

Filed by:

G. Scott Shepherd, Site Development Specialist II - SBA Communications 134 Flanders Rd., Suite 125, Westborough, MA 01581 508.251.0720 x 3807 - gshepherd@sbasite.com

November 18, 2020

Melanie A. Bachman Executive Director Connecticut Siting Council Ten Franklin Square New Britain, CT 06051

RE: Notice of Exempt Modification

99 Knowlton Hill Rd., Ashford, CT 06278

Latitude: 41 50 26.8 Longitude: 41 50 26.8

T-Mobile Site #: CT11519D\_L600

Dear Ms. Bachman:

T-Mobile currently maintains nine (9) antennas at the 147-foot level of the existing 149-foot Monopole Tower at 99 Knowlton Hill Rd., CT. The 147-foot tower is owned by SBA Towers V, LLC. The property is owned by Thomas E. Knowlton. T-Mobile now intends to remove three (3) new L700/L600 MHz antennas and replace with three (3) new L700/L600 MHz antennas. The new antennas would be installed at the 147-foot level of the tower.

**Please note:** Per the Connecticut Siting Council Website: CSC COVID 19 Guidelines. In order to prevent the spread of Coronavirus and protect the health and safety of our members and staff, as of March 18, 2020, the Connecticut Siting Council shall convert to full remote operations until March 30, 2020. Please be advised that during this time period, all hard copy filing requirements will be waived in lieu of an electronic filing. Please also be advised that the March 26, 2020 regular meeting shall be held via teleconference. The Council's website is not equipped with an on-line filing fee receipt service. Therefore, filing fees and/or direct cost charges associated with matters received electronically during the above-mentioned time period will be directly invoiced at a later date.

Planned Modifications:

**TOWER** 

# Remove:

N/A

# Remove and Replace:

• (3) RFS LNX-6515DS antannas (remove) – (3) RFS APXVAARR24\_43-U-NA20 antennas (replace)



# **Install New:**

- (3) Ericsson Radio 4449 B71+B12 RRU
- (1) Sitepro PRK-1245L (Platform kit)
- (1) Sitepro HRK12-U (Platform Kit)
- (1) 1-5/8" fiber

# **Existing Equipment to Remain:**

- (3) Ericsson KRY 112 489/2 TMA
- 12' Low Profile Platform
- (6) 1-5/8" coax

### **Entitlements:**

- (3) Ericsson KRY 112 489/2 TMA
- (12) 1-5/8" coax
- (2) 1-5/8" fiber

# **GROUND**

### Install New:

• Equipment inside existing RBS 6201 Equipment cabinet

This facility was approved by the Council on October 26, 2004 under Docket 291. Approval was given for a monopole no taller than necessary to provide the proposed telecommunications services, sufficient to accommodate the antennas of the carrier and other entities, both public and private, but not to exceed 150-feet above ground level, including appurtenances. The tower and foundation were to be designed and constructed with the ability to be extended to 180-feet above ground level. A recalculated radio frequency report was to be provided when a change in operation caused a change in power density levels. Upon the establishment of any new State or federal radio frequency standards applicable to the facility, it was to be brought into compliance. The Certificate Holder was to permit public or private entities to share space on the proposed tower for fair consideration or to provide legal, technical, environmental, or economic reasons precluding such sharing. There was to be space provided on the tower for no compensation for any municipal antennas, provide they were compatible with the structural integrity of the tower. And any obsolete antennas were to be removed within 60 days. There were no further post construction stipulations set. Please see attached.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. §16.50j-72(b)(2). In accordance with R.C.S.A. § 16.50j-73, a copy of this letter is being sent to the Town of Ashford's First Selectman, Cathryn E. Silver-Smith, and Building Official James Rupert, as well as to the property owner. (Separate notice is not being sent to tower owner, as it belongs to SBA.)

The planned modifications to the facility fall squarely within those activities explicitly provided for in R.C.S.A. §16.50j-72(b)(2).

- 1. The proposed modifications will not result in an increase in the height of the existing structure.
- 2. The proposed modification will not require the extension of the site boundary.



- 3. The proposed modifications will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
- 4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communications Commission safety standard.
- 5. The proposed modification will not cause a change or alteration in the physical or environmental characteristics of the site.
- 6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-referenced telecommunication facility constitute an exempt modifications under R.C.S.A. § 16-50j-72(b)(2).

Sincerely,

G. Scott Shepherd
Site Development Specialist II
SBA COMMUNICATIONS CORPORATION
134 Flanders Rd., Suite 125
Westborough, MA 01581
508.251.0720 x3807 + T
508.366.2610 + F
508.868.6000 + C
gshepherd@sbasite.com

# **Attachments**

cc: Cathryn E. Silver-Smith, First Selectman / with attachments

Town of Ashford, Town Hall, 5 Town Hall Road, Ashford, CT 06278

James Rupert, Building Official / with attachments

Town of Ashford, Town Hall, 5 Town Hall Road, Ashford, CT 06278

Thomas E. Knowlton / with attachments

317 Squaw Hollow Road, Ashford CT 06278 (SBA address on file)

99 Knowlton Hill Rd., Ashford, CT 06278 (Town address on file)

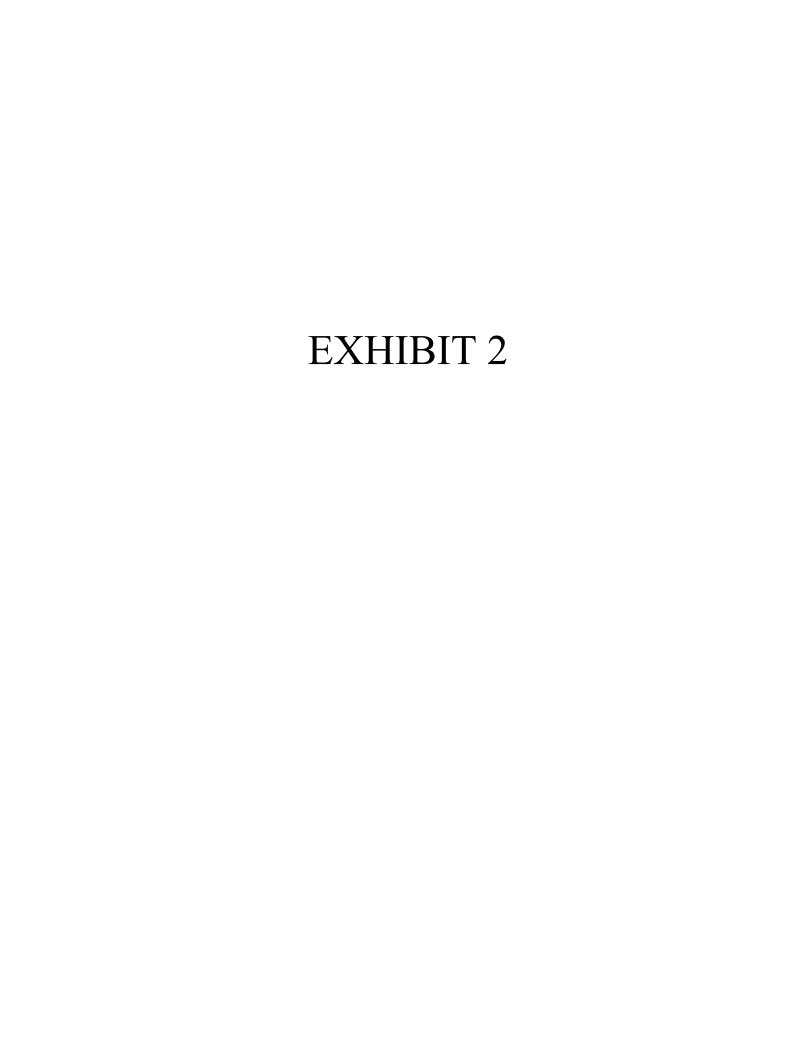


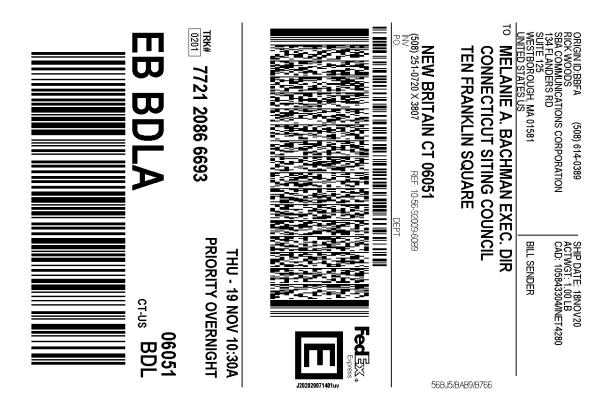
# **EXHIBIT LIST**

Exhibit 1	Check Copy	x To be invoiced at a later date per Covid guidelines.
Exhibit 2	Notification Receipts	X
Exhibit 3	Property Card	X
Exhibit 4	Property Map	X
Exhibit 5	Original Zoning Approval	CSC 10/26/04
Exhibit 6	Construction Drawings	Chappell Engineering 11/9/20
Exhibit 7	Modification Drawings	Geo Structural 6/19/19
Exhibit 8	Structural Analysis	TES 9/6/19
Exhibit 9	EME Report	Transcom Engineering 6/10/19

# EXHIBIT 1

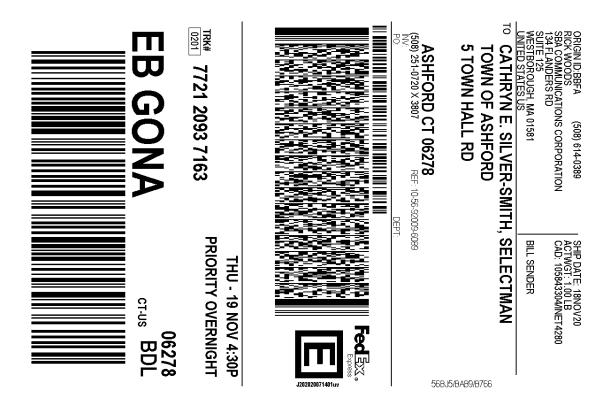
Normally, Exhibit 1 would contain a copy of the check for the filing fee.





