPGRADE
81 MONTEVIDEO ROAD
AVON, CT
ANALYSIS RESULTS AND RECOMMENDATIONS

Dear Virginia:

As requested, Tectonic Engineering & Surveying Consultants P.C. has completed an inspection and detailed structural analysis of the existing 150' guyed tower at the above referenced site. The overall configuration of the tower is shown in Figure 1, attached.

The tower was inspected by representatives of Tectonic on 11/28/06. Based on our inspection, there are several items of concern with respect to the physical condition of the tower, as follows:

- Significant external corrosion of the tower mast, especially between the 100' and 130' levels.
- Considering the age of the tower and the use of light gauge steel tubing for the tower legs, we anticipate that internal corrosion may be present. Due to the thin walls of the tubing, even moderate corrosion can significantly reduce the structural capacity of the tower legs.
- The bracing members are very small diameter steel rods. Several are bent, which reduces their effectiveness.
- Some of the leg splice bolts are significantly corroded.
- The guys are small diameter and are generally not sufficiently taut.
- The antenna mounts at the 108' level are heavily corroded.
- Numerous existing cables are inadequately secured, and they are not efficiently bundled to minimize wind loading.

This analysis was performed using Revision G of the TIA-222 standard, as previously discussed. We find that the structure is significantly overloaded in its existing condition, despite the fact that the tower was assumed to be in "like-new" condition for the purpose

of the analysis. In other words, the capacities of the tower members and guys were not adjusted (i.e. reduced) to reflect their actual condition.

As a result of our analysis, we find that:

1. The existing tower is unable to withstand the required wind loading of 95 mph (3-second gust) with no ice accumulation.
2. The maximum wind speed that the tower can safely withstand is less than 60 mph.
3. The existing tower cannot support the required ice loading, even with no wind load acting on it at all.

The primary reasons for the large degree of overstress are:

a. The small size and limited capacity of the leg and bracing members, as well as the guys.
b. The inclusion of a topographic factor based on the site location, as required by TIA-222-G.
c. The larger ice thickness required by TIA-222-G, compared to the previous version of the standard.

We note that previous Structural Analysis reports by Tectonic and at least two (2) other firms show that there were problems related to the capacity of the tower under Revision F of the TIA-222 standard.

Since no information on the existing tower foundation and guy anchors was available, we could not assess their actual capacities. Based on the large overstress in the tower mast and guys, we anticipate that the existing foundations may not have sufficient capacity to resist the required loads.

The tower will need to be upgraded to satisfy the current code requirements, and to provide capacity for modified antenna configurations in the future. Reinforcement is expected to involve the installation of heavy external bracing in approximately the lower half of the tower, and replacing the guys at several levels with larger size guy wires and matching hardware. It will also involve modifications to the tower base foundation and all guy anchors.

Although upgrading the tower to meet these requirements may be technically feasible, it may be as costly and disruptive as the construction of a new replacement tower.

We recommend the installation of a new, sturdier tower on a new foundation, and relocation of all existing antennas thereto, as a better alternative. We understand that

replacement of the structure does bring up other issues, but it will probably be more economical in the long run.

Anticipating that the processes for approval, procurement, and installation of a replacement tower may take a substantial amount of time, we recommend that the following corrective actions be taken as soon as possible:

1. Retension all of the guys to be between 8% and 10% of their breaking strength.
2. Clean and repaint the deteriorated portions of the tower to minimize further corrosion.
3. Bundle and securely fasten the cables to the tower legs or faces.
4. Perform a simple visual inspection of the structure after any severe wind or ice storm, to verify that no damage has occurred.

In closing, we foresee no problem with removing the two (2) existing T-Mobile panel antennas that are mounted at the 138' level, and reinstalling one (1) panel antenna at a lower elevation, as suggested.

Please contact me if there are any questions on the above.

Sincerely,
TECTONIC

Jeffrey B. Kirby, P.E.
Chief Structural Engineer

file AvonTowerUpgradeLtr.doc

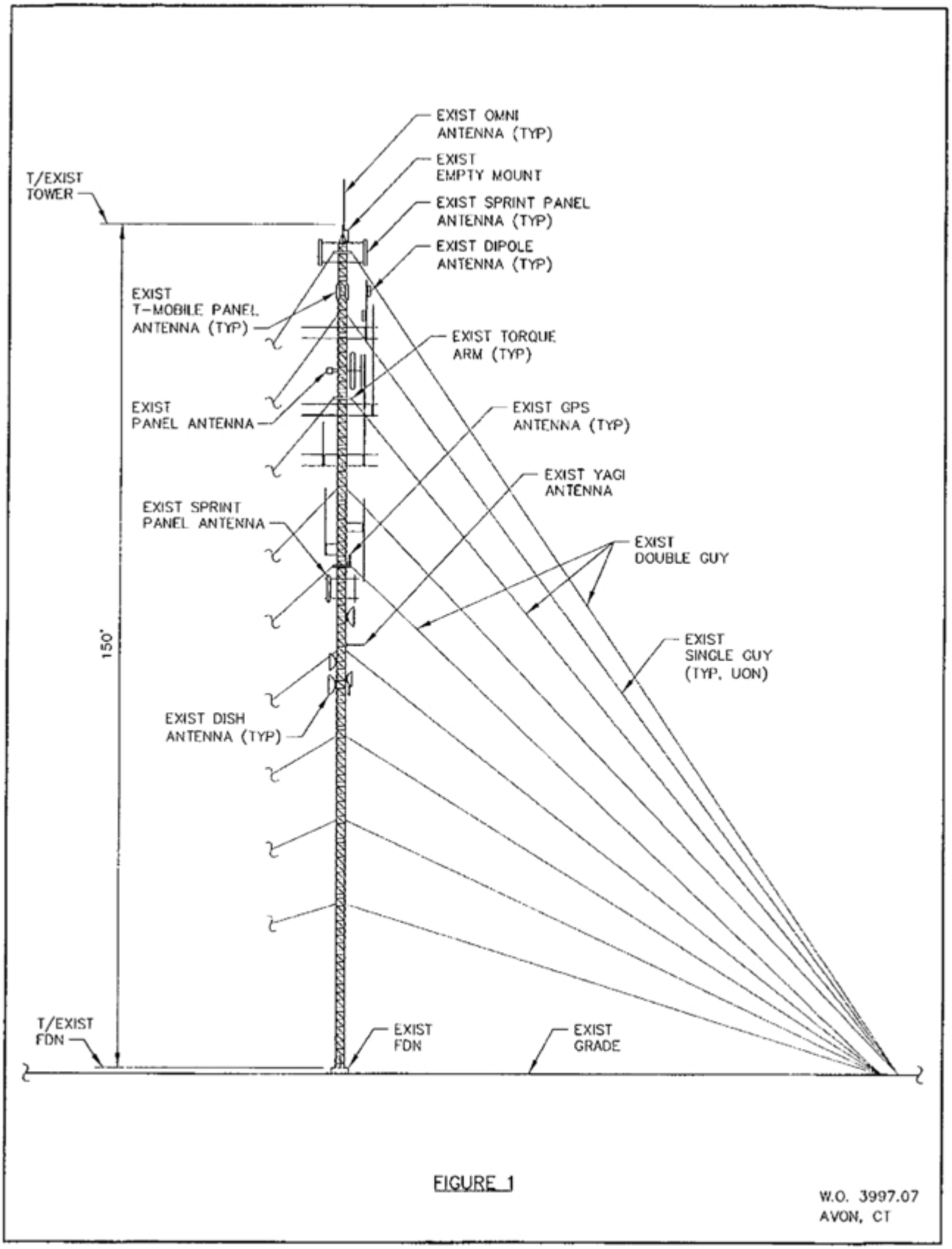
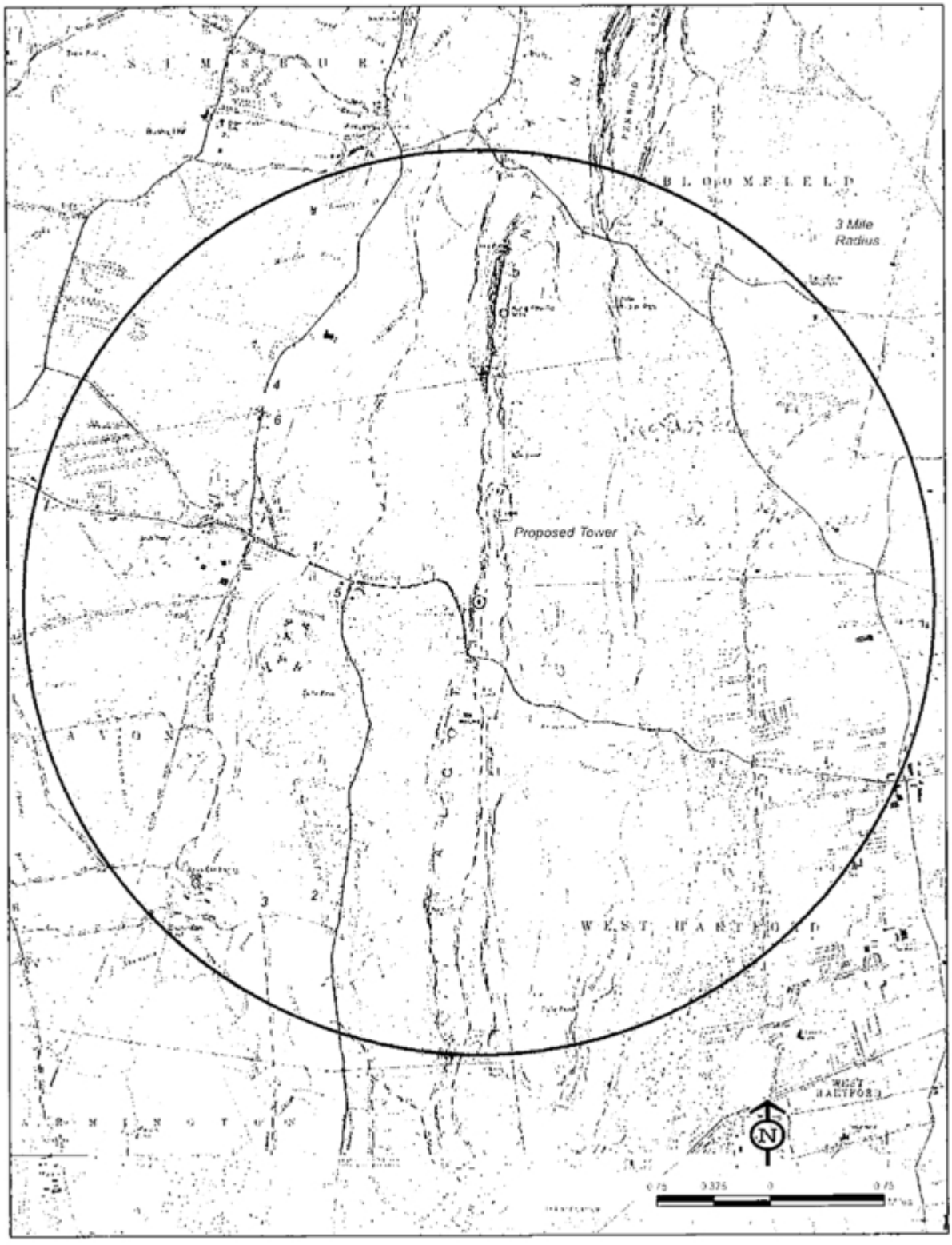


FIGURE 1

W.O. 3997.07
AVON, CT

Photolog Documentation

Town of
Avon
Connecticut

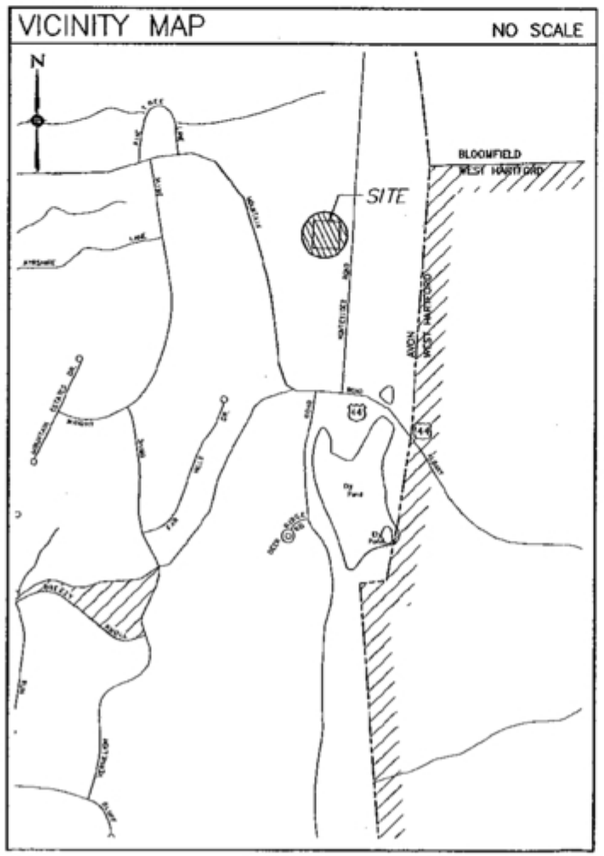


MESSAGE CENTER MANAGEMENT

AVON TOWER REPLACEMENT

81 MONTEVIDEO ROAD
AVON, CONNECTICUT

VICINITY MAP — NO SCALE



PROJECT SUMMARY

SITE NAME:	AVON
SITE ADDRESS:	81 MONTEVIDEO ROAD AVON, CONNECTICUT
CONTACT PERSON:	MESSAGE CENTER MANAGEMENT VIRGINIA KING (860) 727-5799
GOVERNING CODE:	CONNECTICUT STATE BUILDING AND LIFE SAFETY CODE
APPLICANT:	MESSAGE CENTER MANAGEMENT 40 WOODLAND STREET HARTFORD, CONNECTICUT 06105
ARCHITECT:	URS CORPORATION AES 500 ENTERPRISE DRIVE ROCKY HILL, CT 06067
M/E/P ENGINEER:	URS CORPORATION AES 500 ENTERPRISE DRIVE ROCKY HILL, CT 06067
SURVEYOR:	URS CORPORATION A.E.S. 500 ENTERPRISE DRIVE, SUITE 38 ROCKY HILL, CT 06067
GEODETIC COORDINATES:	LATITUDE: 41°-48'-11.00" LONGITUDE: 72°-45'-4.95" NAD 83
GROUND ELEVATION:	ELEVATION 747 NGVD 29

LEGEND

SYMBOL	DESCRIPTION
	SECTION OR DETAIL NUMBER SHEET WHERE DETAIL/SECTION OCCURS
	ELEVATION NUMBER SHEET WHERE ELEVATION OCCURS

ABBREVIATIONS

MIN.	MINIMUM
V.I.F.	VERIFY IN FIELD
O.C.	ON CENTER
PSF	POUND/SQUARE FOOT
TYP.	TYPICAL
FT.	FEET
SQ.FT.	SQUARE FEET
N/A	NOT APPLICABLE

SHEET INDEX

MESSAGE CENTER MANAGEMENT
40 WOODLAND STREET
HARTFORD, CONNECTICUT
06105

A&E FIRM
URS CORPORATION AES
500 ENTERPRISE DRIVE
ROCKY HILL, CONNECTICUT
1-(860)-529-8882

STATE OF CONNECTICUT
IGNACIO C. ARIAS
No. 8704
LICENSED ARCHITECT

PROJECT NO: 36924843

JOB NO: MCM 007

DRAWN BY: RRH

CHECKED BY:

ISSUED FOR

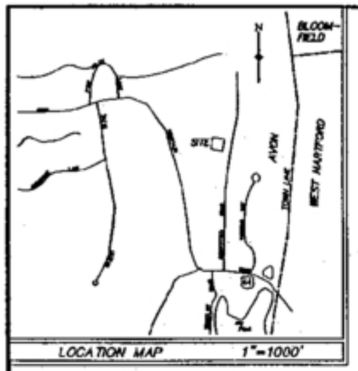
0	08/08/07	REVIEW
1	08/10/07	FINAL

AVON
81 MONTEVIDEO ROAD
AVON, CONNECTICUT

SCALE: NONE

TITLE SHEET-
GENERAL NOTES
AND LEGENDS

T-1



LEGEND

Bush — Mailbox
Catch Basin — Manhole
Coniferous Tree — Parking Meter
Deciduous Tree — Sign
Gas Gate — Utility Pole
Hydrant — Water Gate
Light Pole

N/F Avon Land Trust

N18°51'14"E 83.97'
N21°39'44"E 70.36'
N11°31'17"E 77.67'
N11°22'45"W 110.87'

Land of Monte LLC

N/F Scott Fanning & Kimberly Greer

N/F Ralph S. & Ellen H. Gobell

EXISTING TOWER LOCATION
LATITUDE – 41°-48'-11.00"
LONGITUDE – 72°-48'-04.95"
NAD 83
ELEVATION 747
NGVD 29

PROPOSED TOWER LOCATION
LATITUDE – 41°-48'-11.00"
LONGITUDE – 72°-48'-04.75"
NAD 83
ELEVATION 746
NGVD 29

Existing Building
Gravel Drive
Proposed Street Line per Map in Note 4A.
Stone Wall
Edge of Pavement
N10°43'14"W 47.03'
S11°55'14"E 34.21'
S07°08'14"W 74.41'
146.30' S10°08'44"W
325.03' N19°51'46"W 308.85'
313.46' S75°54'26"E

MONTEVIDEO ROAD
(PRIVATE ROAD)

GRAPHIC SCALE 1"=30'

NOTES:

1. THIS SURVEY AND MAP HAVE BEEN PREPARED IN ACCORDANCE WITH THE REGULATIONS OF CONNECTICUT STATE AGENCIES, SECTIONS 20-300b-1 THRU 20-300b-20, AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. ON SEPTEMBER 26, 1996. THE TYPE OF SURVEY IS A PROPERTY SURVEY AND A T-2 TOPOGRAPHIC SURVEY, THE BOUNDARY DETERMINATION CATEGORY IS A RESURVEY, THE HORIZONTAL AND VERTICAL ACCURACY CONFORMS TO CLASS A-2 & V-2 ACCURACY.
2. BEARINGS REFER TO THE CONNECTICUT COORDINATE SYSTEM (NAD 83) ESTABLISHED WITH GPS.
3. ELEVATIONS REFER TO THE NATIONAL GEODETIC VERTICAL DATUM OF 1929 (NGVD 29) BASED UPON CGS MONUMENT 7569 HOLDING THE PUBLISHED ELEVATION OF 314.17.
4. REFERENCE IS MADE TO THE FOLLOWING MAPS:
   A. "PROPERTY OF E. ALBERT LEHMER AND THEODORE NELSON MONTEVIDEO ROAD AVON, CONNECTICUT", BY F. PERRY CLOSE, SCALE 1"=100', DATED MAY 1994.
   B. MAP SHOWING LAND OWNED BY RICHARD W. & CAROLYN W. NELSON MONTEVIDEO ROAD AVON, CONNECTICUT", BY HEWITT WHITTEMORE, SCALE 1"=30', DATED FEBRUARY 1967.
   C. "PROPERTY OF HENRY M. ZIKONS ET AL 85 MONTEVIDEO ROAD AVON, CONNECTICUT", BY PETERSEN & NOTMAN, SCALE 1"=80', DATED AUGUST 1988, REVISED JUNE 1992.
5. THE PROPERTY IS TOGETHER WITH AND SUBJECT TO RIGHTS AND EASEMENTS AS DESCRIBED IN VOLUME 34, PAGE 241 OF THE AVON LAND RECORDS.
6. REFERENCE IS MADE TO A GRANT OF VARIANCE AND SPECIAL EXCEPTION RECORDED IN VOLUME 279, PAGE 140 OF THE AVON LAND RECORDS.
7. REFERENCE IS MADE TO A GRANT OF VARIANCE AND SPECIAL EXCEPTION RECORDED IN VOLUME 330, PAGE 728 OF THE AVON LAND RECORDS.
8. REFERENCE IS MADE TO A GRANT OF VARIANCE AND SPECIAL EXCEPTION RECORDED IN VOLUME 358, PAGE 88 OF THE AVON LAND RECORDS.
9. REFERENCE IS MADE TO A GRANT OF VARIANCE AND SPECIAL EXCEPTION RECORDED IN VOLUME 373, PAGE 361 OF THE AVON LAND RECORDS.
10. REFERENCE IS MADE TO A MEMORANDUM OF LICENSE AGREEMENT RECORDED IN VOLUME 406, PAGE 738 OF THE AVON LAND RECORDS.
11. REFERENCE IS MADE TO A MEMORANDUM OF LICENSE AGREEMENT RECORDED IN VOLUME 408, PAGE 731 OF THE AVON LAND RECORDS.
12. REFERENCE IS MADE TO A MEMORANDUM OF LICENSE AGREEMENT RECORDED IN VOLUME 439, PAGE 724 OF THE AVON LAND RECORDS.
13. UNDERGROUND UTILITY, STRUCTURE AND FACILITY LOCATIONS DEPICTED HEREON MAY BE BASED COMPILED, IN PART, FROM RECORD MAPPING AND OTHER DATA SUPPLIED BY THE RESPECTIVE UTILITY COMPANIES, GOVERNMENTAL AGENCIES AND/OR OTHER SOURCES. THESE LOCATIONS MUST BE CONSIDERED APPROXIMATE IN NATURE. ADDITIONALLY, OTHER SUCH UNDERGROUND FEATURES MAY EXIST ON THE SITE. THE EXISTENCE OF WHICH ARE UNKNOWN TO URS CORPORATION AES. THE EXISTENCE, SIZE, TYPE AND LOCATION OF ALL SUCH FEATURES MUST BE DETERMINED AND VERIFIED IN THE FIELD BY THE APPROPRIATE AUTHORITIES PRIOR TO CONSTRUCTION. CALL BEFORE YOU DIG 1-800-922-4455.

PROPERTY & TOPOGRAPHIC SURVEY
LAND OF
MONTE LLC
81 MONTEVIDEO ROAD
AVON, CONNECTICUT
PREPARED FOR
MESSAGE CENTER MANAGEMENT, INC.

No.	Date	Revision description
1	6-19-07	Proposed Tower Location Added

TO MY KNOWLEDGE AND BELIEF THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON

MICHAEL S. TALARICO, L.S. LICENSE NO. 14206

TRUE AND VALID COPIES OF THIS MAP OR PLAN MUST BEAR THE ORIGINAL SIGNATURE AND EMBOSSED SEAL OF THE ABOVE NAMED LAND SURVEYOR. UNAUTHORIZED REPRODUCTION OR ALTERATION IS FORBIDDEN.

Embossed seal

URS

Surveying and Mapping by:
URS Corporation AES
500 Enterprise Drive, Suite 3B
Rocky Hill, Connecticut 06067-4002
Tel. (860) 529-8882

Scale: 1" = 20'
Date: APRIL 2007

Field book #	Drew Chief	Project #
1659-52	M. TORRENT	36924643
Sheet #	Drawn by	Checked by	Map File #
4205	KC	MST	7155-3

S:\SURVEY\ACTIVE\36924643\SURVEY\36924643.DWG

SEDIMENTATION CONTROL FENCE SPECIFICATIONS

MAINTENANCE

1. SILT FENCES SHALL BE INSPECTED IMMEDIATELY AFTER EACH RAINFALL AND AT LEAST DAILY DURING PROLONGED RAINFALL. ANY REPAIRS THAT ARE REQUIRED SHALL BE MADE IMMEDIATELY.
2. IF THE FABRIC ON A SILT FENCE SHOULD DECOMPOSE OR BECOME INEFFECTIVE DURING THE EXPECTED LIFE OF THE FENCE, THE FABRIC SHALL BE REPLACED PROMPTLY.
3. SEDIMENT DEPOSITS SHOULD BE INSPECTED AFTER EVERY STORM EVENT. THE DEPOSITS SHOULD BE REMOVED WHEN THEY REACH APPROXIMATELY ONE-HALF THE HEIGHT OF THE BARRIER.
4. SEDIMENT DEPOSITS THAT ARE REMOVED OR LEFT IN PLACE AFTER THE FABRIC HAS BEEN REMOVED SHALL BE GRADED TO CONFORM WITH THE EXISTING TOPOGRAPHY AND VEGETATION.

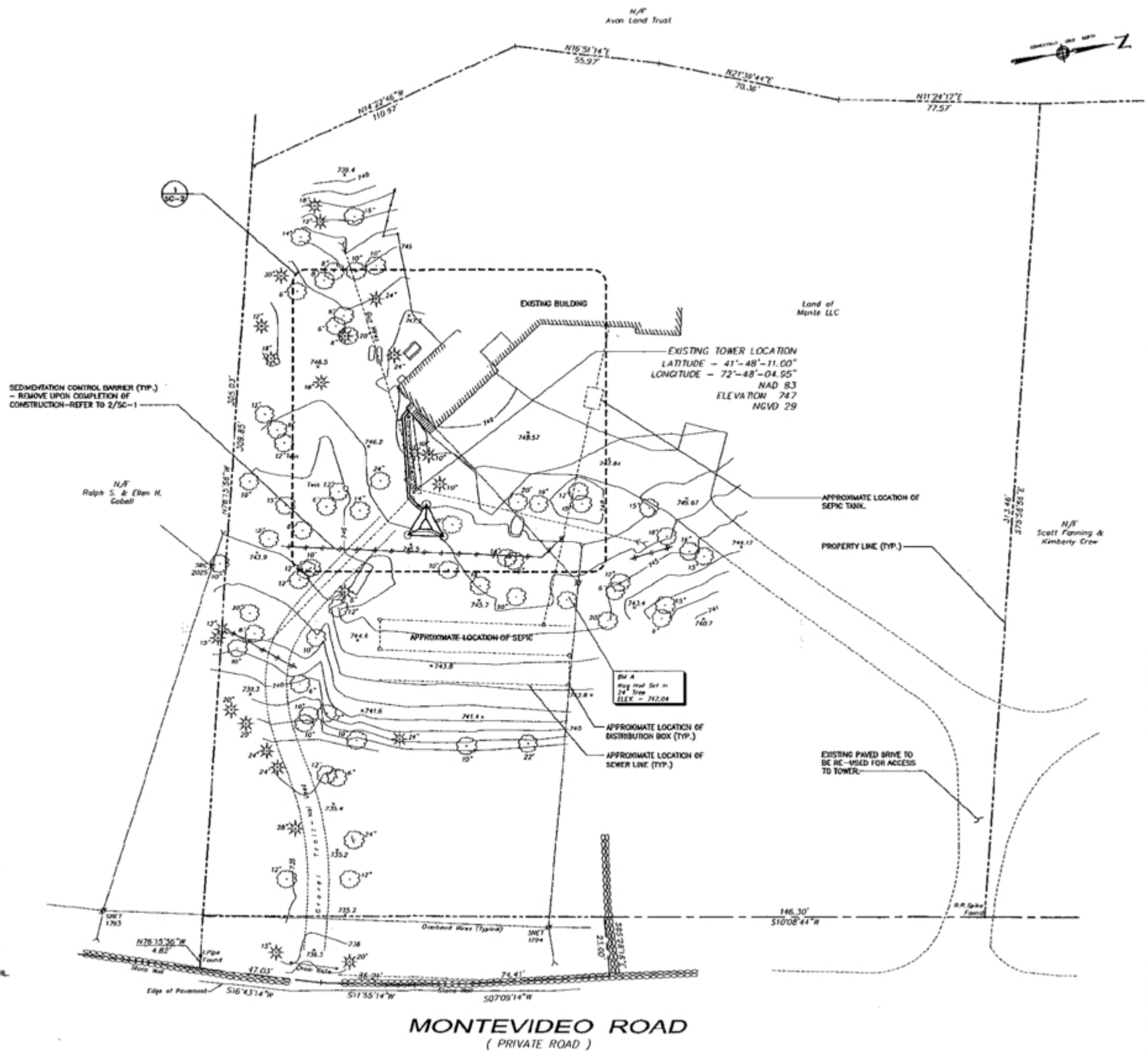
CONSTRUCTION SEQUENCE

1. THE GEOTEXTILE FABRIC SHALL MEET THE DESIGN CRITERIA FOR SILT FENCES
2. THE FABRIC SHALL BE EMBEDDED A MINIMUM OF 8 INCHES INTO THE GROUND AND THE SOIL COMPACTED OVER THE EMBEDDED FABRIC.
3. WOVEN WIRE FENCES SHALL BE FASTENED SECURELY TO THE FENCE POSTS WITH WIRE TIES OR STAPLES.
4. FILTER CLOTH SHALL BE FASTENED SECURELY TO THE WOVEN WIRE FENCE WITH TIES SPACED EVERY 24 INCHES AT THE TOP, MID-SECTION, AND BOTTOM.
5. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED BY 6 INCHES, FOLDED AND STAPLED.
6. FENCE POSTS SHALL BE A MINIMUM OF 36 INCHES LONG AND DRIVEN A MINIMUM OF 16 INCHES INTO THE GROUND. WOOD POSTS SHALL BE OF SOUND QUALITY HARDWOOD AND SHALL HAVE A MINIMUM CROSS SECTIONAL AREA OF 3.0 SQUARE INCHES.
7. MAINTENANCE SHALL BE PERFORMED AS NEEDED TO PREVENT BULGES IN THE SILT FENCE DUE TO DEPOSITION OF SEDIMENT.

EROSION CONTROL NOTES

1. DURING CONSTRUCTION AND THEREAFTER EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NOTED. NOT GREATER THAN 80,000 SQ. FT. OF LAND SHALL BE EXPOSED AT ANY ONE TIME DURING DEVELOPMENT. WHEN LAND IS EXPOSED DURING DEVELOPMENT, THE EXPOSURE SHOULD BE KEPT TO THE SHORTEST PRACTICAL PERIOD OF TIME AND SHALL NOT EXCEED 90 DAYS. LAND SHOULD NOT BE LEFT EXPOSED DURING THE WINTER MONTHS.
2. SILTATION FENCING SHALL BE INSTALLED WHERE SHOWN PRIOR TO ANY ON SITE GRADING OR DISTURBANCE OF EXISTING SURFACE MATERIAL. IT SHOULD BE MAINTAINED DURING AND AFTER DEVELOPMENT TO REMOVE SEDIMENT FROM RUNOFF WATER AND FROM LAND UNDERGOING DEVELOPMENT. WHERE POSSIBLE NATURAL DRAINAGE-WAYS SHOULD BE UTILIZED AND LEFT OPEN TO REMOVE EXCESS SURFACE WATER.
3. ALL DISTURBED AREAS AND SIDE SLOPES WHICH ARE FINISH GRADED WITH NO FURTHER CONSTRUCTION TO TAKE PLACE SHALL BE LOAMED AND SEEDED. A MINIMUM OF 4" OF LOAM SHALL BE INSTALLED.
4. ANY DISTURBED AREAS WHICH ARE TO BE LEFT TEMPORARILY, AND WHICH WILL BE REGRADED LATER DURING CONSTRUCTION SHALL BE MACHINE HAY MULCHED AND SEEDED WITH RYE GRASS TO PREVENT EROSION. HAY OR STRAW MULCH SHALL BE APPLIED TO ALL FRESHLY SEEDED AREAS AT A RATE OF 2 TONS PER ACRE. BALES SHALL BE UNSPOILED, AIR-DRIED, AND FREE FROM WEED, SEEDS AND ANY COARSE MATERIAL.

2 / SC-2 SEDIMENTATION CONTROL BARRIER - SILT FENCE
SCALE: N.T.S.



1 / SC-1 SITE PLAN
SCALE: 1" = 20'-0"
0 10 20 40

MESSAGE CENTER MANAGEMENT
40 WOODLAND STREET
HARTFORD, CONNECTICUT
06105

A&E FIRM
URS CORPORATION AES
500 ENTERPRISE DRIVE
ROCKY HILL, CONNECTICUT
1-(860)-529-6882

STATE OF CONNECTICUT
IGNACIO C. ARIAS
No. 8706
LICENSED ARCHITECT

PROJECT NO: 36924843
JOB NO: MCM 007
DRAWN BY: RRH
CHECKED BY:

ISSUED FOR		
0	08/08/07	REVIEW
1	08/10/07	FINAL

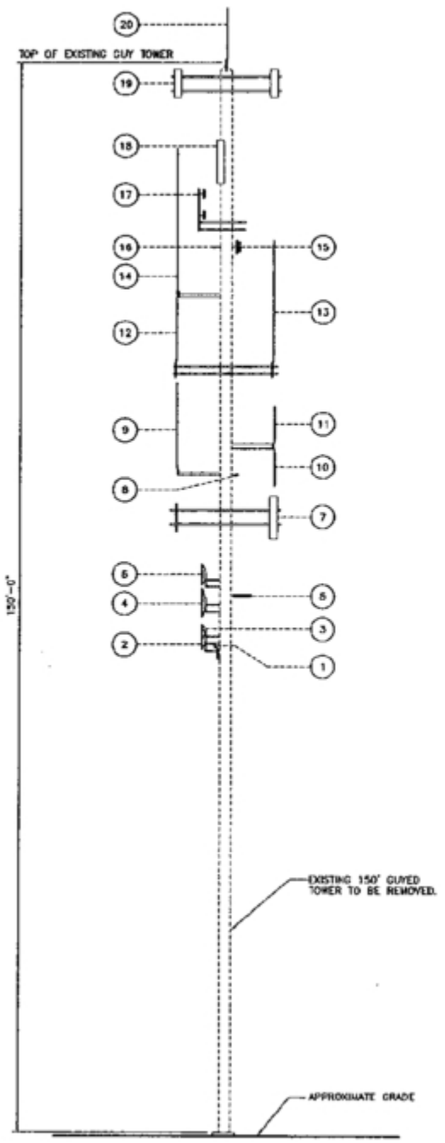
AVON
81 MONTEVIDEO ROAD
AVON, CONNECTICUT

SCALE: AS NOTED

SITE PLAN AND SEDIMENTATION DETAILS

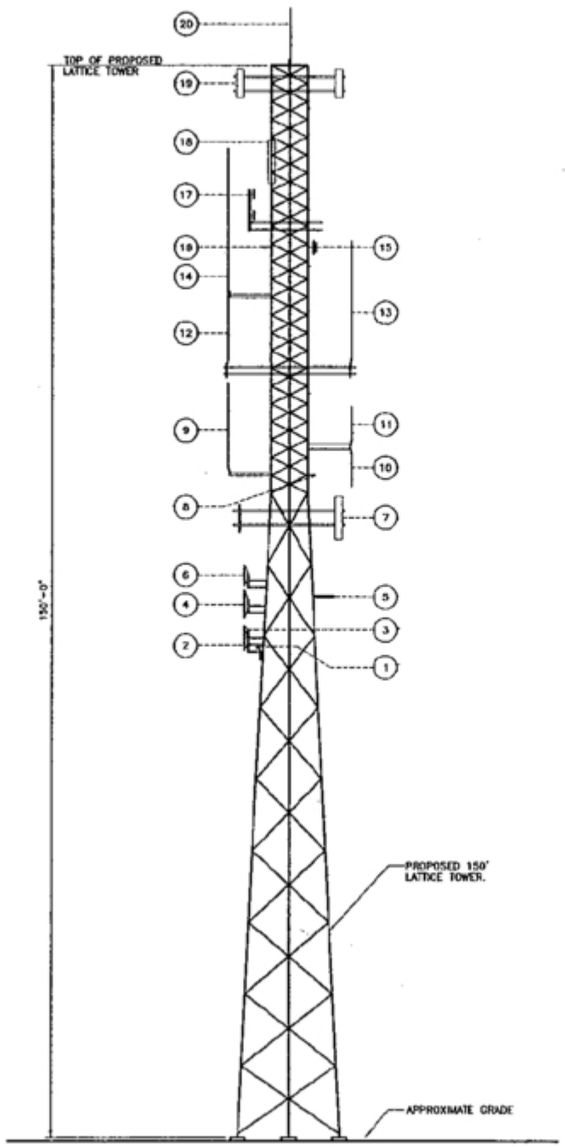
SC-1

LEGEND		
DESCRIPTION	EXISTING	PROPOSED
PROPERTY LINE		
LEASE LINE		
CHAIN LINK FENCE		
CONTOUR LINES	165	
UNDERGROUND UTILITIES		E/T
UTILITY POLE	ø	ø
TREE LINE		
SEDIMENTATION FENCE		



1. GUY WIRES NOT SHOWN FOR CLARITY.

3 SC-2 EXISTING TOWER ELEVATION
SCALE: 1" = 10'-0"



PROPOSED TOWER & FOUNDATION DESIGN BY OTHERS.