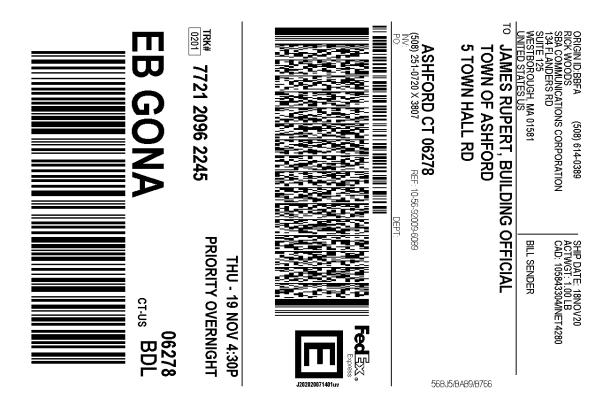
- 1. Use the 'Print' button on this page to print your label to your laser or inkjet printer.
- 2. Fold the printed page along the horizontal line.
- 3. Place label in shipping pouch and affix it to your shipment so that the barcode portion of the label can be read and scanned.

**Warning**: Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.



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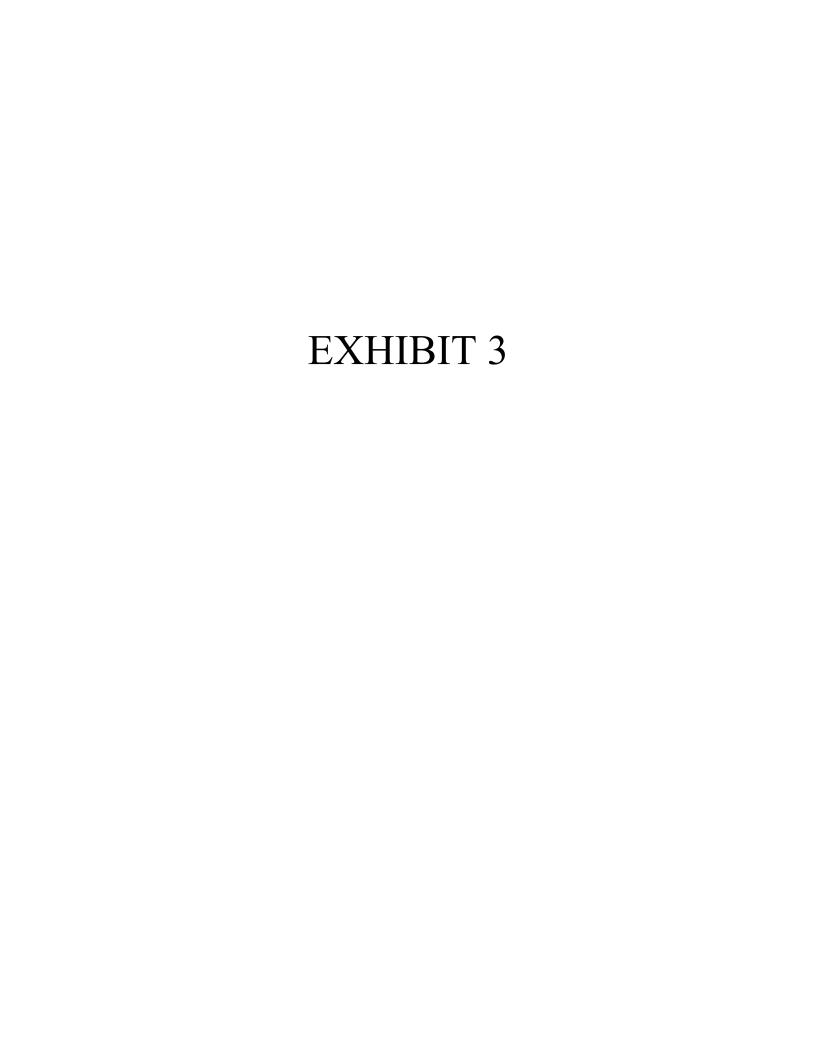
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# 99 KNOWLTON HILL RD

**Location** 99 KNOWLTON HILL RD **Mblu** 43/ D/ 1.2/ /

Acct# 00107500 Owner KNOWLTON THOMAS E

**Taxable Status Assessment** \$113,300

**Appraisal** \$161,700 **PID** 1004

**Building Count** 1 **Legal Description** 

> **Lot Type** topoTopo Clear

**Location** Rural

# **Current Value**

Appraisal							
Valuation Year	Building	Extra Features	Outbuildings	Land	Total		
2018	\$99,000	\$2,000	\$500 \$60,200		\$161,700		
		Assessment					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total		
2018	\$69,300	\$1,400	\$400	\$42,200	\$113,300		

# **Parcel Addreses**

Additional Addresses				
Address City, State Zip Type				
99 KNOWLTON HILL RD		Primary		

# **Owner of Record**

KNOWLTON THOMAS E Sale Price Owner \$0 Co-Owner Certificate С

**Book & Page** 140/835 Care Of Address 99 KNOWLTON HILL RD Sale Date 12/22/2003

ASHFORD, CT 06278

Qualified

# **Ownership History**

Ownership History					
Owner	Sale Price	Certificate	Book & Page	Sale Date	
KNOWLTON ROYAL O EST OF ET AL	\$0		105/ 827	01/10/1995	

# **Building 1 : Section 1**

Year Built: 1800 Living Area: 2,112 Replacement Cost: \$197,907 Building Percent 50

Good:

**Replacement Cost** 

**Less Depreciation:** \$99,000

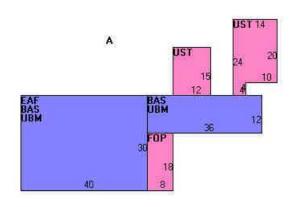
Building At	tributes
Field	Description
Style	Cape Cod
Model	Residential
Grade:	C+
Stories:	1.25
Occupancy	1
Exterior Wall 1	Clapboard
Exterior Wall 2	
Roof Structure:	Gable
Roof Cover	Arch. Shingles
Interior Wall 1	Plastered
Interior Wall 2	
Interior Flr 1	Hardwood
Interior Flr 2	
Heat Fuel	Oil
Heat Type:	Hot Water
AC Type:	None
Total Bedrooms:	3 Bedrooms
Total Bthrms:	1
Total Half Baths:	0
Total Xtra Fixtrs:	0
Total Rooms:	8
Bath Style:	Average
Kitchen Style:	Average
Bsmt. Garages	0

# **Building Photo**



(http://images.vgsi.com/photos/AshfordCTPhotos//\00\00\07/31

# **Building Layout**



(http://images.vgsi.com/photos/AshfordCTPhotos//Sketches/100-

Building Sub-Areas (sq ft) <u>Legend</u>						
Code	Description	Gross Area	Living Area			
BAS	First Floor	1,632	1,632			
EAF	Expansion Attic Finished	1,200	480			
FOP	Framed Open Porch	144	0			
UBM	Unfinished Basement	1,632	0			
UST	Utility Storage	476	0			
		5,084	2,112			

# **Extra Features**

Extra Features <u>Legend</u>

Code	Description	Sub Code	Sub Description	Size	Value	Assessed Value	Bldg #	Comment
FPL2	Fireplace 1.5 Sty			1 units	\$2,000	\$1,400	1	

# **Parcel Information**

Use Code 101

**Description** Single Family

**Deeded Acres** 2.02

# Land

Land Use Land Line Valuation

**Use Code** 101 **Size (Acres)** 2.02

DescriptionSingle FamilyFrontageZoneRADepth

NeighborhoodAssessed Value\$42,200Alt Land ApprNoAppraised Value\$60,200

Category

# Outbuildings

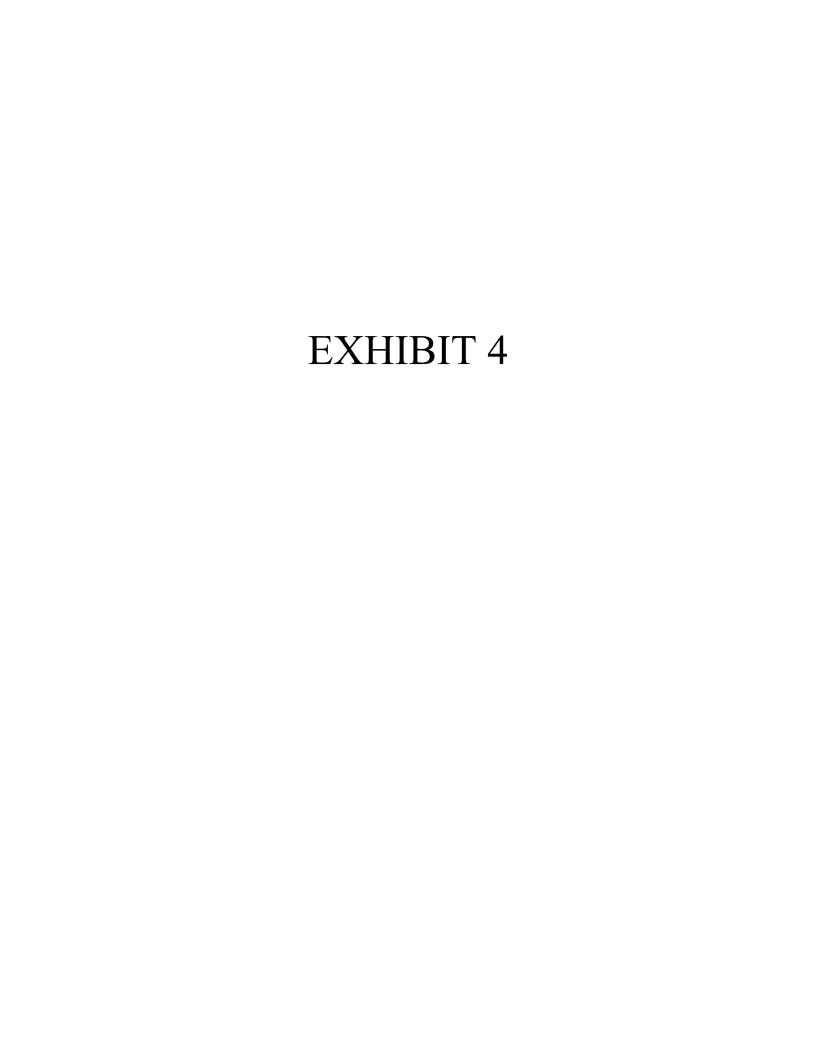
Outbuildings <u>Leg</u>						<u>Legend</u>		
Code	Code Description Sub Code Sub Description Size Value Assessed Value Bldg #				Bldg #	Comment		
SHD1	Shed	FR	Frame	180 S.F.	\$500	\$400	1	А

# **Valuation History**

Appraisal							
Valuation Year	Building	Extra Features	Outbuildings	Land	Total		
2018	\$99,000	\$2,000	\$500	\$60,200	\$161,700		
2017	\$99,000	\$2,000	\$500	\$60,200	\$161,700		
2016	\$99,000	\$2,000	\$500	\$60,200	\$161,700		

Assessment						
Valuation Year	Building	Extra Features	Outbuildings	Land	Total	
2018	\$69,300	\$1,400	\$400	\$42,200	\$113,300	
2017	\$69,300	\$1,400	\$400	\$42,200	\$113,300	
2016	\$69,300	\$1,400	\$400	\$42,200	\$113,300	

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# Google Maps 99 Knowlton Hill Rd





# 99 Knowlton Hill Rd

Ashford, CT 06278





Save



Nearby



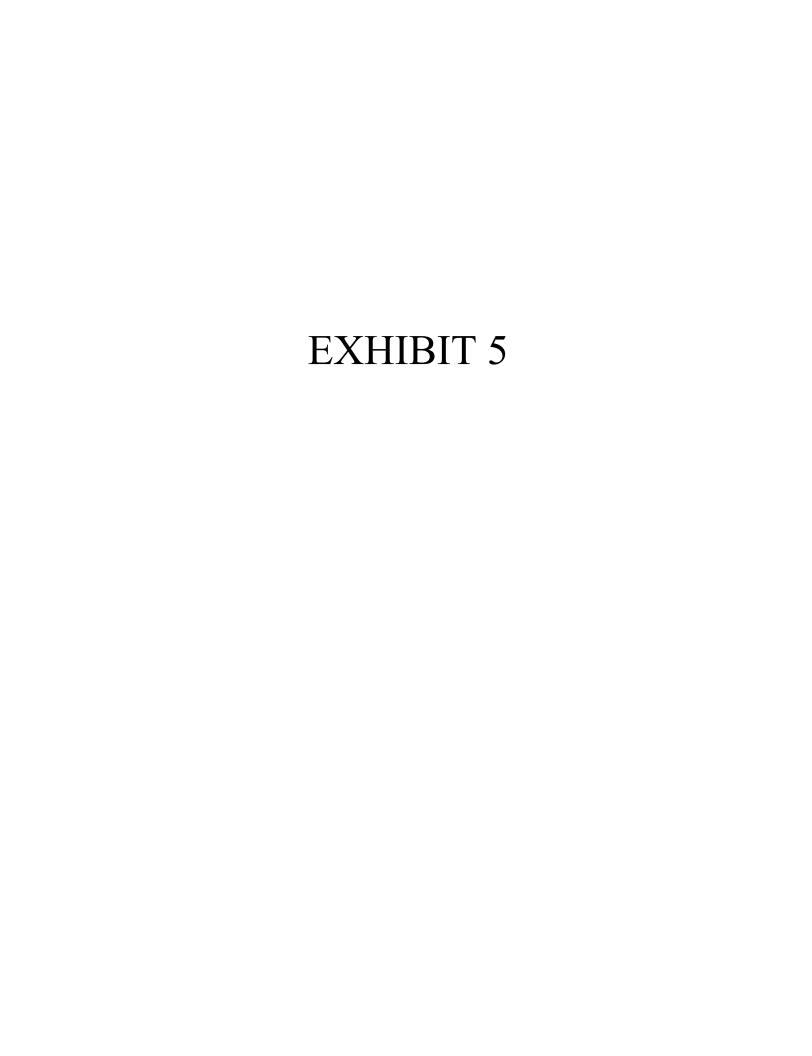


Send to your phone

Share

# **Photos**





# **Connecticut Siting Council**

# **Decisions**

<b>DOCKET NO. 291</b> - National Grid Communications, Inc. application for a Certificate of Environmental	}	Connecticut
Compatibility and Public Need for the construction, maintenance and operation of a wireless	}	Siting
telecommunications facility at one of two sites located on Knowlton Hill Road, Ashford, Connecticut	}	Council
		October 26, 2004

### **Decision and Order**

Pursuant to the foregoing Findings of Fact and Opinion, the Connecticut Siting Council (Council) finds that the effects associated with the construction, operation, and maintenance of a telecommunications facility including effects on the natural environment; ecological integrity and balance; public health and safety; scenic, historic, and recreational values; forests and parks; air and water purity; and fish and wildlife are not disproportionate either alone or cumulatively with other effects when compared to need, are not in conflict with the policies of the State concerning such effects, and are not sufficient reason to deny the application and therefore directs that a Certificate of Environmental Compatibility and Public Need, as provided by General Statutes § 16-50k, be issued to Tower Ventures II, LLC, hereinafter reffered to as the Certificate Holder, at Site A-1, located on parcel 43/E/4, Knowlton Hill Road, Ashford, Connecticut. The Council denies certification of Site A-2, located on parcel 43/E/4, Knowlton Hill Road, Ashford, Connecticut.

The facility shall be constructed, operated, and maintained substantially as specified in the Council's record in this matter, and subject to the following conditions:

- 1. The tower shall be constructed as a monopole, no taller than necessary to provide the proposed telecommunications services, sufficient to accommodate the antennas of Omnipoint Communications and other entities, both public and private, but such tower shall not exceed a height of 150 feet above ground level, including appurtenances. The tower and foundation shall be designed and constructed with the ability to be extended to 180 feet above ground level, with such extension subject to Council approval by petition for a declaratory ruling, pursuant to Sections 16-50j-38 through 16-50j-40 of the Regulations of Connecticut State Agencies.
  - 2. The Certificate Holder shall prepare a Development and Management (D&M) Plan for this site in compliance with Sections 16-50j-75 through 16-50j-77 of the Regulations of Connecticut State Agencies. The D&M Plan shall be served on the Town of Ashford, for comment, and all parties and intervenors as listed in the service list, and submitted to and approved by the Council prior to the commencement of facility construction and shall include:
    - a final site plan(s) of site development to include specifications for the tower, tower foundation, antennas, equipment building, access road, utility line, and landscaping; and
    - b. construction plans for site clearing, water drainage, and erosion and sedimentation control consistent with the <u>2002 Connecticut Guidelines for Soil Erosion and Sediment Control</u>, as amended.

3. The Certificate Holder shall, prior to the commencement of operation, provide the Council worst-case

modeling of electromagnetic radio frequency power density of all proposed entities' antennas at the closest point of uncontrolled access to the tower base, consistent with Federal Communications Commission, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holder shall ensure a recalculated report of electromagnetic radio frequency power density is submitted to the Council if and when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this Decision and Order.

- 4. Upon the establishment of any new State or federal radio frequency standards applicable to frequencies of this facility, the facility granted herein shall be brought into compliance with such standards.
- 5. The Certificate Holder shall permit public or private entities to share space on the proposed tower for fair consideration, or shall provide any requesting entity with specific legal, technical, environmental, or economic reasons precluding such tower sharing.
- 6. The Certificate Holder shall provide reasonable space on the tower for no compensation for any municipal antennas, provided such antennas are compatible with the structural integrity of the tower.
- 7. If the facility does not initially provide wireless services within one year of completion of construction or ceases to provide wireless services for a period of one year, this Decision and Order shall be void, and the Certificate Holder shall dismantle the tower and remove all associated equipment or reapply for any continued or new use to the Council before any such use is made.
- 8. Any antenna that becomes obsolete and ceases to function shall be removed within 60 days after such antennas become obsolete and cease to function.
- 9. Unless otherwise approved by the Council, this Decision and Order shall be void if the facility authorized herein is not operational within one year of the effective date of this Decision and Order or within one year after all appeals to this Decision and Order have been resolved. Any request for extension of this period shall be filed with the Council not later than sixty days prior to the expiration date of this Certificate and shall be served on all parties and intervenors and the Town of Ashford, as listed in the service list. Any proposed modifications to this Decision and Order shall likewise be so served.

Pursuant to General Statutes § 16-50p, the Council hereby directs that a copy of the Findings of Fact, Opinion, and Decision and Order be served on each person listed below, and notice of issuance shall be published in the <u>Hartford Courant</u> and the <u>Willimantic Chronicle</u>.

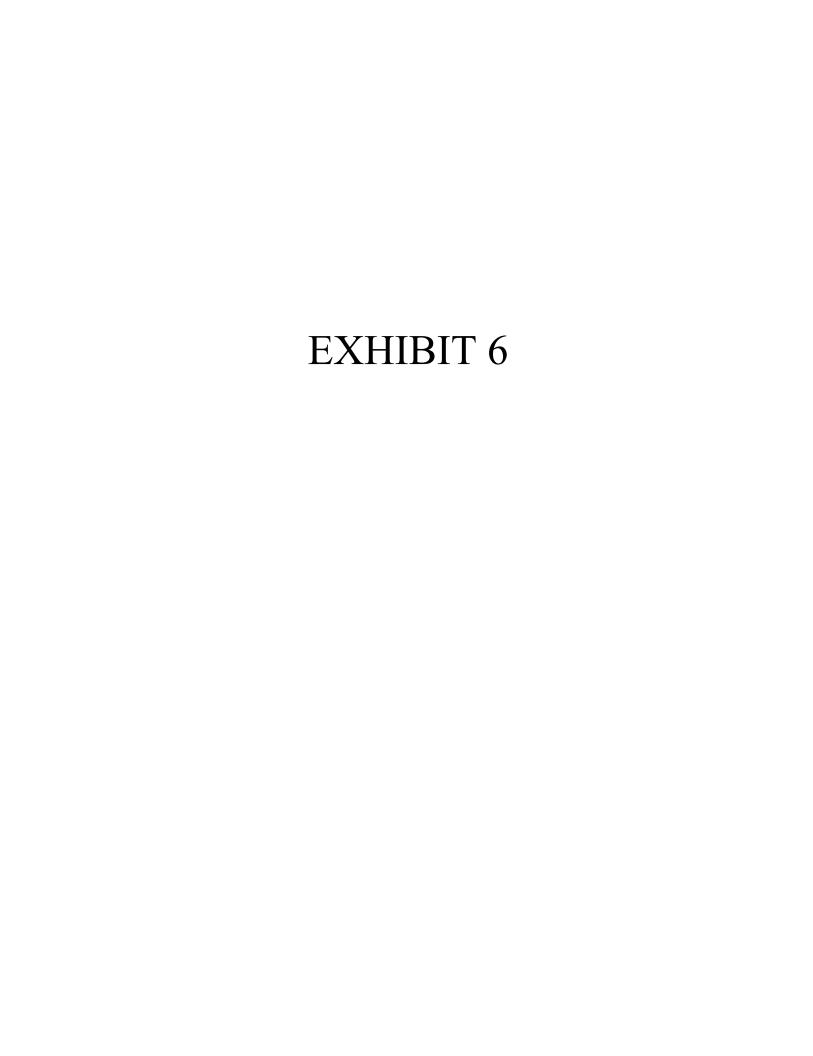
By this Decision and Order, the Council disposes of the legal rights, duties, and privileges of each party named or admitted to the proceeding in accordance with Section 16-50j-17 of the Regulations of Connecticut State Agencies.