2 SC-2 PROPOSED TOWER ELEVATION
SCALE: 1" = 10'-0"

0 5 10 20 30

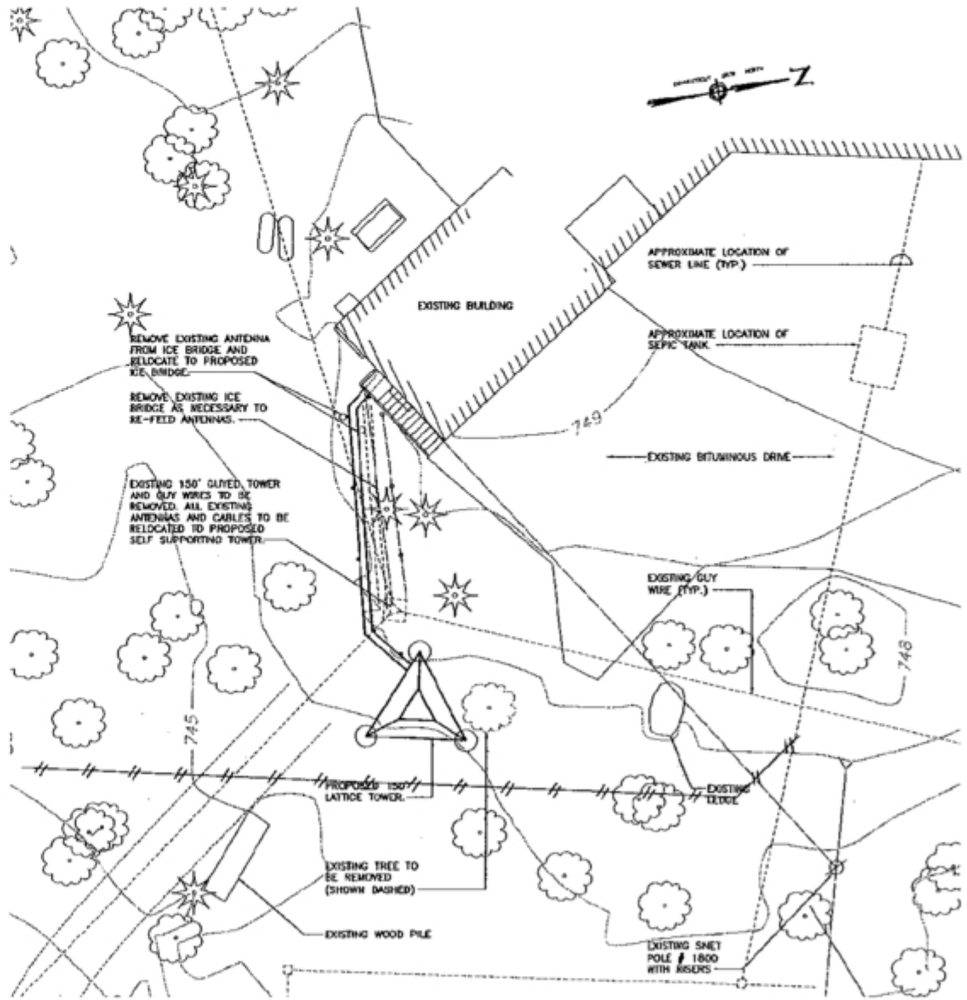
ANTENNA INFORMATION:

1. ANTENNA: LEG MOUNTED GPS ELEVATION: (AGL) 68'-0"
2. ANTENNA: 3' SOLID DISH ELEVATION: (AGL) 69'-0"
3. ANTENNA: 2' SOLID DISH ELEVATION: (AGL) 70'-0"
4. ANTENNA: 4' SOLID DISH ELEVATION: (AGL) 74'-0"
5. ANTENNA: (1) YAGI ON 3' SIDE ARM ELEVATION: (AGL) 75'-0"
6. ANTENNA: 3' SOLID DISH ELEVATION: (AGL) 78'-0"
7. ANTENNA: (1'x8') PANEL ANTENNA ON 6' SIDE ARM ELEVATION: (AGL) 86'-0"
8. ANTENNA: LEG MOUNTED GPS ELEVATION: (AGL) 92'-0"
9. ANTENNA: 12' WHIP ANTENNA ON A 6'-0" SIDE ARM ELEVATION: (AGL) 93'-0"
10. ANTENNA: 8' WHIP ANTENNA INVERTED ON EXISTING SIDE ARM ELEVATION: (AGL) 96'-0"

ANTENNA INFORMATION:

11. ANTENNA: 5' WHIP ANTENNA ON A 6'-0" SIDE ARM ELEVATION: (AGL) 98'-0"
12. ANTENNA: 10' WHIP ANTENNA ON A 6'-0" SIDE ARM ELEVATION: (AGL) 108'-0"
13. ANTENNA: 17' WHIP ANTENNA ON A 6'-0" SIDE ARM ELEVATION: (AGL) 108'-0"
14. ANTENNA: 20' WHIP ANTENNA ON A 6'-0" SIDE ARM ELEVATION: (AGL) 118'-0"
15. ANTENNA: (1) DB225 ON 6' SIDE ARM ELEVATION: (AGL) 124'-0"
16. ANTENNA: (1) DB8731H105 PANEL ANTENNA ON 3' SIDE ARM ELEVATION: (AGL) 124'-0"
17. ANTENNA: (1) DB222 ON 3' SIDE ARM ELEVATION: (AGL) 130'-0"
18. ANTENNA: (2) APX18-208513-C PANEL ANTENNAS ON (2) 6' SIDE ARM ELEVATION: (AGL) 136'-0"
19. ANTENNA: (3) 7187-05 PANEL ANTENNAS ON (3) 6' SIDE ARM ELEVATION: (AGL) 147'-0"
20. ANTENNA: (1) 8' WHIP ANTENNA (LEG MOUNTED) ELEVATION: (AGL) 150'-0"

NOTE: ALL MEASUREMENTS ARE ABOVE GROUND LEVEL.



1 SC-2 PARTIAL SITE PLAN
SCALE: 1" = 10'-0"

0 5 10 20 30

MESSAGE CENTER MANAGEMENT
40 WOODLAND STREET
HARTFORD, CONNECTICUT
06105

A&E FIRM
URS CORPORATION AES
500 ENTERPRISE DRIVE
ROCKY HILL, CONNECTICUT
1-(860)-529-8882

PROJECT NO: 36924843
JOB NO: MCM 007
DRAWN BY: RRH
CHECKED BY:

ISSUED FOR		
0	08/08/07	REVIEW
1	08/10/07	FINAL

AVON
81 MONTEVIDEO ROAD
AVON, CONNECTICUT

SCALE: AS NOTED

PARTIAL SITE PLAN AND TOWER ELEVATION

SC-2

Tower

Town of Avon, Conn.

BUILDING PERMIT

This card must be posted and visible from the road

08-0916 3-24-08 J. Williams

CERTIFICATE OF APPROVAL

TOWN OF AVON
Tel. 860.409.4316 Fax 860.409.4321
60 West Main Street Avon Connecticut 06001-3743

LOCATION OF JOB	GIS NO.	ZONE	TYPE OF PERMIT
81 MONTEVIDEO ROAD	**3180081**	**RU2A**	**BUILDING, ELECTRICAL**

OWNER	STREET	CITY	STATE & ZIPCODE
MONTE LLC	**40 WOODLAND STREET**	**HARTFORD**	**CT 06105**

THIS IS TO CERTIFY THAT REPLACEMENT OF EXISTING 150FT. TELECOMMUNICATIONS TOWER AND FENCE

DONE UNDER PERMIT # 08-1405,08-0916,08-0941 ISSUED 7/3/08,3/24/08,3/13/08

SUBSTANTIALLY COMPLIES WITH THE STATE OF CONNECTICUT BUILDING CODE IN EFFECT

DECEMBER 31, 2005, AND IS HEREBY APPROVED.

CONDITIONS OF APPROVAL: NONE

Zoning Approval 10-30-08

BUILDING OFFICIAL

FIRE MARSHAL APPROVAL N/A

DATE 11/3/08

Engineering Dept. Approval LTP 11/3/08

Wetlands Approval

DISTRIBUTION: ORIGINAL: OWNER COPY: ASSESSOR COPY: BUILDING DEPT.

Final Report of Special Inspections

Project: *Message Center Management: MCM-007 / 36924843*

Location: *81 Montevideo Road, Avon, Connecticut 06001*

Owner: *Message Center Management*

Owner's Address: *40 Woodland Street*
Hartford, CT 06105

Architect of Record: *URS Corporation*

Structural Engineer of Record: *Valmont Structures-Pirod, Inc.*
1545 Pidco Drive, Plymouth, Indiana 46563

To the best of my information, knowledge and belief, the Special Inspections required for this project, and itemized in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved other than the following:

Comments:

(Attach continuation sheets if required to complete the description of corrections.)

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,

Valmont Structures-Pirod, Inc., 1545 Pidco Drive
Plymouth, Indiana 46563

William R. Heiden III, P.E.
(Type or print name)

Signature

8-13-08
Date

STATE OF CONNECTICUT
WILLIAM HEIDEN III
No. 23038
LICENSED
PROFESSIONAL ENGINEER

AUG 13 2008

Licensed Professional Seal

Final Report of Special Inspections

Agent's Final Report

Project: *Message Center Management: MCM-007 / 36924843*
81 Montevideo Road, Avon, Connecticut 06001

Agent: *URS Corporation A.E.S., 500 Enterprise Drive, Rocky Hill, CT 06067*

Special Inspector: *URS Corporation*
Testing Agency: *JGI Eastern, Inc.*

To the best of my information, knowledge and belief, the Special Inspections or testing required for this project, and designated for this Agent in the *Statement of Special Inspections* submitted for permit, have been performed and all discovered discrepancies have been reported and resolved other than the following:

Comments:

(Attach continuation sheets if required to complete the description of corrections.)

Interim reports submitted prior to this final report form a basis for and are to be considered an integral part of this final report.

Respectfully submitted,
Agent of the Special Inspector
URS Corporation, 500 Enterprise Drive – Suite 3B
Rocky Hill, CT 06067

Richard Sambor, P.E.
(Type or print name)

Signature Date

Licensed Professional Seal or Certification

URS

SPECIAL INSPECTION REPORT #1 Date: 05/29/08

Client Name:	**Message center Management**
Site Address:	**81 Montevideo Road, Avon, CT**
Site Type:	**Construction of new U14.0x150' Valmont Lattice Tower**
Site Visit Type:	**Special Inspection**
Project Number:	**36924843.00000 / MCM-007**
Date of Visit:	**May 21, 2008**
Attendees:	**Jason Mead - URS Corporation** Ray Bayer - Construction Services of Branford, LLC Rob Olah, P.E., JGI Eastern, Inc/Terracon (Testing Agency)
Weather:	**52º Cloudy, 07:30am**
Prepared by:	**Jason Mead**
Progress / Activities:	**Foundation Sub-grade and Material Inspection**

The following was noted:

1. Sub-grade: Sub-grade material corresponded with that identified within the geotechnical report prepared by Clarence Welti, P.E., P.C, dated November 6th, 2007. Material consisted of moraine soils; fine to medium sand. Base was compacted with a hand-maneuvered vibratory compactor. Excavation was dry and free of deleterious materials. Contractor had installed a layer of 3/8" crushed stone at the base of the excavation, per the recommendations of the geotechnical report and was confirmed by JGI. Base of foundation excavation measured 6'-0"+/-. Finished grade shall differentiate by 1 foot vertically from East to West side of the tower base per URS design drawing C-1 and C-2 dated 01/25/08.

2. Rebar/Anchors: Rebar inventory was observed and found to be in conformance with the requirements of the Valmont design documents, i.e., size, quantity and material specification (ASTM A615, Grade 60). Rebar was supplied by Barker Steel Company, Inc. Anchor bolt assembly (x3) provided by the tower manufacturer (Valmont).

cc: Virginia King – Message Center Management
Jim Maher – Message Center Management
James Sansone – Building Official Town of Avon
Ray Bayer/John Centore, Construction Services of Branford, LLC
William R. Heiden III, P.E Valmont Structures

ICA/AA/CF/Book - URS

URS Corporation
500 Enterprise Drive, Suite 3B
Rocky Hill, CT 06067
Tel: 860.529.8882
Fax: 860.529.3991

URS

SPECIAL INSPECTION REPORT #2 Date: 05/29/08

Client Name:	**Message center Management**
Site Address:	**81 Montevideo Road, Avon, CT**
Site Type:	**Construction of new U14.0x150' Valmont Lattice Tower**
Site Visit Type:	**Special Inspection**
Project Number:	**36924843.00000 / MCM-007**
Date of Visit:	**May 27, 2008**
Attendees:	**Jason Mead - URS Corporation** Ray Bayer - Construction Services of Branford, LLC Fred Hart - JGI Eastern, Inc/Terracon (Testing Agency) Jim Maher – Message Center Management James Sansone – Building Official – Town of Avon
Weather:	**82º Sunny, 12:00noon**
Prepared by:	**Jason Mead**
Progress / Activities:	**Rebar Inspection and Concrete Placement**

The following was noted:

1. **Rebar/Anchors:** Rebar installation was observed and found to be in conformance with the requirements of the Valmont design documents, i.e., size, quantity, spacing and material specification (ASTM A615, Grade 60). Rebar was supplied by Barker Steel. Precast masonry blocks were utilized to attain 3" min bottom cover. Type 26, #5 standees were utilized in accordance with the Valmont design documents to support the top temperature steel from the bottom flexural reinforcement (main bars). Twelve (12) #12 vertical L bars were installed at each pier location and secured at top with one (1) #4 tie. Remaining ties shall be installed once main footing concrete has initially cured. URS noticed that the design documents prepared by Valmont did not include a provision for a minimum of two (2) #4 ties within the top 5in of the pier/column s, per ACI 318 Section 7.10.5.6. URS contacted Valmont to discuss discrepancy. Valmont concurred with URS that an additional tie shall be required. URS notified G.C and building official of this change. Anchor bolt assemblies (x3) were in place and leveled by laser.

2. **Concrete Placement:** Concrete specification utilized was 4000psi. Min design requirement was 3000psi. URS observed six deliveries out of a total of seven and received batch slips. First concrete delivery arrived at 1:25pm and concrete was placed approximately every 30 minutes thereafter.

URS Corporation
500 Enterprise Drive, Suite 3B
Rocky Hill, CT 06067
Tel: 860.529.8882
Fax: 860.529.3991

URS

Concrete Placement continued: Date: 05/29/08

Concrete was vibrated and evenly distributed. Slump tests and cylinder samples were performed by JGI Eastern, results of which shall be provided under separate cover. A seventh concrete delivery was required to finish the mat pour. Concrete top surface was leveled. Finished concrete was reportedly covered with a tarpaulin to prevent anticipated rain infiltration. URS departed from site at approximately 3:30pm.

cc: Virginia King – Message Center Management
Jim Maher – Message Center Management
James Sansone – Building Official Town of Avon
Ray Bayer/John Centore, Construction Services of Branford, LLC
William R. Heiden III, P.E Valmont Structures
ICA/AA/CF/Book - URS

URS

SPECIAL INSPECTION REPORT #3 Date: 05/29/08

Client Name:	**Message center Management**
Site Address:	**81 Montevideo Road, Avon, CT**
Site Type:	**Construction of new U14.0x150' Valmont Lattice Tower**
Site Visit Type:	**Special Inspection**
Project Number:	**36924843.00000 / MCM-007**
Date of Visit:	**May 28, 2008**
Attendees:	**Jason Mead - URS Corporation** Ray Bayer - Construction Services of Branford, LLC Jeff Chapo JGI Eastern, Inc/Terracon (Testing Agency) Jim Maher – Message Center Management James Sansone – Building Official – Town of Avon
Weather:	**65º Sunny, 11:00am**
Prepared by:	**Jason Mead**
Progress / Activities:	**Concrete Placement (Three piers)**

The following was noted:

Concrete Placement: Sonotube forms were installed and adjusted to maintain 3" min side cover from pier vertical rebar. Anchor bolts were checked for alignment and clearance. Two #4 binder ties installed within top 5" per ACI 318 Section 7.10.5.6 and 6" o.c thereafter, for a total of seven (7).Unitex Pro Poxy 204 medium viscosity, multi purpose two component epoxy bonding adhesive was applied to the hardened mat concrete prior to placing of pier concrete. Concrete specification utilized was 4000psi. Min design requirement was 3000psi. Concrete placement consisted of one delivery of approximately 6 cu yds. Batch slip checked for conformance and retained by G.C and JGI Eastern/Terracon. Concrete delivery arrived at 12:10pm. Concrete was placed and evenly distributed. Slump tests, cylinder samples and an air entrainment test were conducted by JGI Eastern, results of which shall be provided under separate cover. Concrete surface was floated and crowned to drain. All three pier concrete placement completed by 1:10pm. URS departed from site at 1:20pm.

cc: Virginia King – Message Center Management
Jim Maher – Message Center Management
James Sansone – Building Official Town of Avon
Ray Bayer/John Centore, Construction Services of Branford, LLC
William R. Heiden III, P.E Valmont Structures

ICA/AA/CF/Book - URS

URS Corporation
500 Enterprise Drive, Suite 3B
Rocky Hill, CT 06067
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Fax: 860.529.3991

EXHIBIT 6

AVON_1

81 MONTEVIDEO ROAD
AVON, CT 06001
HARTFORD COUNTY

SITE NO.: CT11284A

SITE TYPE: 150' SELF-SUPPRT TOWER

RF DESIGN GUIDELINE: 67D5A992DB OUTDOOR

APPROVALS

PROJECT MANAGER:	DATE:	ZONING/SITE ACQ.:	DATE:
CONSTRUCTION:	DATE:	OPERATIONS:	DATE:
RF ENGINEERING:	DATE:	TOWER OWNER:	DATE:

T-MOBILE TECHNICIAN SITE SAFETY NOTES

LOCATION	SPECIAL RESTRICTIONS
SECTOR A:	ACCESS BY CERTIFIED CLIMBER
SECTOR B:	ACCESS BY CERTIFIED CLIMBER
SECTOR C:	ACCESS BY CERTIFIED CLIMBER
SECTOR D:	ACCESS BY CERTIFIED CLIMBER
GPS/LMU:	UNRESTRICTED
RADIO CABINETS:	UNRESTRICTED
PPC DISCONNECT:	UNRESTRICTED
MAIN CIRCUIT D/C:	UNRESTRICTED
NIU/T DEMARC:	UNRESTRICTED
OTHER/SPECIAL:	NONE

SITE NOTES

1. THIS IS AN UNMANNED AND RESTRICTED ACCESS TELECOMMUNICATION FACILITY, AND IS NOT FOR HUMAN HABITATION. IT WILL BE USED FOR THE TRANSMISSION OF RADIO SIGNAL FOR THE PURPOSE OF PROVIDING PUBLIC CELLULAR SERVICE.
   - ADA COMPLIANCE NOT REQUIRED.
   - POTABLE WATER OR SANITARY SERVICE IS NOT REQUIRED.
   - NO OUTDOOR STORAGE OR ANY SOLID WASTE RECEPTACLES REQUIRED.
2. CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON JOB SITE. CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT/ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK. FAILURE TO NOTIFY THE ARCHITECT/ENGINEER PLACE THE RESPONSIBILITY ON THE CONTRACTOR TO CORRECT THE DISCREPANCIES AT THE CONTRACTOR'S EXPENSE.
3. NEW CONSTRUCTION WILL CONFORM TO ALL APPLICABLE CODES AND ORDINANCES.
   - BUILDING CODE: 2018 CONNECTICUT STATE BUILDING CODE
   - ELECTRICAL CODE: 2017 NATIONAL ELECTRICAL CODE
   - STRUCTURAL CODE: TIA/EIA-222-G STRUCTURAL STANDARDS FOR ANTENNA SUPPORTING STRUCTURES AND ANTENNAS.

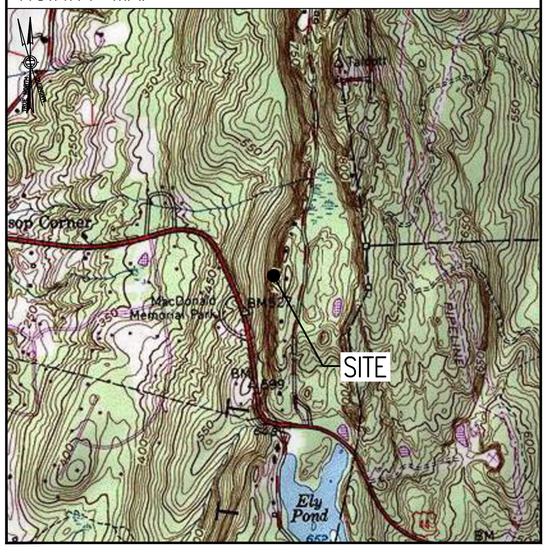
GENERAL NOTES

1. THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY, MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS, AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES.
2. THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONTRACT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMISSIONS OR ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF THESE DOCUMENTS.
3. THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE OMNIPOINT REPRESENTATIVE OF ANY CONFLICTS, ERRORS, OR OMISSIONS PRIOR TO THE SUBMISSION OF CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK. IN THE EVENT OF DISCREPANCIES THE CONTRACTOR SHALL PRICE THE MORE COSTLY OR EXTENSIVE WORK, UNLESS DIRECTED IN WRITING OTHERWISE.
4. THE SCOPE OF WORK SHALL INCLUDE FURNISHING ALL MATERIALS, EQUIPMENT, LABOR AND ALL OTHER MATERIALS AND LABOR DEEMED NECESSARY TO COMPLETE THE WORK/PROJECT AS DESCRIBED HEREIN.
5. THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
6. THE CONTRACTOR SHALL OBTAIN AUTHORIZATION TO PROCEED WITH CONSTRUCTION PRIOR TO STARTING WORK ON ANY ITEM NOT CLEARLY DEFINED BY THE CONSTRUCTION DRAWINGS/CONTRACT DOCUMENTS.
7. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS ACCORDING TO THE MANUFACTURER'S/VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES TAKE PRECEDENCE.
8. THE CONTRACTOR SHALL PROVIDE A FULL SET OF CONSTRUCTION DOCUMENTS AT THE SITE UPDATED WITH THE LATEST REVISIONS AND ADDENDUMS OR CLARIFICATIONS AVAILABLE FOR THE USE BY ALL PERSONNEL INVOLVED WITH THE PROJECT.
9. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.
10. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL NECESSARY CONSTRUCTION CONTROL SURVEYS, ESTABLISHING AND MAINTAINING ALL LINES AND GRADES REQUIRED TO CONSTRUCT ALL IMPROVEMENTS AS SHOWN HEREIN.
11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS WHICH MAY BE REQUIRED FOR THE WORK BY THE ARCHITECT/ENGINEER, THE STATE, COUNTY OR LOCAL GOVERNMENT AUTHORITY.
12. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS, EASEMENTS, PAVING, CURBING, ETC. DURING CONSTRUCTION. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL REPAIR ANY DAMAGE THAT MAY HAVE OCCURRED DUE TO CONSTRUCTION ON OR ABOUT THE PROPERTY.
13. THE CONTRACTOR SHALL KEEP THE GENERAL WORK AREA CLEAN AND HAZARD FREE DURING CONSTRUCTION AND DISPOSE OF ALL DIRT, DEBRIS, RUBBISH AND REMOVE EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY. PREMISES SHALL BE LEFT IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF ANY NATURE.
14. THE CONTRACTOR SHALL COMPLY WITH ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJECT.
15. THE CONTRACTOR SHALL NOTIFY THE PROJECT OWNER'S REPRESENTATIVE WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE LESSEE/LICENSEE REPRESENTATIVE.
16. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, PROPERTY LINES, ETC. ON THE JOB.
17. ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND EXISTING PLANS OF RECORD. THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK.

AT LEAST 72 HOURS PRIOR TO DIGGING, THE CONTRACTOR IS REQUIRED TO CALL DIG SAFE AT 811

VICINITY MAP

SCALE: 1" = 1000'-0"



DO NOT SCALE DRAWINGS

CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

SHEET INDEX

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A-4	EQUIPMENT DETAILS	1
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SPECIAL ZONING NOTE:
BASED ON INFORMATION PROVIDED BY T-MOBILE REGULATORY COMPLIANCE PROFESSIONALS AND LEGAL COUNSEL, THIS TELECOMMUNICATIONS EQUIPMENT DEPLOYMENT IS CONSIDERED AN ELIGIBLE FACILITY UNDER THE MIDDLE CLASS TAX RELIEF AND JOB CREATION ACT OF 2012, 47 USC 1455(A), SECTION 6409(A), AND IS SUBJECT TO AN ELIGIBLE FACILITY REQUEST, EXPEDITED REVIEW, AND LIMITED/PARTIAL ZONING PRE-EMPTION FOR LOCAL DISCRETIONARY PERMITS (VARIANCE, SPECIAL PERMIT, SITE PLAN REVIEW, OR ADMINISTRATIVE REVIEW).

PROJECT SUMMARY

SITE NUMBER:	CT11284A
SBA SITE NUMBER:	CT22071-A
SBA SITE NAME:	AVON (MONTEVIDEO)
SITE ADDRESS:	81 MONTEVIDEO ROAD AVON, CT 06001
PROPERTY OWNER:	MONTE LLC. 40 WOODLAND STREET HARTFORD, CT 06105
TOWER OWNER:	SBA TOWERS, LLC 8501 CONGRESS AVENUE BOCA RATON, FL 33487 PHONE: 561-226-9523
COUNTY:	HARTFORD
ZONING DISTRICT:	RU2A (RURAL RESIDENTAIL)
STRUCTURE TYPE:	SELF-SUPPRT TOWER
STRUCTURE HEIGHT:	150'±
APPLICANT:	T-MOBILE NORTHEAST LLC 15 COMMERCE WAY, SUITE B NORTON, MA 02766
SBA RSM:	STEPHEN ROTH PHONE: 860-539-4920 EMAIL: SRoth@sbasite.com
ARCHITECT:	CHAPPELL ENGINEERING ASSOCIATES, LLC. 201 BOSTON POST ROAD WEST, SUITE 101 MARLBOROUGH, MA 01752
STRUCTURAL ENGINEER:	CHAPPELL ENGINEERING ASSOCIATES, LLC. 201 BOSTON POST ROAD WEST, SUITE 101 MARLBOROUGH, MA 01752
SITE CONTROL POINT:	LATITUDE: N.41.8031° (41° 48' 11.00") LONGITUDE W.72.8013° (72° 48' 04.69")

T-MOBILE NORTHEAST LLC
15 COMMERCE WAY, SUITE B
NORTON, MA 02766
(508) 286-2700

SBA COMMUNICATIONS CORP.
134 FLANDERS ROAD, SUITE 125
WESTBOROUGH, MA 01581
(508) 251-0720

CHAPPELL ENGINEERING ASSOCIATES, LLC
Civil ·Structural ·Land Surveying
R.K. EXECUTIVE CENTRE
201 BOSTON POST ROAD WEST, SUITE 101
MARLBOROUGH, MA 01752
(508) 481-7400
www.chappellengineering.com

STATE OF CONNECTICUT
JAMES M. FITZGERALD
No. 25997
LICENSED
PROFESSIONAL CIVIL ENGINEER

CHECKED BY: JMT

APPROVED BY: JMT

SUBMITTALS

REV.	DATE	DESCRIPTION	BY
1	09/03/20	ISSUED FOR CONSTRUCTION	CMC
0	07/20/20	ISSUED FOR REVIEW	JRV

SITE NUMBER:
CT11284A

SITE ADDRESS:
81 MONTEVIDEO ROAD
AVON, CT 06001

SHEET TITLE
TITLE SHEET

SHEET NUMBER
T-1

1815.142

GENERAL NOTES:

1. FOR THE PURPOSE OF CONSTRUCTION DRAWINGS, THE FOLLOWING DEFINITIONS SHALL APPLY:
   CONTRACTOR – T–MOBILE
   SUBCONTRACTOR – GENERAL CONTRACTOR (CONSTRUCTION)
   OWNER – T–MOBILE
   OEM – ORIGINAL EQUIPMENT MANUFACTURER

2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.

3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK.

4. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL, STATE AND FEDERAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.

5. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.

6. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CONTRACTOR.

9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER, T1 CABLES AND GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR AND/OR LANDLORD PRIOR TO CONSTRUCTION.

10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE OWNER.

11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY.

12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION AND RETURN DISTURBED AREAS TO ORIGINAL CONDITIONS.

13. THE SUBCONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE SUBCONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.

14. SUBCONTRACTOR SHALL NOTIFY CHAPPELL ENGINEERING ASSOCIATES, LLC 48 HOURS IN ADVANCE OF POURING CONCRETE OR BACKFILLING TRENCHES, SEALING ROOF AND WALL PENETRATIONS AND POST DOWNS, FINISHING NEW WALLS OR FINAL ELECTRICAL CONNECTIONS FOR ENGINEERING REVIEW.

15. CONSTRUCTION SHALL COMPLY WITH ALL T–MOBILE STANDARDS AND SPECIFICATIONS.

16. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.

17. THE EXISTING CELL SITES ARE IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.

18. IF THE EXISTING CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

SITE WORK GENERAL NOTES:

1. THE SUBCONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.

2. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY ENGINEERS. EXTREME CAUTION SHOULD BE USED BY THE SUBCONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. SUBCONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION.

3. ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS AND PROJECT SPECIFICATIONS.

4. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.

5. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE BTS EQUIPMENT AND TOWER AREAS.

6. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

7. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.

8. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF ENGINEERING, OWNER AND/OR LOCAL UTILITIES.

9. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE AND STABILIZED TO PREVENT EROSION AS SPECIFIED IN THE PROJECT SPECIFICATIONS.

10. SUBCONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.

11. THE SUBCONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE T–MOBILE SPECIFICATION FOR SITE SIGNAGE.

CONCRETE AND REINFORCING STEEL NOTES:

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST–IN–PLACE CONCRETE.

2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE. A HIGHER STRENGTH (4000PSI) MAY BE USED. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 381 CODE REQUIREMENTS

3. REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE. SPLICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD, UNO.

4. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
   CONCRETE CAST AGAINST EARTH.....3 IN.
   CONCRETE EXPOSED TO EARTH OR WEATHER:
   #6 AND LARGER .....2 IN.
   #5 AND SMALLER & WWF .....1½ IN.
   CONCRETE NOT EXPOSED TO EARTH OR WEATHER
   OR NOT CAST AGAINST THE GROUND:
   SLAB AND WALL .....¾ IN.
   BEAMS AND COLUMNS .....1½ IN.

5. A CHAMFER ¾" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

6. INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHORS SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO THE MANUFACTURERS RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION/WEDGE ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED. EXPANSION BOLTS SHALL BE PROVIDED BY SIMPSON OR APPROVED EQUAL.

7. CONCRETE CYLINDER TIES ARE NOT REQUIRED FOR SLAB ON GRADE WHEN CONCRETE IS LESS THAN 50 CUBIC YARDS (IBC1905.6.2.3) IN THAT EVENT THE FOLLOWING RECORDS SHALL BE PROVIDED BY THE CONCRETE SUPPLIER;
   (A) RESULTS OF CONCRETE CYLINDER TEST PERFORMED AT THE SUPPLIERS PLANT.
   (B) CERTIFICATION OF MINIMUM COMPRESSIVE STRENGTH FOR THE CONCRETE GRADE SUPPLIED.
FOR GREATER THAN 50 CUBIC YARDS THE GC SHALL PERFORM THE CONCRETE CYLINDER TEST.

8. AS AN ALTERNATIVE TO ITEM 7. TEST CYLINDERS SHALL BE TAKEN INITIALLY AND THEREAFTER FOR EVERY 50 YARDS OF CONCRETE FROM EACH DIFFERENT BATCH PLANT.

9. EQUIPMENT SHALL NOT BE PLACED ON NEW PADS FOR SEVEN DAYS AFTER PAD IS POURED, UNLESS IT IS VERIFIED BY CYLINDER TESTS THAT COMPRESSIVE STRENGTH HAS BEEN ATTAINED.

STRUCTURAL STEEL NOTES:

1. ALL STEEL WORK SHALL BE PAINTED OR GALVANIZED IN ACCORDANCE WITH THE DRAWINGS AND T–MOBILE SPECIFICATIONS UNLESS OTHERWISE NOTED. STRUCTURAL STEEL SHALL BE ASTM–A–36 UNLESS OTHERWISE NOTED ON THE SITE SPECIFIC DRAWINGS. STEEL DESIGN, INSTALLATION AND BOLTING SHALL BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "MANUAL OF STEEL CONSTRUCTION".

2. ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND AWS D1.1. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION", 9TH EDITION. PAINTED SURFACES SHALL BE TOUCHED UP.

3. BOLTED CONNECTIONS SHALL USE BEARING TYPE ASTM A325 BOLTS (¾"ø) AND SHALL HAVE MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE GALVANIZED OR STAINLESS STEEL.

4. NON–STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE ⅝" DIA. ASTM A 307 BOLTS (GALV) UNLESS NOTED OTHERWISE.

5. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER REVIEW & APPROVAL ON PROJECTS REQUIRING STRUCTURAL STEEL

6. ALL STRUCTURAL STEEL WORK SHALL BE DONE IN ACCORDANCE WITH AISC SPECIFICATIONS.

SOIL COMPACTION NOTES FOR SLAB ON GRADE:

1. EXCAVATE AS REQUIRED TO REMOVE VEGETATION AND TOPSOIL TO EXPOSE NATURAL SUBGRADE AND PLACE CRUSHED STONE AS REQUIRED.

2. COMPACTION CERTIFICATION: AN INSPECTION AND WRITTEN CERTIFICATION BY A QUALIFIED GEOTECHNICAL TECHNICIAN OR ENGINEER IS ACCEPTABLE.

3. AS AN ALTERNATE TO INSPECTION AND WRITTEN CERTIFICATION, THE "UNDISTURBED SOIL" BASE SHALL BE COMPACTED WITH "COMPACTION EQUIPMENT", LISTED BELOW, TO AT LEAST 90% MODIFIED PROCTOR MAXIMUM DENSITY PER ASTM D 1557 METHOD C.

4. COMPACTED SUBBASE SHALL BE UNIFORM AND LEVELED. PROVIDE 6" MINIMUM CRUSHED STONE OR GRAVEL COMPACTED IN 3" LIFTS ABOVE COMPACTED SOIL. GRAVEL SHALL BE NATURAL OR CRUSHED WITH 100% PASSING #1 SIEVE.

5. AS AN ALTERNATE TO ITEMS 2 AND 3, THE SUBGRADE SOILS WITH 5 PASSES OR A MEDIUM SIZED VIBRATORY PLATE COMPACTOR (SUCH AS BOMAG BPR 30/38) OR HAND–OPERATED SINGLE DRUM VIBRATORY ROLLER (SUCH AS BOMAG BW 55E). AND SOFT AREAS THAT ARE ENCOUNTERED SHOULD BE REMOVED AND REPLACED WITH A WELL–GRADED GRANULAR FILL AND COMPACTED AS STATED ABOVE.

COMPACTION EQUIPMENT:

1. HAND OPERATED DOUBLE DRUN, VIBRATORY ROLLER, VIBRATORY PLATE COMPACTOR OR JUMPING JACK COMPACTOR.

CONSTRUCTION NOTES:

1. FIELD VERIFICATION:
SUBCONTRACTOR SHALL FIELD VERIFY SCOPE OF WORK, T–MOBILE ANTENNA PLATFORM LOCATION AND UTILITY TRENCHWORK.

2. COORDINATION OF WORK:
SUBCONTRACTOR SHALL COORDINATE RF WORK AND PROCEDURES WITH CONTRACTOR.

3. CABLE LADDER RACK:
SUBCONTRACTOR SHALL FURNISH AND INSTALL CABLE LADDER RACK, CABLE TRAY AND/OR ICE BRIDGE, AND CONDUIT AS REQUIRED TO SUPPORT CABLES TO THE NEW BTS LOCATION.

ELECTRICAL INSTALLATION NOTES:

1. WIRING, RACEWAY, AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC AND TELCORDIA.

2. SUBCONTRACTOR SHALL MODIFY OR INSTALL CABLE TRAY SYSTEM AS REQUIRED TO SUPPORT RF AND TRANSPORT CABLING TO THE NEW BTS EQUIPMENT. SUBCONTRACTOR SHALL SUBMIT MODIFICATIONS TO CONTRACTOR FOR APPROVAL.

3. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC AND TELCORDIA.

4. CABLES SHALL NOT BE ROUTED THROUGH LADDER–STYLE CABLE TRAY RUNGS.

5. EACH END OF EVERY POWER, GROUNDING, AND T1 CONDUCTOR AND CABLE SHALL BE LABELED WITH COLOR–CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA, AND MATCH INSTALLATION REQUIREMENTS.

6. POWER PHASE CONDUCTORS (I.E., HOTS) SHALL BE LABELED WITH COLOR–CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, ½ INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). PHASE CONDUCTOR COLOR CODES SHALL CONFORM WITH THE NEC AND OSHA.

7. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS. ALL EQUIPMENT SHALL BE LABELED WITH THEIR VOLTAGE RATING, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING, AND BRANCH CIRCUIT ID NUMBERS (I.E., PANELBOARD AND CIRCUIT ID'S).

8. PANELBOARDS (ID NUMBERS) AND INTERNAL CIRCUIT BREAKERS (CIRCUIT ID NUMBERS) SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS.

9. ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.

10. POWER, CONTROL, AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE CONDUCTOR (#34 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN–2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.

11. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE CONDUCTOR (#6 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN–2 GREEN INSULATION, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.

12. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED OUTDOORS, OR BELOW GRADE, SHALL BE SINGLE CONDUCTOR #2 AWG SOLID TINNED COPPER CABLE, UNLESS OTHERWISE SPECIFIED.

13. POWER AND CONTROL WIRING, NOT IN TUBING OR CONDUIT, SHALL BE MULTI–CONDUCTOR, TYPE TC CABLE (#34 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN–2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; WITH OUTER JACKET; LISTED OR LABELED FOR THE LOCATION USED, UNLESS OTHERWISE SPECIFIED.

14. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP–STYLE, COMPRESSION WIRE LUGS AND WIRENUTS BY HARGER (OR EQUAL). LUGS AND WIRENUTS SHALL BE RATED FOR OPERATION AT NO LESS THAN 75°C (90°C IF AVAILABLE).

15. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.

16. NEW RACEWAY OR CABLE TRAY WILL MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.

17. ELECTRICAL METALLIC TUBING (EMT) OR RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80 FOR LOCATIONS SUBJECT TO PHYSICAL DAMAGE) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

18. ELECTRICAL METALLIC TUBING (EMT), ELECTRICAL NONMETALLIC TUBING (ENT), OR RIGID NONMETALLIC CONDUIT (RIGID PVC, SCHEDULE 40) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.

19. GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE GRADE.

20. RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80) SHALL BE USED UNDERGROUND; DIRECT BURIED, IN AREAS OF OCCASIONAL LIGHT VEHICLE TRAFFIC OR ENCASED IN REINFORCED CONCRETE IN AREAS OF HEAVY VEHICLE TRAFFIC.