The parties and intervenors to this proceeding are:

<u>Applicant</u>	Its Representative
Tower Ventures II, LLC	David Vivian Senior Vice President Tower Ventures II, LLC 733 Chapin Street, Suite 200F Ludlow, MA 01056

	Benjamin Proto, Esq. 2090 Cutspring Road Stratford, CT 06614 Kenneth Ira Spigle, Esq. 170 Westminster Street, Suite 701 Providence, RI 02903
Intervenor	Its Representative

Content Last Modified on 6/14/2005 9:29:58 AM



# CT519/TVI ASHFORD

# 99 KNOWLTON HILL ROAD ASHFORD, CT 06278 WINDHAM COUNTY

# SITE NO.: CT11519D

SITE TYPE: 150'± MONOPOLE

RF DESIGN GUIDELINE: 67D04G OUTD00R

# SITE NOTES

- THIS IS AN UNMANNED AND RESTRICTED ACCESS TELECOMMUNICATION FACILITY, AND IS NOT FOR HUMAN HABITATION. IT WILL BE USED FOR THE TRANSMISSION OF RADIO SIGNAL FOR THE PURPOSE OF PROVIDING PUBLIC CELLULAR SERVICE.
- ADA COMPLIANCE NOT REQUIRED.
- POTABLE WATER OR SANITARY SERVICE IS NOT REQUIRED.
- NO OUTDOOR STORAGE OR ANY SOLID WASTE RECEPTACLES REQUIRED.
- ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK. FAILURE TO NOTIFY THE ARCHITECT/ENGINEER PLACE THE RESPONSIBILITY ON THE CONTRACTOR TO CORRECT THE DISCREPANCIES AT THE CONTRACTOR'S EXPENSE.
- NEW CONSTRUCTION WILL CONFORM TO ALL APPLICABLE CODES AND ORDINANCES.
- BUILDING CODE: 2018 CONNECTICUT STATE BUILDING CODE
- ELECTRICAL CODE: 2017 NATIONAL ELECTRICAL CODE
- STRUCTURAL CODE: TIA/EIA-222-G STRUCTURAL STANDARDS FOR ANTENNA SUPPORTING STRUCTURES AND ANTÉNNAS.

# GENERAL NOTES

PPC DISCONNECT: UNRESTRICTED

MAIN CIRCUIT D/C: UNRESTRICTED

**APPROVALS** 

**PROJECT MANAGER:** 

**CONSTRUCTION:** 

RF ENGINEERING:

**LOCATION** 

SECTOR A:

SECTOR B:

SECTOR C:

SECTOR D:

RADIO CABINETS:

NIU/T DEMARC:

OTHER/SPECIAL:

GPS/LMU:

DATE:

DATE:

DATE:

-MOBILE TECHNICIAN SITE SAFETY NOTES

SPECIAL RESTRICTIONS

ACCESS BY CERTIFIED CLIMBER

ACCESS BY CERTIFIED CLIMBER

ACCESS BY CERTIFIED CLIMBER

ACCESS BY CERTIFIED CLIMBER

UNRESTRICTED

UNRESTRICTED

UNRESTRICTED

NONE

- THE CONTRACTOR SHALL GIVE ALL NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY, MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS, AND LOCAL AND STATE JURISDICTIONAL CODES BEARING ON THE PERFORMANCE OF THE WORK. THE WORK PERFORMED ON THE PROJECT AND THE MATERIALS INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS. AND ORDINANCES.
- THE ARCHITECT/ENGINEER HAVE MADE EVERY EFFORT TO SET FORTH IN THE CONSTRUCTION AND CONTRACT DOCUMENTS THE COMPLETE SCOPE OF WORK. THE CONTRACTOR BIDDING THE JOB IS NEVERTHELESS CAUTIONED THAT MINOR OMISSIONS OR ERRORS IN THE DRAWINGS AND OR SPECIFICATIONS SHALL NOT EXCUSE SAID CONTRACTOR FROM COMPLETING THE PROJECT AND IMPROVEMENTS IN ACCORDANCE WITH THE INTENT OF THESE DOCUMENTS.
- THE CONTRACTOR OR BIDDER SHALL BEAR THE RESPONSIBILITY OF NOTIFYING (IN WRITING) THE OMNIPOINT REPRESENTATIVE OF ANY CONFLICTS, ERRORS, OR OMISSIONS PRIOR TO THE SUBMISSION OF CONTRACTOR'S PROPOSAL OR PERFORMANCE OF WORK. IN THE EVENT OF DISCREPANCIES THE CONTRACTOR SHALL PRICE THE MORE COSTLY OR EXTENSIVE WORK, UNLESS DIRECTED IN WRITING OTHERWISE.
- THE SCOPE OF WORK SHALL INCLUDE FURNISHING ALL MATERIALS, EQUIPMENT, LABOR AND ALL OTHER MATERIALS AND LABOR DEEMED NECESSARY TO COMPLETE THE WORK/PROJECT AS DESCRIBED
- THE CONTRACTOR SHALL VISIT THE JOB SITE PRIOR TO THE SUBMISSION OF BIDS OR PERFORMING WORK TO FAMILIARIZE HIMSELF WITH THE FIELD CONDITIONS AND TO VERIFY THAT THE PROJECT CAN BE CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- THE CONTRACTOR SHALL OBTAIN AUTHORIZATION TO PROCEED WITH CONSTRUCTION PRIOR TO STARTING WORK ON ANY ITEM NOT CLEARLY DEFINED BY THE CONSTRUCTION DRAWINGS/CONTRACT DOCUMENTS.
- THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS ACCORDING TO THE MANUFACTURER'S/VENDOR'S SPECIFICATIONS UNLESS NOTED OTHERWISE OR WHERE LOCAL CODES OR ORDINANCES
- THE CONTRACTOR SHALL PROVIDE A FULL SET OF CONSTRUCTION DOCUMENTS AT THE SITE UPDATED WITH THE LATEST REVISIONS AND ADDENDUMS OR CLARIFICATIONS AVAILABLE FOR THE USE BY ALL PERSONNEL INVOLVED WITH THE PROJECT.
- THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.
- 10. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL NECESSARY CONSTRUCTION CONTROL SURVEYS, ESTABLISHING AND MAINTAINING ALL LINES AND GRADES REQUIRED TO CONSTRUCT ALL IMPROVEMENTS AS SHOWN HEREIN.

11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS WHICH MAY BE REQUIRED FOR THE WORK BY THE ARCHITECT/ENGINEER, THE STATE, COUNTY OR LOCAL GOVERNMENT AUTHORITY.

ZONING/SITE ACQ.:

**OPERATIONS:** 

TOWER OWNER:

DATE:

DATE:

DATE:

- 12. THE CONTRACTOR SHALL MAKE NECESSARY PROVISIONS TO PROTECT EXISTING IMPROVEMENTS, EASEMENTS, PAVING, CURBING, ETC. DURING CONSTRUCTION. UPON COMPLETION OF WORK, THE CONTRACTOR SHALL REPAIR ANY DAMAGE THAT MAY HAVE OCCURRED DUE TO CONSTRUCTION ON OR ABOUT THE PROPERTY.
- 13. THE CONTRACTOR SHALL KEEP THE GENERAL WORK AREA CLEAN AND HAZARD FREE DURING CONSTRUCTION AND DISPOSE OF ALL DIRT, DEBRIS, RUBBISH AND REMOVE EQUIPMENT NOT SPECIFIED AS REMAINING ON THE PROPERTY. PREMISES SHALL BE LEFT IN CLEAN CONDITION AND FREE FROM PAINT SPOTS, DUST, OR SMUDGES OF
- 14. THE CONTRACTOR SHALL COMPLY WITH ALL OSHA REQUIREMENTS AS THEY APPLY TO THIS PROJECT.
- 15. THE CONTRACTOR SHALL NOTIFY THE PROJECT OWNER'S REPRESENTATIVE WHERE A CONFLICT OCCURS ON ANY OF THE CONTRACT DOCUMENTS. THE CONTRACTOR IS NOT TO ORDER MATERIAL OR CONSTRUCT ANY PORTION OF THE WORK THAT IS IN CONFLICT UNTIL CONFLICT IS RESOLVED BY THE LESSEE/LICENSEE
- 16. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, PROPERTY LINES, ETC. ON THE JOB.
- 17. ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND EXISTING PLANS OF RECORD. THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK.

AT LEAST 72 HOURS PRIOR TO DIGGING, THE CONTRACTOR IS REQUIRED TO CALL DIG SAFE AT 811



# VICINITY MAP SCALE: 1" = 1000' - 0"· Knowlton

# DO NOT SCALE DRAWINGS

CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE PROJECT OWNER'S REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.

SHEET INDEX			
SHEET NO.	DESCRIPTION	REV. NO.	
T-1	TITLE SHEET	1	
GN-1	GENERAL NOTES	1	
A-1	COMPOUND & EQUIPMENT PLAN	1	
A-2	TOWER ELEVATIONS & ANTENNA PLAN	1	
A-3	SITE DETAILS	1	
E-1	ELECTRIC & GROUNDING DETAILS	1	

CHEEL INDEA

**SPECIAL ZONING NOTE:** BASED ON INFORMATION PROVIDED BY T-MOBILE REGULATORY COMPLIANCE PROFESSIONALS AND LEGAL COUNSEL, THIS TELECOMMUNICATIONS EQUIPMENT DEPLOYMENT IS CONSIDERED AN <u>ELIGIBLE FACILITY</u> UNDER THE MIDDLE CLASS TAX RELIEF AND JOB CREATION ACT OF 2012, 47 USC 1455(A), SECTION 6409(A), AND IS SUBJECT TO AN ELIGIBLE FACILITY REQUEST, EXPEDITED REVIEW, AND LIMITED/PARTIAL ZONING PRE-EMPTION FOR LOCAL DISCRETIONARY PERMITS (VARIANCE, SPECIAL PERMIT, SITE PLAN REVIEW, OR ADMINISTRATIVE REVIEW).

# PROJECT SUMMARY

SITE NUMBER:	CT11519D
SBA SITE NUMBER:	CT13614-A
SBA SITE NAME:	KNOWLTON
SITE ADDRESS:	99 KNOWLTON HILL ROAD ASHFORD, CT 06278
PROPERTY OWNER:	KNOWLTON THOMAS E. 317 SQUAW HOLLOW ROAD ASHFORD, CT 06278
TOWER OWNER:	SBA TOWERS V, LLC 8501 CONGRESS AVENUE BOCA RATON, FL 33487 PHONE: 561–226–9523
COUNTY:	WINDHAM
ZONING DISTRICT:	N/A
STRUCTURE TYPE:	MONOPOLE
STRUCTURE HEIGHT:	150'±
APPLICANT:	T-MOBILE NORTHEAST LLC 15 COMMERCE WAY, SUITE B NORTON, MA 02766
SBA RSM:	STEPHEN ROTH

PHONE: 860-539-4920 EMAIL: SRoth@sbasite.com ARCHITECT: CHAPPELL ENGINEERING ASSOCIATES, LLC.

SBA RSM:

STRUCTURAL ENGINEER:

SITE CONTROL POINT:

201 BOSTON POST ROAD WEST, SUITE 101 MARLBOROUGH, MA 01752

> CHAPPELL ENGINEERING ASSOCIATES, LLC. 201 BOSTON POST ROAD WEST, SUITE 101 MARLBOROUGH, MA 01752

LATITUDE: N.41.840773° N41°50'26.78" LONGITUDE W.72.207521° W72°12'27.08"

# T-MOBILE NORTHEAST LLC

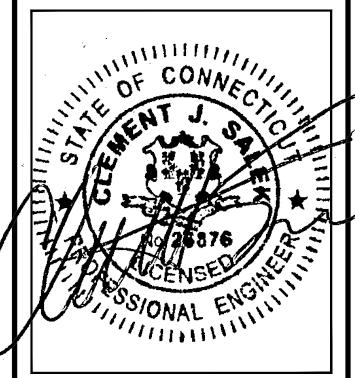
15 COMMERCE WAY, SUITE B NORTON, MA 02766 (508) 286-2700



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R.K. EXECUTIVE CENTRE 201 BOSTON POST ROAD WEST, SUITE 101 MARLBOROUGH, MA 01752 (508) 481-7400 www.chappellengineering.com



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	SUBMITTALS			
REV.	DATE	DESCRIPTION	BY	
1	11/09/20	ISSUED FOR CONSTRUCTION	CMC	
0	05/29/19	ISSUED FOR REVIEW	JRV	

SITE NUMBER: CT11519D

SITE ADDRESS: 99 KNOWLTON HILL ROAD ASHFORD, CT 06278

SHEET TITLE

TITLE SHEET

SHEET NUMBER

# **GENERAL NOTES:**

- 1. FOR THE PURPOSE OF CONSTRUCTION DRAWINGS, THE FOLLOWING DEFINITIONS SHALL APPLY: CONTRACTOR - T-MOBILE SUBCONTRACTOR - GENERAL CONTRACTOR (CONSTRUCTION) OWNER - T-MOBILE
  - OEM ORIGINAL EQUIPMENT MANUFACTURER
- 2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
- 3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS. AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK.
- 4. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL, STATE AND FEDERAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 5. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
- 6. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- 7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CONTRACTOR.
- 9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER, T1 CABLES AND GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY, SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR AND/OR LANDLORD PRIOR TO CONSTRUCTION.
- 10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE OWNER.
- 11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY.
- 12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION AND RETURN DISTURBED AREAS TO ORIGINAL CONDITIONS.
- 13. THE SUBCONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN. THE SUBCONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.
- 14. SUBCONTRACTOR SHALL NOTIFY CHAPPELL ENGINEERING ASSOCIATES, LLC 48 HOURS IN ADVANCE OF POURING CONCRETE OR BACKFILLING TRENCHES, SEALING ROOF AND WALL PENETRATIONS AND POST DOWNS, FINISHING NEW WALLS OR FINAL ELECTRICAL CONNECTIONS FOR ENGINEERING REVIEW.
- 15. CONSTRUCTION SHALL COMPLY WITH ALL T-MOBILE STANDARDS AND SPECIFICATIONS.
- 16. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
- 17. THE EXISTING CELL SITES ARE IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
- 18. IF THE EXISTING CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

# SITE WORK GENERAL NOTES:

- 1. THE SUBCONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.
- 2. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY ENGINEERS. EXTREME CAUTION SHOULD BE USED BY THE SUBCONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. SUBCONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION.
- 3. ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS AND PROJECT SPECIFICATIONS.
- 4. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
- 5. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE BTS EQUIPMENT AND TOWER AREAS.
- 6. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.
- 7. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
- 8. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF ENGINEERING, OWNER AND/OR LOCAL UTILITIES.
- 9. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER. EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE AND STABILIZED TO PREVENT EROSION AS SPECIFIED IN THE PROJECT SPECIFICATIONS.
- 10. SUBCONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
- 11. THE SUBCONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE T-MOBILE SPECIFICATION FOR SITE SIGNAGE.

# **CONCRETE AND REINFORCING STEEL NOTES:**

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST-IN-PLACE CONCRETE.
- 2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE. A HIGHER STRENGTH (400PSI) MAY BE USED. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 381 CODE
- 3. REINFORCING STEEL SHALL CONFORM TO ASTM A 615. GRADE 60. DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE. SPLICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD, UNO.
- 4. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
- CONCRETE CAST AGAINST EARTH.......3 IN. CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 AND LARGER ......2 IN.
- #5 AND SMALLER & WWF ............. 1½ IN. CONCRETE NOT EXPOSED TO EARTH OR WEATHER
- OR NOT CAST AGAINST THE GROUND: SLAB AND WALL ..... BEAMS AND COLUMNS ......1½ IN.
- 5. A CHAMFER 34" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE WITH ACI 301 SECTION
- 6. INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHORS SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO THE MANUFACTURERS RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERNING CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION/WEDGE ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED. EXPANSION BOLTS SHALL BE PROVIDED BY SIMPSON OR APPROVED EQUAL.
- 7. CONCRETE CYLINDER TIES ARE NOT REQUIRED FOR SLAB ON GRADE WHEN CONCRETE IS LESS THAN 50 CUBIC YARDS (IBC1905.6.2.3) IN THAT EVENT THE FOLLOWING RECORDS SHALL BE PROVIDED BY THE CONCRETE SUPPLIER;
- (A) RESULTS OF CONCRETE CYLINDER TEST PERFORMED AT THE SUPPLIERS PLANT.
- (B) CERTIFICATION OF MINIMUM COMPRESSIVE STRENGTH FOR THE CONCRETE GRADE SUPPLIED. FOR GREATER THAN 50 CUBIC YARDS THE GC SHALL PERFORM THE CONCRETE CYLINDER TEST.
- 8. AS AN ALTERNATIVE TO ITEM 7. TEST CYLINDERS SHALL BE TAKEN INITIALLY AND THEREAFTER FOR EVERY 50 YARDS OF CONCRETE FROM EACH DIFFERENT BATCH PLANT.
- 9. EQUIPMENT SHALL NOT BE PLACED ON NEW PADS FOR SEVEN DAYS AFTER PAD IS POURED, UNLESS IT IS VERIFIED BY CYLINDER TESTS THAT COMPRESSIVE STRENGTH HAS BEEN ATTAINED.

# STRUCTURAL STEEL NOTES:

- 1. ALL STEEL WORK SHALL BE PAINTED OR GALVANIZED IN ACCORDANCE WITH THE DRAWINGS AND T-MOBILE SPECIFICATIONS UNLESS OTHERWISE NOTED. STRUCTURAL STEEL SHALL BE ASTM-A-36 UNLESS OTHERWISE NOTED ON THE SITE SPECIFIC DRAWINGS. STEEL DESIGN, INSTALLATION AND BOLTING SHALL BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "MANUAL OF STEEL CONSTRUCTION".
- 2. ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND AWS D1.1 WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION", 9TH EDITION, PAINTED SURFACES SHALL BE TOUCHED UP.
- 3. BOLTED CONNECTIONS SHALL USE BEARING TYPE ASTM A325 BOLTS (3/4"0) AND SHALL HAVE MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE GALVANIZED OR STAINLESS STEEL.
- 4. NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE %" DIA. ASTM A 307 BOLTS (GALV) UNLESS NOTED OTHERWISE.
- 5. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER REVIEW & APPROVAL ON PROJECTS REQUIRING STRUCTURAL
- 6. ALL STRUCTURAL STEEL WORK SHALL BE DONE IN ACCORDANCE WITH AISC SPECIFICATIONS.

# SOIL COMPACTION NOTES FOR SLAB ON GRADE:

- 1. EXCAVATE AS REQUIRED TO REMOVE VEGETATION AND TOPSOIL TO EXPOSE NATURAL SUBGRADE AND PLACE CRUSHED STONE AS REQUIRED.
- 2. COMPACTION CERTIFICATION: AN INSPECTION AND WRITTEN CERTIFICATION BY A QUALIFIED GEOTECHNICAL TECHNICIAN OR ENGINEER IS ACCEPTABLE.
- 3. AS AN ALTERNATE TO INSPECTION AND WRITTEN CERTIFICATION, THE "UNDISTURBED SOIL" BASE SHALL BE COMPACTED WITH "COMPACTION EQUIPMENT". LISTED BELOW, TO AT LEAST 90% MODIFIED PROCTOR MAXIMUM DENSITY PER ASTM D 1557 METHOD C.
- 4. COMPACTED SUBBASE SHALL BE UNIFORM AND LEVELED. PROVIDE 6" MINIMUM CRUSHED STONE OR GRAVEL COMPACTED IN 3" LIFTS ABOVE COMPACTED SOIL. GRAVEL SHALL BE NATURAL OR CRUSHED WITH 100% PASSING #1 SIEVE.
- 5. AS AN ALTERNATE TO ITEMS 2 AND 3, THE SUBGRADE SOILS WITH 5 PASSES OR A MEDIUM SIZED VIBRATORY PLATE COMPACTOR (SUCH AS BOMAG BPR 30/38) OR HAND-OPERATED SINGLE DRUM VIBRATORY ROLLER (SUCH AS BOMAG BW 55E). AND SOFT AREAS THAT ARE ENCOUNTERED SHOULD BE REMOVED AND REPLACED WITH A WELL-GRADED GRANULAR FILL AND COMPACTED AS STATED ABOVE.