21. LIQUID–TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID–TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.

22. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION–TYPE AND APPROVED FOR THE LOCATION USED. SETSCREW FITTINGS ARE NOT ACCEPTABLE.

23. CABINETS, BOXES AND WIREWAYS SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.

24. CABINETS, BOXES AND WIREWAYS TO MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.

25. WIREWAYS SHALL BE EPOXY–COATED (GRAY) AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARD; SHALL BE PANDUIT TYPE E (OR EQUAL); AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.

26. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE GALVANIZED OR EPOXY–COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50, AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.

27. METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY–COATED, OR NON–CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.

28. NONMETALLIC RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.

29. THE SUBCONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CONTRACTOR BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.

30. THE SUBCONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD AGAINST LIFE AND PROPERTY.

31. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE LOCAL CODES.

32. CONDUIT ROUTINGS ARE SCHEMATIC. SUBCONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED.

T-MOBILE
NORTHEAST LLC

15 COMMERCE WAY, SUITE B
NORTON, MA 02766
(508) 286–2700

SBA COMMUNICATIONS CORP.
134 FLANDERS ROAD, SUITE 125
WESTBOROUGH, MA 01581
(508) 251–0720

CHAPPELL ENGINEERING ASSOCIATES, LLC
Civil ·Structural ·Land Surveying
R.K. EXECUTIVE CENTRE
201 BOSTON POST ROAD WEST, SUITE 101
MARLBOROUGH, MA 01752
(508) 481–7400
www.chappellengineering.com

STATE OF CONNECTICUT
JAMES M. FITZGERALD
No. 25997
LICENSED
PROFESSIONAL CIVIL ENGINEER

CHECKED BY: JMT

APPROVED BY: JMT

SUBMITTALS

REV.	DATE	DESCRIPTION	BY
1	09/03/20	ISSUED FOR CONSTRUCTION	CMC
0	07/20/20	ISSUED FOR REVIEW	JRV

SITE NUMBER:
CT11284A

SITE ADDRESS:
81 MONTEVIDEO ROAD
AVON, CT 06001

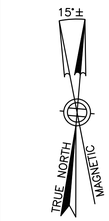
SHEET TITLE
GENERAL NOTES

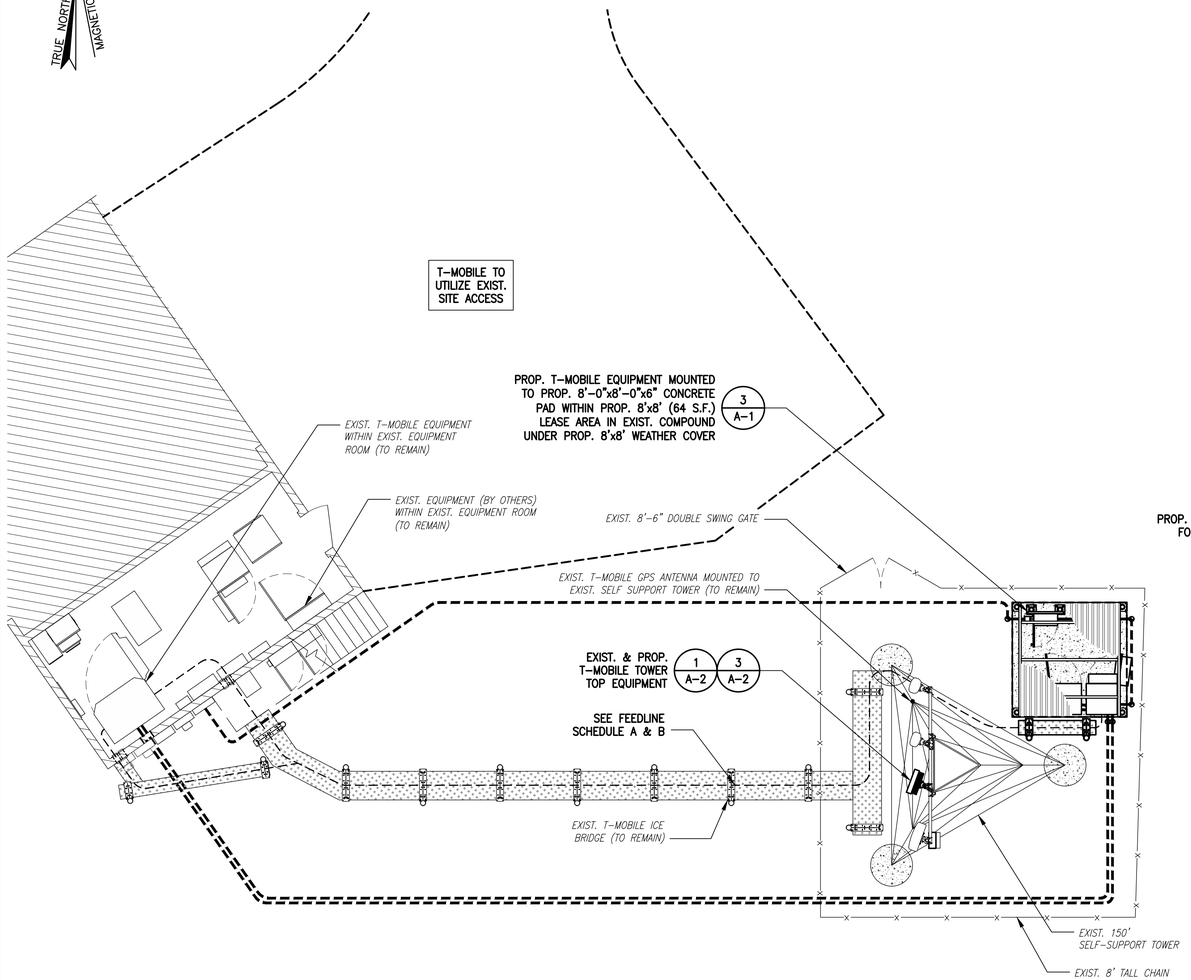
SHEET NUMBER
GN-1

1815.142

FEEDLINE SCHEDULE	FEEDLINES		LOCATION
A	*EXISTING TO REMAIN:*	*(2) 1-5/8" COAX CABLES* *(1) 1/2" COAX FOR GPS* *(1) 6x12 (1-3/8") HCS FIBER CABLE*	ROUTED PER TOWER STRUCTURAL ANALYSIS
	EXISTING TO BE REMOVED:	*(4) 1-5/8" COAX CABLES*	
B	**PROPOSED:**	**(1) 6x12 (1-5/8") HCS FIBER CABLES**	

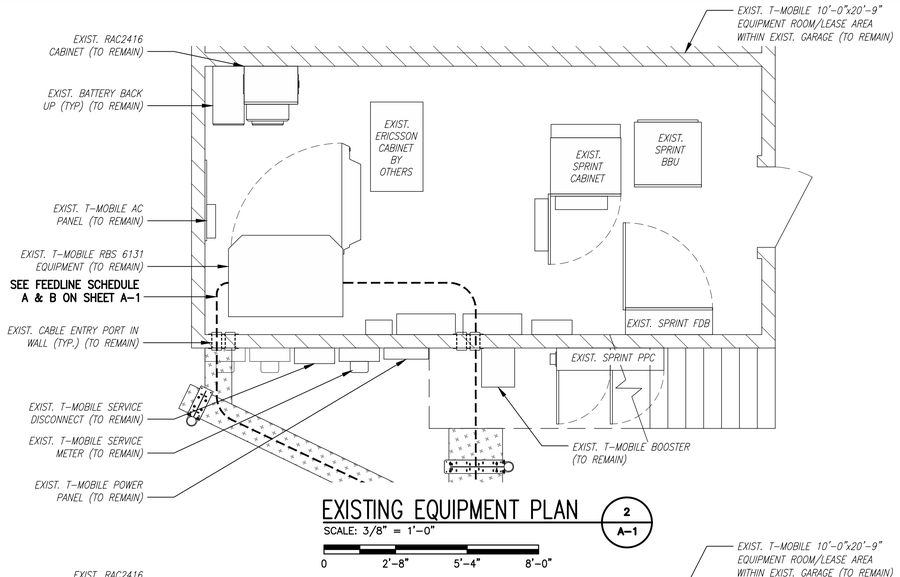
NOTE:
EXISTING T-MOBILE EQUIPMENT FEEDLINE INVENTORY BASED ON OBSERVED FIELD CONDITIONS. RFDS AND FEEDLINE LEASING ENTITLEMENTS MAY DIFFER.



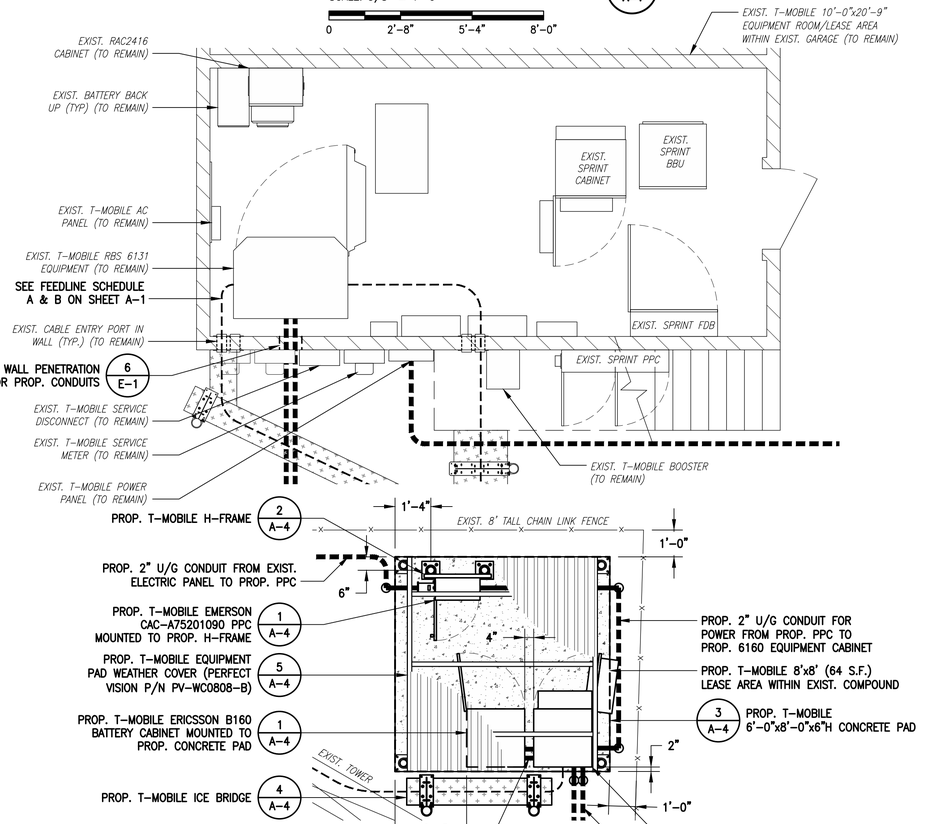


COMPOUND PLAN
1 A-1
SCALE: 1" = 5'-0"
0 5' 10' 15'

SPECIAL PRE-CONSTRUCTION WORK NOTE (SBA-PROVIDED TOWER STRUCTURAL ANALYSIS SPECIAL EQUIPMENT INSTALLATION REQUIREMENTS):
GENERAL CONTRACTOR SHALL FURNISH AND INSTALL ALL SPECIAL OR SUPPLEMENTAL ADDITIONAL TOWER-MOUNTED EQUIPMENT PER RECOMMENDATIONS FROM SBA-PROVIDED TOWER STRUCTURAL ANALYSIS FOR ANY SPECIAL SHIELDING OF TOWER TOP EQUIPMENT AND FOR ANY SPECIAL FEEDLINE BUNDLING OR RELOCATION.



EXISTING EQUIPMENT PLAN
2 A-1
SCALE: 3/8" = 1'-0"
0 2'-8" 5'-4" 8'-0"



PROPOSED EQUIPMENT PLAN
3 A-1
SCALE: 3/8" = 1'-0"
0 2'-8" 5'-4" 8'-0"

T-MOBILE NORTHEAST LLC
15 COMMERCE WAY, SUITE B
NORTON, MA 02766
(508) 286-2700

SBA COMMUNICATIONS CORP.
134 FLANDERS ROAD, SUITE 125
WESTBOROUGH, MA 01581
(508) 251-0720

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Civil ·Structural ·Land Surveying
R.K. EXECUTIVE CENTRE
201 BOSTON POST ROAD WEST, SUITE 101
MARLBOROUGH, MA 01752
(508) 481-7400
www.chappellengineering.com

STATE OF CONNECTICUT
JAMES M. FITZGERALD
No. 25937
LICENSED
PROFESSIONAL CIVIL ENGINEER

CHECKED BY: JMT

APPROVED BY: JMT

SUBMITTALS

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SITE ADDRESS:
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AVON, CT 06001

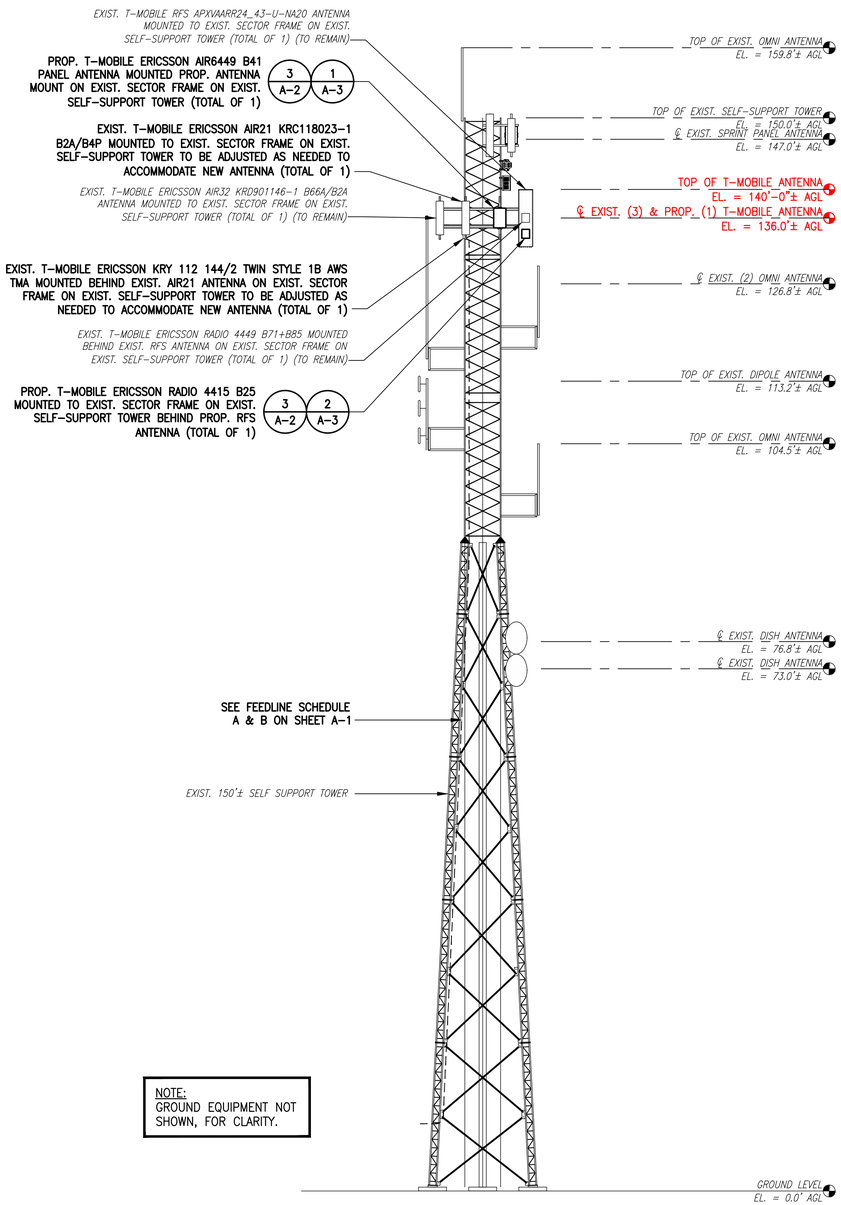
SHEET TITLE
COMPOUND & EQUIPMENT PLANS

SHEET NUMBER
A-1

1815.142

SPECIAL PRE-CONSTRUCTION WORK NOTE (SBA-PROVIDED TOWER STRUCTURAL ANALYSIS SPECIAL EQUIPMENT INSTALLATION REQUIREMENTS):
GENERAL CONTRACTOR SHALL FURNISH AND INSTALL ALL SPECIAL OR SUPPLEMENTAL ADDITIONAL TOWER-MOUNTED EQUIPMENT PER RECOMMENDATIONS FROM SBA-PROVIDED TOWER STRUCTURAL ANALYSIS FOR ANY SPECIAL SHIELDING OF TOWER TOP EQUIPMENT AND FOR ANY SPECIAL FEEDLINE BUNDLING OR RELOCATION.

RAD CENTER NOTE:
T-MOBILE RAD CENTER SHOWN IN RED TEXT BASED ON SBA-PROVIDED CO-LOCATION APPLICATION, EQUIPMENT DATABASE, AND STRUCTURAL ANALYSIS. THE SBA-PROVIDED ANTENNA RAD CENTER SHALL SUPERSEDE ANY CONFLICTING INFORMATION DERIVED FROM THE T-MOBILE RFDS.



NOTE:
GROUND EQUIPMENT NOT SHOWN, FOR CLARITY.

SELF-SUPPORT TOWER ELEVATION 1 A-2

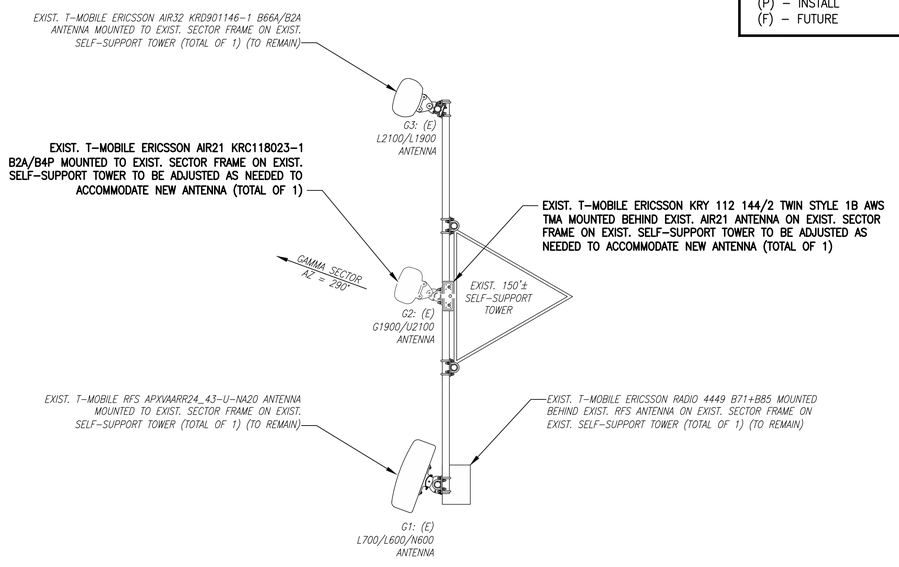
SCALE: 1" = 10'

0 5' 10' 20' 30'

ANTENNA STATUS LEGEND:

EMPTY – EMPTY PIPE

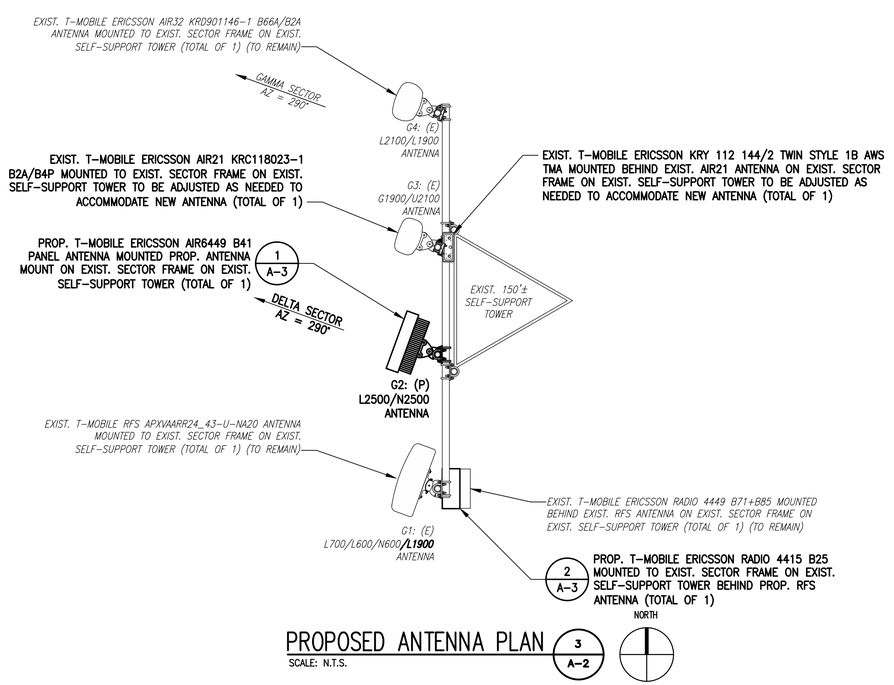
(E) – EXISTING
(P) – INSTALL
(F) – FUTURE



EXISTING ANTENNA PLAN 2 A-2

SCALE: N.T.S.

NORTH



PROPOSED ANTENNA PLAN 3 A-2

SCALE: N.T.S.

NORTH

GENERAL CONTRACTOR SHALL REFER TO MOUNT STRUCTURAL ANALYSIS AND ANY MOUNT MODIFICATION DESIGN PROVIDED BY SBA

NOTE:
VERIFY PROPOSED AZIMUTHS WITH RF ENGINEER PRIOR TO INSTALLATION.

T-MOBILE NORTHEAST LLC
15 COMMERCE WAY, SUITE B
NORTON, MA 02766
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WESTBOROUGH, MA 01581
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SHEET TITLE
TOWER ELEVATION & ANTENNA PLANS

SHEET NUMBER
A-2

1815.142

FINAL ANTENNA CONFIGURATION

SECTOR	ANTENNA	RAD CENTER	AZIMUTH (TRUE NORTH)	MECHANICAL DOWNTILT	ELECTRICAL DOWNTILT	BAND	TMA/RADIOS	CABLES
GAMMA	*RFS APXVAARR24_43-U-NA20*	*136'-0"± AGL*	*290°*	*0°*	*6°*	*L600/N600/L700*	*ERICSSON RADIO 4449 B71+B85*	*(2) 1-5/8" COAX CABLES* *(1) 6x12 (1-5/8") HCS FIBER CABLE* **(1) 6x12 (1-5/8") HCS FIBER CABLES**
					6°	**L1900**	**ERICSSON RADIO 4415 B25**	
	ERICSSON M-MIMO AIR6449 B41	**136'-0"± AGL**	**290°**	**0°**	**0°**	**L2500/N2500**	**–**	
	ERICSSON AIR21 KRC118023-1 B2A/B4P	*136'-0"± AGL*	*290°*	*0°*	*4°*	*G1900/U2100*	*ERICSSON KRY 112 144/2 TWIN STYLE 1B AWS TMA*	
	ERICSSON AIR32 KRD901046-1 B66A/B2A	*136'-0"± AGL*	*290°*	*0°*	*6°*	*L2100/L1900*	*–*	

CABLE NOTE: EXISTING (4) 1-5/8" COAX CABLES TO BE REMOVED. SEE FEEDLINE SCHEDULE A & B ON SHEET A-1.

NOTE: RFDS REV6 – 05/11/20

ERICSSON M-MIMO AIR6449 B41 PANEL ANTENNA
DIMENSIONS: 33.1"H x 20.5"W x 8.3"D
WEIGHT: 103.0 lbs
QUANTITY: TOTAL OF 1

ANTENNA DETAILS (1 / A-3)
SCALE: N.T.S.

ERICSSON RADIO 4415 B25
DIMENSIONS: 16.5"H x 13.4"W x 5.9"D
WEIGHT: 46 LBS
QUANTITY: TOTAL OF 1

RADIO DETAILS (2 / A-3)
SCALE: N.T.S.

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15 COMMERCE WAY, SUITE B
NORTON, MA 02766
(508) 286-2700

SBA COMMUNICATIONS CORP.
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WESTBOROUGH, MA 01581
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www.chappellengineering.com

STATE OF CONNECTICUT
JAMES M. FITZGERALD
No. 25997
LICENSED PROFESSIONAL CIVIL ENGINEER

CHECKED BY: JMT

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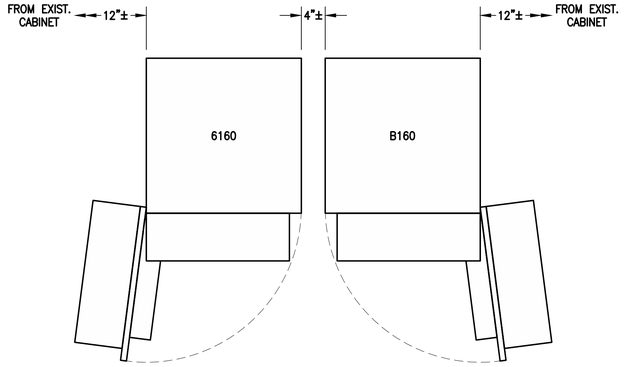
SHEET TITLE
ANTENNA DETAILS

SHEET NUMBER
A-3

1815.142

ERICSSON CAC-A75201090 PPC

DIMENSIONS: 40.0"H x 20.0"W x 10.0"D
QUANTITY: TOTAL OF 1



ERICSSON 6160 SITE SUPPORT CABINET

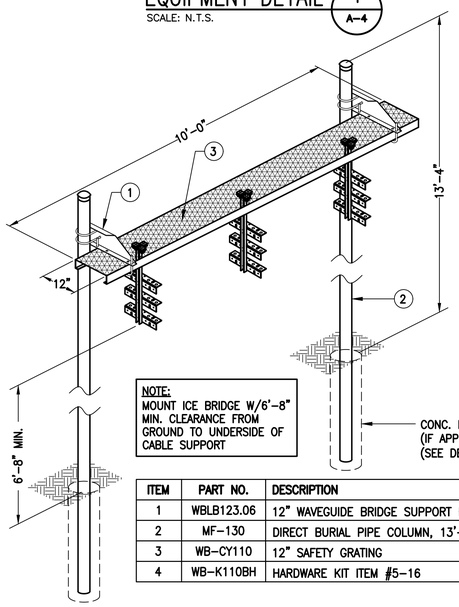
DIMENSIONS: 63.25"H x 26.0"W x 34.0"D
QUANTITY: TOTAL OF 1

ERICSSON B160 BATTERY CABINET

DIMENSIONS: 63.25"H x 26.0"W x 34.0"D
QUANTITY: TOTAL OF 1

EQUIPMENT DETAIL (1 / A-4)

SCALE: N.T.S.

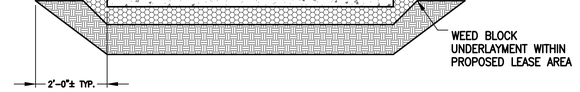


NOTE:
MOUNT ICE BRIDGE W/6'-8" MIN. CLEARANCE FROM GROUND TO UNDERSIDE OF CABLE SUPPORT

ITEM	PART NO.	DESCRIPTION	QTY.
1	WBLB123.06	12" WAVEGUIDE BRIDGE SUPPORT BRACKET	2
2	MF-130	DIRECT BURIAL PIPE COLUMN, 13'-4"	2
3	WB-CY110	12" SAFETY GRATING	1
4	WB-K110BH	HARDWARE KIT ITEM #5-16	1

CABLE BRIDGE DETAIL (4 / A-4)

SCALE: NTS



CONCRETE PAD DETAIL (3 / A-4)

SCALE: 1/2" = 1'-0"

CONCRETE GENERAL NOTES

1. ALL CONCRETE WORK SHALL CONFORM TO ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND TO THE PROJECT SPECIFICATIONS.
2. ALL CONCRETE IS TO BE NORMAL DENSITY CONCRETE WITH A MAXIMUM SLUMP OF 4 INCHES. MAXIMUM AGGREGATE SIZE ¾ INCH. NO ADDITIONAL WATER SHALL BE ADDED TO THE CONCRETE AT THE JOB SITE.
3. PROVIDE AIR ENTRAINMENT OF 4 TO 6 PERCENT IN ALL EXPOSED CONCRETE WORK WITH AIR-ENTRAINING ADMIXTURE COMPLYING WITH ASTM C 260. AT TROWEL-FINISHED FLOORS, DO NOT EXCEED AIR-ENTRAINMENT CONTENT OF 3 PERCENT.
4. NO HOLES OR SLEEVES SHALL BE MADE THROUGH CONCRETE WORK OTHER THAN THOSE INDICATED ON THE STRUCTURAL DRAWINGS WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.
5. ALL FORMWORK OFFSET TOLERANCES (PER ACI 117) TO BE CLASS A.
6. FLOOR SLAB TOLERANCES TO ASTM E1155; SPECIFIED OVERALL MINIMUM VALUE OF FLATNESS F F=25 WITH LOCAL MINIMUM F F=17, AND MINIMUM VALUE OF LEVELNESS F F=20 WITH LOCAL MINIMUM F I AND F F WITHIN 72 HOURS OF SLAB CONSTRUCTION.
7. CABINETS ON SLAB (IF APPLICABLE). ALLOWABLE CAPACITY OF CONCRETE USED IN DESIGN MIN. 4000 PSI.

FOUNDATION NOTES:

1. DESIGN INFORMATION AND GENERAL REQUIREMENTS

1.1 CODES

A. DESIGN CONFORMS TO INTERNATIONAL BUILDING CODE 2012.

B. AMERICAN CONCRETE INSTITUTE "BUILDING COADE REQUIREMENTS FOR REINFORCED CONCRETE," ACI 318-08.

2. EARTHWORK

2.1 FOUNDATIONS

A. FOUNDATIONS HAVE BEEN DESIGNED TO BEAR ON (UNDISTURBED RESIDUAL SOILS/COMPACTED STRUCTURAL FILL), CAPABLE OF SAFELY SUPPORTING A NET ALLOWABLE BEARING PRESSURE OF 2000 PSF. IF FOUNDATION CONDITIONS PROVE UNACCEPTABLE AT ELEVATIONS SHOWN, EXCAVATION SHALL BE CARRIED DEEPER AND SHALL BE BACKFILLED WITH LEAN CONCRETE TO PLAN FOOTING BOTTOM, OR REDESIGN OF FOUNDATIONS WILL BE REQUIRED AT THE DIRECTION OF THE ENGINEER.

B. DESIGN, FURNISH AND INSTALL ALL TEMPORARY SHEETING, SHORING AND DRAINAGE NECESSARY TO MAINTAIN THE EXCAVATION AND PROTECT SURROUNDING STRUCTURES AND UTILITIES.

C. THOROUGHLY COMPACT ALL BOTTOM OF FOOTINGS PRIOR TO PLACING ANY CONCRETE.

3. CONCRETE

3.1 FORMWORK

A. CONCRETE CONSTRUCTION SHALL CONFORM TO "SPECIFICATIONS FOR STRUCTURAL, CONCRETE FOR BUILDINGS," (ACI 301-89).

B. FORMWORK SHALL CONFORM TO ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS."

3.2 REINFORCEMENT

A. REINFORCING STEEL ASTM A615, GRADE 60. WELDED WIRE ASTM A185 (FLAT SHEET). LAPS 40 BAR DIAMETERS UNLESS NOTED. BARS SHALL BE SECURELY HELD IN ACCURATE POSITION BY SUITABLE ACCESSORIES, TIE BARS, SUPPORT BARS, ETC. HOOK LENGTHS SHALL BE 12 BAR DIAMETERS.

B. CONCRETE COVER FOR REINFORCING BARS SHALL BE AS FOLLOWS, UNLESS OTHERWISE NOTED:

FOOTINGS & SLABS CAST AGAINST GROUND	3"
CONCRETE TO BE IN CONTACT WITH GROUND OR WEATHER AT BARS GREATER THAN #5	2"
AT BARS #5 OR LESS	1-1/2"
CONCRETE NOT TO BE EXPOSED TO GROUND OR WEATHER BEAMS, GIRDERS & COLUMNS	1-1/2"
SLABS & WALLS	3/4"

3.3 CAST-IN-PLACE-CONCRETE

A. MINIMUM 28 DAY CYLINDER STRENGHT AND MAXIMUM SLUMP, PRIOR TO ADDITION OF SUPER PLASTICIZERS, AS FOLLOWS:

	F'C (PSI)	SLUMP
CLASS I FOOTINGS	4000	3"
CLASS II FOOTINGS	4000	3"
CLASS III INTERIOR ELEVATED SLABS & WALLS	4000	4"
CLASS V OTHER WORK	4000	4"
CLASS VI LEAN CONCRETE FOR OVER EXCAVATION OF FOUNDATIONS	2000	N/A

B. MIX DESIGN TO BE IN ACCORDANCE WITH ACI 318, CHAPTER 5. NO CALCIUM CHLORIDE OR ADMIXTURE CONTAINING CHLORIDES SHALL BE USED IN ANY CONCRETE.

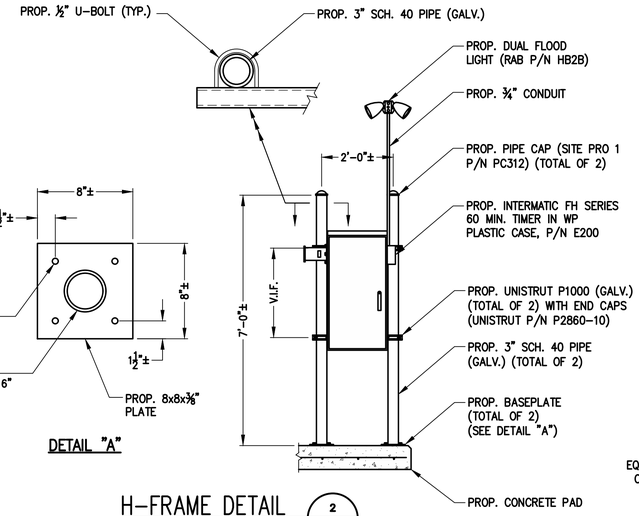
C. COARSE AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C33 SIZE #57. COARSE AGGREGATE FOR LIGHT WEIGHT CONCRETE SHALL CONFORM TO ASTM C330 GRADED 3/4" TO 1/4".

D. COLD WEATHER PLACEMENT SHALL COMPLY WITH ACI 306.1.

E. HOT WEATHER PLACEMENT SHALL COMPLY WITH ACI 305 R.

F. CHAMFER ALL EXPOSED EDGES 3/4".

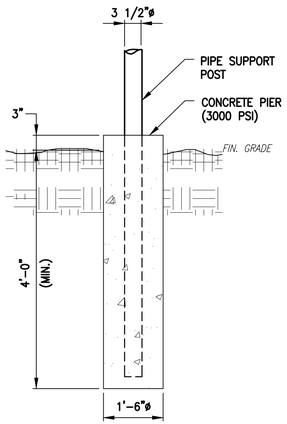
G. THE MAXIMUM TEMPERATURE OF ALL CONCRETE AT DELIVERY TO THE SITE SHALL BE 85°F. TOTAL DELIVERY TIME SHALL BE LESS THAN 75 MINUTES.



DETAIL "A"

H-FRAME DETAIL (2 / A-4)

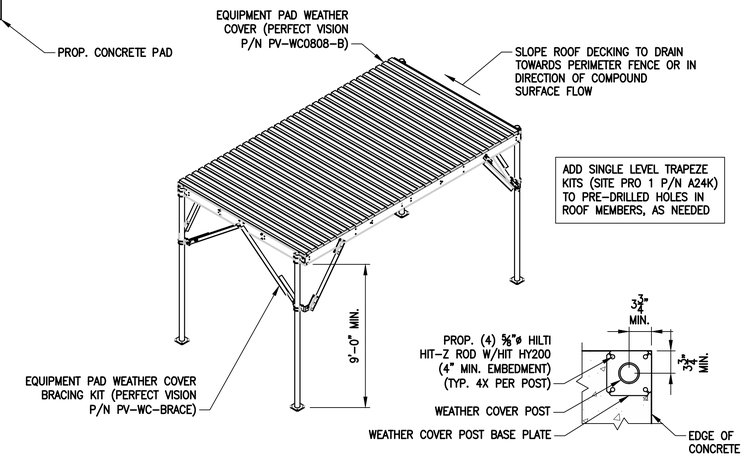
SCALE: N.T.S.



NOTES:

1. FOR EXPOSED LEDGE, PROVIDE GROUT LEVELING PAD. INSTALL (2)-⅝"ø EXPANSION ANCHORS, (6" LONG).
2. FOR BURIED LEDGE AT LESS THAN 3'-6" BELOW FINISH GRADE, CORE 8"ø HOLE INTO LEDGE 18" DEEP. FILL AROUND PIPE WITH NON-SHRINK GROUT. USE COAL TAR ON BURIED LENGTH OF PIPE, AND BACKFILL TO FINISH GRADE
3. FOR CONCRETE, FASTEN BASEPLATE WITH (2)-⅝"ø EXPANSION ANCHORS, (6" LONG).
4. FOR POSTS ON CONCRETE OR EXPOSED LEDGE, PROVIDE 4"x8"x⅜" BASE PLATE WITH (2)-1⅛"ø HOLES @ 6" O.C.

DETAIL "B"



EQUIPMENT PAD WEATHER COVER DETAIL (5 / A-4)

SCALE: N.T.S.