# **COMPACTION EQUIPMENT:**

1. HAND OPERATED DOUBLE DRUN, VIBRATORY ROLLER, VIBRATORY PLATE COMPACTOR OR JUMPING JACK COMPACTOR.

# **CONSTRUCTION NOTES:**

1. FIELD VERIFICATION:

- SUBCONTRACTOR SHALL FIELD VERIFY SCOPE OF WORK, T-MOBILE ANTENNA PLATFORM LOCATION AND UTILITY TRENCHWORK.
- 2. COORDINATION OF WORK:
- SUBCONTRACTOR SHALL COORDINATE RF WORK AND PROCEDURES WITH CONTRACTOR.

# 3. CABLE LADDER RACK:

SUBCONTRACTOR SHALL FURNISH AND INSTALL CABLE LADDER RACK, CABLE TRAY AND/OR ICE BRIDGE, AND CONDUIT AS REQUIRED TO SUPPORT CABLES TO THE NEW BTS LOCATION.

# **ELECTRICAL INSTALLATION NOTES:**

- 1. WIRING, RACEWAY, AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC AND TELCORDIA.
- 2. SUBCONTRACTOR SHALL MODIFY OR INSTALL CABLE TRAY SYSTEM AS REQUIRED TO SUPPORT RF AND TRANSPORT CABLING TO THE NEW BTS EQUIPMENT. SUBCONTRACTOR SHALL SUBMIT MODIFICATIONS TO CONTRACTOR FOR APPROVAL.
- 3. ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC AND
- 4. CABLES SHALL NOT BE ROUTED THROUGH LADDER-STYLE CABLE TRAY RUNGS.
- 5. EACH END OF EVERY POWER, GROUNDING, AND T1 CONDUCTOR AND CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA, AND MATCH INSTALLATION REQUIREMENTS.
- 6. POWER PHASE CONDUCTORS (I.E., HOTS) SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, ½ INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). PHASE CONDUCTOR COLOR CODES SHALL CONFORM WITH THE NEC AND OSHA.
- 7. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS. ALL EQUIPMENT SHALL BE LABELED WITH THEIR VOLTAGE RATING, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING, AND BRANCH CIRCUIT ID NUMBERS (I.E., PANELBOARD AND CIRCUIT ID'S).
- 8. PANELBOARDS (ID NUMBERS) AND INTERNAL CIRCUIT BREAKERS (CIRCUIT ID NUMBERS) SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS.
- 9. ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.
- 10. POWER, CONTROL, AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE CONDUCTOR (#34 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE
- 11. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE CONDUCTOR (#6 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN-2 GREEN INSULATION, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.
- 12. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED OUTDOORS, OR BELOW GRADE, SHALL BE SINGLE CONDUCTOR #2 AWG SOLID TINNED COPPER CABLE, UNLESS OTHERWISE SPECIFIED.
- 13. POWER AND CONTROL WIRING, NOT IN TUBING OR CONDUIT, SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#34 AWG OR LARGER), 600 V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; WITH OUTER JACKET; LISTED OR LABELED FOR THE LOCATION USED, UNLESS OTHERWISE SPECIFIED.
- 14. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRENUTS BY HARGER (OR EQUAL). LUGS AND WIRENUTS SHALL BE RATED FOR OPERATION AT NO LESS THAN 75°C (90°C IF AVAILABLE).
- 15. RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
- 16. NEW RACEWAY OR CABLE TRAY WILL MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.
- 17. ELECTRICAL METALLIC TUBING (EMT) OR RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80 FOR LOCATIONS SUBJECT TO PHYSICAL DAMAGE) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.
- 18. ELECTRICAL METALLIC TUBING (EMT). ELECTRICAL NONMETALLIC TUBING (ENT), OR RIGID NONMETALLIC CONDUIT (RIGID PVC, SCHEDULE 40) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
- 19. GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE
- 20. RIGID NONMETALLIC CONDUIT (I.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80) SHALL BE USED UNDERGROUND; DIRECT BURIED, IN AREAS OF OCCASIONAL LIGHT VEHICLE TRAFFIC OR ENCASED IN REINFORCED CONCRETE IN AREAS OF HEAVY VEHICLE TRAFFIC.
- 21. LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
- 22. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION—TYPE AND APPROVED FOR THE LOCATION
- 23. CABINETS, BOXES AND WIREWAYS SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA,
- UL, ANSI/IEEE AND NEC. 24. CABINETS, BOXES AND WIREWAYS TO MATCH THE EXISTING INSTALLATION WHERE POSSIBLE.

USED. SETSCREW FITTINGS ARE NOT ACCEPTABLE.

- 25. WIREWAYS SHALL BE EPOXY-COATED (GRAY) AND INCLUDE A HINGED COVER. DESIGNED TO SWING OPEN DOWNWARD: SHALL BE PANDUIT TYPE E (OR EQUAL): AND RATED NEMA 1 (OR BETTER) INDOORS. OR NEMA 3R (OR BETTER) OUTDOORS.
- 26. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50, AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.
- 27. METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED, OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.
- 28. NONMETALLIC RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.
- 29. THE SUBCONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CONTRACTOR BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
- 30. THE SUBCONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD AGAINST LIFE AND PROPERTY.
- 31. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE LOCAL CODES.
- 32. CONDUIT ROUTINGS ARE SCHEMATIC. SUBCONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED.

# T-MOBILE NORTHEAST LLC

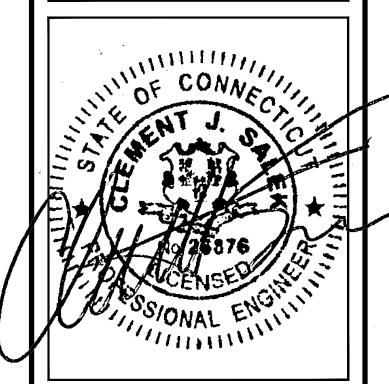
15 COMMERCE WAY, SUITE B NORTON, MA 02766 (508) 286-2700



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CHECKED BY:

APPROVED BY:

SUBMITTALS REV. DATE RY DESCRIPTION 1 | 11/09/20 | ISSUED FOR CONSTRUCTION | CMC

0 | 05/29/19 | ISSUED FOR REVIEW

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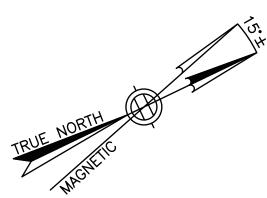
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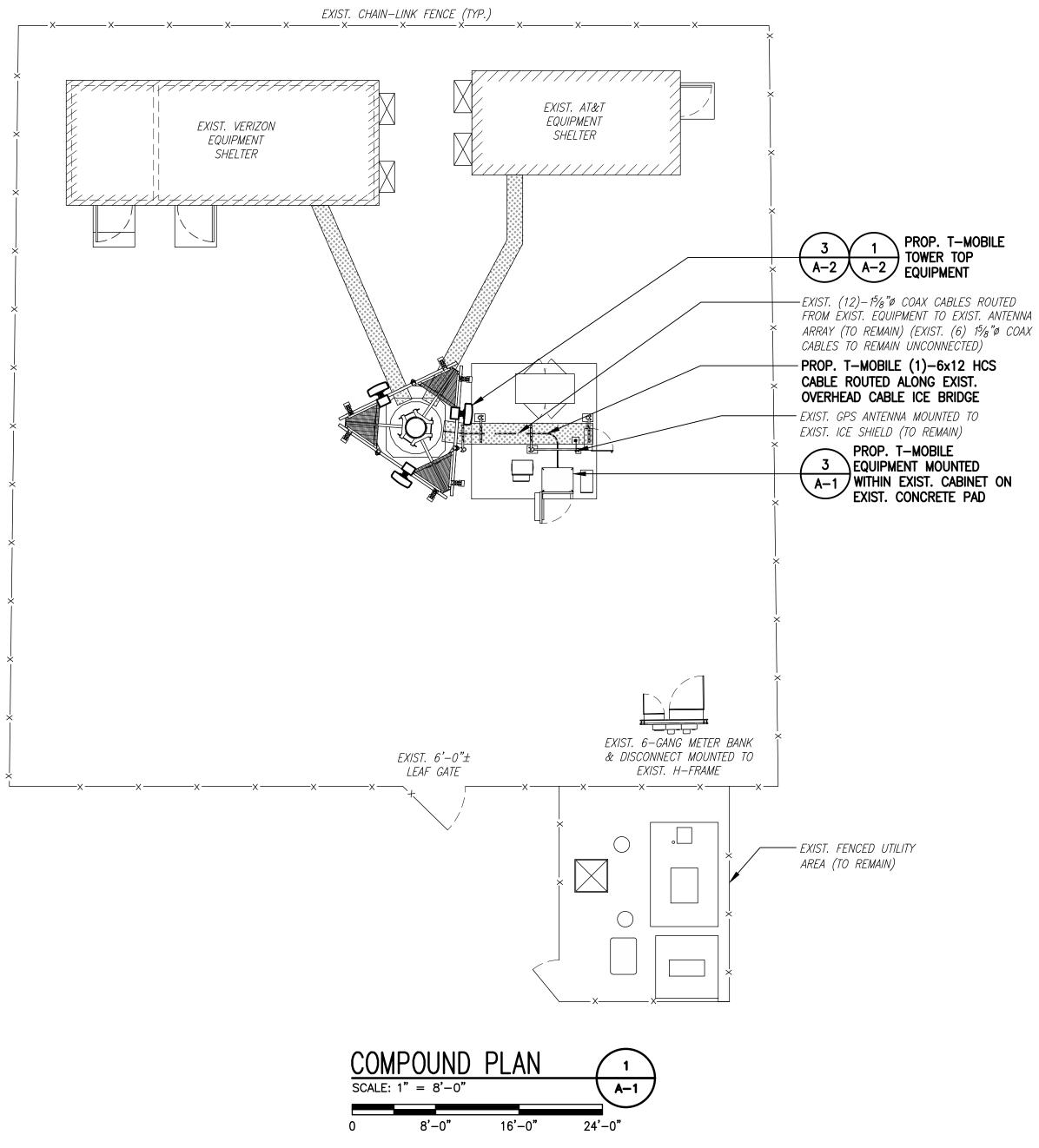
**GENERAL NOTES** 