T-MOBILE NORTHEAST LLC

15 COMMERCE WAY, SUITE B
NORTON, MA 02766
(508) 286-2700

SBA COMMUNICATIONS CORP.
134 FLANDERS ROAD, SUITE 125
WESTBOROUGH, MA 01581
(508) 251-0720

CHAPPELL ENGINEERING ASSOCIATES, LLC
Civil ·Structural ·Land Surveying
R.K. EXECUTIVE CENTRE
201 BOSTON POST ROAD WEST, SUITE 101
MARLBOROUGH, MA 01752
(508) 481-7400
www.chappellengineering.com

STATE OF CONNECTICUT
JAMES M. FITZGERALD
No. 25997
LICENSED
PROFESSIONAL CIVIL ENGINEER

CHECKED BY: JMT

APPROVED BY: JMT

SUBMITTALS

REV.	DATE	DESCRIPTION	BY
1	09/03/20	ISSUED FOR CONSTRUCTION	CMC
0	07/20/20	ISSUED FOR REVIEW	JRV

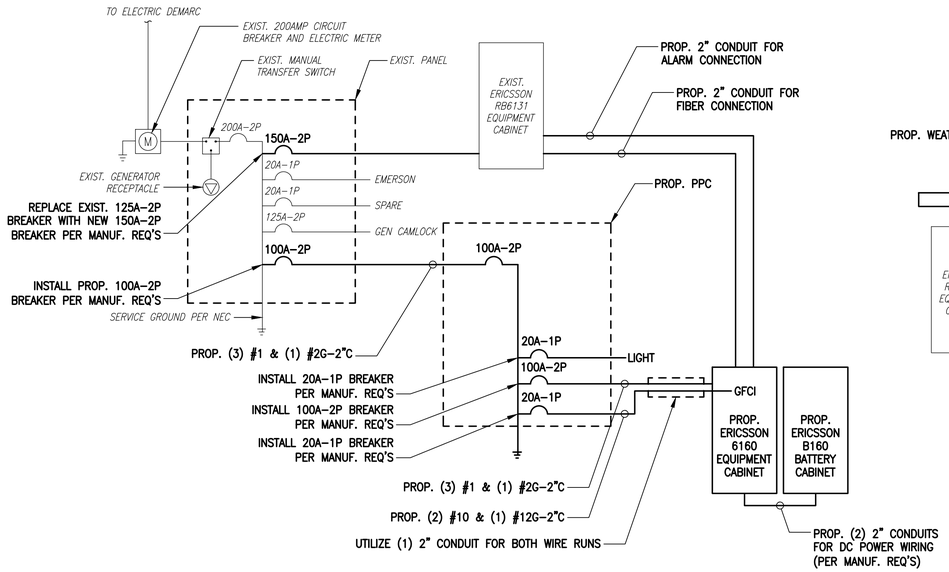
SITE NUMBER:
CT11284A

SITE ADDRESS:
81 MONTEVIDEO ROAD
AVON, CT 06001

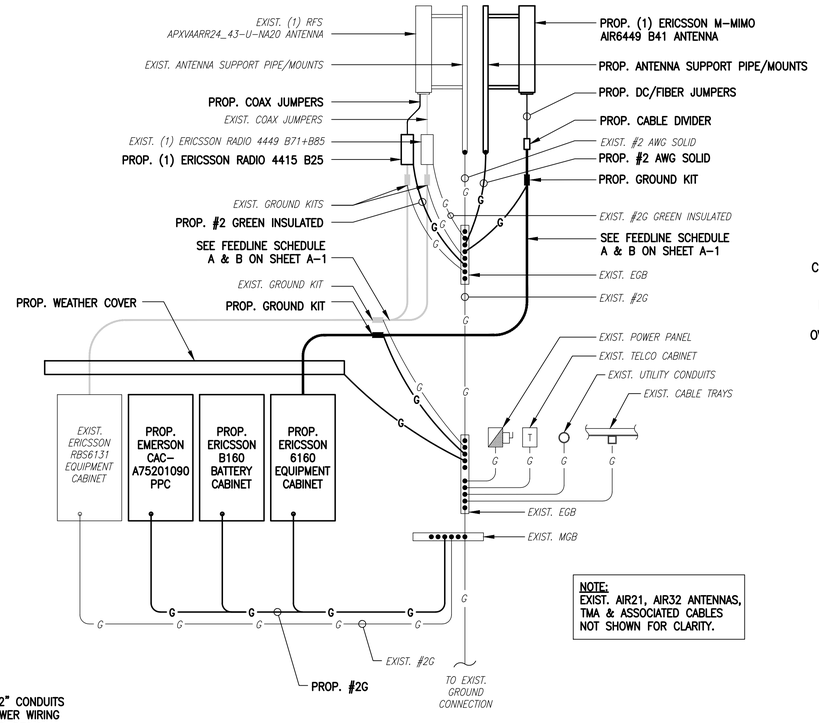
SHEET TITLE
EQUIPMENT DETAILS

SHEET NUMBER
A-4

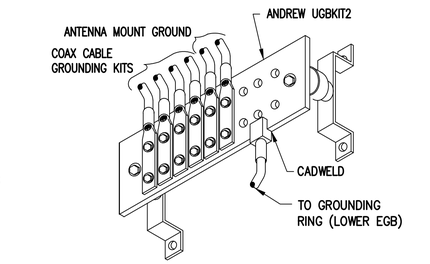
1815.142



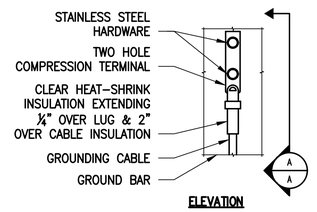
ONE LINE DIAGRAM (1 / E-1)
SCALE: NOT TO SCALE



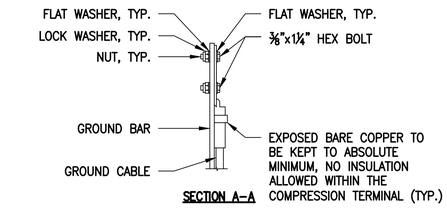
GROUNDING RISER DIAGRAM (2 / E-1)
SCALE: NOT TO SCALE



GROUND BAR (EGB) (5 / E-1)
SCALE: NOT TO SCALE



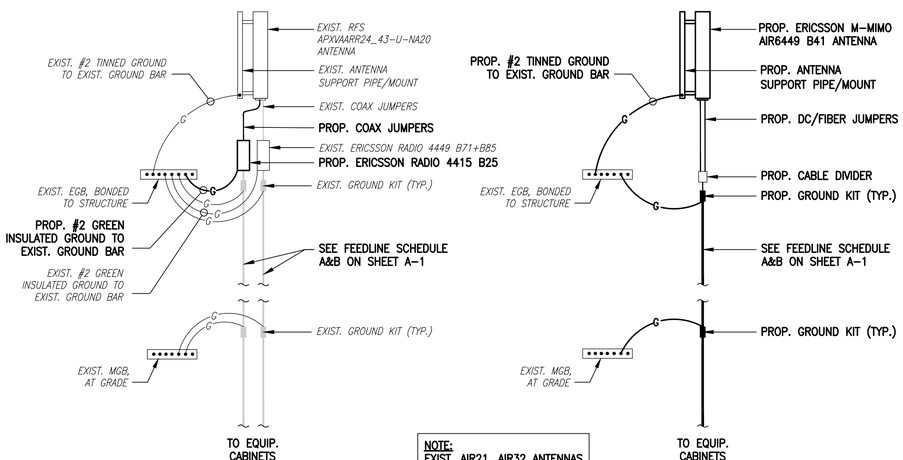
ELEVATION



SECTION A-A

NOTES:
1. "DOUBLING UP" OR "STACKING" OF CONNECTION IS NOT PERMITTED.
2. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.
3. CADWELL DOWNLEADS FROM UPPER EGB, LOWER EGB AND MGB.

TYPICAL GROUND BAR CONNECTIONS DETAIL (3 / E-1)
SCALE: NOT TO SCALE

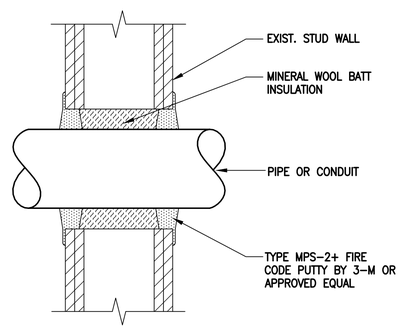


L700/L600/N600/L1900 ANTENNA

NOTE: EXIST. AIR21, AIR32 ANTENNAS, TMA & ASSOCIATED CABLES NOT SHOWN FOR CLARITY.

L2500/N2500 ANTENNA

COAX CABLE CONNECTION AND GROUNDING DETAIL (4 / E-1)
SCALE: NOT TO SCALE



U.L. SYSTEM NO. C-AJ-1009 THROUGH PENETRATION FIRE STOP SYSTEM

INSTALLATION REQUIREMENTS DETAILED IN THE U.L. (UNDERWRITERS LABORATORIES INC.) FIRE RESISTANCE DIRECTORY SHOULD BE STRICTLY ADHERED TO DURING INSTALLATION

CONDUIT WALL PENETRATION (6 / E-1)
SCALE: NTS

ELECTRICAL AND GROUNDING NOTES

1. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC) AS WELL AS APPLICABLE STATE AND LOCAL CODES.
2. ALL ELECTRICAL ITEMS SHALL BE U.L. APPROVED OR LISTED AND PROCURED PER SPECIFICATION REQUIREMENTS.
3. THE ELECTRICAL WORK INCLUDES ALL LABOR AND MATERIAL DESCRIBED BY DRAWINGS AND SPECIFICATION INCLUDING INCIDENTAL WORK TO PROVIDE COMPLETE OPERATING AND APPROVED ELECTRICAL SYSTEM.
4. GENERAL CONTRACTOR SHALL PAY FEES FOR PERMITS, AND IS RESPONSIBLE FOR OBTAINING SAID PERMITS AND COORDINATION OF INSPECTIONS.
5. ELECTRICAL AND TELCO WIRING OUTSIDE A BUILDING AND EXPOSED TO WEATHER SHALL BE IN WATER TIGHT GALVANIZED RIGID STEEL CONDUITS OR SCHEDULE 80 PVC (AS PERMITTED BY CODE) AND WHERE REQUIRED IN LIQUID TIGHT FLEXIBLE METAL OR NONMETALLIC CONDUITS.
6. BURIED CONDUIT SHALL BE SCHEDULE 40 PVC.
7. ELECTRICAL WIRING SHALL BE COPPER WITH TYPE XHHW, THWN, OR THININSULATION.
8. RUN ELECTRICAL CONDUIT OR CABLE BETWEEN ELECTRICAL UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE PPC AS INDICATED ON THIS DRAWING. PROVIDE FULL LENGTH PULL ROPE. COORDINATE INSTALLATION WITH UTILITY COMPANY.
9. RUN TELCO CONDUIT OR CABLE BETWEEN TELEPHONE UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE TELCO CABINET AND BTS CABINET AS INDICATED ON THIS DRAWING PROVIDE FULL LENGTH PULL ROPE IN INSTALLED TELCO CONDUIT. PROVIDE GREENLEE CONDUIT MEASURING TAPE AT EACH END.
10. WHERE CONDUIT BETWEEN BTS AND PROJECT OWNER CELL SITE PPC AND BETWEEN BTS AND PROJECT OWNER CELL SITE TELCO SERVICE CABINET ARE UNDERGROUND USE PVC, SCHEDULE 40 CONDUIT. ABOVE THE GROUND PORTION OF THESE CONDUITS SHALL BE PVC CONDUIT.
11. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.
12. PPC SUPPLIED BY PROJECT OWNER.
13. GROUNDING SHALL COMPLY WITH NEC ART. 250. ADDITIONALLY, GROUNDING, BONDING AND LIGHTNING PROTECTION SHALL BE DONE IN ACCORDANCE WITH "T-MOBILE BTS SITE GROUNDING STANDARDS".
14. GROUND COAXIAL CABLE SHIELDS MINIMUM AT BOTH ENDS USING MANUFACTURERS COAX CABLE GROUNDING KITS SUPPLIED BY PROJECT OWNER.
15. USE #6 COPPER STRANDED WIRE WITH GREEN COLOR INSULATION FOR ABOVE GRADE GROUNDING (UNLESS OTHERWISE SPECIFIED) AND #2 SOLID TINNED BARE COPPER WIRE FOR BELOW GRADE GROUNDING AS INDICATED ON THE DRAWING.
16. ALL GROUND CONNECTIONS TO BE BURNDY HYGROUND COMPRESSION TYPE CONNECTORS OR CADWELD EXOTHERMIC WELD. DO NOT ALLOW BARE COPPER WIRE TO BE IN CONTACT WITH GALVANIZED STEEL.
17. ROUTE GROUNDING CONDUCTORS ALONG THE SHORTEST AND STRAIGHTEST PATH POSSIBLE, EXCEPT AS OTHERWISE INDICATED. GROUNDING LEADS SHOULD NEVER BE BENT AT RIGHT ANGLE. ALWAYS MAKE AT LEAST 12" RADIUS BENDS. #6 WIRE CAN BE BENT AT 6" RADIUS WHEN NECESSARY. BOND ANY METAL OBJECTS WITHIN 6 FEET OF PROJECT OWNER EQUIPMENT OR CABINET TO MASTER GROUND BAR OR GROUNDING RING.
18. CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO HOLE COMPRESSION TYPE COPPER LUGS. APPLY OXIDE INHIBITING COMPOUND TO ALL LOCATIONS.
19. APPLY OXIDE INHIBITING COMPOUND TO ALL COMPRESSION TYPE GROUND CONNECTIONS.
20. CONTRACTOR SHALL PROVIDE AND INSTALL OMNI DIRECTIONAL ELECTRONIC MARKER SYSTEM (EMS) BALLS OVER EACH GROUND ROD AND BONDING POINT BETWEEN EXIST. TOWER/ MONOPOLE GROUNDING RING AND EQUIPMENT GROUNDING RING.
21. CONTRACTOR SHALL TEST COMPLETED GROUND SYSTEM AND RECORD RESULTS FOR PROJECT CLOSE-OUT DOCUMENTATION. 5 OHMNS MINIMUM RESISTANCE REQUIRED.
22. CONTRACTOR SHALL CONDUCT ANTENNA, COAX, AND LNA RETURN-LOSS AND DISTANCE- TO-FAULT MEASUREMENTS (SWEEP TESTS) AND RECORD RESULTS FOR PROJECT CLOSE OUT.

T-MOBILE NORTHEAST LLC
15 COMMERCE WAY, SUITE B
NORTON, MA 02766
(508) 286-2700

SBA
SBA COMMUNICATIONS CORP.
134 FLANDERS ROAD, SUITE 125
WESTBOROUGH, MA 01581
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STATE OF CONNECTICUT · JAMES M. FITZGERALD · No. 25997 · LICENSED PROFESSIONAL CIVIL ENGINEER

CHECKED BY: JMT

APPROVED BY: JMT

SUBMITTALS

REV.	DATE	DESCRIPTION	BY
1	09/03/20	ISSUED FOR CONSTRUCTION	CMC
0	07/20/20	ISSUED FOR REVIEW	JRV

SITE NUMBER:
CT11284A

SITE ADDRESS:
81 MONTEVIDEO ROAD
AVON, CT 06001

SHEET TITLE
ELECTRIC & GROUNDING DETAILS

SHEET NUMBER
E-1

1815.142

EXHIBIT 7

Tower Engineering Solutions
Phone (972) 483-0607, Fax (972) 975-9615
1320 Greenway Drive, Suite 600, Irving, Texas 75038

Structural Analysis Report

Existing 150 ft PIROD Self Supporting Tower
Customer Name: SBA Communications Corp
Customer Site Number: CT22071-A
Customer Site Name: Avon (Montevideo)
Carrier Name: T-Mobile (App#: 134550, v1)
Carrier Site ID / Name: CT11284A / SBA Avon/RT 1777
Site Location: 81 Montevideo Road
Avon, Connecticut
HARTFORD County
Latitude: 41.803100
Longitude: -72.801300

Analysis Result:

Max Structural Usage: 90.0% [Pass]
Max Foundation Usage: 75.0% [Pass]
Additional Usage Caused by New Mount/Mount Modification: N/A

Report Prepared by: Matthew Baker

Introduction

The purpose of this report is to summarize the analysis results on the 150 ft PIROD Self Supporting Tower to support the proposed antennas and transmission lines in addition to those currently installed. Any modification listed under Sources of Information was assumed completed and was included in this analysis.

Sources of Information

Tower Drawings	Valmont Eng. File # A-123251-, Archive # F-1010188, dated 11/29/2007
Foundation Drawing	Valmont Eng. File # A-123251-, Archive # F-1010188, dated 11/29/2007
Geotechnical Report	Dr. Clarence Welti, P.E., P.C Geotechnical Report (for URS Corp. Project # 36924843, Job # MCM 007), dated 11/06/2007
Modification Drawings	N/A

Analysis Criteria

The rigorous analysis was performed in accordance with the requirements and stipulations of the TIA-222-G-2**.** In accordance with this standard, the structure was analyzed using **TESTowers,** a proprietary analysis software. The program considers the structure as an elastic 3-D model with second-order effects and temperature effects incorporated in the analysis. The analysis was performed using multiple wind directions.

Wind Speed Used in the Analysis:	Ultimate Design Wind Speed V_{ult} = 120.0 mph (3-Sec. Gust)/ Nominal Design Wind Speed V_{asd} = 93.0 mph (3-Sec. Gust)
Wind Speed with Ice:	50 mph (3-Sec. Gust) with 1" radial ice concurrent
Operational Wind Speed:	60 mph + 0" Radial ice
Standard/Codes:	TIA-222-G-2 / 2015 IBC / 2018 Connecticut State Building Code
Exposure Category:	B
Structure Class:	II
Topographic Category:	4
Crest Height:	520 ft
Seismic Parameters:	S_S = 0.181, S_1 = 0.064

This structural analysis is based upon the tower being classified as a Structure Class II; however, if a different classification is required subsequent to the date hereof, the tower classification will be changed to meet such requirement and a new structural analysis will be run.

Existing Antennas, Mounts and Transmission Lines

The table below summarizes the antennas, mounts and transmission lines that were considered in the analysis as existing on the tower.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
1	159.8	1	10' Omni	Direct	(1) 7/8"	Unknown
2	147.0	2	RFS - APXVSPP18-C-A20 - Panel	(3) T-Frames	(4) 1-1/4" Fiber	Sprint Nextel
3		3	RFS - APXVSPP18-C-A20 - Panel			
4		3	Alcatel Lucent - RRH2x20-25 - RRH			
5	143.7	3	Alcatel Lucent - 800 MHz RRH	Direct		
6	140.1	3	Alcatel Lucent - 1900 MHz RRH	Direct		
-	136.0	1	Ericsson - AIR 21 - Panel	(1) Sector Frame	(6) 1 5/8" (2) 1 1/4" Hybrid	T-Mobile
-		1	Ericsson - AIR 32 KRD901146-1_B66A - Panel			
-		1	RFS - APXVAARR24_43-U-NA20 - Panel			
-		1	Ericsson - KRY 112 144/2 - TMA			
-		1	Ericsson - 4449 B71 + B12 - RRH			
14	126.8	1	Omni 10'	(1) Standoff	(1) 1 1/4"	Unknown
15		1	Omni 20'	(1) Standoff	(1) 7/8"	
16	113.2	1	Dipole 10'	(1) Standoff	(1) 7/8"	
17	104.5	1	Omni 15'	(1) Standoff	(1) 7/8"	
18	99.8	1	Element 6' - Yagi	Direct	(1) 1/2"	
19	76.8	1	4.5' Parabolic Dish	(1) Standoff	(1) 1/4"	
20	73.0	1	4.5' Parabolic Dish	(1) Standoff	(1) 1/4"	
21	72.4	1	3.0"x2.5' GPS	Direct	(1) 1/2"	
22	67.3	1	3.0"x1.2' GPS	Direct	(1) 1/4"	
23	13.9	1	3.0"x1.5' GPS	(1) Standoff	(2) 1/2"	

Proposed Carrier's Final Configuration of Antennas, Mounts and Transmission Lines

Information pertaining to the proposed carrier's final configuration of antennas and transmission lines was provided by SBA Communications Corp. The proposed antennas and lines are listed below.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
7	136.0	1	RFS - APXVAARR24_43-U-NA20 (Octa) - Panel	(1) Sector Frame	(5) 1 5/8" (3) 1 1/4" Hybrid	T-Mobile
8		1	Ericsson - AIR32 KRD901146-1_B66A (Octa) - Panel			
9		1	Ericsson - AIR 21 - Panel			
10		1	Ericsson - AIR6449 B41 - Panel			
11		1	Ericsson - KRY 112 144/2 - TMA			
12		1	Ericsson - Radio 4449 B71 + B12 - RRH			
13		1	Ericsson - Radio 4415 B25 - RRH			

See the attached coax layout for the line placement considered in the analysis.

Analysis Results

The results of the structural analysis, performed for the wind and ice loading and antenna equipment as defined above, are summarized as the following:

Tower Component	Legs	Diagonals	Horizontals
Max. Usage:	**90.0%**	**87.9%**	**41.8%**
Pass/Fail	**Pass**	**Pass**	**Pass**

Foundations

	Compression (Kips)	Uplift (Kips)	Shear (Kips)
Analysis Reactions	278.9	255.2	27.4

The foundation has been investigated using the supplied documents and soils report and was found adequate. Therefore, no modification to the foundation will be required.

Operational Condition (Rigidity):

The maximum twist and sway of the microwave dishes under the operational wind speed as specified in the Analysis Criteria are listed in the table below:

Elevation (ft)	Antenna / Dish	Carrier	Twist (deg)	Sway (deg)
76.8	4.5' Parabolic Dish	Unknown	0.447	0.261
73.0	4.5' Parabolic Dish	Unknown	0.414	0.216

It is recommended that the carriers review the twist and sway values of the microwave dishes.

Conclusions

Based on the analysis results, the existing structure and its foundation were found to be adequate to safely support the existing and proposed equipment and meet the minimum requirements per the ANSI/TIA/EIA 222-G Standard under the design basic wind speed as specified in the Analysis Criteria.

Standard Conditions

1. This analysis was performed based on the information supplied to **(TES) Tower Engineering Solutions, LLC.** Verification of the information provided was not included in the Scope of Work for **TES**. The accuracy of the analysis is dependent on the accuracy of the information provided.

2. The structural analysis was performance based upon the evidence available at the time of this report. All information provided by the client is considered to be accurate.

3. The analyses will be performed based on the codes as specified by the client or based on the best knowledge of the engineering staff of **TES**. In the absence of information to the contrary, all work will be performed in accordance with the latest relevant revision of ANSI/TIA-222. If wind speed and/or ice loads are different from the minimum values recommended by the EIA/TIA-222 standard or other codes, **TES** should be notified in writing and the applicable minimum values provided by the client.

4. The configuration of the existing mounts, antennas, coax and other appurtenances were supplied by the customer for the current structural analysis. **TES** has not visited the tower site to verify the adequacy of the information provided. If there is any discrepancy found in the report regarding the existing conditions, **TES** should be notified immediately to evaluate the effect of the discrepancy on the analysis results.

5. The client will assume responsibility for rework associated with the differences in initially provided information, including tower and foundation information, existing and/or proposed equipment and transmission lines.

6. If a feasibility analysis was performed, final acceptance of changed conditions shall be based upon a rigorous structural analysis.

Structure: CT22071-A-SBA

Site Name:	Avon (Montevideo)			**Code:**	EIA/TIA-222-G	6/30/2020
Type:	Self Support	**Base Shape:**	Triangle	**Basic WS:**	93.00	
Height:	150.00 (ft)	**Base Width:**	14.00	**Basic Ice WS:**	50.00	
Base Elev:	0.00 (ft)	**Top Width:**	5.00	**Operational WS:**	60.00	Page: 1

Tower Engineering Solutions

Section Properties

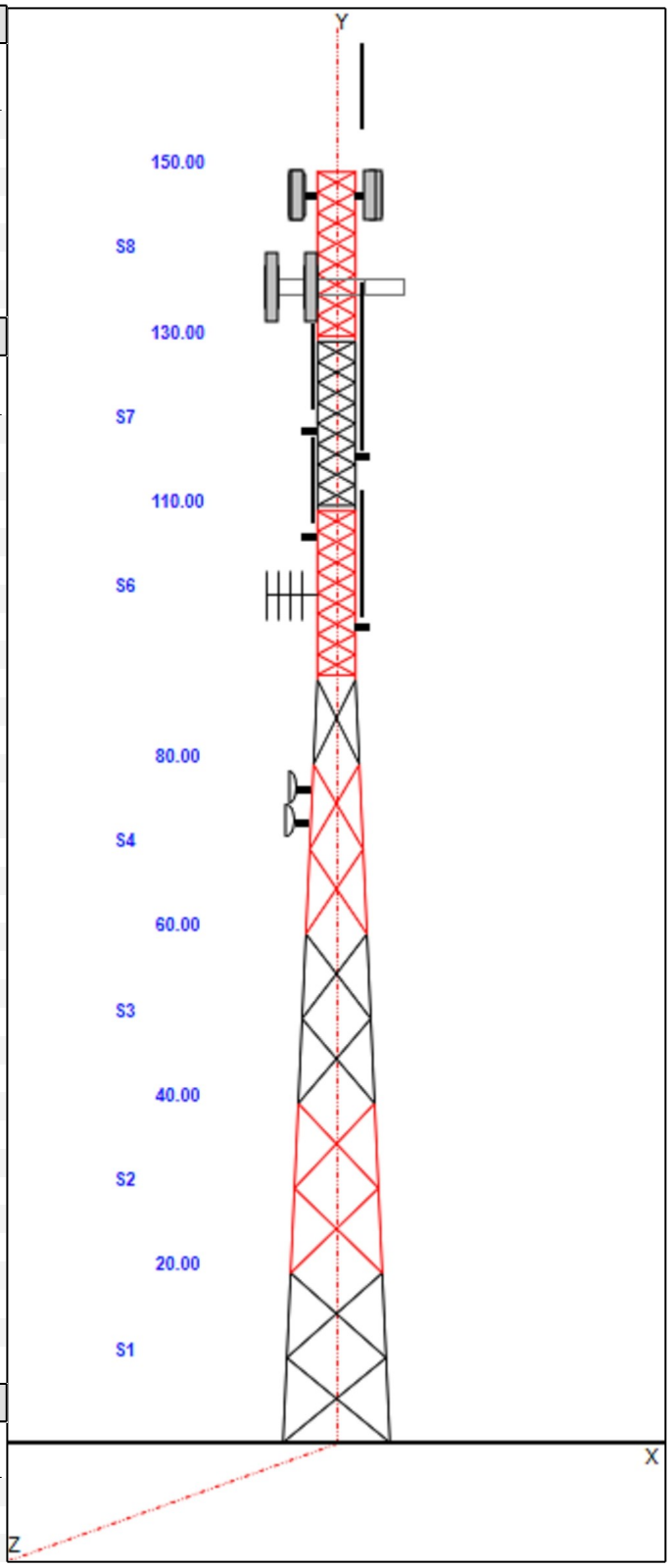
Sect	Leg Members	Diagonal Members	Horizontal Members
1	12B 12"BD 1.75"	SAE 3X3X0.3125	
2	12B 12"BD 1.75"	SAE 3X3X0.1875	
3	12B 12"BD 1.75"	SAE 2.5X2.5X0.1875	
4-5	12B 12"BD 1.5"	SAE 2.5X2.5X0.1875	
6	SOL 2 1/4" SOLID	SOL 1" SOLID	SOL 1" SOLID
7	SOL 2" SOLID	SOL 7/8" SOLID	SOL 7/8" SOLID
8	SOL 1 3/4" SOLID	SOL 7/8" SOLID	SOL 7/8" SOLID

Discrete Appurtenances

Attach Elev (ft)	Force Elev (ft)	Qty	Description
150.00	150.00	1	Lightning Rod
150.00	150.00	1	Beacon
150.00	159.80	1	10' Omni
147.00	147.00	3	T-Frames
147.00	147.00	2	APXVSPP18-C-A20
147.00	147.00	3	APXV9TM14-ALU-I20
147.00	147.00	3	RRH2x20-25
143.70	143.70	3	800 MHz RRH
140.10	140.10	3	1900MHz RRH
136.00	136.00	1	Sector Frame
136.00	136.00	1	APXVAARR24_43-U-NA20
136.00	136.00	1	AIR32
136.00	136.00	1	AIR 21
136.00	136.00	1	AIR6449 B41
136.00	136.00	1	KRY 112 144/2
136.00	136.00	1	4449
136.00	136.00	1	RRUS 4415 B25
119.30	119.30	1	Side Arm (M. Heavy)
119.30	126.80	1	10' Omni
116.30	116.30	1	Side Arm (M. Heavy)
116.30	126.80	1	20' Omni
106.60	106.60	1	Side Arm (M. Heavy)
106.60	113.20	1	10' Dipole
99.80	99.80	1	6' Yagi
96.00	96.00	1	Side Arm (M. Heavy)
96.00	104.50	1	15' Omni
76.80	76.80	1	Side Arm (L. Heavy)
76.80	76.80	1	4.5' Std. Dish
73.00	73.00	1	Side Arm (M. Heavy)
73.00	73.00	1	4' Std. Dish
72.40	72.40	1	3.05 x 2.5 GPS
72.40	72.40	1	3.05 x 1.2 GPS
12.30	13.90	1	Antenna Mount, 24" Short Strai
12.30	12.30	1	3.05 x 1.5 GPS

Linear Appurtenances

Elev From (ft)	Elev To (ft)	Qty	Description
0.00	150.00	1	7/8" Coax
0.00	147.00	4	1-1/4" Fiber
0.00	147.00	1	W/G Ladder
0.00	136.00	3	1 1/4" Coax
0.00	136.00	5	1 5/8" Coax



0.00	136.00	1	W/G Ladder
0.00	119.30	1	1 1/4" Coax
0.00	116.30	1	7/8" Coax
0.00	106.60	1	7/8" Coax
0.00	99.80	1	1/2" Coax
0.00	96.00	1	7/8" Coax
0.00	76.80	1	1/4" Coax
0.00	73.00	1	1/4" Coax
0.00	72.40	1	1/2" Coax
0.00	67.30	1	1/4" Coax
0.00	13.90	2	1/2" Coax

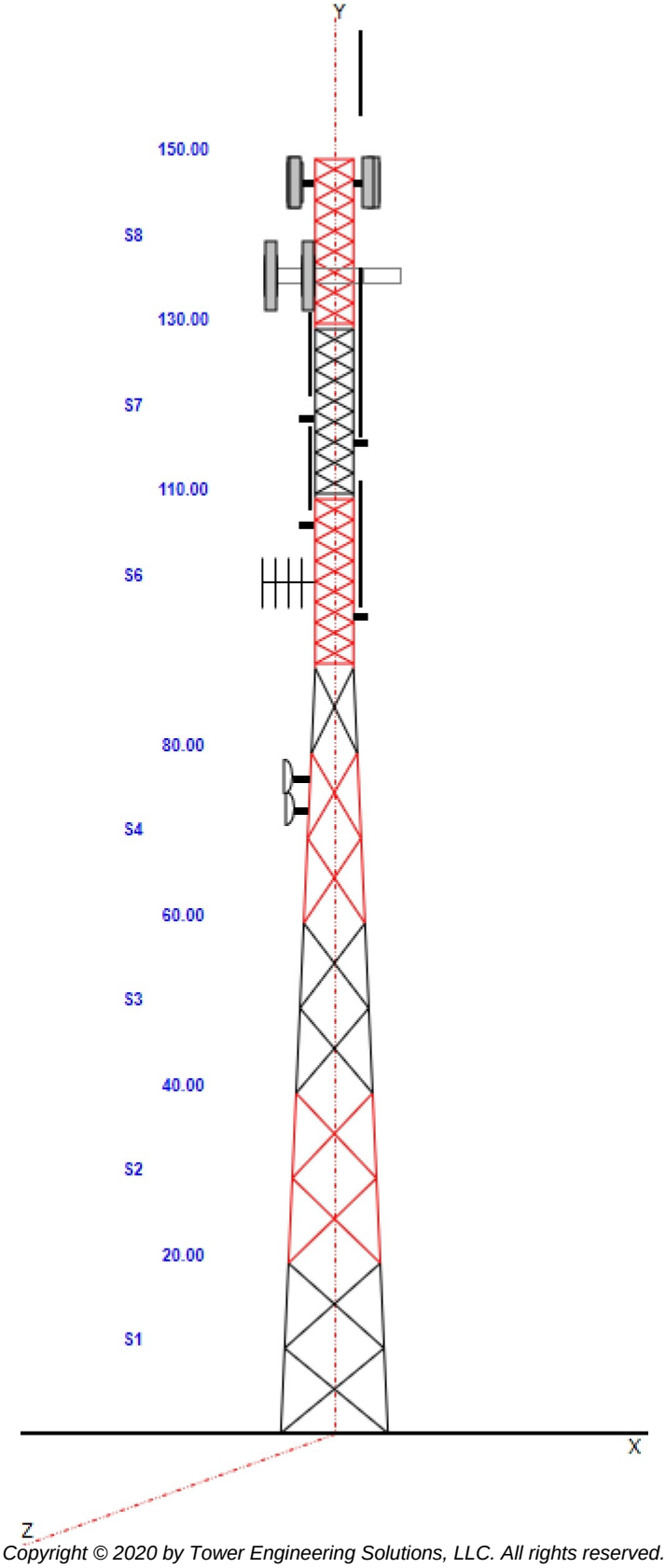
Base Reactions

Leg			Overturning		
Max Uplift:	-255.24	(kips	Moment:	3268.65	(ft-kips)
Max Down:	278.90	(kips	Total Down:	27.91	(kips)
Max Shear:	27.40	(kips	Total Shear:	39.75	(kips)

Structure: CT22071-A-SBA

Site Name:	Avon (Montevideo)			**Code:**	EIA/TIA-222-G	6/30/2020
Type:	Self Support	**Base Shape:**	Triangle	**Basic WS:**	93.00	
Height:	150.00 (ft)	**Base Width:**	14.00	**Basic Ice WS:**	50.00	
Base Elev:	0.00 (ft)	**Top Width:**	5.00	**Operational WS:**	60.00	Page: 3

Tower Engineering Solutions

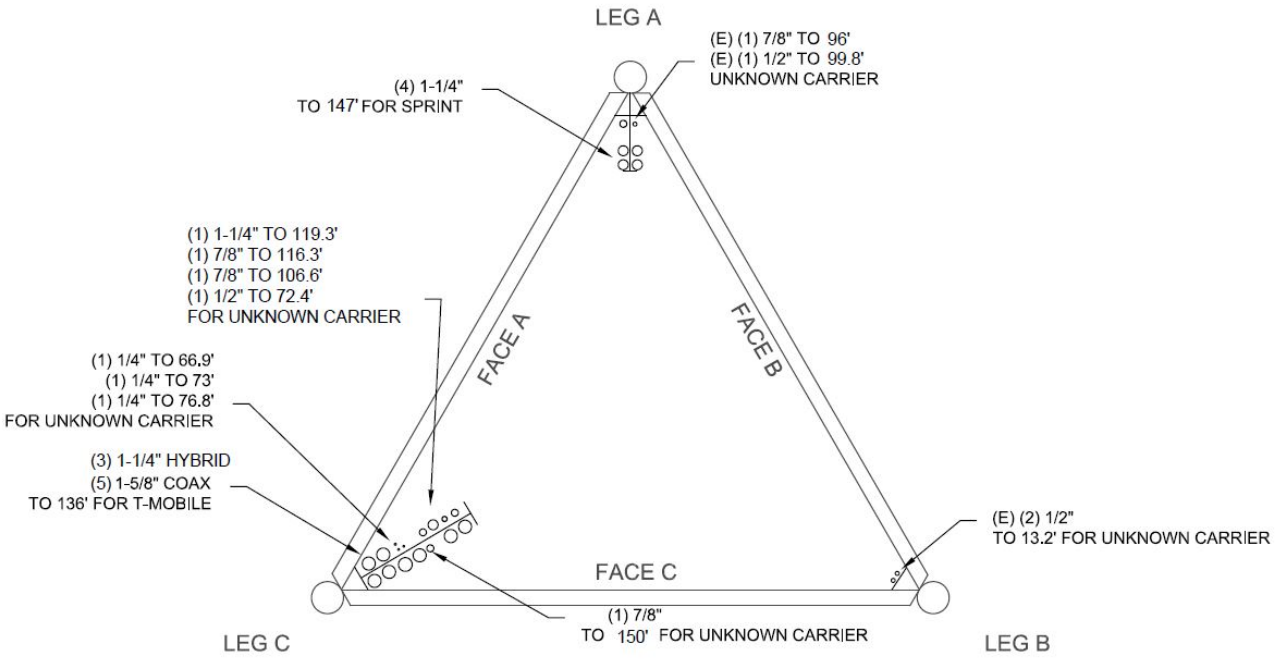


Structure: CT22071-A-SBA - Coax Line Placement

Type: Self Support
Site Name: Avon (Montevideo)
Height: 150.00 (ft)

6/30/2020

Tower Engineering Solutions



Loading Summary

Structure:	CT22071-A-SBA		**Code:**	EIA/TIA-222-G	6/30/2020
Site Name:	Avon (Montevideo)		**Exposure:**	B	
Height:	150.00 (ft)		**Crest Height:**	520.00	
Base Elev:	0.000 (ft)		**Site Class:**	C - Very Dense Soil	
Gh:	0.85	**Topography:** 4	**Struct Class:**	II	Page: 5