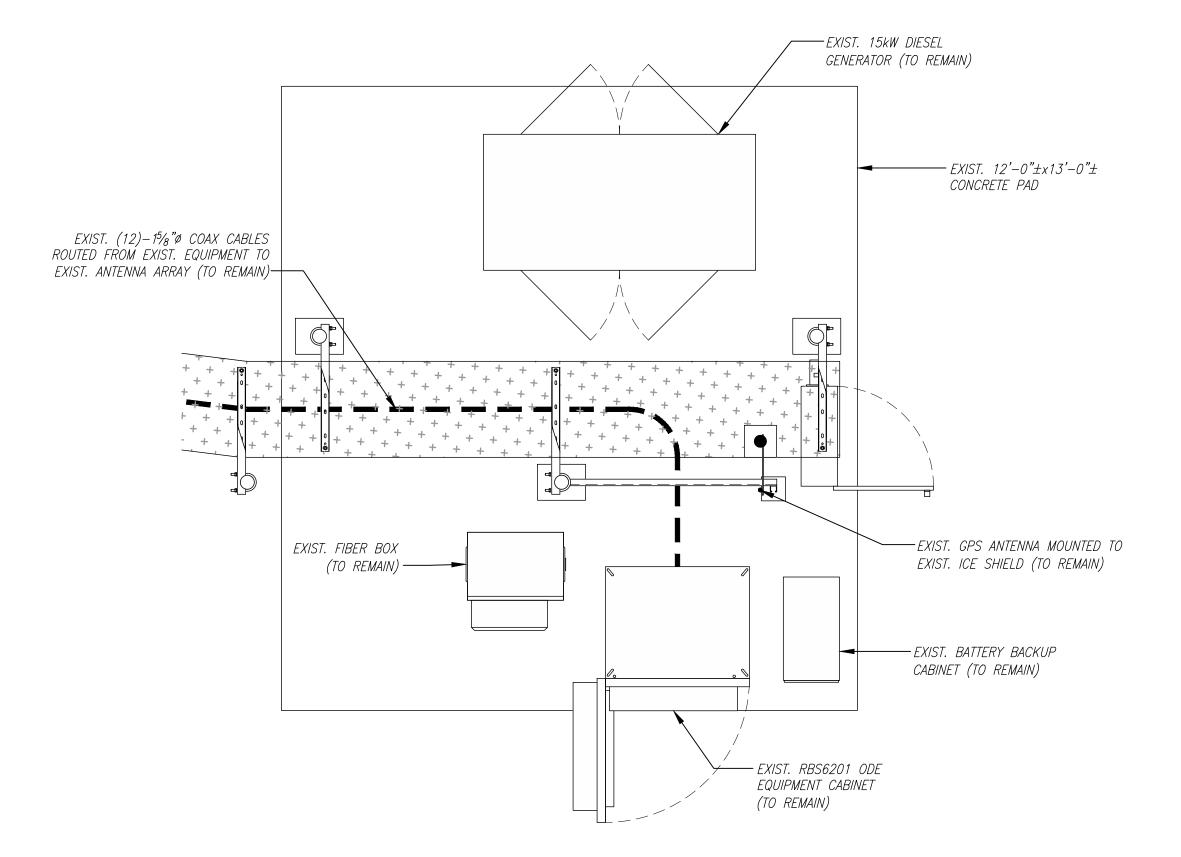
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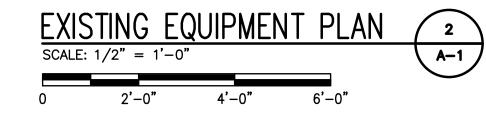
GN-1

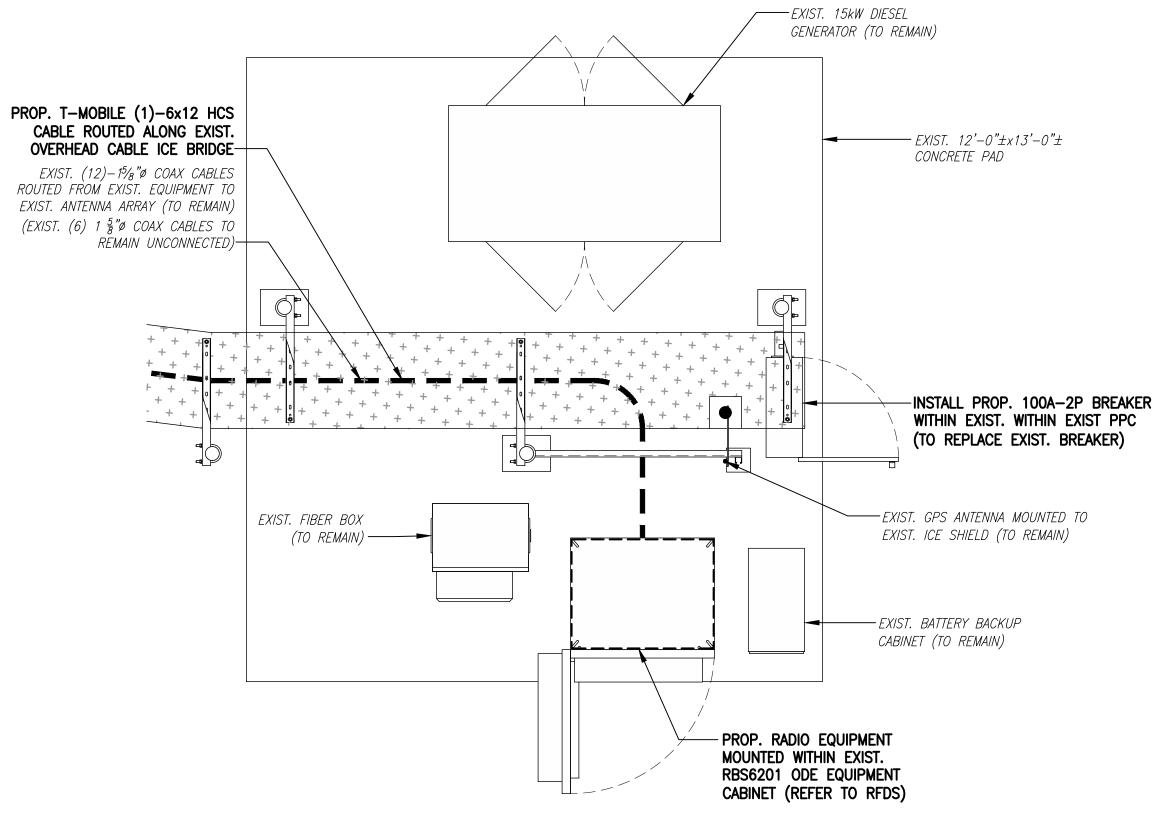
SPECIAL PRE-CONSTRUCTION WORK NOTE (SBA-PROVIDED TOWER STRUCTURAL ANALYSIS SPECIAL EQUIPMENT INSTALLATION REQUIREMENTS):
GENERAL CONTRACTOR SHALL FURNISH AND INSTALL ALL SPECIAL OR SUPPLEMENTAL ADDITIONAL TOWER-MOUNTED EQUIPMENT PER RECOMMENDATIONS FROM
SBA-PROVIDED TOWER STRUCTURAL ANALYSIS FOR ANY SPECIAL SHIELDING OF TOWER TOP EQUIPMENT AND FOR ANY SPECIAL FEEDLINE BUNDLING OR RELOCATION.

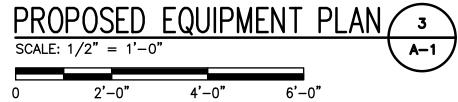












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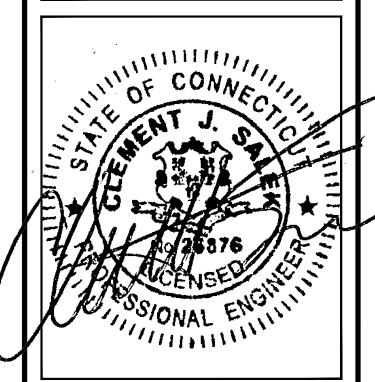
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COMPOUND & EQUIPMENT PLAN

SHEET NUMBER

**A**-

1815.09

SPECIAL PRE-CONSTRUCTION WORK NOTE (SBA-PROVIDED TOWER STRUCTURAL ANALYSIS SPECIAL EQUIPMENT INSTALLATION REQUIREMENTS):
GENERAL CONTRACTOR SHALL FURNISH AND INSTALL ALL SPECIAL OR SUPPLEMENTAL ADDITIONAL TOWER-MOUNTED EQUIPMENT PER RECOMMENDATIONS FROM SBA-PROVIDED TOWER STRUCTURAL ANALYSIS FOR ANY SPECIAL SHIELDING OF TOWER TOP EQUIPMENT AND FOR ANY SPECIAL FEEDLINE BUNDLING OR RELOCATION.

GENERAL CONTRACTOR TO REMOVE ANY EXISTING TMAS THAT ARE NOT SHOWN ON THESE CONSTRUCTION DRAWINGS.

RAD CENTER NOTE:
T-MOBILE RAD CENTER SHOWN IN RED TEXT BASED ON SBA-PROVIDED CO-LOCATION APPLICATION, EQUIPMENT DATABASE, AND STRUCTURAL ANALYSIS. THE SBA-PROVIDED ANTENNA RAD CENTER SHALL SUPERSEDE ANY CONFLICTING INFORMATION DERIVED FROM THE T-MOBILE RFDS.