Tower Engineering Solutions

Discrete Appurtenances Properties

Attach Elev (ft)	Description	Qty	No Ice Weight (lb)	No Ice CaAa (sf)	Ice Weight (lb)	Ice CaAa (sf)	Len (in)	Width (in)	Depth (in)	Ka	Orientation Factor	Vert Ecc (ft)
150.00	Lightning Rod	1	5.00	0.500	40.77	3.481	72.000	1.000	1.000	1.00	1.00	0.000
150.00	Beacon	1	36.00	2.720	263.52	4.342	28.000	17.500	17.500	1.00	1.00	0.000
150.00	10' Omni	1	25.00	3.000	154.92	9.160	120.000	3.000	3.000	1.00	1.00	9.800
147.00	T-Frames	3	120.00	4.500	298.87	13.461	0.000	0.000	0.000	0.75	0.75	0.000
147.00	APXVSPP18-C-A20	2	57.00	8.020	352.26	12.792	72.000	11.800	7.000	0.80	0.83	0.000
147.00	APXV9TM14-ALU-I20	3	55.10	6.340	335.52	8.275	56.300	12.600	6.300	0.80	0.78	0.000
147.00	RRH2x20-25	3	70.00	4.050	287.75	5.514	26.100	18.600	6.700	0.80	0.67	0.000
143.70	800 MHz RRH	3	53.00	2.490	179.34	4.444	19.700	13.000	10.800	1.00	1.00	0.000
140.10	1900MHz RRH	3	44.00	3.800	230.47	6.174	23.000	13.000	17.000	1.00	1.00	0.000
136.00	Sector Frame	1	400.00	10.000	876.98	24.906	0.000	0.000	0.000	1.00	1.00	0.000
136.00	APXVAARR24_43-U-NA20	1	128.00	20.240	929.62	23.522	95.900	24.000	7.800	0.90	0.70	0.000
136.00	AIR32	1	132.20	6.510	441.18	8.459	57.000	12.900	8.700	0.90	0.86	0.000
136.00	AIR 21	1	91.00	6.090	420.30	8.051	56.000	12.100	7.900	0.90	0.85	0.000
136.00	AIR6449 B41	1	104.00	5.680	351.41	7.367	33.100	20.600	8.600	0.90	0.72	0.000
136.00	KRY 112 144/2	1	11.00	0.410	29.42	1.222	6.900	6.100	2.700	0.90	0.50	0.000
136.00	4449	1	70.00	1.650	209.00	2.640	15.000	13.200	9.300	0.90	0.67	0.000
136.00	RRUS 4415 B25	1	46.00	1.640	116.21	2.520	15.000	13.200	5.400	0.90	0.67	0.000
119.30	Side Arm (M. Heavy)	1	160.00	6.000	397.81	17.914	0.000	0.000	0.000	1.00	1.00	0.000
119.30	10' Omni	1	25.00	3.000	154.55	9.143	120.000	3.000	3.000	1.00	1.00	7.500
116.30	Side Arm (M. Heavy)	1	160.00	6.000	397.81	17.914	0.000	0.000	0.000	1.00	1.00	0.000
116.30	20' Omni	1	55.00	6.000	311.62	18.085	240.000	3.000	3.000	1.00	1.00	10.50
106.60	Side Arm (M. Heavy)	1	160.00	6.000	396.58	17.853	0.000	0.000	0.000	1.00	1.00	0.000
106.60	10' Dipole	1	30.00	3.760	219.08	13.928	120.000	3.000	3.000	1.00	1.00	6.600
99.80	6' Yagi	1	25.00	8.950	457.06	49.456	72.000	60.000	3.000	1.00	1.00	0.000
96.00	Side Arm (M. Heavy)	1	160.00	6.000	396.58	17.853	0.000	0.000	0.000	1.00	1.00	0.000
96.00	15' Omni	1	40.00	4.500	232.11	13.571	180.000	3.000	3.000	1.00	1.00	8.500
76.80	Side Arm (L. Heavy)	1	120.00	4.500	294.78	13.256	0.000	0.000	0.000	1.00	1.00	0.000
76.80	4.5' Std. Dish	1	188.00	26.000	543.97	32.392	48.000	48.000	0.000	1.00	1.00	0.000
73.00	Side Arm (M. Heavy)	1	160.00	6.000	393.04	17.675	0.000	0.000	0.000	1.00	1.00	0.000
73.00	4' Std. Dish	1	188.00	26.000	543.97	32.392	48.000	48.000	0.000	1.00	1.00	0.000
72.40	3.05 x 2.5 GPS	1	0.30	0.380	4.44	2.232	2.600	3.000	0.000	1.00	1.00	0.000
72.40	3.05 x 1.2 GPS	1	0.30	0.240	4.44	1.410	2.600	3.000	0.000	1.00	1.00	0.000
12.30	Antenna Mount, 24" Short Strai	1	50.00	1.500	175.38	4.008	0.000	0.000	0.000	1.00	1.00	1.600
12.30	3.05 x 1.5 GPS	1	0.30	0.260	3.87	1.351	2.600	3.000	0.000	1.00	1.00	0.000
	Totals:	**45**	**3,710.40**		**13,460.73**						**Number of Appurtenances :**	**34**

Loading Summary

Structure:	CT22071-A-SBA		**Code:**	EIA/TIA-222-G	6/30/2020
Site Name:	Avon (Montevideo)		**Exposure:**	B	
Height:	150.00 (ft)		**Crest Height:**	520.00	
Base Elev:	0.000 (ft)		**Site Class:**	C - Very Dense Soil	
Gh:	0.85	**Topography:** 4	**Struct Class:**	II	Page: 6

Tower Engineering Solutions

Linear Appurtenances Properties

Elev. From (ft)	Elev. To (ft)	Description	Qty	Width (in)	Weight (lb/ft)	Pct In Block	Spread On Faces	Bundling Arrangement	Cluster Dia (in)	Out of Zone	Spacing (in)	Orientation Factor	Ka Override
0.00	150.00	7/8" Coax	1	1.11	0.52	100.00	2	Individual NR		N	0.50	1.00	
0.00	147.00	1-1/4" Fiber	4	1.25	0.95	50.00	3	Block		N	0.50	0.96	
0.00	147.00	W/G Ladder	1	1.00	6.00	100.00	3	Individual NR		N	0.50	1.00	
0.00	136.00	1 1/4" Coax	3	1.55	0.66	50.00	2	Block		N	0.50	1.00	
0.00	136.00	1 5/8" Coax	5	1.98	1.04	50.00	2	Block		N	0.50	1.00	
0.00	136.00	W/G Ladder	1	1.00	6.00	100.00	2	Individual NR		N	0.50	1.00	
0.00	119.30	1 1/4" Coax	1	1.55	0.66	100.00	2	Individual NR		N	0.50	1.00	
0.00	116.30	7/8" Coax	1	1.11	0.52	100.00	2	Individual NR		N	0.50	1.00	
0.00	106.60	7/8" Coax	1	1.11	0.52	100.00	2	Individual NR		N	0.50	1.00	
0.00	99.80	1/2" Coax	1	0.65	0.16	100.00	3	Individual NR		N	0.50	1.00	0
0.00	96.00	7/8" Coax	1	1.11	0.52	100.00	3	Individual NR		N	0.50	1.00	
0.00	76.80	1/4" Coax	1	0.25	0.04	100.00	2	Individual NR		N	0.50	1.00	0
0.00	73.00	1/4" Coax	1	0.25	0.04	100.00	2	Individual NR		N	0.50	1.00	0
0.00	72.40	1/2" Coax	1	0.65	0.16	100.00	2	Individual NR		N	0.50	1.00	0
0.00	67.30	1/4" Coax	1	0.25	0.04	100.00	2	Individual NR		N	0.50	1.00	0
0.00	13.90	1/2" Coax	2	0.65	0.16	100.00	1	Individual IR		N	0.50	1.00	

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 7	

Tower Engineering Solutions

Load Case: 1.2D + 1.6W Normal Wind - P1

1.2D + 1.6W 93 mph Wind at Normal To Face - P1

Wind Load Factor: 1.60
Dead Load Factor: 1.20
Ice Dead Load Factor: 0.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	1.00	1.00	0.00	24.00	40.81	0.00	4,620.9	0.0	3290.14	1560.21	4,850.35
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	1.00	1.00	0.00	22.72	39.30	0.00	3,969.0	0.0	2919.06	1444.14	4,363.20
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	1.00	1.00	0.00	18.97	39.30	0.00	3,769.5	0.0	2661.35	1601.37	4,262.72
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	1.00	1.00	0.00	17.75	38.41	0.00	3,257.4	0.0	2545.55	1691.46	4,237.01
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	1.00	1.00	0.00	8.70	18.48	0.00	1,608.8	0.0	1214.76	867.24	2,082.00
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	1.00	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	1.00	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	1.00	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	1218.32	694.40	1,912.72
														23,458.4	**0.0**			**27,607.02**

Load Case: 1.2D + 1.6W Normal Wind - P2

1.2D + 1.6W 93 mph Wind at Normal To Face - P2

Wind Load Factor: 1.60
Dead Load Factor: 1.20
Ice Dead Load Factor: 0.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	19.45	15.937	18.83	0.00	0.13	2.85	1.00	1.00	0.00	24.00	40.81	0.00	4,620.9	0.0	1809.58	858.12	2,667.69
1 2	30.0	18.62	14.403	18.83	0.00	0.14	2.79	1.00	1.00	0.00	22.72	39.30	0.00	3,969.0	0.0	1605.49	794.28	2,399.76
1 3	50.0	20.65	10.827	18.83	0.00	0.16	2.75	1.00	1.00	0.00	18.97	39.30	0.00	3,769.5	0.0	1463.74	880.75	2,344.50
1 4	70.0	21.82	9.814	17.23	0.00	0.18	2.66	1.00	1.00	0.00	17.75	38.41	0.00	3,257.4	0.0	1400.05	930.30	2,330.36
1 5	85.0	22.39	4.578	8.61	0.00	0.22	2.52	1.00	1.00	0.00	8.70	18.48	0.00	1,608.8	0.0	668.12	476.98	1,145.10
1 6	100.0	22.78	0.000	15.46	0.00	0.15	2.78	1.00	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	767.91	934.66	1,702.57
1 7	120.0	23.12	0.000	13.66	0.00	0.13	2.84	1.00	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	700.84	841.05	1,541.89
1 8	140.0	23.31	0.000	12.85	0.00	0.12	2.87	1.00	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	670.07	381.92	1,051.99
														23,458.4	**0.0**			**15,183.86**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 8	

Tower Engineering Solutions

Load Case: 1.2D + 1.6W Normal Wind - P3

1.2D + 1.6W 93 mph Wind at Normal To Face - P3

Wind Load Factor: 1.60
Dead Load Factor: 1.20
Ice Dead Load Factor: 0.00

Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	1.00	1.00	0.00	24.00	40.81	0.00	4,620.9	0.0	3290.14	1560.21	4,850.35
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	1.00	1.00	0.00	22.72	39.30	0.00	3,969.0	0.0	2919.06	1444.14	4,363.20
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	1.00	1.00	0.00	18.97	39.30	0.00	3,769.5	0.0	2661.35	1601.37	4,262.72
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	1.00	1.00	0.00	17.75	38.41	0.00	3,257.4	0.0	2545.55	1691.46	4,237.01
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	1.00	1.00	0.00	8.70	18.48	0.00	1,608.8	0.0	1214.76	867.24	2,082.00
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	1.00	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	1.00	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	1.00	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	1218.32	694.40	1,912.72
														23,458.4	**0.0**			**27,607.02**

Load Case: 1.2D + 1.6W 60° Wind - P1

1.2D + 1.6W 93 mph Wind at 60° From Face - P1

Wind Load Factor: 1.60
Dead Load Factor: 1.20
Ice Dead Load Factor: 0.00

Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	0.80	1.00	0.00	20.82	40.81	0.00	4,620.9	0.0	2853.28	1560.21	4,413.49
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	0.80	1.00	0.00	19.83	39.30	0.00	3,969.0	0.0	2548.90	1444.14	3,993.04
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	0.80	1.00	0.00	16.80	39.30	0.00	3,769.5	0.0	2357.56	1601.37	3,958.93
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	0.80	1.00	0.00	15.79	38.41	0.00	3,257.4	0.0	2264.13	1691.46	3,955.59
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	0.80	1.00	0.00	7.78	18.48	0.00	1,608.8	0.0	1086.90	867.24	1,954.14
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	0.80	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	0.80	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	0.80	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	1218.32	694.40	1,912.72
														23,458.4	**0.0**			**26,086.93**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 9	

Tower Engineering Solutions

Load Case: 1.2D + 1.6W 60° Wind - P2 — 1.2D + 1.6W 93 mph Wind at 60° From Face - P2

Wind Load Factor: 1.60 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.20

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	19.45	15.937	18.83	0.00	0.13	2.85	0.80	1.00	0.00	20.82	40.81	0.00	4,620.9	0.0	1569.30	858.12	2,427.42
1 2	30.0	18.62	14.403	18.83	0.00	0.14	2.79	0.80	1.00	0.00	19.83	39.30	0.00	3,969.0	0.0	1401.89	794.28	2,196.17
1 3	50.0	20.65	10.827	18.83	0.00	0.16	2.75	0.80	1.00	0.00	16.80	39.30	0.00	3,769.5	0.0	1296.66	880.75	2,177.41
1 4	70.0	21.82	9.814	17.23	0.00	0.18	2.66	0.80	1.00	0.00	15.79	38.41	0.00	3,257.4	0.0	1245.27	930.30	2,175.57
1 5	85.0	22.39	4.578	8.61	0.00	0.22	2.52	0.80	1.00	0.00	7.78	18.48	0.00	1,608.8	0.0	597.80	476.98	1,074.78
1 6	100.0	22.78	0.000	15.46	0.00	0.15	2.78	0.80	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	767.91	934.66	1,702.57
1 7	120.0	23.12	0.000	13.66	0.00	0.13	2.84	0.80	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	700.84	841.05	1,541.89
1 8	140.0	23.31	0.000	12.85	0.00	0.12	2.87	0.80	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	670.07	381.92	1,051.99
														23,458.4	**0.0**			**14,347.81**

Load Case: 1.2D + 1.6W 60° Wind - P3 — 1.2D + 1.6W 93 mph Wind at 60° From Face - P3

Wind Load Factor: 1.60 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.20

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	0.80	1.00	0.00	20.82	40.81	0.00	4,620.9	0.0	2853.28	1560.21	4,413.49
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	0.80	1.00	0.00	19.83	39.30	0.00	3,969.0	0.0	2548.90	1444.14	3,993.04
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	0.80	1.00	0.00	16.80	39.30	0.00	3,769.5	0.0	2357.56	1601.37	3,958.93
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	0.80	1.00	0.00	15.79	38.41	0.00	3,257.4	0.0	2264.13	1691.46	3,955.59
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	0.80	1.00	0.00	7.78	18.48	0.00	1,608.8	0.0	1086.90	867.24	1,954.14
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	0.80	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	0.80	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	0.80	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	1218.32	694.40	1,912.72
														23,458.4	**0.0**			**26,086.93**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 10	

Tower Engineering Solutions

Load Case: 1.2D + 1.6W 90° Wind - P1 — 1.2D + 1.6W 93 mph Wind at 90° From Face - P1

Wind Load Factor: 1.60 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.20

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	0.85	1.00	0.00	21.61	40.81	0.00	4,620.9	0.0	2962.49	1560.21	4,522.71
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	0.85	1.00	0.00	20.56	39.30	0.00	3,969.0	0.0	2641.44	1444.14	4,085.58
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	0.85	1.00	0.00	17.35	39.30	0.00	3,769.5	0.0	2433.51	1601.37	4,034.88
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	0.85	1.00	0.00	16.28	38.41	0.00	3,257.4	0.0	2334.49	1691.46	4,025.94
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	0.85	1.00	0.00	8.01	18.48	0.00	1,608.8	0.0	1118.87	867.24	1,986.10
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	0.85	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	0.85	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	0.85	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	1218.32	694.40	1,912.72
														23,458.4	**0.0**			**26,466.95**

Load Case: 1.2D + 1.6W 90° Wind - P2 — 1.2D + 1.6W 93 mph Wind at 90° From Face - P2

Wind Load Factor: 1.60 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.20

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	19.45	15.937	18.83	0.00	0.13	2.85	0.85	1.00	0.00	21.61	40.81	0.00	4,620.9	0.0	1629.37	858.12	2,487.49
1 2	30.0	18.62	14.403	18.83	0.00	0.14	2.79	0.85	1.00	0.00	20.56	39.30	0.00	3,969.0	0.0	1452.79	794.28	2,247.07
1 3	50.0	20.65	10.827	18.83	0.00	0.16	2.75	0.85	1.00	0.00	17.35	39.30	0.00	3,769.5	0.0	1338.43	880.75	2,219.19
1 4	70.0	21.82	9.814	17.23	0.00	0.18	2.66	0.85	1.00	0.00	16.28	38.41	0.00	3,257.4	0.0	1283.97	930.30	2,214.27
1 5	85.0	22.39	4.578	8.61	0.00	0.22	2.52	0.85	1.00	0.00	8.01	18.48	0.00	1,608.8	0.0	615.38	476.98	1,092.36
1 6	100.0	22.78	0.000	15.46	0.00	0.15	2.78	0.85	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	767.91	934.66	1,702.57
1 7	120.0	23.12	0.000	13.66	0.00	0.13	2.84	0.85	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	700.84	841.05	1,541.89
1 8	140.0	23.31	0.000	12.85	0.00	0.12	2.87	0.85	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	670.07	381.92	1,051.99
														23,458.4	**0.0**			**14,556.82**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 11	

Tower Engineering Solutions

Load Case: 1.2D + 1.6W 90° Wind - P3 — 1.2D + 1.6W 93 mph Wind at 90° From Face - P3

Wind Load Factor: 1.60 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.20

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	0.85	1.00	0.00	21.61	40.81	0.00	4,620.9	0.0	2962.49	1560.21	4,522.71
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	0.85	1.00	0.00	20.56	39.30	0.00	3,969.0	0.0	2641.44	1444.14	4,085.58
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	0.85	1.00	0.00	17.35	39.30	0.00	3,769.5	0.0	2433.51	1601.37	4,034.88
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	0.85	1.00	0.00	16.28	38.41	0.00	3,257.4	0.0	2334.49	1691.46	4,025.94
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	0.85	1.00	0.00	8.01	18.48	0.00	1,608.8	0.0	1118.87	867.24	1,986.10
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	0.85	1.00	0.00	8.93	34.80	0.00	2,534.1	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	0.85	1.00	0.00	7.85	29.53	0.00	2,073.5	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	0.85	1.00	0.00	7.37	13.29	0.00	1,625.1	0.0	1218.32	694.40	1,912.72
														23,458.4	**0.0**			**26,466.95**

Load Case: 0.9D + 1.6W Normal Wind — 0.9D + 1.6W 93 mph Wind at Normal To Face

Wind Load Factor: 1.60 — **Wind Importance Factor:** 1.00

Dead Load Factor: 0.90

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	1.00	1.00	0.00	24.00	40.81	0.00	3,465.7	0.0	3290.14	1560.21	4,850.35
1 2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	1.00	1.00	0.00	22.72	39.30	0.00	2,976.7	0.0	2919.06	1444.14	4,363.20
1 3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	1.00	1.00	0.00	18.97	39.30	0.00	2,827.2	0.0	2661.35	1601.37	4,262.72
1 4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	1.00	1.00	0.00	17.75	38.41	0.00	2,443.1	0.0	2545.55	1691.46	4,237.01
1 5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	1.00	1.00	0.00	8.70	18.48	0.00	1,206.6	0.0	1214.76	867.24	2,082.00
1 6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	1.00	1.00	0.00	8.93	34.80	0.00	1,900.6	0.0	1396.20	1699.39	3,095.59
1 7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	1.00	1.00	0.00	7.85	29.53	0.00	1,555.1	0.0	1274.26	1529.18	2,803.43
1 8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	1.00	1.00	0.00	7.37	13.29	0.00	1,218.8	0.0	1218.32	694.40	1,912.72
														17,593.8	**0.0**			**27,607.02**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 12	

Tower Engineering Solutions

Load Case: 0.9D + 1.6W 60° Wind — 0.9D + 1.6W 93 mph Wind at 60° From Face

Wind Load Factor: 1.60
Dead Load Factor: 0.90
Ice Dead Load Factor: 0.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect	Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	0.80	1.00	0.00	20.82	40.81	0.00	3,465.7	0.0	2853.28	1560.21	4,413.49
1	2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	0.80	1.00	0.00	19.83	39.30	0.00	2,976.7	0.0	2548.90	1444.14	3,993.04
1	3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	0.80	1.00	0.00	16.80	39.30	0.00	2,827.2	0.0	2357.56	1601.37	3,958.93
1	4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	0.80	1.00	0.00	15.79	38.41	0.00	2,443.1	0.0	2264.13	1691.46	3,955.59
1	5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	0.80	1.00	0.00	7.78	18.48	0.00	1,206.6	0.0	1086.90	867.24	1,954.14
1	6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	0.80	1.00	0.00	8.93	34.80	0.00	1,900.6	0.0	1396.20	1699.39	3,095.59
1	7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	0.80	1.00	0.00	7.85	29.53	0.00	1,555.1	0.0	1274.26	1529.18	2,803.43
1	8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	0.80	1.00	0.00	7.37	13.29	0.00	1,218.8	0.0	1218.32	694.40	1,912.72
															17,593.8	**0.0**			**26,086.93**

Load Case: 0.9D + 1.6W 90° Wind — 0.9D + 1.6W 93 mph Wind at 90° From Face

Wind Load Factor: 1.60
Dead Load Factor: 0.90
Ice Dead Load Factor: 0.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect	Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1	1	10.0	35.36	15.937	18.83	0.00	0.13	2.85	0.85	1.00	0.00	21.61	40.81	0.00	3,465.7	0.0	2962.49	1560.21	4,522.71
1	2	30.0	33.86	14.403	18.83	0.00	0.14	2.79	0.85	1.00	0.00	20.56	39.30	0.00	2,976.7	0.0	2641.44	1444.14	4,085.58
1	3	50.0	37.55	10.827	18.83	0.00	0.16	2.75	0.85	1.00	0.00	17.35	39.30	0.00	2,827.2	0.0	2433.51	1601.37	4,034.88
1	4	70.0	39.67	9.814	17.23	0.00	0.18	2.66	0.85	1.00	0.00	16.28	38.41	0.00	2,443.1	0.0	2334.49	1691.46	4,025.94
1	5	85.0	40.71	4.578	8.61	0.00	0.22	2.52	0.85	1.00	0.00	8.01	18.48	0.00	1,206.6	0.0	1118.87	867.24	1,986.10
1	6	100.0	41.42	0.000	15.46	0.00	0.15	2.78	0.85	1.00	0.00	8.93	34.80	0.00	1,900.6	0.0	1396.20	1699.39	3,095.59
1	7	120.0	42.04	0.000	13.66	0.00	0.13	2.84	0.85	1.00	0.00	7.85	29.53	0.00	1,555.1	0.0	1274.26	1529.18	2,803.43
1	8	140.0	42.38	0.000	12.85	0.00	0.12	2.87	0.85	1.00	0.00	7.37	13.29	0.00	1,218.8	0.0	1218.32	694.40	1,912.72
															17,593.8	**0.0**			**26,466.95**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 13	

Tower Engineering Solutions

Load Case: 1.2D + 1.0Di + 1.0Wi Normal Wind — 1.2D + 1.0Di + 1.0Wi 50 mph Wind at Normal From Face

Wind Load Factor: 1.00
Dead Load Factor: 1.20
Ice Dead Load Factor: 1.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	10.22	15.937	63.00	44.17	0.28	2.34	1.00	1.00	2.51	53.37	88.99	83.59	13,408.	8787.9	1084.49	1030.20	2,114.70
1 2	30.0	9.79	14.403	64.56	45.73	0.33	2.22	1.00	1.00	2.76	53.73	85.23	91.86	13,474.	9505.9	990.98	966.17	1,957.14
1 3	50.0	10.85	10.827	63.56	44.72	0.37	2.12	1.00	1.00	2.86	50.57	86.92	95.25	13,106.	9337.2	988.10	1072.49	2,060.58
1 4	70.0	11.47	9.814	60.41	43.18	0.44	1.98	1.00	1.00	2.91	49.45	86.96	82.29	12,146.	8888.9	956.43	1047.86	2,004.29
1 5	85.0	11.77	4.578	29.61	21.00	0.53	1.86	1.00	1.00	2.94	25.40	42.98	29.39	5,734.6	4125.8	472.80	453.34	926.14
1 6	100.0	11.97	0.000	83.90	68.44	0.74	1.78	1.00	1.00	2.96	70.62	84.09	45.54	11,571.	9037.5	1281.73	485.94	1,767.67
1 7	120.0	12.15	0.000	82.45	68.79	0.73	1.78	1.00	1.00	2.97	68.72	79.08	17.64	10,157.	8083.7	1263.75	419.52	1,683.28
1 8	140.0	12.25	0.000	81.84	68.99	0.73	1.78	1.00	1.00	2.98	68.02	39.12	9.94	8,282.0	6656.9	1260.57	217.39	1,477.96
														87,882.0	**64423.6**			**13,991.75**

Load Case: 1.2D + 1.0Di + 1.0Wi 60° Wind — 1.2D + 1.0Di + 1.0Wi 50 mph Wind at 60° From Face

Wind Load Factor: 1.00
Dead Load Factor: 1.20
Ice Dead Load Factor: 1.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	10.22	15.937	63.00	44.17	0.28	2.34	0.80	1.00	2.51	50.18	88.99	83.59	13,408.	8787.9	1019.73	1030.20	2,049.93
1 2	30.0	9.79	14.403	64.56	45.73	0.33	2.22	0.80	1.00	2.76	50.85	85.23	91.86	13,474.	9505.9	937.85	966.17	1,904.02
1 3	50.0	10.85	10.827	63.56	44.72	0.37	2.12	0.80	1.00	2.86	48.41	86.92	95.25	13,106.	9337.2	945.79	1072.49	2,018.28
1 4	70.0	11.47	9.814	60.41	43.18	0.44	1.98	0.80	1.00	2.91	47.49	86.96	82.29	12,146.	8888.9	918.46	1047.86	1,966.33
1 5	85.0	11.77	4.578	29.61	21.00	0.53	1.86	0.80	1.00	2.94	24.48	42.98	29.39	5,734.6	4125.8	455.76	453.34	909.09
1 6	100.0	11.97	0.000	83.90	68.44	0.74	1.78	0.80	1.00	2.96	70.62	84.09	45.54	11,571.	9037.5	1281.73	485.94	1,767.67
1 7	120.0	12.15	0.000	82.45	68.79	0.73	1.78	0.80	1.00	2.97	68.72	79.08	17.64	10,157.	8083.7	1263.75	419.52	1,683.28
1 8	140.0	12.25	0.000	81.84	68.99	0.73	1.78	0.80	1.00	2.98	68.02	39.12	9.94	8,282.0	6656.9	1260.57	217.39	1,477.96
														87,882.0	**64423.6**			**13,776.55**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 14	

Tower Engineering Solutions

Load Case: 1.2D + 1.0Di + 1.0Wi 90° Wind — 1.2D + 1.0Di + 1.0Wi 50 mph Wind at 90° From Face

Wind Load Factor: 1.00
Dead Load Factor: 1.20
Ice Dead Load Factor: 1.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	10.22	15.937	63.00	44.17	0.28	2.34	0.85	1.00	2.51	50.98	88.99	83.59	13,408.	8787.9	1035.92	1030.20	2,066.12
1 2	30.0	9.79	14.403	64.56	45.73	0.33	2.22	0.85	1.00	2.76	51.57	85.23	91.86	13,474.	9505.9	951.13	966.17	1,917.30
1 3	50.0	10.85	10.827	63.56	44.72	0.37	2.12	0.85	1.00	2.86	48.95	86.92	95.25	13,106.	9337.2	956.37	1072.49	2,028.85
1 4	70.0	11.47	9.814	60.41	43.18	0.44	1.98	0.85	1.00	2.91	47.98	86.96	82.29	12,146.	8888.9	927.95	1047.86	1,975.82
1 5	85.0	11.77	4.578	29.61	21.00	0.53	1.86	0.85	1.00	2.94	24.71	42.98	29.39	5,734.6	4125.8	460.02	453.34	913.35
1 6	100.0	11.97	0.000	83.90	68.44	0.74	1.78	0.85	1.00	2.96	70.62	84.09	45.54	11,571.	9037.5	1281.73	485.94	1,767.67
1 7	120.0	12.15	0.000	82.45	68.79	0.73	1.78	0.85	1.00	2.97	68.72	79.08	17.64	10,157.	8083.7	1263.75	419.52	1,683.28
1 8	140.0	12.25	0.000	81.84	68.99	0.73	1.78	0.85	1.00	2.98	68.02	39.12	9.94	8,282.0	6656.9	1260.57	217.39	1,477.96
														87,882.0	**64423.6**			**13,830.35**

Load Case: 1.0D + 1.0W Normal Wind — 1.0D + 1.0W 60 mph Wind at Normal To Face

Wind Load Factor: 1.00
Dead Load Factor: 1.00
Ice Dead Load Factor: 0.00
Wind Importance Factor: 1.00
Ice Importance Factor: 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	14.72	15.937	18.83	0.00	0.13	2.85	1.00	1.00	0.00	25.97	40.81	0.00	3,850.8	0.0	926.12	405.88	1,332.00
1 2	30.0	14.09	14.403	18.83	0.00	0.14	2.79	1.00	1.00	0.00	24.56	39.30	0.00	3,307.5	0.0	821.15	375.69	1,196.84
1 3	50.0	15.63	10.827	18.83	0.00	0.16	2.75	1.00	1.00	0.00	20.85	39.30	0.00	3,141.3	0.0	761.12	416.59	1,177.71
1 4	70.0	16.51	9.814	17.23	0.00	0.18	2.66	1.00	1.00	0.00	19.26	38.41	0.00	2,714.5	0.0	718.45	440.03	1,158.47
1 5	85.0	16.94	4.578	8.61	0.00	0.22	2.52	1.00	1.00	0.00	9.37	18.48	0.00	1,340.7	0.0	340.48	225.61	566.09
1 6	100.0	17.24	0.000	15.46	0.00	0.15	2.78	1.00	1.00	0.00	8.93	34.80	0.00	2,111.8	0.0	363.22	442.09	805.30
1 7	120.0	17.50	0.000	13.66	0.00	0.13	2.84	1.00	1.00	0.00	7.85	29.53	0.00	1,727.9	0.0	331.49	397.81	729.30
1 8	140.0	17.64	0.000	12.85	0.00	0.12	2.87	1.00	1.00	0.00	7.37	13.29	0.00	1,354.2	0.0	316.94	180.65	497.58
														19,548.6	**0.0**			**7,463.30**

Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 15	

Tower Engineering Solutions

Load Case: 1.0D + 1.0W 60° Wind — 1.0D + 1.0W 60 mph Wind at 60° From Face

Wind Load Factor: 1.00 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.00

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	14.72	15.937	18.83	0.00	0.13	2.85	0.80	1.00	0.00	22.79	40.81	0.00	3,850.8	0.0	812.48	405.88	1,218.36
1 2	30.0	14.09	14.403	18.83	0.00	0.14	2.79	0.80	1.00	0.00	21.68	39.30	0.00	3,307.5	0.0	724.86	375.69	1,100.54
1 3	50.0	15.63	10.827	18.83	0.00	0.16	2.75	0.80	1.00	0.00	18.69	39.30	0.00	3,141.3	0.0	682.09	416.59	1,098.68
1 4	70.0	16.51	9.814	17.23	0.00	0.18	2.66	0.80	1.00	0.00	17.30	38.41	0.00	2,714.5	0.0	645.24	440.03	1,085.26
1 5	85.0	16.94	4.578	8.61	0.00	0.22	2.52	0.80	1.00	0.00	8.46	18.48	0.00	1,340.7	0.0	307.22	225.61	532.83
1 6	100.0	17.24	0.000	15.46	0.00	0.15	2.78	0.80	1.00	0.00	8.93	34.80	0.00	2,111.8	0.0	363.22	442.09	805.30
1 7	120.0	17.50	0.000	13.66	0.00	0.13	2.84	0.80	1.00	0.00	7.85	29.53	0.00	1,727.9	0.0	331.49	397.81	729.30
1 8	140.0	17.64	0.000	12.85	0.00	0.12	2.87	0.80	1.00	0.00	7.37	13.29	0.00	1,354.2	0.0	316.94	180.65	497.58
														19,548.6	**0.0**			**7,067.85**

Load Case: 1.0D + 1.0W 90° Wind — 1.0D + 1.0W 60 mph Wind at 90° From Face

Wind Load Factor: 1.00 — **Wind Importance Factor:** 1.00

Dead Load Factor: 1.00

Ice Dead Load Factor: 0.00 — **Ice Importance Factor:** 1.00

Sect Seq	Wind Height (ft)	qz (psf)	Total Flat Area (sqft)	Total Round Area (sqft)	Ice Round Area (sqft)	Sol Ratio	Cf	Df	Dr	Ice Thick (in)	Eff Area (sqft)	Linear Area (sqft)	Ice Linear Area (sqft)	Total Weight (lb)	Weight Ice (lb)	Struct Force (lb)	Linear Force (lb)	Total Force (lb)
1 1	10.0	14.72	15.937	18.83	0.00	0.13	2.85	0.85	1.00	0.00	23.58	40.81	0.00	3,850.8	0.0	840.89	405.88	1,246.77
1 2	30.0	14.09	14.403	18.83	0.00	0.14	2.79	0.85	1.00	0.00	22.40	39.30	0.00	3,307.5	0.0	748.93	375.69	1,124.62
1 3	50.0	15.63	10.827	18.83	0.00	0.16	2.75	0.85	1.00	0.00	19.23	39.30	0.00	3,141.3	0.0	701.84	416.59	1,118.43
1 4	70.0	16.51	9.814	17.23	0.00	0.18	2.66	0.85	1.00	0.00	17.79	38.41	0.00	2,714.5	0.0	663.54	440.03	1,103.56
1 5	85.0	16.94	4.578	8.61	0.00	0.22	2.52	0.85	1.00	0.00	8.68	18.48	0.00	1,340.7	0.0	315.53	225.61	541.14
1 6	100.0	17.24	0.000	15.46	0.00	0.15	2.78	0.85	1.00	0.00	8.93	34.80	0.00	2,111.8	0.0	363.22	442.09	805.30
1 7	120.0	17.50	0.000	13.66	0.00	0.13	2.84	0.85	1.00	0.00	7.85	29.53	0.00	1,727.9	0.0	331.49	397.81	729.30
1 8	140.0	17.64	0.000	12.85	0.00	0.12	2.87	0.85	1.00	0.00	7.37	13.29	0.00	1,354.2	0.0	316.94	180.65	497.58
														19,548.6	**0.0**			**7,166.71**

Force/Stress Compression Summary

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 16	

Tower Engineering Solutions

LEG MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing % X	Y	Z	KL/R	Fy (ksi)	Mem Cap (kips)	Leg Use %	Controls
1	20	12B - 12"BD 1.75"	-269.66	1.2D + 1.6W Normal Wind - P1	10.02	100	100	100	25.99	50.00	308.82	87.3	Member X
2	40	12B - 12"BD 1.75"	-242.62	1.2D + 1.6W Normal Wind - P1	10.02	100	100	100	25.99	50.00	308.82	78.6	Member X
3	60	12B - 12"BD 1.75"	-210.98	1.2D + 1.6W Normal Wind - P1	10.02	100	100	100	25.99	50.00	308.82	68.3	Member X
4	80	12B - 12"BD 1.5"	-176.93	1.2D + 1.6W Normal Wind - P1	10.02	100	100	100	30.32	50.00	222.99	79.3	Member X
5	90	12B - 12"BD 1.5"	-140.13	1.2D + 1.6W Normal Wind - P1	10.02	100	100	100	30.32	50.00	222.99	62.8	Member X
6	110	SOL - 2 1/4" SOLID	-132.52	1.2D + 1.6W Normal Wind - P1	2.42	100	100	100	51.55	50.00	147.32	90.0	Member X
7	130	SOL - 2" SOLID	-65.93	1.2D + 1.6W Normal Wind - P1	2.42	100	100	100	58.00	50.00	110.55	59.6	Member X
8	150	SOL - 1 3/4" SOLID	-19.89	1.2D + 1.6W Normal Wind - P1	2.42	100	100	100	66.28	50.00	78.50	25.3	Member X

Splices

		Top Splice					Bottom Splice				
Sect	Top Elev	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts
1	20	1.2D + 1.6W Normal Wind - P1	250.56	0.00	0.0			1.2D + 1.6W Normal Wind - P1	279.46	0.00			
2	40	1.2D + 1.6W Normal Wind - P1	219.60	0.00	0.0			1.2D + 1.6W Normal Wind - P1	250.56	0.00		1 A325	6
3	60	1.2D + 1.6W Normal Wind - P1	186.64	0.00	0.0			1.2D + 1.6W Normal Wind - P1	219.60	0.00		1 A325	6
4	80	1.2D + 1.6W Normal Wind - P1	150.60	0.00	0.0			1.2D + 1.6W Normal Wind - P1	186.64	0.00		1 A325	6
5	90	1.2D + 1.6W Normal Wind - P1	138.26	0.00	0.0			1.2D + 1.6W Normal Wind - P1	150.60	0.00		1 A325	6
6	110	1.2D + 1.6W Normal Wind - P1	69.66	0.00	0.0			1.2D + 1.6W Normal Wind - P1	138.26	0.00		1 A325	6
7	130	1.2D + 1.6W Normal Wind - P1	22.46	0.00	0.0			1.2D + 1.6W Normal Wind - P1	69.66	0.00			
8	150	1.2D + 1.0Di + 1.0Wi 90° Wind	0.51	0.00	0.0			1.2D + 1.6W Normal Wind - P1	22.46	0.00			

HORIZONTAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing % X	Y	Z	KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	Use %	Controls
1	20										0.00	0	0				
2	40										0.00	0	0				
3	60										0.00	0	0				
4	80										0.00	0	0				
5	90										0.00	0	0				
6	110	SOL - 1" SOLID	-1.12	1.2D + 1.6W 60° Wind - P1	5.00	100	100	100	168.00	50.00	6.29	0	0			18	Member X
7	130	SOL - 7/8" SOLID	-1.54	1.2D + 1.6W Normal Wind - P1	5.00	100	100	100	191.96	50.00	3.69	0	0			42	Member X
8	150	SOL - 7/8" SOLID	-1.15	1.2D + 1.6W Normal Wind - P1	5.00	100	100	100	191.96	50.00	3.69	0	0			31	Member X

DIAGONAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Len (ft)	Bracing % X	Y	Z	KL/R	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	Use %	Controls
1	20	SAE - 3X3X0.3125	-7.74	1.2D + 1.6W Normal Wind - P1	16.80	50	50	50	171.17	36.00	13.73	1	1	31.81	29.9	56	Member Z
2	40	SAE - 3X3X0.1875	-6.15	1.2D + 1.6W 60° Wind - P1	15.24	50	50	50	153.45	36.00	10.46	1	1	31.81	17.9	59	Member Z
3	60	SAE - 2.5X2.5X0.1875	-6.40	1.2D + 1.6W 60° Wind - P1	13.80	50	50	50	167.23	36.00	7.29	1	1	31.81	17.9	88	Member Z
4	80	SAE - 2.5X2.5X0.1875	-6.52	1.2D + 1.6W 60° Wind - P1	12.50	50	50	50	151.56	36.00	8.87	1	1	31.81	17.9	74	Member Z
5	90	SAE - 2.5X2.5X0.1875	-7.11	1.2D + 1.6W 90° Wind - P1	11.42	50	50	50	138.38	36.00	10.64	1	1	31.81	17.9	67	Member Z
6	110	SOL - 1" SOLID	-6.00	1.2D + 1.6W 90° Wind - P1	5.55	50	50	50	119.95	50.00	12.33	0	0			49	Member X
7	130	SOL - 7/8" SOLID	-4.82	1.2D + 1.6W 90° Wind - P1	5.55	50	50	50	137.06	50.00	7.23	0	0			67	Member X
8	150	SOL - 7/8" SOLID	-3.28	1.2D + 1.6W 90° Wind - P1	5.55	50	50	50	137.06	50.00	7.23	0	0			45	Member X

Force/Stress Tension Summary

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 17	

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LEG MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Leg Use %	Controls
1	20	12B - 12"BD 1.75"	247.94	0.9D + 1.6W 60° Wind	50	324.45	76.4	Member
2	40	12B - 12"BD 1.75"	224.00	0.9D + 1.6W 60° Wind	50	324.45	69.0	Member
3	60	12B - 12"BD 1.75"	195.76	0.9D + 1.6W 60° Wind	50	324.45	60.3	Member
4	80	12B - 12"BD 1.5"	164.22	0.9D + 1.6W 60° Wind	50	238.50	68.9	Member
5	90	12B - 12"BD 1.5"	130.38	0.9D + 1.6W 60° Wind	50	238.50	54.7	Member
6	110	SOL - 2 1/4" SOLID	127.92	0.9D + 1.6W 60° Wind	50	178.92	71.5	Member
7	130	SOL - 2" SOLID	63.34	0.9D + 1.6W 60° Wind	50	141.37	44.8	Member
8	150	SOL - 1 3/4" SOLID	19.52	0.9D + 1.6W 60° Wind	50	108.24	18.0	Member

Splices

		Top Splice					Bottom Splice				
Sect	Top Elev	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts	Load Case	Force (kips)	Cap (kips)	Use %	Bolt Type	Num Bolts
1	20	0.9D + 1.6W 60° Wind	230.20	0.00	0.0			0.9D + 1.6W 60° Wind	256.6	0.00			
2	40	0.9D + 1.6W 60° Wind	202.50	0.00	0.0			0.9D + 1.6W 60° Wind	230.2	318.06	72.4	1 A325	6
3	60	0.9D + 1.6W 60° Wind	172.51	0.00	0.0			0.9D + 1.6W 60° Wind	202.5	318.06	63.7	1 A325	6
4	80	0.9D + 1.6W 60° Wind	139.48	0.00	0.0			0.9D + 1.6W 60° Wind	172.5	318.06	54.2	1 A325	6
5	90	0.9D + 1.6W 60° Wind	127.65	0.00	0.0			0.9D + 1.6W 60° Wind	139.4	318.06	43.9	1 A325	6
6	110	0.9D + 1.6W 60° Wind	63.27	0.00	0.0			0.9D + 1.6W 60° Wind	127.6	318.06	40.1	1 A325	6
7	130	0.9D + 1.6W 60° Wind	19.47	0.00	0.0			0.9D + 1.6W 60° Wind	63.27	0.00			
8	150		0.00	0.00	0.0			0.9D + 1.6W 60° Wind	19.47	0.00			

HORIZONTAL MEMBERS

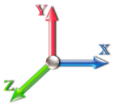
Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	20	-			36	0.00	0	0					
2	40	-			36	0.00	0	0					
3	60	-			36	0.00	0	0					
4	80	-			36	0.00	0	0					
5	90	-			36	0.00	0	0					
6	110	SOL - 1" SOLID	1.52	1.2D + 1.6W Normal W	50	35.34	0	0				4.3	Member
7	130	SOL - 7/8" SOLID	1.56	1.2D + 1.6W 60° Wind ·	50	27.06	0	0				5.8	Member
8	150	SOL - 7/8" SOLID	1.04	1.2D + 1.6W 60° Wind ·	50	27.06	0	0				3.8	Member

DIAGONAL MEMBERS

Sect	Top Elev	Member	Force (kips)	Load Case	Fy (ksi)	Mem Cap (kips)	Num Bolts	Num Holes	Shear Cap (kips)	Bear Cap (kips)	B.S. Cap (kips)	Use %	Controls
1	20	SAE - 3X3X0.3125	7.52	0.9D + 1.6W 60° Wind	36	46.60	1	1	31.81	29.91	19.47	38.6	Blck Shear
2	40	SAE - 3X3X0.1875	6.45	0.9D + 1.6W 60° Wind	36	28.68	1	1	31.81	17.94	11.68	55.2	Blck Shear
3	60	SAE - 2.5X2.5X0.1875	6.49	0.9D + 1.6W 60° Wind	36	22.55	1	1	31.81	17.94	10.66	60.9	Blck Shear
4	80	SAE - 2.5X2.5X0.1875	7.70	0.9D + 1.6W 60° Wind	36	22.55	1	1	31.81	17.94	10.66	72.2	Blck Shear
5	90	SAE - 2.5X2.5X0.1875	6.88	0.9D + 1.6W 90° Wind	36	22.55	1	1	31.81	17.94	10.66	64.5	Blck Shear
6	110	SOL - 1" SOLID	6.40	1.2D + 1.6W 60° Wind ·	50	35.34	0	0				18.1	Member
7	130	SOL - 7/8" SOLID	4.74	1.2D + 1.6W 90° Wind ·	50	27.06	0	0				17.5	Member
8	150	SOL - 7/8" SOLID	3.34	1.2D + 1.6W Normal W	50	27.06	0	0				12.3	Member

Seismic Section Forces

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 18	



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Load Case: 1.2D + 1.0E

Dead Load Factor	1.20	**Sds**	0.144	**Ss**	0.1810	**Fa**	1.2000	**Ke**	0.0000
Seismic Load Factor	1.00	**Sd1**	0.072	**S1**	0.0640	**Fv**	1.7000	**Kg**	0.0000
Seismic Importance Factor	1.00	**SA**	0.105	**R**	3.0000	**Vs**	0.9799	**f1**	1.4514

Sect #	Elev (ft)	Wz (lb)	a	b	c	Lateral Fsz (lb)
1	10.00	3901.0	0.01	0.05	0.03	14.30
2	30.00	3307.4	0.08	0.07	0.04	23.64
3	50.00	3141.2	0.21	0.06	0.02	35.27
4	70.00	3371.1	0.41	0.01	0.01	51.64
5	85.00	1340.7	0.61	-0.06	0.02	23.29
6	100.00	2526.7	0.84	-0.12	0.07	51.58
7	120.00	2127.9	1.21	0.01	0.26	75.51
8	140.00	3542.7	1.65	0.93	0.73	273.91

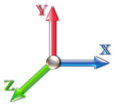
Load Case: 0.9D + 1.0E

Dead Load Factor	0.90	**Sds**	0.144	**Ss**	0.1810	**Fa**	1.2000	**Ke**	0.0000
Seismic Load Factor	1.00	**Sd1**	0.072	**S1**	0.0640	**Fv**	1.7000	**Kg**	0.0000
Seismic Importance Factor	1.00	**SA**	0.105	**R**	3.0000	**Vs**	0.9799	**f1**	1.4514

Sect #	Elev (ft)	Wz (lb)	a	b	c	Lateral Fsz (lb)
1	10.00	3901.0	0.01	0.05	0.03	14.30
2	30.00	3307.4	0.08	0.07	0.04	23.64
3	50.00	3141.2	0.21	0.06	0.02	35.27
4	70.00	3371.1	0.41	0.01	0.01	51.64
5	85.00	1340.7	0.61	-0.06	0.02	23.29
6	100.00	2526.7	0.84	-0.12	0.07	51.58
7	120.00	2127.9	1.21	0.01	0.26	75.51
8	140.00	3542.7	1.65	0.93	0.73	273.91

Support Forces Summary

Structure:	CT22071-A-SBA	**Code:**	EIA/TIA-222-G	6/30/2020	
Site Name:	Avon (Montevideo)	**Exposure:**	B		
Height:	150.00 (ft)	**Crest Height:**	520.00		
Base Elev:	0.000 (ft)	**Site Class:**	C - Very Dense Soil		
Gh:	0.85 **Topography:** 4	**Struct Class:**	II	Page: 19	



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Load Case	Node	FX (kips)	FY (kips)	FZ (kips)	(-) = Uplift (+) = Down
1.2D + 1.6W Normal Wind - P1	1	-0.02	278.90	-27.40	
	1a	11.07	-125.51	-6.16	
	1b	-11.05	-125.47	-6.19	
1.2D + 1.6W Normal Wind - P2	1	-0.02	209.75	-19.57	
	1a	7.78	-90.94	-3.87	
	1b	-7.77	-90.90	-3.89	
1.2D + 1.6W Normal Wind - P3	1	-0.02	278.90	-27.40	
	1a	11.07	-125.51	-6.16	
	1b	-11.05	-125.47	-6.19	
1.2D + 1.6W 60° Wind - P1	1	-0.32	141.58	-13.63	
	1a	-11.31	139.71	7.60	
	1b	-21.48	-253.37	-13.08	
1.2D + 1.6W 60° Wind - P2	1	-0.05	108.22	-9.92	
	1a	-7.96	106.35	5.98	
	1b	-14.93	-186.66	-9.30	
1.2D + 1.6W 60° Wind - P3	1	-0.32	141.58	-13.63	
	1a	-11.31	139.71	7.60	
	1b	-21.48	-253.37	-13.08	
1.2D + 1.6W 90° Wind - P1	1	-0.42	9.69	-0.71	
	1a	-19.68	237.23	12.35	
	1b	-18.51	-219.01	-11.64	
1.2D + 1.6W 90° Wind - P2	1	-0.08	9.68	-0.75	
	1a	-13.95	178.94	9.22	
	1b	-12.68	-160.71	-8.47	
1.2D + 1.6W 90° Wind - P3	1	-0.42	9.69	-0.71	
	1a	-19.68	237.23	12.35	
	1b	-18.51	-219.01	-11.64	
0.9D + 1.6W Normal Wind	1	-0.02	276.06	-27.19	
	1a	11.23	-127.59	-6.27	
	1b	-11.21	-127.54	-6.29	
0.9D + 1.6W 60° Wind	1	-0.33	138.95	-13.42	
	1a	-11.14	137.22	7.49	
	1b	-21.64	-255.24	-13.18	
0.9D + 1.6W 90° Wind	1	-0.43	7.27	-0.51	
	1a	-19.51	234.60	12.24	
	1b	-18.68	-220.94	-11.73	
1.2D + 1.0Di + 1.0Wi Normal Wind	1	-0.03	165.66	-13.24	
	1a	5.17	-32.46	-2.67	
	1b	-5.14	-32.55	-2.72	
1.2D + 1.0Di + 1.0Wi 60° Wind	1	-0.02	99.70	-6.84	
	1a	-5.63	97.57	3.82	
	1b	-10.30	-96.63	-6.18	

1.2D + 1.0Di + 1.0Wi 90° Wind	1	-0.01	34.50	-0.57
	1a	-9.67	145.35	6.05
	1b	-8.79	-79.20	-5.48
1.2D + 1.0E	1	0.00	14.08	3.84
	1a	3.84	6.92	-2.17
	1b	-3.84	6.92	-2.17
0.9D + 1.0E	1	0.00	11.74	4.04
	1a	4.02	4.59	-2.27
	1b	-4.02	4.59	-2.27
1.0D + 1.0W Normal Wind	1	0.00	79.11	-7.78
	1a	2.53	-27.92	-1.42
	1b	-2.53	-27.93	-1.43
1.0D + 1.0W 60° Wind	1	-0.11	42.87	-4.11
	1a	-3.43	42.08	2.24
	1b	-5.32	-61.70	-3.24
1.0D + 1.0W 90° Wind	1	-0.13	8.06	-0.67
	1a	-5.66	67.82	3.51
	1b	-4.53	-52.63	-2.85

Max Reactions

Leg			Overturning		
Max Uplift:	-255.24	(kips)	Moment:	3268.65	(ft-kips)
Max Down:	278.90	(kips)	Total Down:	27.91	(kips)
Max Shear:	27.40	(kips)	Total Shear:	39.75	(kips)

Max Reactions

Leg			Overturning		
Max Uplift:	-255.24	(kips)	Moment:	3268.65	(ft-kips)
Max Down:	278.90	(kips)	Total Down:	27.91	(kips)
Max Shear:	27.40	(kips)	Total Shear:	39.75	(kips)

Anchor Bolts

Bolt Size (in.):	1.00	Number Bolts:	6
Yield Strength (Ksi):	105.00	Tensile Strength (Ksi):	150.00
Detail Type:	C		

Interaction Ratio: 0.70

Max Usages

Max Leg: 90.0% (1.2D + 1.6W Normal Wind - P1 - Sect 6)
Max Diag: 87.9% (1.2D + 1.6W 60° Wind - P1 - Sect 3)
Max Horiz: 41.8% (1.2D + 1.6W Normal Wind - P1 - Sect 7)

Max Deflection, Twist and Sway

Load Case	Elevation (ft)	Deflection (ft)	Twist (deg)	Sway (deg)
0.9D + 1.0E - Normal To Face	10.00	0.0044	0.0000	0.0135
	70.00	0.0089	-0.0004	0.0159
	80.00	0.0102	0.0000	0.0183
	95.38	0.0179	-0.0006	0.0274
	100.21	0.0186	0.0000	0.0297
	107.46	0.0225	0.0000	0.0306
	115.38	0.0291	-0.0003	0.0365
	120.21	0.0305	0.0000	0.0377
	135.38	0.0427	0.0001	0.0409
	140.21	0.0461	-0.0001	0.0404
	142.63	0.0478	-0.0001	0.0404
	147.46	0.0512	0.0000	0.0403
	150.00	0.0530	0.0000	0.0409
0.9D + 1.6W 93 mph Wind at 60° From Face	10.00	0.0116	0.2021	0.1047
	70.00	0.4299	1.5820	0.8149
	80.00	0.5710	1.7051	0.9791
	95.38	0.8390	0.2206	1.1005
	100.21	0.9487	1.8686	1.2518
	107.46	1.1130	1.9853	1.2391
	115.38	1.3031	0.7624	1.3282
	120.21	1.4361	2.1420	1.4232
	135.38	1.8485	2.1437	1.5946
	140.21	1.9812	2.1438	1.5917
	142.63	2.0476	2.1437	1.5849
	147.46	2.1811	2.1437	1.6765
	150.00	2.2508	2.1438	1.5939

0.9D + 1.6W 93 mph Wind at 90° From Face	10.00	0.0105	0.2002	0.1019
	70.00	0.4304	1.5669	0.8234
	80.00	0.5710	1.6866	0.9960
	95.38	0.8312	0.0261	0.9808
	100.21	0.9493	1.6808	1.2455
	107.46	1.1126	1.6799	1.1686
	115.38	1.2909	0.0347	0.9833
	120.21	1.4341	1.6788	1.3111
	135.38	1.8446	1.6775	1.5839
	140.21	1.9763	1.6774	1.5759
	142.63	2.0424	1.6775	1.5756
	147.46	2.1748	1.6774	1.7624
	150.00	2.2438	1.6775	1.5845
0.9D + 1.6W 93 mph Wind at Normal To Face	10.00	0.0123	0.2033	0.1089
	70.00	0.4377	1.5942	0.7571
	80.00	0.5816	1.7192	0.8870
	95.38	0.8555	-0.0356	1.3748
	100.21	0.9579	1.7123	1.2736
	107.46	1.1259	1.7097	1.4137
	115.38	1.3291	-0.0354	1.9744
	120.21	1.4550	1.7059	1.7004
	135.38	1.8801	1.7033	1.6362
	140.21	2.0162	1.7035	1.6218
	142.63	2.0840	1.7035	1.6181
	147.46	2.2205	1.7032	1.6194
	150.00	2.2946	1.7039	2.5229
1.0D + 1.0W 60 mph Wind at 60° From Face	10.00	0.0029	0.0523	0.0278
	70.00	0.1136	0.4094	0.2145
	80.00	0.1508	0.4410	0.2570
	95.38	0.2209	0.0156	0.2884
	100.21	0.2499	0.4515	0.3283
	107.46	0.2930	0.4596	0.3246
	115.38	0.3427	0.0530	0.3466
	120.21	0.3779	0.4703	0.3725
	135.38	0.4864	0.4703	0.4191
	140.21	0.5213	0.4703	0.4181
	142.63	0.5388	0.4703	0.4172
	147.46	0.5740	0.4703	0.4424
	150.00	0.5923	0.4703	0.4196
1.0D + 1.0W 60 mph Wind at 90° From Face	10.00	0.0030	0.0521	0.0270
	70.00	0.1135	0.4077	0.2160
	80.00	0.1504	0.4388	0.2611
	95.38	0.2186	0.0063	0.2575
	100.21	0.2496	0.4376	0.3262
	107.46	0.2924	0.4376	0.3063
	115.38	0.3390	0.0071	0.2587
	120.21	0.3766	0.4376	0.3439
	135.38	0.4843	0.4374	0.4156
	140.21	0.5189	0.4374	0.4136
	142.63	0.5362	0.4374	0.4137
	147.46	0.5710	0.4373	0.4622
	150.00	0.5891	0.4373	0.4160
1.0D + 1.0W 60 mph Wind at Normal To Face	10.00	0.0034	0.0529	0.0293
	70.00	0.1162	0.4145	0.2011
	80.00	0.1543	0.4471	0.2347
	95.38	0.2263	-0.0094	0.3623
	100.21	0.2536	0.4466	0.3359
	107.46	0.2979	0.4465	0.3730
	115.38	0.3512	-0.0093	0.5192
	120.21	0.3848	0.4463	0.4478
	135.38	0.4966	0.4461	0.4313
	140.21	0.5326	0.4460	0.4277
	142.63	0.5505	0.4460	0.4270
	147.46	0.5865	0.4460	0.4271
	150.00	0.6060	0.4460	0.6624

1.2D + 1.0Di + 1.0Wi 50 mph Wind at 60° From Face	10.00	0.0069	0.0455	0.0556
	70.00	0.2139	0.3522	0.3877
	80.00	0.2853	0.3787	0.4727
	95.38	0.4224	0.0623	0.5573
	100.21	0.4773	0.4294	0.6341
	107.46	0.5604	0.4657	0.6154
	115.38	0.6569	0.2130	0.6648
	120.21	0.7232	0.5100	0.7098
	135.38	0.9299	0.5108	0.7988
	140.21	0.9966	0.5108	0.7959
	142.63	1.0300	0.5107	0.7955
	147.46	1.0970	0.5107	0.8480
	150.00	1.1321	0.5108	0.8006
1.2D + 1.0Di + 1.0Wi 50 mph Wind at 90° From Face	10.00	0.0062	0.0448	0.0523
	70.00	0.2120	0.3471	0.3842
	80.00	0.2825	0.3723	0.4760
	95.38	0.4160	0.0133	0.4872
	100.21	0.4740	0.3708	0.6265
	107.46	0.5559	0.3714	0.5641
	115.38	0.6464	0.0180	0.4710
	120.21	0.7165	0.3721	0.6441
	135.38	0.9205	0.3719	0.7876
	140.21	0.9861	0.3719	0.7831
	142.63	1.0191	0.3719	0.7859
	147.46	1.0850	0.3718	0.8883
	150.00	1.1194	0.3719	0.7906
1.2D + 1.0Di + 1.0Wi 50 mph Wind at Normal From Face	10.00	0.0047	0.0462	0.0486
	70.00	0.2174	0.3595	0.3817
	80.00	0.2906	0.3878	0.4465
	95.38	0.4299	-0.0169	0.7076
	100.21	0.4810	0.3873	0.6464
	107.46	0.5663	0.3872	0.7345
	115.38	0.6704	-0.0164	1.0178
	120.21	0.7326	0.3870	0.8644
	135.38	0.9453	0.3868	0.8199
	140.21	1.0142	0.3868	0.8143
	142.63	1.0485	0.3868	0.8133
	147.46	1.1171	0.3867	0.8133
	150.00	1.1538	0.3869	1.3168
1.2D + 1.0E - Normal To Face	10.00	0.0043	0.0000	0.0134
	70.00	0.0089	-0.0004	0.0160
	80.00	0.0103	0.0000	0.0182
	95.38	0.0179	-0.0006	0.0275
	100.21	0.0186	0.0000	0.0297
	107.46	0.0226	0.0000	0.0307
	115.38	0.0292	-0.0003	0.0366
	120.21	0.0306	0.0000	0.0377
	135.38	0.0428	0.0001	0.0409
	140.21	0.0462	-0.0001	0.0405
	142.63	0.0479	0.0001	0.0405
	147.46	0.0513	0.0000	0.0404
	150.00	0.0531	0.0000	0.0410
1.2D + 1.6W 93 mph Wind at 60° From Face - P1	10.00	0.0115	0.2021	0.1047
	70.00	0.4308	1.5820	0.8168
	80.00	0.5723	1.7051	0.9815
	95.38	0.8411	0.2213	1.1043
	100.21	0.9511	1.8692	1.2558
	107.46	1.1159	1.9863	1.2434
	115.38	1.3067	0.7652	1.3334
	120.21	1.4402	2.1436	1.4285
	135.38	1.8541	2.1453	1.6006
	140.21	1.9873	2.1454	1.5977
	142.63	2.0540	2.1453	1.5909
	147.46	2.1879	2.1452	1.6823
	150.00	2.2579	2.1454	1.5999

1.2D + 1.6W 93 mph Wind at 60° From Face - P2	10.00	0.0083	0.2027	0.0794
	70.00	0.3286	1.5859	0.6513
	80.00	0.4384	1.7097	0.7823
	95.38	0.6471	0.1783	0.8617
	100.21	0.7362	1.8420	0.9973
	107.46	0.8675	1.9362	0.9815
	115.38	1.0182	0.6174	1.0604
	120.21	1.1276	2.0631	1.1371
	135.38	1.4621	2.0644	1.2964
	140.21	1.5699	2.0645	1.2945
	142.63	1.6238	2.0644	1.2890
	147.46	1.7323	2.0643	1.3828
	150.00	1.7889	2.0644	1.2965
1.2D + 1.6W 93 mph Wind at 60° From Face - P3	10.00	0.0115	0.2021	0.1047
	70.00	0.4308	1.5820	0.8168
	80.00	0.5723	1.7051	0.9815
	95.38	0.8411	0.2213	1.1043
	100.21	0.9511	1.8692	1.2558
	107.46	1.1159	1.9863	1.2434
	115.38	1.3067	0.7652	1.3334
	120.21	1.4402	2.1436	1.4285
	135.38	1.8541	2.1453	1.6006
	140.21	1.9873	2.1454	1.5977
	142.63	2.0540	2.1453	1.5909
	147.46	2.1879	2.1452	1.6823
	150.00	2.2579	2.1454	1.5999
1.2D + 1.6W 93 mph Wind at 90° From Face - P1	10.00	0.0106	0.2002	0.1020
	70.00	0.4312	1.5668	0.8250
	80.00	0.5721	1.6865	0.9982
	95.38	0.8330	0.0263	0.9841
	100.21	0.9514	1.6808	1.2490
	107.46	1.1152	1.6801	1.1724
	115.38	1.2941	0.0354	0.9879
	120.21	1.4378	1.6790	1.3158
	135.38	1.8496	1.6777	1.5893
	140.21	1.9818	1.6777	1.5812
	142.63	2.0480	1.6777	1.5809
	147.46	2.1809	1.6777	1.7677
	150.00	2.2501	1.6777	1.5899
1.2D + 1.6W 93 mph Wind at 90° From Face - P2	10.00	0.0077	0.2013	0.0783
	70.00	0.3287	1.5740	0.6595
	80.00	0.4378	1.6952	0.7969
	95.38	0.6383	0.0216	0.7381
	100.21	0.7357	1.6908	0.9908
	107.46	0.8659	1.6903	0.9080
	115.38	1.0048	0.0302	0.6966
	120.21	1.1241	1.6896	1.0226
	135.38	1.4565	1.6885	1.2854
	140.21	1.5632	1.6885	1.2791
	142.63	1.6167	1.6885	1.2788
	147.46	1.7242	1.6885	1.4662
	150.00	1.7800	1.6885	1.2863
1.2D + 1.6W 93 mph Wind at 90° From Face - P3	10.00	0.0106	0.2002	0.1020
	70.00	0.4312	1.5668	0.8250
	80.00	0.5721	1.6865	0.9982
	95.38	0.8330	0.0263	0.9841
	100.21	0.9514	1.6808	1.2490
	107.46	1.1152	1.6801	1.1724
	115.38	1.2941	0.0354	0.9879
	120.21	1.4378	1.6790	1.3158
	135.38	1.8496	1.6777	1.5893
	140.21	1.9818	1.6777	1.5812
	142.63	2.0480	1.6777	1.5809
	147.46	2.1809	1.6777	1.7677
	150.00	2.2501	1.6777	1.5899

1.2D + 1.6W 93 mph Wind at Normal To Face - P1	10.00	0.0123	0.2033	0.1092
	70.00	0.4387	1.5942	0.7594
	80.00	0.5831	1.7192	0.8895
	95.38	0.8578	-0.0357	1.3788
	100.21	0.9606	1.7123	1.2780
	107.46	1.1291	1.7097	1.4182
	115.38	1.3331	-0.0354	1.9799
	120.21	1.4594	1.7059	1.7061
	135.38	1.8862	1.7033	1.6423
	140.21	2.0228	1.7035	1.6279
	142.63	2.0909	1.7035	1.6243
	147.46	2.2278	1.7032	1.6255
	150.00	2.3023	1.7039	2.5291
1.2D + 1.6W 93 mph Wind at Normal To Face - P2	10.00	0.0090	0.2034	0.0802
	70.00	0.3326	1.5940	0.5898
	80.00	0.4447	1.7191	0.6943
	95.38	0.6601	-0.0276	1.1308
	100.21	0.7418	1.7137	1.0173
	107.46	0.8764	1.7118	1.1569
	115.38	1.0401	-0.0277	1.6875
	120.21	1.1422	1.7087	1.4097
	135.38	1.4885	1.7066	1.3361
	140.21	1.5995	1.7067	1.3235
	142.63	1.6547	1.7067	1.3200
	147.46	1.7660	1.7065	1.3218
	150.00	1.8270	1.7070	2.2234
1.2D + 1.6W 93 mph Wind at Normal To Face - P3	10.00	0.0123	0.2033	0.1092
	70.00	0.4387	1.5942	0.7594
	80.00	0.5831	1.7192	0.8895
	95.38	0.8578	-0.0357	1.3788
	100.21	0.9606	1.7123	1.2780
	107.46	1.1291	1.7097	1.4182
	115.38	1.3331	-0.0354	1.9799
	120.21	1.4594	1.7059	1.7061
	135.38	1.8862	1.7033	1.6423
	140.21	2.0228	1.7035	1.6279
	142.63	2.0909	1.7035	1.6243
	147.46	2.2278	1.7032	1.6255
	150.00	2.3023	1.7039	2.5291

Mat Foundation Design for Self Supporting Tower

			Date: 6/30/2020
Customer Name:	SBA Communications Corp	EIA/TIA Standard:	EIA-222-G
Site Name:		Structure Height (Ft.):	150
Site Nmber:	CT22071-A-SBA	Engineer Name:	M. Baker
Engr. Number:	94598	Engineer Login ID:	

Foundation Info Obtained from: Drawings/Calculations

Analysis or Design? Analysis

Number of Tower Legs: 3 Legs

Base Reactions (Factored):

(1). Individual Leg:

Axial Load (Kips):	278.9	Uplift Force (Kips):	255.2
Shear Force (Kips):	27.4		

(2). Tower Base:

Total Vertical Load (Kips):	27.9	Total Shear Force (Kips):	39.8
Moment (Kips-ft):	3268.7		

Foundation Geometries:

Leg distance (Center-to-Center ft.):		14.0	Mods required -Yes/No ?:	No	
Diameter of Pier (ft.):	Round	4.0	Pier Height A. G. (ft.):	0.50	
Tower center to mat center (ft):		2.02083	Depth of Base BG (ft.):	6.0	
Length of Pad (ft.):		23	Width of Pad (ft.):	23	(W)
Thickness of Pad (ft):		2.50			

Material Properties and Reabr Info:

Concrete Strength (psi):	3000	Steel Elastic Modulus:	29000	ksi
Vertical bar yield (ksi)	60	Tie steel yield (ksi):	60	
Vertical Rebar Size #:	8	Tie / Stirrup Size #:	4	
Qty. of Vertical Rebars:	12	Tie Spacing (in):	6.0	
Pad Rebar Yield (Ksi):	60	Pad Steel Rebar Size (#):	8	
Concrete Cover (in.):	3	Unit Weight of Concrete:	150.0	pcf

Rebar at the bottom of the concrete pad:

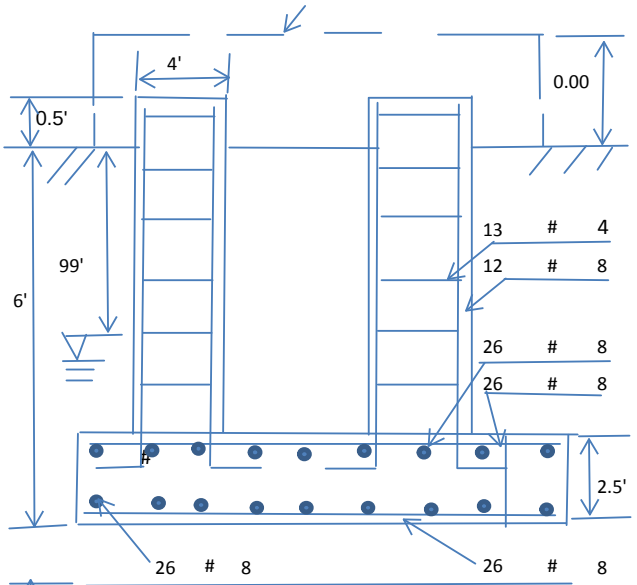
Qty. of Rebar in Pad (L):	26	Qty. of Rebar in Pad (W):	26

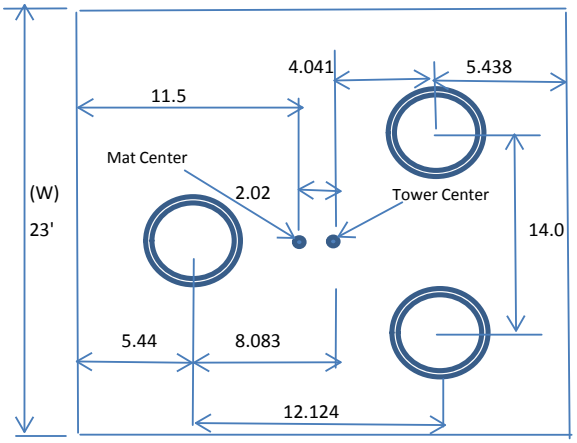
Rebar at the top of the concrete pad:

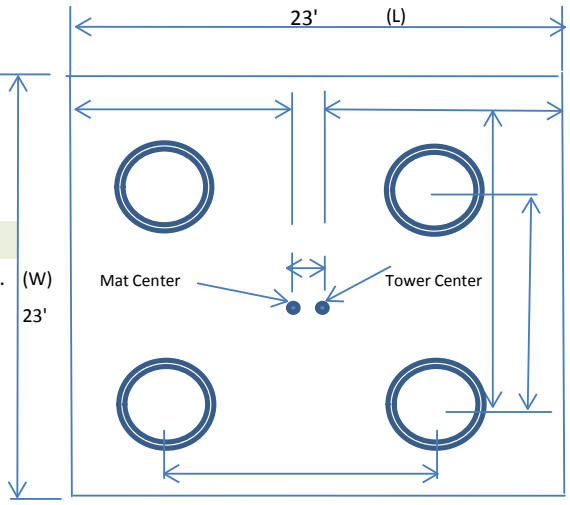
Qty. of Rebar in Pad (L):	26	Qty. of Rebar in Pad (W):	26

Soil Design Parameters:

Soil Unit Weight (pcf):	120.0	Soil Buoyant Weight:	50.0	Pcf
Water Table B.G.S. (ft):	99.0	Unit Weight of Water:	62.4	pcf
Ultimate Bearing Pressure (psf):	12000	Consider ties in concrete shear strength:		Yes
Consider Soil Lateral Resistance ?	Yes	Enter soil C (psf) or Phi (deg.):	30.0	Deg.
		Depth to ignor lateral resistance	1.0	Ft.







Apply 1.35 for e/w per G/H: 1.35

Foundation Analysis and Design:	Uplift Strength Reduction Factor:	0.75	Compression Strength Reduction Factor:	0.75			
Total Dry Soil Volume (cu. Ft.):		1719.55	Total Dry Soil Weight (Kips):	206.35			
Total Buoyant Soil Volume (cu. Ft.):		0.00	Total Buoyant Soil Weight (Kips):	0.00			
Total Effective Soil Weight (Kips):		206.35	Weight from the Concrete Block at Top (K):	0.00			
Total Dry Concrete Volume (cu. Ft.):		1473.30	Total Dry Concrete Weight (Kips):	220.99			
Total Buoyant Concrete Volume (cu. Ft.):		0.00	Total Buoyant Concrete Weight (Kips):	0.00			
Total Effective Concrete Weight (Kips):		220.99	Total Vertical Load on Base (Kips):	455.25			
Check Soil Capacities:						Load/ Capacity Ratio	
Calculated Maxium Net Soil Pressure under the base (psf):		3334.67	<	Allowable Factored Soil Bearing (psf):	9000	0.37	OK!
Allowable Foundation Overturning Resistance (kips-ft.):		4744.0	>	Design Factored Momont (kips-ft):	3550	0.75	OK!
Factor of Safety Against Overturning (O. R. Moment/Design Moment):		1.34	OK!				
Check the capacities of Reinforceing Concrete:							
Strength reduction factor (Flexure and axial tension):		0.90	Strength reduction factor (Shear):		0.75		
Strength reduction factor (Axial compresion):		0.65	Wind Load Factor on Concrete Design:		1.00		
(1) Concrete Pier:						Load/ Capacity Ratio	
	Vertical Steel Rebar Area (sq. in./each):	0.79		Tie / Stirrup Area (sq. in./each):	0.20		
	Calculated Moment Capacity (Mn,Kips-Ft):	459.6	>	Design Factored Moment (Mu, Kips-Ft)	106.8	0.23	OK!
	Calculated Shear Capacity (Kips):	215.1	>	Design Factored Shear (Kips):	27.4	0.13	OK!
	Calculated Tension Capacity (Tn, Kips):	511.9	>	Design Factored Tension (Tu Kips):	255.2	0.50	OK!
	Calculated Compression Capacity (Pn, Kips):	2386.9	>	Design Factored Axial Load (Pu Kips):	278.9	0.12	OK!
	Moment & Tension Strength Combination:	0.23	OK!	Check Tie Spacing (Design/Req'd):	0.50		
	Pier Reinforcement Ratio:	0.005		Reinforcement Ratio is too small			
(2).Concrete Pad:							
	One-Way Design Shear Capacity (L or W Direction, Kips):	600.9	>	One-Way Factored Shear (L/W-Dir Kips	155.1	0.26	OK!
	One-Way Design Shear Capacity (Diagonal Dir., Kips):	511.0	>	One-Way Factored Shear (Dia. Dir, Kips	189.7	0.37	OK!
	Lower Steel Pad Reinforcement Ratio (L or W-Direct.):	0.0028		Lower Steel Reinf. Ratio (Dia. Dir.):	0.0025		
	Lower Steel Pad Moment Capacity (L or W-Dir. Kips-ft):	2368.5	>	Moment at Bottom (L-Direct. K-Ft):	498.4	0.21	OK!
	Lower Steel Pad Moment Capacity (Dia. Direction,K-ft):	2291.7	>	Moment at Bottom (Dia. Dir. K-Ft):	1147.3	0.50	OK!
	Upper Steel Pad Reinforcement Ratio (L or W -Direction):	0.0028		Upper Steel Reinf. Ratio (Dia. Dir.):	0.0025		
	Upper Steel Pad Moment Capacity (L or W-Dir., Kips-ft):	2368.5	>	Moment at the top (L-Dir Kips-Ft):	194.4	0.08	OK!
	Upper Steel Pad Moment Capacity (Dia. Direction, K-ft):	2291.7	>	Moment at the top (Dia. Dir., K-Ft):	352.9	0.15	OK!
	Punching Failure Capacity (Kips):	1019.1	>	Punch. Failure Factored Shear (K):	278.9	0.27	OK!

EXHIBIT 8

Tower Engineering Solutions
Phone (972) 483-0607, Fax (972) 975-9615
1320 Greenway Drive, Suite 600, Irving, Texas 75038

Antenna Mount Analysis Report

Existing 150-Ft Self Support Tower

Customer Name: SBA Communications Corp

Customer Site Number: CT22071-A-SBA / Avon (Montevideo)

Customer Site Name: Avon (Montevideo)

Carrier Name: T-Mobile (App#: 134550, V1)

Carrier Site ID / Name: CT11284A / SBA Avon/RT 1777

Site Location: 81 Montevideo Road

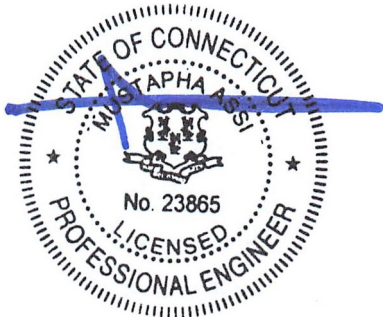
Avon, Connecticut

HARTFORD County

Latitude: 41.803100

Longitude: -72.801300

Exp.01/31/2021



07/08/2020

Analysis Result:

Max Structural Usage: 61.5%[Pass]

Report Prepared By: Neena Thapa

Introduction

The purpose of this report is to summarize the analysis results on the (1) Sector Frame at 136.00' elevation to support the proposed antenna configuration. Any modification listed under Sources of Information was assumed completed and was included in this analysis.

Sources of Information

Mount Drawings	Mount Mapping by SGS, dated 6/24/2020
Antenna Loading	Provided by SBA Application #: 134550, v1, dated 6/17/2020
Modification Drawings	N/A

Analysis Criteria

Basic Wind Speed Used in the Analysis: V_{ULT} = 120 mph (3-Sec. Gust) / Equivalent to
V_{ASD} = 93 mph (3-Sec. Gust)
Basic Wind Speed with Ice: 50 mph (3-Sec. Gust) with 1" radial ice concurrent
Operational Wind Speed: 60 mph +0" Radial ice
Standard/Codes: ANSI/TIA/EIA 222-G/2015 IBC/2018 CSBC
Exposure Category: B
Structure Class: II
Topographic Category: 4
Crest Height (Ft): 135

The site is a Risk Category II structure per IBC Table 1604.5. This site does not support emergency communication equipment for first responders such as fire departments, police, hospitals, ambulance services or any of the facilities listed for Risk Categories III and IV. The scope of work detailed in this structural analysis does not include items that are a part of emergency service as the 911 or essential facility service of an emergency response system.

Mount Information

(1) Sector Frame at 136.00'

Final Antenna Configuration

1	RFS APXVAARR24_43-U-NA20 (Octa)
1	Ericsson AIR32 KRD901146-1_B66A (Octa)
1	Ericsson AIR 21
1	Ericsson AIR6449 B41
1	Ericsson KRY 112 144/2
1	Ericsson Radio 4449 B71 + B12
1	Ericsson Radio 4415 B25

In addition to the proposed equipment loading, a 500 lb serviceability load was also considered in this analysis in accordance with TIA requirements.

Analysis Results

Our calculations have determined that under design wind load the existing mounts will be structurally adequate to support the proposed antenna configuration. The maximum structural usage is 61.5%, which occurs in the face horizontal. The proposed equipment must be installed as stipulated in the Final Antenna Configuration section of this report. The analysis results are void if the proposed equipment is not installed in accordance with this report.

Attachments

1. Mount Photos
2. Antenna Placement Diagram
3. Mount Mapping Information
4. Analysis Calculations

Standard Conditions

1. The loading configuration as analyzed in this report is as provided from the customer. Any deviation from this design shall be communicated to TES to verify deviation will not adversely impact the analysis.

2. The analysis is based on the presumption that the antenna mount members and components along with any existing reinforcement items have been correctly and properly designed, manufactured, installed and maintained.

3. All the existing structural members were assumed to be in good condition with no physical damage or deterioration associated with corrosion. The mount analysis is not a condition assessment of the mount.

4. The mount analysis was performed in accordance with the loading provided, and if applicable the modification required to support the additional loading.

5. If the mount is modified, installation must adhere to the configuration communicated in the modification drawings.

6. The modification drawings are not intended to convey means or methods. These are the responsibility of the installing contractor.

7. Rigging plan review is available if the contractor requires for a construction class IV or other if required. Review fee would apply.

8. The mount modification package was created based upon information provided for the mount loading. The underlying tower is assumed to provide support and sufficient rigidity to support the mount loads as a tower analysis was not part of the mount analysis.

9. TES is not responsible for modifications to climbing facilities unless communicated to TES in writing.


06/24/2020 14:22

Structure: CT22071-A-SBA - Avon (Montevideo)

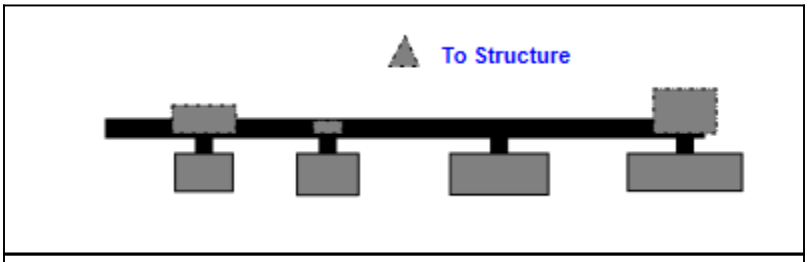
Sector: A

Structure Type: Self Support

Mount Elev: 136.00

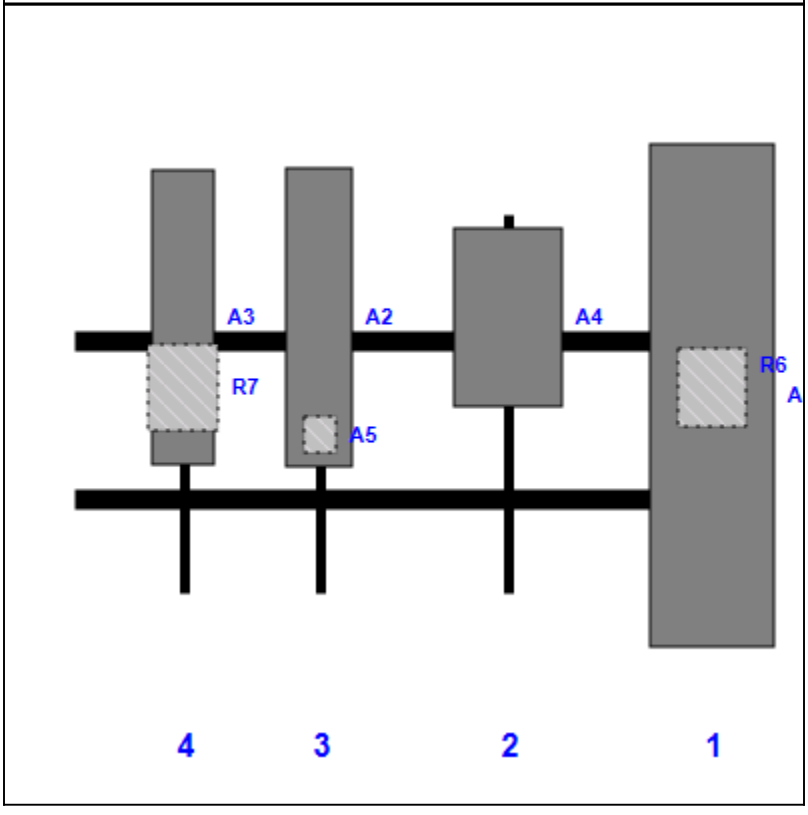
7/8/2020

Plan View



Front View

Looking Toward Structure



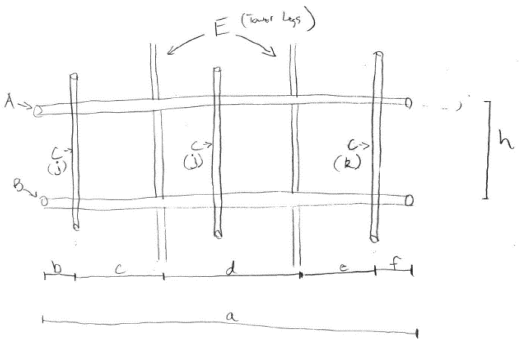
Ref #	Model	Height (in)	Width (in)	H Dist Left	Pipe #	Pipe Pos V	Pos	From Top	H Offset	Status	Validation
A1	RFS	95.90	24.00	122.0	1	a	Front	34.50			
R6	Ericsson Radio 4449 B71 +	15.00	13.20	122.0	1	a	Behind	33.00			
A4	Ericsson AIR6449 B41	33.90	20.60	83.00	2	a	Front	19.50			
A2	Ericsson AIR32	57.00	12.90	47.00	3	a	Front	19.50			
A5	Ericsson KRY 112 144/2	6.90	6.10	47.00	3	a	Behind	42.00			
A3	Ericsson AIR 21	56.00	12.10	21.00	4	a	Front	19.50			
R7	Ericsson Radio 4415 B25	16.50	13.40	21.00	4	a	Behind	33.00			

Antenna Mount Type "Other" Mapping Form (PATENT PENDING)

TES Tower Engineering Solutions

FCC # Valmont Structures

Tower Owner:	SBA Communications	**Mapping Date:**	6/24/20
Site Name:	Avon (Montevideo)	**Structure Type:**	3-Sided S.S. Tower
Site Number or ID:	CT22071	**Structure Height (Ft.):**	150
Mapping Contractor:	SGS	**Mount Height (Ft.):**	135



Geometries (Unit: inches)									
a	126	**e**	29	**j**	72	**o**		**s**	
b	21	**f**	4	**k**	96	**p**		**t**	
c	12	**g**		**m**		**q**		**u ***	17-21
d	60	**h**	36	**n**		**r**		**v ***	

Members (Unit: inches)						* - See Ant. Layout for "u", "v" and member "K" (pipe)			
Items	**Member**	**Lx (O.D.)**	**Ly (I.D.)**	**T**	**F**	**Member**	**Lx (O.D.)**	**Ly (I.D.)**	**T**
A	2.375 OD x 0.154 Pipe	2.375	2.067	0.154	**F**				
B	2.375 OD x 0.154 Pipe	2.375	2.067	0.154	**G**				
C	2.375 OD x 0.218 Pipe	2.375	1.939	0.218	**H**				
D					**J**				
E	1.75" Solid Rod	1.75	1.75	N/A	**K (pipe)***				

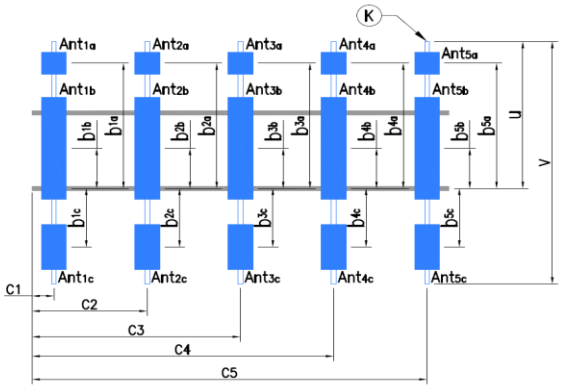
Distance from top of bottom support rail to lowest tip of ant./eqpt. of Carrier above. (N/A if > 10 ft.)	5'-4"
Distance from top of bottom support rail to highest tip of ant./eqpt. of Carrier below. (N/A if > 10 ft.)	N/A

Please enter the infomation below if members can't be found from the drop down lists

*Only one mount connected to C-A face of tower at 290 degree azimuths

Tower Face Width at the mount (ft.):	5'	Tower Leg Size at the mount (in.):	1.75"

Climbing facility is , at 230 Degree Azimuth

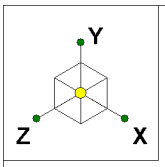


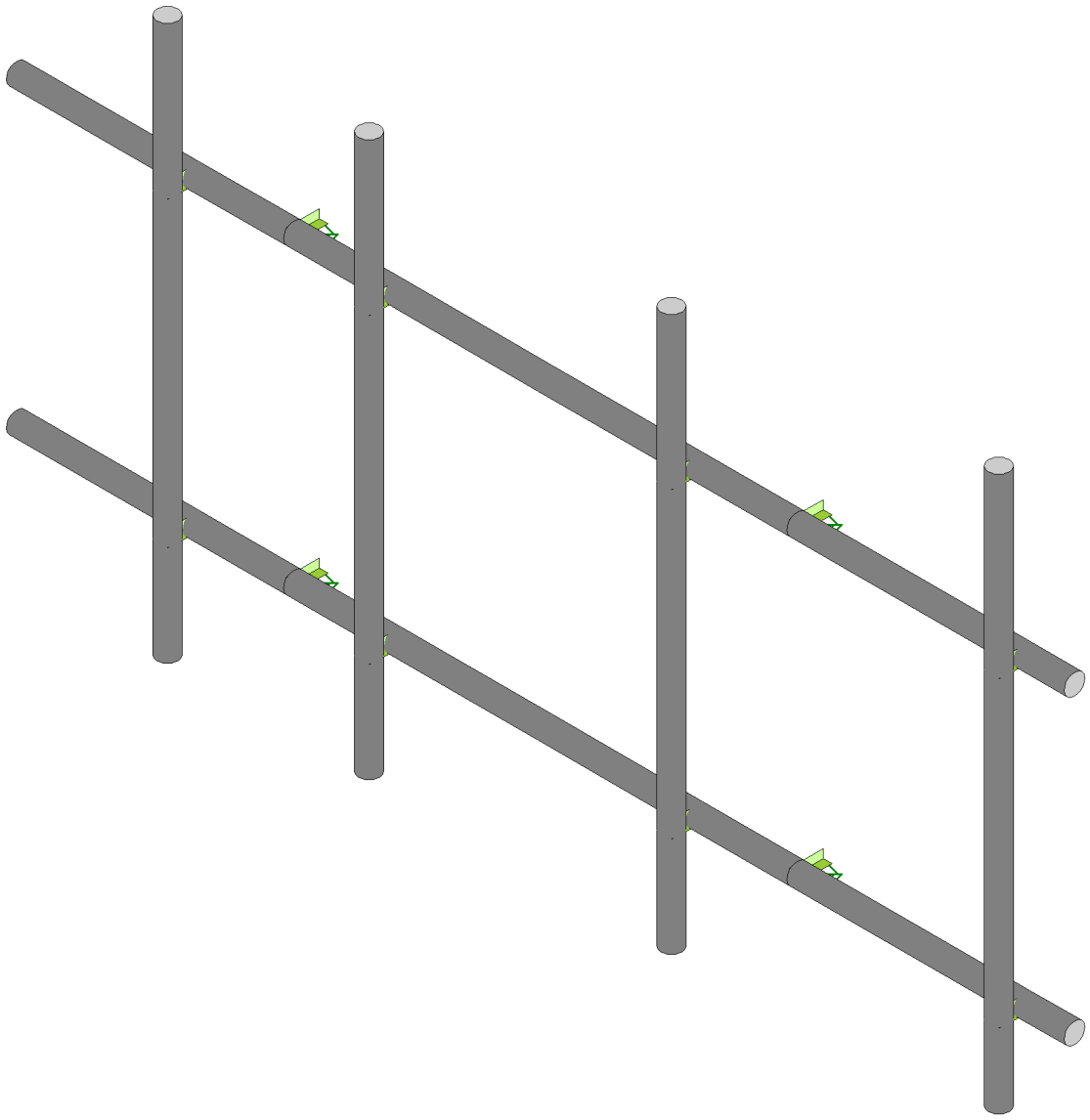
Antenna Layout

Azimuth (Degree) of Each Sector and Climbing Information

Sector A:		Deg	
Sector B:		Deg	
Sector C:	290	Deg	
Climbing	230	Deg	
Climbing Facility	Corrosion Type:	Good condition	
	Access:	Climbing path was obstructed.	
	Condition:	N/A	

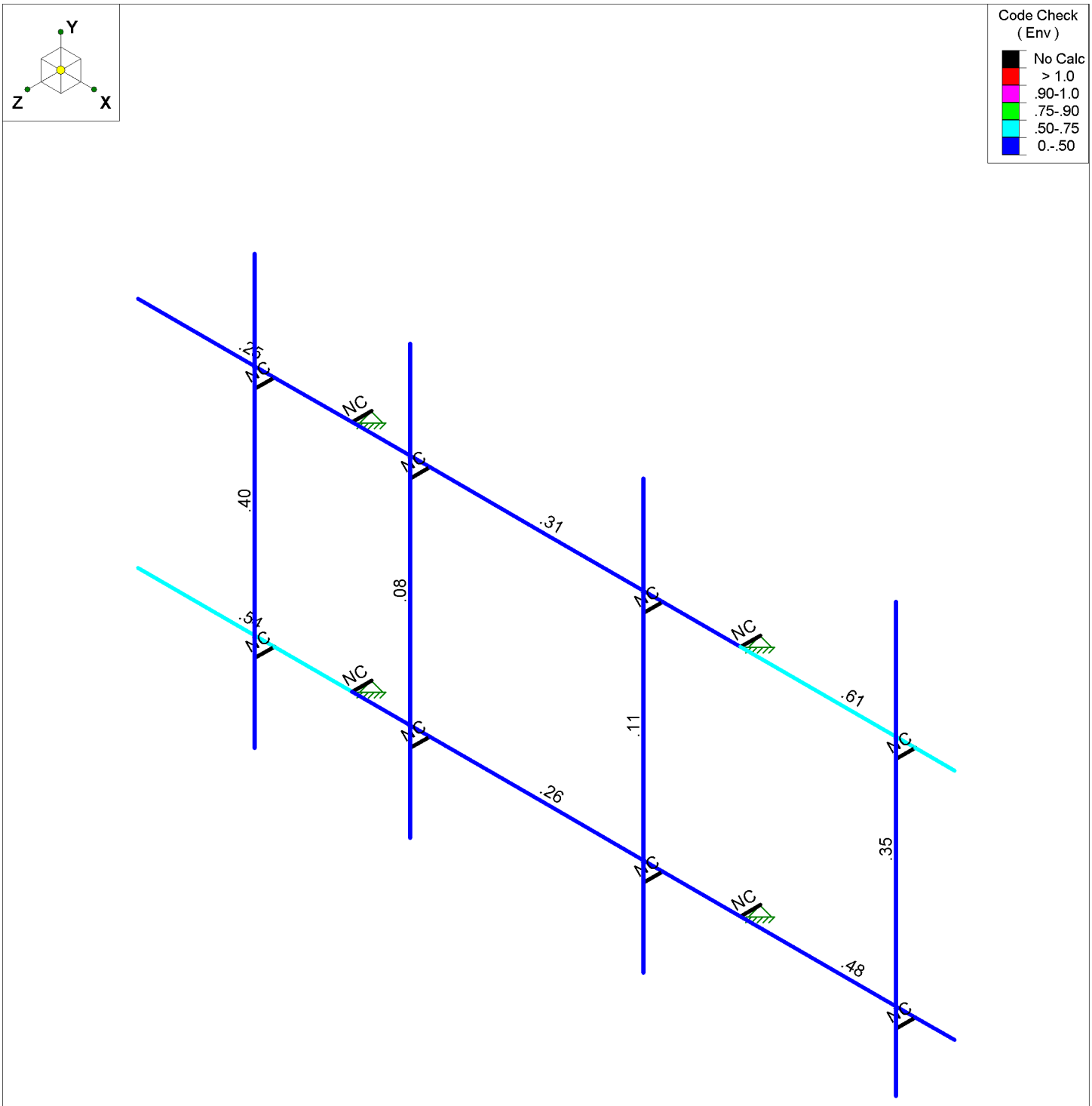
Ants. Items	Enter antenna model. If not labled, enter "Unknown". If no antenna at specified location, enter "N/A". If antennas and the locations are the same on all three sectors, only enter one sector.	Width (in.)	Depth (in.)	Height (in.)	Coax Size and Qty	Mounting Locations (Unit: inches) Vertical Distances"b_{1a}, b_{2a}, b_{3a}, b_{1b}...." (In.)	Horiz. offset (Use "-" if Ant. is inside)	Horiz. offset "C_1, C_2, C_3, C_4, C_5" (in.)	Photos of antennas Photo Numbers
	Sector A								
Ant_{1a}									
Ant_{1b}									
Ant_{1c}									
Ant_{2a}									
Ant_{2b}									
Ant_{2c}									
Ant_{3a}									
Ant_{3b}									
Ant_{3c}									
Ant_{4a}									
Ant_{4b}									
Ant_{4c}									
Ant_{5a}									
Ant_{5b}									
Ant_{5c}									
Are Ant same as sector A?		N/A	**Sector B**						
Ant_{1a}									
Ant_{1b}									
Ant_{1c}									
Ant_{2a}									
Ant_{2b}									
Ant_{2c}									
Ant_{3a}									
Ant_{3b}									
Ant_{3c}									
Ant_{4a}									
Ant_{4b}									
Ant_{4c}									
Ant_{5a}									
Ant_{5b}									
Ant_{5c}									





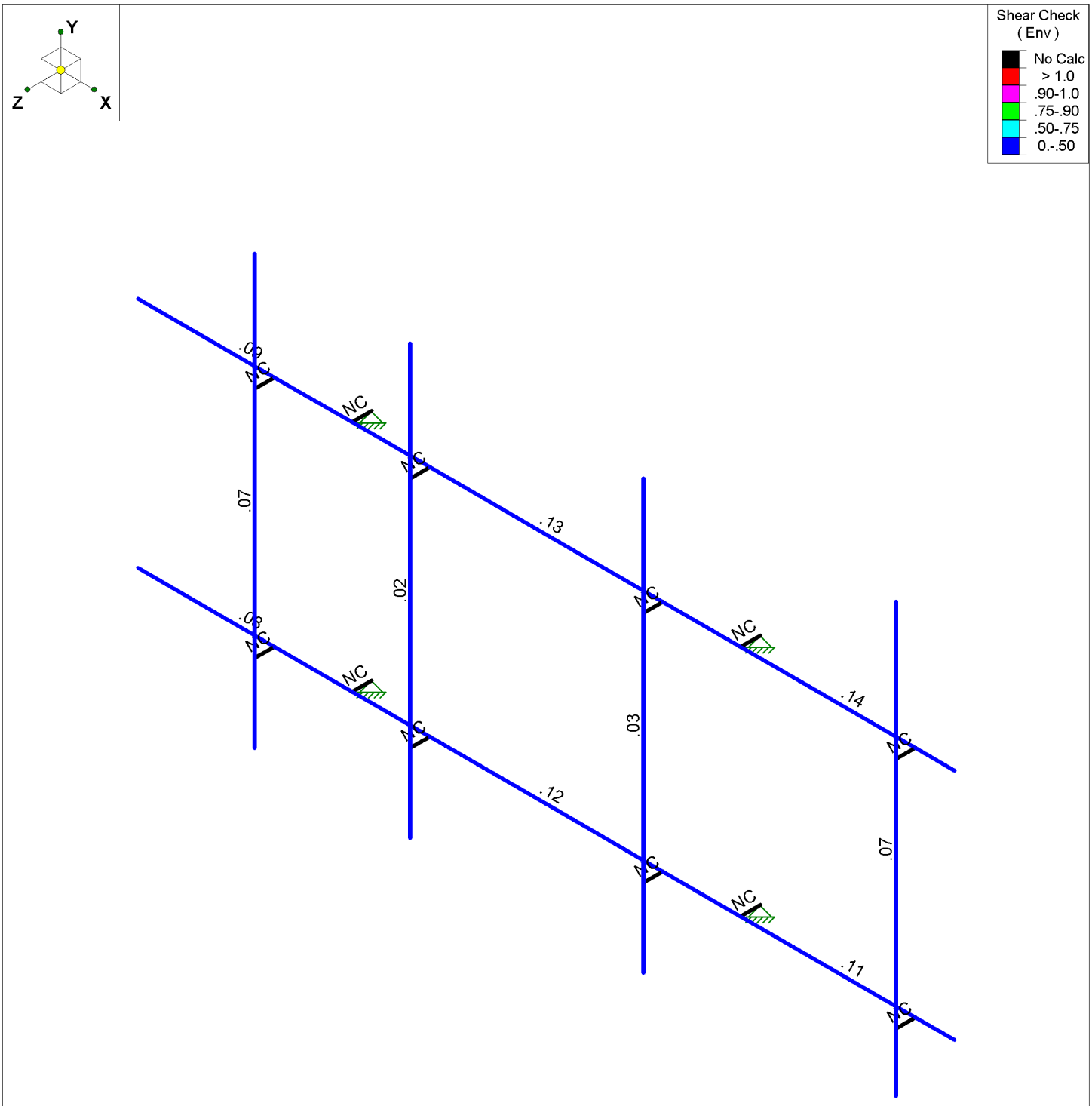
Loads: BLC 24,

Tower Engineering Solutio...	CT22071-A-SBA_MT_LOT_Loads Only_Sector A_G	SK - 1
		July 8, 2020 at 2:40 PM
TES Project No. 94597		CT22071-A-SBA_94597_G_RISA_L...



Member Code Checks Displayed (Enveloped)
Loads: BLC 24,
Results for LC 1, 1.2D+1.6W (Front)

Tower Engineering Solutio...	CT22071-A-SBA_MT_LOT_Loads Only_Sector A_G	SK - 2
		July 8, 2020 at 2:41 PM
TES Project No. 94597		CT22071-A-SBA_94597_G_RISA_L...



Member Shear Checks Displayed (Enveloped)
Loads: BLC 24,
Results for LC 1, 1.2D+1.6W (Front)

Tower Engineering Solutio...	CT22071-A-SBA_MT_LOT_Loads Only_Sector A_G	SK - 3
		July 8, 2020 at 2:41 PM
TES Project No. 94597		CT22071-A-SBA_94597_G_RISA_L...

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed	Area(Me...	Surface(P...
1	Antenna D	None					11			
2	Antenna Di	None					11			
3	Antenna W Front	None					11			
4	Antenna Wi Front	None					11			
5	Antenna W Side	None					11			
6	Antenna Wi Side	None					11			
7	Service Lm1	None					1			
8	Service Lm2	None					1			
9	Structure D	None		-1						
10	Structure Di	None						10		
11	Structure W Front	None						10		
12	Structure Wi Front	None						10		
13	Structure W Side	None						10		
14	Structure Wi Side	None						10		

Load Combinations

	Description	So..	P...	S...	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac..	BLC	Fac...
1	1.2D+1.6W (Front)	Yes	Y		1	1.2	9	1.2	3	1.6	11	1.6												
2	1.2D+1.6W (Back)	Yes	Y		1	1.2	9	1.2	3	-1.6	11	-1.6												
3	1.2D+1.6W (Left)	Yes	Y		1	1.2	9	1.2	5	1.6	13	1.6												
4	1.2D+1.6W (Right)	Yes	Y		1	1.2	9	1.2	5	-1.6	13	-1.6												
5	1.2D+1.0Di+1.0Wi (Fro...	Yes	Y		1	1.2	9	1.2	2	1	10	1	4	1	12	1								
6	1.2D+1.0Di+1.0Wi (Bac...	Yes	Y		1	1.2	9	1.2	2	1	10	1	4	-1	12	-1								
7	1.2D+1.0Di+1.0Wi (Left)	Yes	Y		1	1.2	9	1.2	2	1	10	1	6	1	14	1								
8	1.2D+1.0Di+1.0Wi (Rig...	Yes	Y		1	1.2	9	1.2	2	1	10	1	6	-1	14	-1								
9	1.2D+1.5L1+.16W (Mai...	Yes	Y		1	1.2	9	1.2	7	1.5	3	.16	11	.16										
10	1.2D+1.5L2+.16W (Mai...	Yes	Y		1	1.2	9	1.2	8	1.5	3	.16	11	.16										
11	1.4D	Yes	Y		1	1.4	9	1.4																

Joint Coordinates and Temperatures

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Diap...
1	N4	0	1.75	1.083333	0	
2	N5	0	4.7499	1.083333	0	
3	N9	-5.25	1.75	1.083333	0	
4	N10	-5.25	4.7499	1.083333	0	
5	N13	5.25	1.75	1.083333	0	
6	N14	5.25	4.7499	1.083333	0	
7	N15	-3.49967	1.75	1.333333	0	
8	N16	-3.49967	4.7499	1.333333	0	
9	N17	4.74967	1.75	1.333333	0	
10	N18	4.74967	4.7499	1.333333	0	
11	N19	-1.5	1.75	1.333333	0	
12	N20	-1.5	4.7499	1.333333	0	
13	N21	-3.49967	6.25	1.333333	0	
14	N22	4.74967	6.5	1.333333	0	

Joint Coordinates and Temperatures (Continued)

	Label	X [ft]	Y [ft]	Z [ft]	Temp [F]	Detach From Diap...
15	N23	-1.5	6.25	1.333333	0	
16	N24	-3.49967	.75	1.333333	0	
17	N25	4.74967	1	1.333333	0	
18	N26	-1.5	.75	1.333333	0	
19	N21A	-2.5	1.75	1.083333	0	
20	N22A	-2.5	4.7499	1.083333	0	
21	N23A	-2.5	1.75	0.833333	0	
22	N24A	-2.5	4.7499	0.833333	0	
23	N23B	2.5	1.75	1.083333	0	
24	N24B	2.5	4.7499	1.083333	0	
25	N25A	2.5	1.75	0.833333	0	
26	N26A	2.5	4.7499	0.833333	0	
27	N27	-3.49967	1.75	1.083333	0	
28	N28	-3.49967	4.7499	1.083333	0	
29	N29	4.74967	1.75	1.083333	0	
30	N30	4.74967	4.7499	1.083333	0	
31	N31	-1.5	1.75	1.083333	0	
32	N32	-1.5	4.7499	1.083333	0	
33	N33	1.5	1.75	1.333333	0	
34	N34	1.5	4.7499	1.333333	0	
35	N35	1.5	6.25	1.333333	0	
36	N36	1.5	.75	1.333333	0	
37	N37	1.5	1.75	1.083333	0	
38	N38	1.5	4.7499	1.083333	0	

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design R...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	M pipe	PIPE_2.0	Beam	Pipe	A500 Gr.B Rect	Typical	1.02	.627	.627	1.25
2	Stabilizer	PIPE_2.0	Beam	Pipe	A500 Gr.B Rect	Typical	1.02	.627	.627	1.25
3	FF	PIPE_2.0	Beam	Pipe	A500 Gr.B Rect	Typical	1.02	.627	.627	1.25
4	Main Pipe	PIPE_4.0	Beam	Pipe	A500 Gr.B Rect	Typical	2.96	6.82	6.82	13.6

Cold Formed Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rules	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	UNISTRUT	1.5CS1.5x063	Beam	CS	A570 Gr.33	Typical	.283	.076	.11	.000374

Aluminum Section Sets

	Label	Shape	Type	Design List	Material	Design Rules	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	AL1A	AACS14X13.9	Beam	AA Channel	3003-H14	Typical	11.8	44.7	401	1.19

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\1E...	Density[k/ft...	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3

Hot Rolled Steel Properties (Continued)

	Label	E [ksi]	G [ksi]	Nu	Therm (\1E..	Density[k/ft...	Yield[ksi]	Ry	Fu[ksi]	Rt
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3

Cold Formed Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (\1E5 F)	Density[k/ft^3]	Yield[ksi]	Fu[ksi]
1	A570 Gr.33	29500	11346	.3	.65	.49	33	52
2	A607 C1 Gr.55	29500	11346	.3	.65	.49	55	70

Aluminum Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (...	Density[...	Table B.4	kt	Ftu[ksi]	Fty[ksi]	Fcy[ksi]	Fsu[ksi]	Ct
1	3003-H14	10100	3787.5	.33	1.3	.173	Table B...	1	19	16	13	12	141
2	6061-T6	10100	3787.5	.33	1.3	.173	Table B...	1	38	35	35	24	141
3	6063-T5	10100	3787.5	.33	1.3	.173	Table B...	1	22	16	16	13	141
4	6063-T6	10100	3787.5	.33	1.3	.173	Table B...	1	30	25	25	19	141
5	5052-H34	10200	3787.5	.33	1.3	.173	Table B...	1	34	26	24	20	141
6	6061-T6 W	10100	3787.5	.33	1.3	.173	Table B...	1	24	15	15	15	141

Member Primary Data

	Label	I Joint	J Joint	K Joint	Rotate(d...	Section/Shape	Type	Design List	Material	Design Rul...
1	M1	N9	N21A			FF	Beam	Pipe	A500 Gr...	Typical
2	M2	N21A	N23B			FF	Beam	Pipe	A500 Gr...	Typical
3	M3	N10	N22A			FF	Beam	Pipe	A500 Gr...	Typical
4	M4	N22A	N24B			FF	Beam	Pipe	A500 Gr...	Typical
5	MP4A	N21	N24			M pipe	Beam	Pipe	A500 Gr...	Typical
6	MP3A	N23	N26			M pipe	Beam	Pipe	A500 Gr...	Typical
7	MP1A	N22	N25			M pipe	Beam	Pipe	A500 Gr...	Typical
8	M10	N24A	N22A			RIGID	Beam	None	RIGID	DR1
9	M11	N23A	N21A			RIGID	Beam	None	RIGID	DR1
10	M10A	N26A	N24B			RIGID	Beam	None	RIGID	DR1
11	M11A	N25A	N23B			RIGID	Beam	None	RIGID	DR1
12	M12	N28	N16			RIGID	Beam	None	RIGID	DR1
13	M13	N27	N15			RIGID	Beam	None	RIGID	DR1
14	M14	N32	N20			RIGID	Beam	None	RIGID	DR1
15	M15	N31	N19			RIGID	Beam	None	RIGID	DR1
16	M16	N30	N18			RIGID	Beam	None	RIGID	DR1
17	M17	N29	N17			RIGID	Beam	None	RIGID	DR1
18	M18	N24B	N14			PIPE_2.0	Beam	Pipe	A53 Gr.B	Typical
19	M19	N23B	N13			PIPE_2.0	Beam	Pipe	A53 Gr.B	Typical
20	MP2A	N35	N36			M pipe	Beam	Pipe	A500 Gr...	Typical
21	M21	N38	N34			RIGID	Beam	None	RIGID	DR1
22	M22	N37	N33			RIGID	Beam	None	RIGID	DR1

Member Advanced Data

	Label	I Release	J Release	I Offset[in]	J Offset[in]	T/C Only	Physical	Analysis ...	Inactive	Seismic Design ...
1	M1						Yes			None
2	M2						Yes			None
3	M3						Yes			None
4	M4						Yes			None
5	MP4A						Yes			None
6	MP3A						Yes			None
7	MP1A						Yes			None
8	M10						Yes			None
9	M11						Yes			None
10	M10A						Yes			None
11	M11A						Yes			None
12	M12						Yes			None
13	M13						Yes			None
14	M14						Yes			None
15	M15						Yes			None
16	M16						Yes			None
17	M17						Yes			None
18	M18						Yes			None
19	M19						Yes			None
20	MP2A						Yes			None
21	M21						Yes			None
22	M22						Yes			None

Hot Rolled Steel Design Parameters

	Label	Shape	Length[ft]	Lbyy[ft]	Lbzz[ft]	Lcomp top[ft]	Lcomp bot[ft]	L-torqu...	Kyy	Kzz	Cb	Function
1	M1	FF	2.75			Lbyy			2.1	2.1		Lateral
2	M2	FF	5			Lbyy			1	1		Lateral
3	M3	FF	2.75			Lbyy			2.1	2.1		Lateral
4	M4	FF	5			Lbyy			1	1		Lateral
5	MP4A	M pipe	5.5			Lbyy						Lateral
6	MP3A	M pipe	5.5			Lbyy						Lateral
7	MP1A	M pipe	5.5			Lbyy						Lateral
8	M18	PIPE_2.0	2.75			Lbyy			2.1	2.1		Lateral
9	M19	PIPE_2.0	2.75			Lbyy			2.1	2.1		Lateral
10	MP2A	M pipe	5.5			Lbyy						Lateral

Cold Formed Steel Design Parameters

Label	Shape	Lengt...	Lbyy[ft]	Lbzz[ft]	Lcomp t...	Lcomp ...	L-torque...	Kyy	Kzz	Cm-...	Cm-...	Cb	R	a[ft]	y sw...	z sw...
No Data to Print ...																

Aluminum Design Parameters

Label	Shape	Length[ft]	Lbyy[ft]	Lbzz[ft]	Lcomp top[ft]	Lcomp bot[ft]	L-torqu...	Kyy	Kzz	Cb	Function
No Data to Print ...											

Joint Loads and Enforced Displacements

Joint Label	L,D,M	Direction	Magnitude[(lb,k-ft), (in,rad), (lb*s^2...
No Data to Print ...			

Member Point Loads (BLC 1 : Antenna D)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	MP1A	Y	-64	.5
2	MP1A	Y	-64	5.25
3	MP3A	Y	-66.1	.25
4	MP3A	Y	-66.1	3
5	MP4A	Y	-45.5	.25
6	MP4A	Y	-45.5	3
7	MP2A	Y	-52	.25
8	MP2A	Y	-52	3
9	MP3A	Y	-11	3.5
10	MP1A	Y	-70	2.75
11	MP4A	Y	-46	2.75

Member Point Loads (BLC 2 : Antenna Di)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	MP1A	Y	-320.969	.5
2	MP1A	Y	-320.969	5.25
3	MP3A	Y	-145.098	.25
4	MP3A	Y	-145.098	3
5	MP4A	Y	-133.521	.25
6	MP4A	Y	-133.521	3
7	MP2A	Y	-124.169	.25
8	MP2A	Y	-124.169	3
9	MP3A	Y	-29.951	3.5
10	MP1A	Y	-110.988	2.75
11	MP4A	Y	-99.27	2.75

Member Point Loads (BLC 3 : Antenna W Front)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	MP1A	Z	-229.006	.5
2	MP1A	Z	-229.006	5.25
3	MP3A	Z	-73.658	.25
4	MP3A	Z	-73.658	3
5	MP4A	Z	-68.906	.25
6	MP4A	Z	-68.906	3
7	MP2A	Z	-65.851	.25
8	MP2A	Z	-65.851	3
9	MP3A	Z	-9.278	3.5
10	MP1A	Z	-37.338	2.75
11	MP4A	Z	-41.637	2.75

Member Point Loads (BLC 4 : Antenna Wi Front)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	MP1A	Z	-75.367	.5
2	MP1A	Z	-75.367	5.25

Member Point Loads (BLC 4 : Antenna Wi Front) (Continued)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
3	MP3A	Z	-26.97	.25
4	MP3A	Z	-26.97	3
5	MP4A	Z	-25.282	.25
6	MP4A	Z	-25.282	3
7	MP2A	Z	-23.831	.25
8	MP2A	Z	-23.831	3
9	MP3A	Z	-5.205	3.5
10	MP1A	Z	-16.152	2.75
11	MP4A	Z	-17.685	2.75

Member Point Loads (BLC 5 : Antenna W Side)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	MP1A	X	92.657	.5
2	MP1A	X	92.657	5.25
3	MP3A	X	53.774	.25
4	MP3A	X	53.774	3
5	MP4A	X	48.768	.25
6	MP4A	X	48.768	3
7	MP2A	X	28.957	.25
8	MP2A	X	28.957	3
9	MP3A	X	3.52	3.5
10	MP1A	X	26.306	2.75
11	MP4A	X	18.56	2.75

Member Point Loads (BLC 6 : Antenna Wi Side)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	MP1A	X	34.513	.5
2	MP1A	X	34.513	5.25
3	MP3A	X	20.584	.25
4	MP3A	X	20.584	3
5	MP4A	X	18.989	.25
6	MP4A	X	18.989	3
7	MP2A	X	11.923	.25
8	MP2A	X	11.923	3
9	MP3A	X	3.378	3.5
10	MP1A	X	12.335	2.75
11	MP4A	X	9.731	2.75

Member Point Loads (BLC 7 : Service Lm1)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	M1	Y	-500	0

Member Point Loads (BLC 8 : Service Lm2)

	Member Label	Direction	Magnitude[lb,k-ft]	Location[ft,%]
1	M19	Y	-500	%100

Member Distributed Loads (BLC 10 : Structure Di)

	Member Label	Direction	Start Magnitude[lb/ft,...	End Magnitude[lb/ft,F...	Start Location[ft,%]	End Location[ft,%]
1	M1	Y	-15.192	-15.192	0	%100
2	M2	Y	-15.192	-15.192	0	%100
3	M3	Y	-15.192	-15.192	0	%100
4	M4	Y	-15.192	-15.192	0	%100
5	MP4A	Y	-15.192	-15.192	0	%100
6	MP3A	Y	-15.192	-15.192	0	%100
7	MP1A	Y	-15.192	-15.192	0	%100
8	M18	Y	-15.192	-15.192	0	%100
9	M19	Y	-15.192	-15.192	0	%100
10	MP2A	Y	-15.192	-15.192	0	%100

Member Distributed Loads (BLC 11 : Structure W Front)

	Member Label	Direction	Start Magnitude[lb/ft,...	End Magnitude[lb/ft,F...	Start Location[ft,%]	End Location[ft,%]
1	M1	PZ	-5.374	-5.374	0	%100
2	M2	PZ	-5.374	-5.374	0	%100
3	M3	PZ	-5.374	-5.374	0	%100
4	M4	PZ	-5.374	-5.374	0	%100
5	MP4A	PZ	-5.374	-5.374	0	%100
6	MP3A	PZ	-5.374	-5.374	0	%100
7	MP1A	PZ	-5.374	-5.374	0	%100
8	M18	PZ	-5.374	-5.374	0	%100
9	M19	PZ	-5.374	-5.374	0	%100
10	MP2A	PZ	-5.374	-5.374	0	%100

Member Distributed Loads (BLC 12 : Structure Wi Front)

	Member Label	Direction	Start Magnitude[lb/ft,...	End Magnitude[lb/ft,F...	Start Location[ft,%]	End Location[ft,%]
1	M1	PZ	-4.868	-4.868	0	%100
2	M2	PZ	-4.868	-4.868	0	%100
3	M3	PZ	-4.868	-4.868	0	%100
4	M4	PZ	-4.868	-4.868	0	%100
5	MP4A	PZ	-4.868	-4.868	0	%100
6	MP3A	PZ	-4.868	-4.868	0	%100
7	MP1A	PZ	-4.868	-4.868	0	%100
8	M18	PZ	-4.868	-4.868	0	%100
9	M19	PZ	-4.868	-4.868	0	%100
10	MP2A	PZ	-4.868	-4.868	0	%100

Member Distributed Loads (BLC 13 : Structure W Side)

	Member Label	Direction	Start Magnitude[lb/ft,...	End Magnitude[lb/ft,F...	Start Location[ft,%]	End Location[ft,%]
1	M1	PX	5.374	5.374	0	%100
2	M2	PX	5.374	5.374	0	%100
3	M3	PX	5.374	5.374	0	%100
4	M4	PX	5.374	5.374	0	%100
5	MP4A	PX	5.374	5.374	0	%100
6	MP3A	PX	5.374	5.374	0	%100
7	MP1A	PX	5.374	5.374	0	%100
8	M18	PX	5.374	5.374	0	%100
9	M19	PX	5.374	5.374	0	%100
10	MP2A	PX	5.374	5.374	0	%100

Member Distributed Loads (BLC 14 : Structure Wi Side)

	Member Label	Direction	Start Magnitude[lb/ft,...	End Magnitude[lb/ft,F...	Start Location[ft,%]	End Location[ft,%]
1	M1	PX	4.868	4.868	0	%100
2	M2	PX	4.868	4.868	0	%100
3	M3	PX	4.868	4.868	0	%100
4	M4	PX	4.868	4.868	0	%100
5	MP4A	PX	4.868	4.868	0	%100
6	MP3A	PX	4.868	4.868	0	%100
7	MP1A	PX	4.868	4.868	0	%100
8	M18	PX	4.868	4.868	0	%100
9	M19	PX	4.868	4.868	0	%100
10	MP2A	PX	4.868	4.868	0	%100

Member Area Loads

Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[ksf]
No Data to Print ...						

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
1	N4						
2	N5						
3	N9						
4	N10						
5	N13						
6	N14						
7	N15						
8	N16						
9	N17						
10	N18						
11	N19						
12	N20						
13	N21						
14	N22						
15	N23						
16	N24						
17	N25						
18	N26						
19	N21A						
20	N22A						
21	N23A	Reaction	Reaction	Reaction			
22	N24A	Reaction	Reaction	Reaction			
23	N23B						
24	N24B						
25	N25A	Reaction	Reaction	Reaction			
26	N26A	Reaction	Reaction	Reaction			
27	N27						
28	N28						
29	N29						
30	N30						
31	N31						
32	N32						

Joint Boundary Conditions (Continued)

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot.[k-ft/rad]	Y Rot.[k-ft/rad]	Z Rot.[k-ft/rad]
33	N33						
34	N34						
35	N35						
36	N36						
37	N37						
38	N38						

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N23A	max	1640.109	2	643.199	6	255.202	5	0	1	0	1	0	1
2		min	-1607.133	1	116.664	10	28.318	3	0	1	0	1	0	1
3	N24A	max	1792.339	2	810.539	9	351.38	1	0	1	0	1	0	1
4		min	-1838.77	1	113.094	10	-489.181	2	0	1	0	1	0	1
5	N25A	max	1678.499	1	1013.513	6	614.271	1	0	1	0	1	0	1
6		min	-1547.882	2	64.78	1	-469.448	2	0	1	0	1	0	1
7	N26A	max	1767.406	1	1019.257	5	893.872	1	0	1	0	1	0	1
8		min	-1884.568	2	60.933	2	-1036.633	2	0	1	0	1	0	1
9	Totals:	max	983.895	4	3230.496	7	1910.742	1						
10		min	-983.895	3	889.529	1	-1910.742	2						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
1	M1	1	max	0	1	0	1	0	1	0	1	0	1	0	1
2			min	0	1	-750	9	0	1	0	1	0	1	0	1
3		2	max	0	1	-3.08	3	5.912	2	0	1	.002	2	.517	9
4			min	0	1	-753.08	9	-5.912	1	0	1	-.002	1	.001	3
5		3	max	0	1	-6.159	3	11.824	2	0	1	.008	2	1.035	9
6			min	0	1	-756.159	9	-11.824	1	0	1	-.008	1	.004	3
7		4	max	532.439	9	-83.137	1	39.447	2	-.014	10	.056	2	.374	9
8			min	-11.987	4	-363.082	6	-139.744	5	-.154	9	-.054	1	-.059	7
9		5	max	532.439	9	-86.217	1	45.359	2	-.014	10	.085	2	.455	9
10			min	-11.987	4	-376.606	6	-143.09	5	-.154	9	-.128	1	.022	10
11	M2	1	max	1674.618	2	334.556	9	128.995	8	.077	6	.274	1	.455	9
12			min	-1574.083	1	7.136	10	-61.86	1	-.041	9	-.325	2	.022	10
13		2	max	1666.983	2	23.112	9	148.515	2	.002	4	.183	1	.046	9
14			min	-1569.175	1	-89.48	10	-142.094	1	-.015	7	-.178	2	-.081	7
15		3	max	1666.983	2	17.604	9	159.264	2	.002	4	.027	5	.043	10
16			min	-1569.175	1	-95.079	10	-152.918	1	-.015	7	-.002	1	-.011	2
17		4	max	1666.983	2	12.004	9	169.621	2	.002	4	.22	2	.165	10
18			min	-1569.175	1	-114.242	7	-163.667	1	-.015	7	-.2	1	-.001	2
19		5	max	1660.173	2	24.236	1	140.225	2	-.038	2	.418	2	.626	6
20			min	-1574.115	1	-401.636	6	-226.067	1	-.185	7	-.472	1	.069	1
21	M3	1	max	0	1	0	1	0	1	0	1	0	1	0	1
22			min	0	1	0	1	0	1	0	1	0	1	0	1
23		2	max	0	1	-3.08	2	5.912	2	0	1	.002	2	.005	6
24			min	0	1	-13.524	8	-5.912	1	0	1	-.002	1	.001	4
25		3	max	0	1	-6.159	2	11.824	2	0	1	.008	2	.019	8
26			min	0	1	-27.048	8	-11.824	1	0	1	-.008	1	.004	2
27		4	max	153.558	3	-83.736	2	330.415	2	-.016	3	.085	2	.004	4

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
28			min	-532.439	9	-842.028	9	-264.678	1	-.087	8	-.087	1	-.347	9
29		5	max	153.558	3	-86.816	2	336.327	2	-.016	3	.314	2	.233	9
30			min	-532.439	9	-845.108	9	-270.59	1	-.087	8	-.271	1	.018	10
31	M4	1	max	1757.831	2	271.483	8	77.939	1	.151	9	.189	1	.233	9
32			min	-1871.82	1	-35.086	9	-163.256	6	.007	2	-.133	2	.018	10
33		2	max	1765.466	2	67.793	9	151.317	2	.021	9	.214	1	.132	9
34			min	-1876.729	1	-101.743	10	-152.928	1	-.021	10	-.225	2	-.083	7
35		3	max	1765.466	2	62.193	9	162.793	2	.021	9	.016	1	.051	9
36			min	-1876.729	1	-107.779	10	-163.676	1	-.021	10	-.034	6	-.013	1
37		4	max	1765.466	2	56.11	9	173.542	2	.021	9	.181	2	.189	10
38			min	-1876.729	1	-115.424	7	-174.008	1	-.021	10	-.194	1	-.023	9
39		5	max	1772.276	2	31.11	2	472.672	2	.034	1	.673	2	.629	5
40			min	-1871.789	1	-404.575	5	-400.864	1	-.2	6	-.625	1	.058	2
41	MP4A	1	max	0	1	.13	8	.029	1	0	1	0	1	0	1
42			min	0	1	-.016	3	-.134	6	0	1	0	1	0	1
43		2	max	215.169	6	89.883	4	122.101	1	0	1	.132	1	.096	3
44			min	60.759	4	-89.868	3	-122.119	2	0	1	-.132	2	-.096	4
45		3	max	79.834	6	22.241	3	127.973	6	.062	9	.031	2	.049	9
46			min	-712.575	9	-532.639	9	-46.497	1	-.022	2	-.159	9	-.041	7
47		4	max	295.291	6	.165	4	123.035	5	.062	9	.126	6	.782	9
48			min	-651.712	9	-532.639	9	-9.971	2	-.022	2	-.04	9	.032	3
49		5	max	0	1	0	3	.01	6	0	1	0	1	0	1
50			min	0	1	-.016	9	-.004	9	0	1	0	1	0	1
51	MP3A	1	max	0	1	.051	8	.051	1	0	1	0	1	0	1
52			min	0	1	-.031	3	-.177	6	0	1	0	1	0	1
53		2	max	251.466	7	97.893	4	129.727	1	0	1	.141	1	.105	3
54			min	85.479	9	-97.893	3	-129.754	2	0	1	-.141	2	-.105	4
55		3	max	201.323	9	61.177	3	150.437	2	.025	6	-.003	2	.018	4
56			min	-24.307	8	-76.261	10	-83.333	1	-.009	1	-.045	9	-.041	9
57		4	max	300.002	9	45.124	4	119.215	5	.025	6	.126	6	.084	10
58			min	83.816	4	-76.261	10	5.916	2	-.009	1	.018	9	-.016	2
59		5	max	0	1	0	2	.011	6	0	1	0	1	0	1
60			min	0	1	-.002	5	0	1	0	1	0	1	0	1
61	MP1A	1	max	0	1	.042	4	.279	1	0	1	0	1	0	1
62			min	0	1	-.867	7	-.746	6	0	1	0	1	0	1
63		2	max	424.817	5	160.117	4	378.513	1	0	1	.329	1	.138	3
64			min	82.959	2	-160.187	3	-378.553	2	0	1	-.329	2	-.138	4
65		3	max	105.139	6	537.017	10	145.056	6	.063	2	.196	1	.241	7
66			min	-391.65	10	84.497	4	-22.868	1	-.055	1	-.229	2	.015	4
67		4	max	132.603	6	537.017	10	138.328	6	.063	2	.173	1	-.059	1
68			min	-385.312	10	96.411	4	-10.917	1	-.055	1	-.116	2	-.539	10
69		5	max	0	1	.753	8	.449	5	0	1	0	1	0	1
70			min	0	1	.014	3	-.041	2	0	1	0	1	0	1
71	M10	1	max	351.38	1	810.359	9	1839.684	1	0	1	0	2	0	7
72			min	-489.181	2	113.077	10	-1791.056	2	0	1	0	1	0	1
73		2	max	351.38	1	810.359	9	1839.684	1	0	1	.115	1	-.007	10
74			min	-489.181	2	113.077	10	-1791.056	2	0	1	-.112	2	-.051	9
75		3	max	351.38	1	810.359	9	1839.684	1	0	1	.23	1	-.014	10
76			min	-489.181	2	113.077	10	-1791.056	2	0	1	-.224	2	-.101	9
77		4	max	351.38	1	810.359	9	1839.684	1	0	1	.345	1	-.021	10
78			min	-489.181	2	113.077	10	-1791.056	2	0	1	-.336	2	-.152	9
79		5	max	351.38	1	810.359	9	1839.684	1	0	1	.46	1	-.028	10

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
80			min	-489.181	2	113.077	10	-1791.056	2	0	1	-.448	2	-.203	9
81	M11	1	max	255.202	5	643.577	6	1607.259	1	0	1	0	1	0	2
82			min	28.318	3	116.695	10	-1640.319	2	0	1	0	2	0	8
83		2	max	255.202	5	643.577	6	1607.259	1	0	1	.1	1	-.007	10
84			min	28.318	3	116.695	10	-1640.319	2	0	1	-.103	2	-.04	6
85		3	max	255.202	5	643.577	6	1607.259	1	0	1	.201	1	-.015	10
86			min	28.318	3	116.695	10	-1640.319	2	0	1	-.205	2	-.08	6
87		4	max	255.202	5	643.577	6	1607.259	1	0	1	.301	1	-.022	10
88			min	28.318	3	116.695	10	-1640.319	2	0	1	-.308	2	-.121	6
89		5	max	255.202	5	643.577	6	1607.259	1	0	1	.402	1	-.029	10
90			min	28.318	3	116.695	10	-1640.319	2	0	1	-.41	2	-.161	6
91	M10A	1	max	893.872	1	1019.162	5	1888.834	2	0	1	0	3	0	1
92			min	-1036.633	2	58.112	2	-1763.667	1	0	1	0	1	0	2
93		2	max	893.872	1	1019.162	5	1888.834	2	0	1	.118	2	-.004	2
94			min	-1036.633	2	58.112	2	-1763.667	1	0	1	-.11	1	-.064	5
95		3	max	893.872	1	1019.162	5	1888.834	2	0	1	.236	2	-.007	2
96			min	-1036.633	2	58.112	2	-1763.667	1	0	1	-.22	1	-.127	5
97		4	max	893.872	1	1019.162	5	1888.834	2	0	1	.354	2	-.011	2
98			min	-1036.633	2	58.112	2	-1763.667	1	0	1	-.331	1	-.191	5
99		5	max	893.872	1	1019.162	5	1888.834	2	0	1	.472	2	-.015	2
100			min	-1036.633	2	58.112	2	-1763.667	1	0	1	-.441	1	-.255	5
101	M11A	1	max	614.271	1	1013.963	6	1549.709	2	0	1	0	1	0	7
102			min	-469.448	2	65.225	1	-1676.163	1	0	1	0	2	0	1
103		2	max	614.271	1	1013.963	6	1549.709	2	0	1	.097	2	-.004	1
104			min	-469.448	2	65.225	1	-1676.163	1	0	1	-.105	1	-.063	6
105		3	max	614.271	1	1013.963	6	1549.709	2	0	1	.194	2	-.008	1
106			min	-469.448	2	65.225	1	-1676.163	1	0	1	-.21	1	-.127	6
107		4	max	614.271	1	1013.963	6	1549.709	2	0	1	.291	2	-.012	1
108			min	-469.448	2	65.225	1	-1676.163	1	0	1	-.314	1	-.19	6
109		5	max	614.271	1	1013.963	6	1549.709	2	0	1	.387	2	-.016	1
110			min	-469.448	2	65.225	1	-1676.163	1	0	1	-.419	1	-.253	6
111	M12	1	max	246.889	1	834.471	9	153.521	3	.616	9	.071	9	-.016	3
112			min	-312.625	2	74.243	2	-532.392	9	.032	4	-.022	3	-.087	8
113		2	max	246.889	1	834.471	9	153.521	3	.616	9	.039	8	-.022	3
114			min	-312.625	2	74.243	2	-532.392	9	.032	4	-.013	3	-.107	8
115		3	max	246.889	1	834.471	9	153.521	3	.616	9	.029	6	-.029	10
116			min	-312.625	2	74.243	2	-532.392	9	.032	4	-.012	1	-.156	9
117		4	max	246.889	1	834.471	9	153.521	3	.616	9	.024	2	-.034	10
118			min	-312.625	2	74.243	2	-532.392	9	.032	4	-.029	9	-.208	9
119		5	max	246.889	1	834.471	9	153.521	3	.616	9	.022	2	-.04	10
120			min	-312.625	2	74.243	2	-532.392	9	.032	4	-.062	9	-.26	9
121	M13	1	max	129.733	5	322.432	6	532.54	9	.981	9	.013	4	-.014	10
122			min	-21.785	2	-645.721	9	-12.023	4	.046	1	-.071	9	-.154	9
123		2	max	129.733	5	322.432	6	532.54	9	.981	9	.012	4	-.02	10
124			min	-21.785	2	-645.721	9	-12.023	4	.046	1	-.039	7	-.114	9
125		3	max	129.733	5	322.432	6	532.54	9	.981	9	.012	1	-.026	10
126			min	-21.785	2	-645.721	9	-12.023	4	.046	1	-.03	7	-.126	8
127		4	max	129.733	5	322.432	6	532.54	9	.981	9	.029	9	-.032	10
128			min	-21.785	2	-645.721	9	-12.023	4	.046	1	-.024	2	-.146	8
129		5	max	129.733	5	322.432	6	532.54	9	.981	9	.062	9	.007	9
130			min	-21.785	2	-645.721	9	-12.023	4	.046	1	-.022	2	-.166	8
131	M14	1	max	224.838	1	302.707	8	170.861	3	.121	9	.033	4	-.018	2

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
132			min	-291.946	2	-109.707	9	-168.05	4	-.056	4	-.045	3	-.131	9
133		2	max	224.838	1	302.707	8	170.861	3	.121	9	.023	4	-.024	2
134			min	-291.946	2	-109.707	9	-168.05	4	-.056	4	-.035	3	-.124	9
135		3	max	224.838	1	302.707	8	170.861	3	.121	9	.012	4	-.029	2
136			min	-291.946	2	-109.707	9	-168.05	4	-.056	4	-.025	7	-.128	5
137		4	max	224.838	1	302.707	8	170.861	3	.121	9	.009	1	-.035	2
138			min	-291.946	2	-109.707	9	-168.05	4	-.056	4	-.024	6	-.147	5
139		5	max	224.838	1	302.707	8	170.861	3	.121	9	.009	1	-.041	2
140			min	-291.946	2	-109.707	9	-168.05	4	-.056	4	-.025	6	-.166	5
141	M15	1	max	125.926	5	306.169	9	76.271	10	.112	10	.023	4	.04	9
142			min	-5.898	2	89.985	4	-56.946	4	-.023	4	-.011	3	-.09	6
143		2	max	125.926	5	306.169	9	76.271	10	.112	10	.022	8	.021	9
144			min	-5.898	2	89.985	4	-56.946	4	-.023	4	-.01	1	-.109	6
145		3	max	125.926	5	306.169	9	76.271	10	.112	10	.023	6	.002	9
146			min	-5.898	2	89.985	4	-56.946	4	-.023	4	-.01	1	-.127	6
147		4	max	125.926	5	306.169	9	76.271	10	.112	10	.024	6	-.017	9
148			min	-5.898	2	89.985	4	-56.946	4	-.023	4	-.009	1	-.146	7
149		5	max	125.926	5	306.169	9	76.271	10	.112	10	.025	6	-.036	9
150			min	-5.898	2	89.985	4	-56.946	4	-.023	4	-.009	1	-.165	7
151	M16	1	max	472.519	1	564.57	10	536.865	10	-.074	3	.027	1	.11	2
152			min	-537.689	2	76.406	2	-129.339	4	-.736	10	-.146	7	-.143	1
153		2	max	472.519	1	564.57	10	536.865	10	-.074	3	.034	1	.106	2
154			min	-537.689	2	76.406	2	-129.339	4	-.736	10	-.116	6	-.154	1
155		3	max	472.519	1	564.57	10	536.865	10	-.074	3	.041	1	.101	2
156			min	-537.689	2	76.406	2	-129.339	4	-.736	10	-.087	6	-.166	1
157		4	max	472.519	1	564.57	10	536.865	10	-.074	3	.048	1	.096	2
158			min	-537.689	2	76.406	2	-129.339	4	-.736	10	-.071	2	-.193	5
159		5	max	472.519	1	564.57	10	536.865	10	-.074	3	.055	1	.091	2
160			min	-537.689	2	76.406	2	-129.339	4	-.736	10	-.063	2	-.228	5
161	M17	1	max	367.386	1	557.518	6	39.879	3	-.095	4	.145	6	.037	1
162			min	-302.216	2	-302.452	10	-536.838	10	-.875	10	-.03	1	-.095	10
163		2	max	367.386	1	557.518	6	39.879	3	-.095	4	.116	6	.032	1
164			min	-302.216	2	-302.452	10	-536.838	10	-.875	10	-.036	1	-.106	6
165		3	max	367.386	1	557.518	6	39.879	3	-.095	4	.087	6	.027	1
166			min	-302.216	2	-302.452	10	-536.838	10	-.875	10	-.042	1	-.141	6
167		4	max	367.386	1	557.518	6	39.879	3	-.095	4	.07	2	.022	1
168			min	-302.216	2	-302.452	10	-536.838	10	-.875	10	-.048	1	-.176	6
169		5	max	367.386	1	557.518	6	39.879	3	-.095	4	.063	2	.017	1
170			min	-302.216	2	-302.452	10	-536.838	10	-.875	10	-.055	1	-.211	6
171	M18	1	max	129.4	4	612.87	5	494.899	1	.143	1	1.145	2	.629	5
172			min	-536.912	10	90.588	2	-559.949	2	-.11	2	-1.066	1	.058	2
173		2	max	129.4	4	599.562	5	488.987	1	.143	1	.763	2	.212	5
174			min	-536.912	10	87.725	2	-554.037	2	-.11	2	-.728	1	-.003	2
175		3	max	129.4	4	586.255	5	483.075	1	.143	1	.384	2	.009	3
176			min	-536.912	10	84.861	2	-548.126	2	-.11	2	-.394	1	-.241	10
177		4	max	129.4	4	572.947	5	477.164	1	.143	1	.025	4	-.056	3
178			min	-536.912	10	81.998	2	-542.214	2	-.11	2	-.121	7	-.63	10
179		5	max	0	1	0	1	0	1	0	1	0	1	0	1
180			min	0	1	0	1	0	1	0	1	0	1	0	1
181	M19	1	max	536.912	10	613.787	6	392.134	1	.095	10	.805	2	.626	6
182			min	-39.81	3	90.31	1	-327.002	2	-.037	1	-.891	1	.069	1
183		2	max	536.912	10	600.479	6	386.222	1	.095	10	.583	2	.214	10

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Momen...	LC	z-z Momen...	LC
184			min	-39.81	3	87.447	1	-321.09	2	-.037	1	-.624	1	.008	1
185		3	max	536.912	10	587.172	6	380.31	1	.095	10	.364	2	-.012	4
186			min	-39.81	3	84.583	1	-315.179	2	-.037	1	-.36	1	-.207	7
187		4	max	536.912	10	573.864	6	374.398	1	.095	10	.149	2	-.077	4
188			min	-39.81	3	81.72	1	-309.267	2	-.037	1	-.101	1	-.605	7
189		5	max	0	1	750	10	0	1	0	1	0	1	0	1
190			min	0	1	0	1	0	1	0	1	0	1	0	1
191	MP2A	1	max	0	1	.009	4	.045	1	0	1	0	1	0	1
192			min	0	1	-.062	7	-.199	6	0	1	0	1	0	1
193		2	max	213.617	5	58.163	4	117.23	1	0	1	.127	1	.06	3
194			min	68.559	9	-58.168	3	-117.256	2	0	1	-.127	2	-.06	4
195		3	max	145.059	2	98.196	10	182.565	6	.009	1	.005	1	.031	10
196			min	-146.894	1	-20.003	4	-81.725	1	-.029	6	-.041	6	-.017	1
197		4	max	244.514	6	98.196	10	155.087	5	.009	1	.178	6	.01	2
198			min	-78.335	1	-26.379	3	35.46	1	-.029	6	.02	1	-.104	10
199		5	max	0	1	.005	5	.015	6	0	1	0	1	0	1
200			min	0	1	0	2	0	1	0	1	0	1	0	1
201	M21	1	max	210.729	1	270.153	5	101.748	3	.035	3	.029	4	.028	1
202			min	-293.267	2	-70.76	2	-89.982	4	-.154	10	-.019	3	-.185	6
203		2	max	210.729	1	270.153	5	101.748	3	.035	3	.023	4	.014	1
204			min	-293.267	2	-70.76	2	-89.982	4	-.154	10	-.013	3	-.198	6
205		3	max	210.729	1	270.153	5	101.748	3	.035	3	.023	8	0	1
206			min	-293.267	2	-70.76	2	-89.982	4	-.154	10	-.009	1	-.211	6
207		4	max	210.729	1	270.153	5	101.748	3	.035	3	.025	6	-.013	1
208			min	-293.267	2	-70.76	2	-89.982	4	-.154	10	-.009	1	-.225	6
209		5	max	210.729	1	270.153	5	101.748	3	.035	3	.029	6	-.027	1
210			min	-293.267	2	-70.76	2	-89.982	4	-.154	10	-.009	1	-.238	6
211	M22	1	max	161.797	5	271.779	6	38.2	3	.009	3	.01	1	-.039	2
212			min	35.251	2	-72.167	1	-98.222	10	-.141	10	-.02	2	-.17	7
213		2	max	161.797	5	271.779	6	38.2	3	.009	3	.009	1	-.043	1
214			min	35.251	2	-72.167	1	-98.222	10	-.141	10	-.02	2	-.187	6
215		3	max	161.797	5	271.779	6	38.2	3	.009	3	.009	1	-.038	1
216			min	35.251	2	-72.167	1	-98.222	10	-.141	10	-.022	6	-.204	6
217		4	max	161.797	5	271.779	6	38.2	3	.009	3	.009	1	-.034	1
218			min	35.251	2	-72.167	1	-98.222	10	-.141	10	-.025	6	-.221	6
219		5	max	161.797	5	271.779	6	38.2	3	.009	3	.009	1	-.029	1
220			min	35.251	2	-72.167	1	-98.222	10	-.141	10	-.029	6	-.238	6

Envelope AISC 14th(360-10): LRFD Steel Code Checks

	Member	Shape	Code Check	Loc[..	LC	Shear ...	Loc[..	Dir	LC	phi*Pn...	phi*Pnt..	phi*Mn...	phi*Mn...	Cb	Eqn
1	M18	PIPE_2.0	.615	0	2	.141	0		1	21540....	32130	1.872	1.872	1...	H1-1b
2	M1	PIPE_2.0	.536	1.747	9	.082	2.75		9	24966....	42228	2.46	2.46	1...	H1-1b
3	M19	PIPE_2.0	.480	0	1	.106	0		6	21540....	32130	1.872	1.872	1...	H1-1b
4	MP4A	PIPE_2.0	.400	4.469	9	.071	4.469		9	26216....	42228	2.46	2.46	2...	H1-1b
5	MP1A	PIPE_2.0	.349	4.698	10	.065	4.698		10	26216....	42228	2.46	2.46	2...	H1-1b
6	M4	PIPE_2.0	.306	5	2	.125	5		6	28478	42228	2.46	2.46	2...	H1-1b
7	M2	PIPE_2.0	.265	5	6	.117	5		7	28478	42228	2.46	2.46	3...	H1-1b
8	M3	PIPE_2.0	.247	1.776	9	.090	2.75		9	24966....	42228	2.46	2.46	2...	H1-1b
9	MP2A	PIPE_2.0	.112	1.547	6	.028	1.547		6	26216....	42228	2.46	2.46	2...	H1-1b
10	MP3A	PIPE_2.0	.081	1.547	6	.023	1.547		6	26216....	42228	2.46	2.46	2...	H1-1b

Envelope AISI S100-10: LRFD Cold Formed Steel Code Checks

Member	Shape	Code Check	Loc[ft]	LC	Shea..	Loc[ft]	Dir	LC	phi*Pn...	phi*Tn...	phi*M...	phi*M...	Cb	Cmyy	Cmzz	Eqn
No Data to Print ...																

Envelope AA ADM1-10: ASD - Building Aluminum Code Checks

Member	Shape	Code C...	Loc[ft]	LC	Shear ...	Loc[ft]	Dir	LC	Pnc/O...	Pnt/Om...	Mny/O...	Mnz/O...	Vny/O...	Vnz/O...	Cb	Eqn
No Data to Print ...																

EXHIBIT 9

EBI Consulting

environmental | engineering | due diligence

RADIO FREQUENCY EMISSIONS ANALYSIS REPORT
EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CT11284A

Avon_1
81 Montevideo Road
Avon, Connecticut 06001

July 9, 2020

EBI Project Number: 6220002987

Site Compliance Summary	
Compliance Status:	**COMPLIANT**
Site total MPE% of FCC general population allowable limit:	**13.69%**

July 9, 2020

T-Mobile
Attn: Jason Overbey, RF Manager
35 Griffin Road South
Bloomfield, Connecticut 06002

Emissions Analysis for Site: CT11284A - Avon_1

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **81 Montevideo Road** in **Avon, Connecticut** for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ($\mu W/cm^2$). The number of $\mu W/cm^2$ calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits; therefore, it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ($\mu W/cm^2$). The general population exposure limits for the 600 MHz and 700 MHz frequency bands are approximately 400 $\mu W/cm^2$ and 467 $\mu W/cm^2$, respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS) and 11 GHz frequency bands is 1000 $\mu W/cm^2$. Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at 81 Montevideo Road in Avon, Connecticut using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was focused at the base of the tower. For this report, the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

1) 2 LTE channels (600 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.

2) 1 NR channel (600 MHz Band) was considered for each sector of the proposed installation. This Channel has a transmit power of 80 Watts.

3) 2 LTE channels (700 MHz Band) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.

4) 4 GSM channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.

5) 4 LTE channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.

6) 2 UMTS channels (AWS Band - 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.

7) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel.

8) 2 LTE channels (BRS Band - 2500 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.

9) 2 NR channels (BRS Band - 2500 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.

10) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.

11) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

12) The antennas used in this modeling are the for the RFS APXVAARR24_43-U-NA20 for the 600 MHz / 600 MHz / 700 MHz / 1900 MHz channel(s), the Ericsson AIR 6449 for the 2500 MHz / 2500 MHz channel(s), the Ericsson AIR 32 for the 1900 MHz / 2100 MHz channel(s), the Ericsson AIR 21 for the 1900 MHz / 2100 MHz channel(s) in Sector C.This is based on feedback from the carrier with regard to anticipated antenna selection. All Antenna gain values and associated transmit power levels are shown in the Site Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufacturer's supplied specifications, minus 10 dB for directional panel antennas and 20 dB for highly focused parabolic microwave dishes, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

13) The antenna mounting height centerline of the proposed antennas is feet above ground level (AGL).

14) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

15) All calculations were done with respect to uncontrolled / general population threshold limits.

T-Mobile Site Inventory and Power Data

Sector:	C
Antenna #:	**1**
Make / Model:	RFS APXVAARR24_43-U-NA20
Frequency Bands:	600 MHz / 600 MHz / 700 MHz / 1900 MHz
Gain:	12.95 dBd / 12.95 dBd / 13.35 dBd / 15.65 dBd
Height (AGL):	feet
Channel Count:	7
Total TX Power (W):	320 Watts
ERP (W):	8,466.41
Antenna C1 MPE %:	**2.74%**
Antenna #:	**2**
Make / Model:	Ericsson AIR 6449
Frequency Bands:	2500 MHz / 2500 MHz
Gain:	22.05 dBd / 22.05 dBd
Height (AGL):	136 feet
Channel Count:	4
Total TX Power (W):	160 Watts
ERP (W):	25,651.93
Antenna C2 MPE %:	**4.99%**
Antenna #:	**3**
Make / Model:	Ericsson AIR 32
Frequency Bands:	1900 MHz / 2100 MHz
Gain:	15.35 dBd / 15.85 dBd
Height (AGL):	136 feet
Channel Count:	4
Total TX Power (W):	240 Watts
ERP (W):	8,728.31
Antenna C3 MPE %:	**1.70%**
Antenna #:	**4**
Make / Model:	Ericsson AIR 21
Frequency Bands:	1900 MHz / 2100 MHz
Gain:	15.35 dBd / 15.35 dBd
Height (AGL):	136 feet
Channel Count:	8
Total TX Power (W):	240 Watts
ERP (W):	8,226.43
Antenna C4 MPE %:	**1.60%**

Site Composite MPE %	
Carrier	MPE %
T-Mobile (Max at Sector C):	11.02%
Sprint	2.67%
Site Total MPE % :	13.69%

T-Mobile MPE % Per Sector	
T-Mobile Sector C Total:	11.02%
Site Total MPE % :	13.69%

T-Mobile Maximum MPE Power Values (Sector C)

T-Mobile Frequency Band / Technology (Sector C)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (μW/cm²)	Frequency (MHz)	Allowable MPE (μW/cm²)	Calculated % MPE
T-Mobile 600 MHz LTE	2	591.73	136.0	2.30	600 MHz LTE	400	0.58%
T-Mobile 600 MHz NR	1	1577.94	136.0	3.07	600 MHz NR	400	0.77%
T-Mobile 700 MHz LTE	2	648.82	136.0	2.52	700 MHz LTE	467	0.54%
T-Mobile 1900 MHz LTE	2	2203.69	136.0	8.57	1900 MHz LTE	1000	0.86%
T-Mobile 2500 MHz LTE	2	6412.98	136.0	24.93	2500 MHz LTE	1000	2.49%
T-Mobile 2500 MHz NR	2	6412.98	136.0	24.93	2500 MHz NR	1000	2.49%
T-Mobile 1900 MHz LTE	2	2056.61	136.0	8.00	1900 MHz LTE	1000	0.80%
T-Mobile 2100 MHz LTE	2	2307.55	136.0	8.97	2100 MHz LTE	1000	0.90%
T-Mobile 1900 MHz GSM	4	1028.30	136.0	8.00	1900 MHz GSM	1000	0.80%
T-Mobile 2100 MHz UMTS	4	1028.30	136.0	8.00	2100 MHz UMTS	1000	0.80%
						Total:	**11.02%**

• NOTE: Totals may vary by approximately 0.01% due to summation of remainders in calculations.

Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector C:	11.02%
T-Mobile Maximum MPE % (Sector C):	11.02%
Site Total:	13.69%
Site Compliance Status:	**COMPLIANT**

The anticipated composite MPE value for this site assuming all carriers present is **13.69%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.