# T-MOBILE NORTHEAST LLC

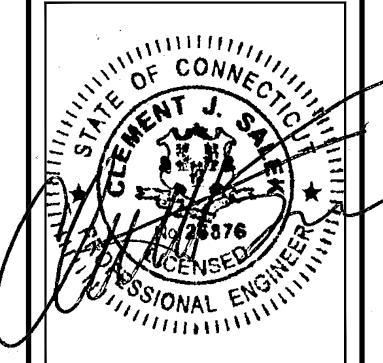
15 COMMERCE WAY, SUITE B NORTON, MA 02766 (508) 286-2700



SBA COMMUNICATIONS CORP.
134 FLANDERS ROAD, SUITE 125
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(508) 251-0720



R.K. EXECUTIVE CENTRE
201 BOSTON POST ROAD WEST, SUITE 101
MARLBOROUGH, MA 01752
(508) 481-7400
www.chappellengineering.com



CHECKED BY: JMT

APPROVED BY: JI

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ı	REV.	DATE	DESCRIPTION	BY
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	1		ISSUED FOR CONSTRUCTION	CMC
	0	05/29/19	ISSUED FOR REVIEW	JR\

# SITE NUMBER: CT11519D

SITE ADDRESS: 99 KNOWLTON HILL ROAD ASHFORD, CT 06278

SHEET TITLE

TOWER ELEVATIONS & ANTENNA PLAN

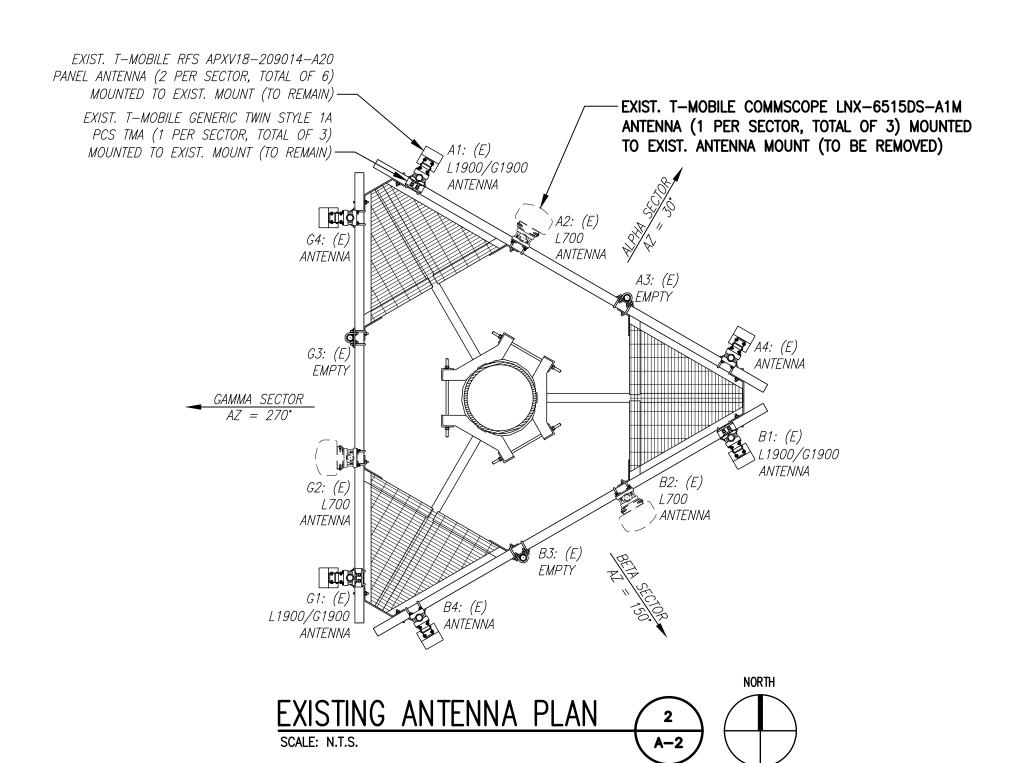
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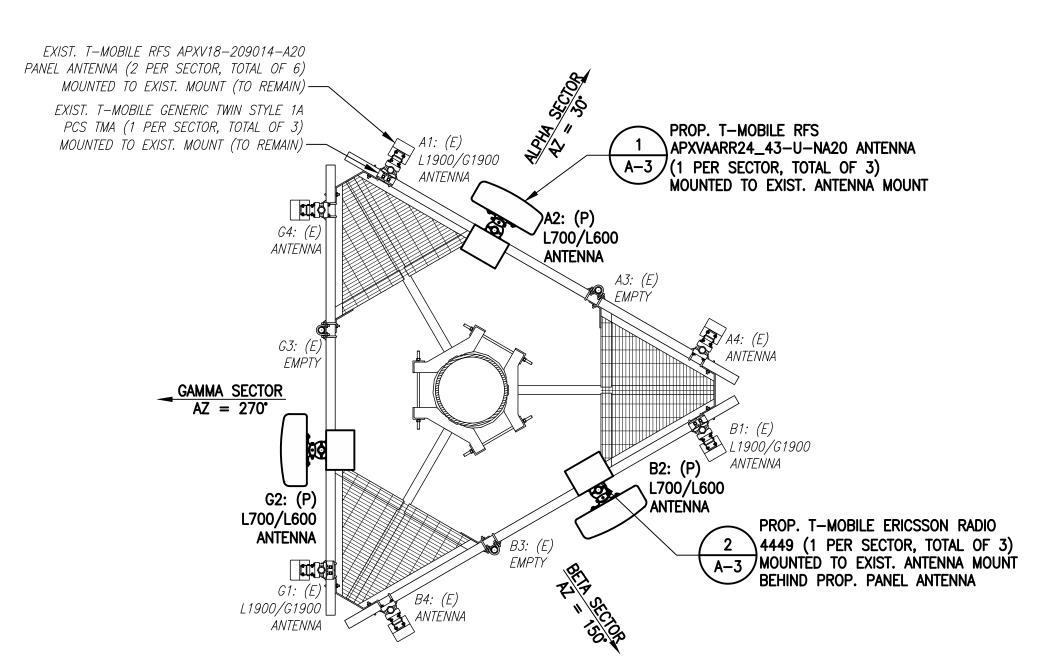
A-2

PROP. T-MOBILE RFS
APXVAARR24\_43-U-NA20 ANTENNA 3 (1 PER SECTOR, TOTAL OF 3)

MOUNTED TO EXIST. ANTENNA MOUNT  $\frac{TOP \ OF \ EXIST. \ MONOPOLE}{EL. = 150.0' \pm \ AGL} \bullet$  $\frac{Q}{EL.} = \frac{EXIST. (6) T - MOBILE ANTENNAS}{EL.} = \frac{147.0' \pm AGL}{EL}$ EXIST. T-MOBILE RFS APXV18-209014-A20 PANEL ANTENNA (2 PER SECTOR, TOTAL OF 6) © PROP. (3) T-MOBILE ANTENNAS MOUNTED TO EXIST. MOUNT (TO REMAIN) - $EL. = 147.0' \pm AGL$ PROP. T-MOBILE ERICSSON RADIO
4449 (1 PER SECTOR, TOTAL OF 3)
MOUNTED TO EXIST. ANTENNA MOUNT
BEHIND PROP. PANEL ANTENNA <u>€\_EXIST. (9) AT&T\_ANTENNAS</u> EL. = 137.5'± AGL EXIST. T-MOBILE GENERIC TWIN STYLE 1A PCS TMA (1 PER SECTOR, TOTAL OF 3) MOUNTED TO EXIST. MOUNT (TO REMAIN) — EXIST. 150'± MONOPOLE — EXIST. (12)—15%" COAX CABLE ROUTED FROM EXIST. EQUIPMENT TO EXIST. ANTENNA ARRAY (TO REMAIN) (EXIST. (6) 15/8"Ø COAX TO REMAIN UNCONNECTED) PROP. T-MOBILE (1) 6x12 HCS CABLE ROUTED UP EXIST. MONOPOLE NOTE: GROUND EQUIPMENT NOT SHOWN, FOR CLARITY. GROUND LEVEL

EL. = 0.0' AGL TOWER ELEVATION A-2

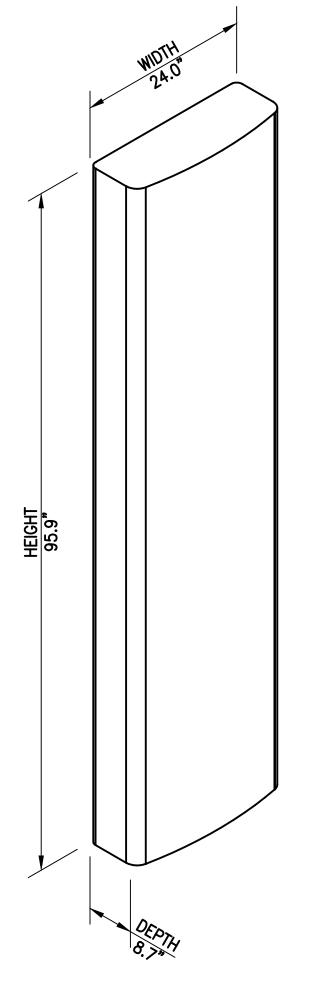






	FINAL ANTENNA CONFIGURATION							
SECTOR	ANTENNA	RAD CENTER	AZIMUTH (TRUE NORTH)	MECHANICAL DOWNTILT	ELECTRICAL DOWNTILT	BAND	RADIOS/TMAS	CABLES
	RFS APXV18-209014-C-A20	147'± AGL	<i>30°</i>	0°	2°	L1900/G1900	GENERIC TWIN STYLE 1A PCS TMA	(2) 15%"ø COAX CABLE
ALPHA	RFS APXVAARR24_43-U-NA20	147'± AGL	30°	0°	2*	L600/L700	ERICSSON RADIO 4449 B71+B12	(1) 6x12 HCS CABLE (SHARED)
ALFTIA	EMPTY	147'± AGL	_	-	-	_	-	-
	RFS APXV18-209014-C-A20	147'± AGL	30°	0°	2°	-	-	_
	RFS APXV18-209014-C-A20	147'± AGL	150°	0°	2°	L1900/G1900	GENERIC TWIN STYLE 1A PCS TMA	(2) 15%"ø COAX CABLE
BETA	RFS APXVAARR24_43-U-NA20	147'± AGL	150°	0°	2*	L600/L700	ERICSSON RADIO 4449 B71+B12	(1) 6x12 HCS CABLE (SHARED)
DETA	EMPTY	147'± AGL	-	-	-	_	-	_
	RFS APXV18-209014-C-A20	147'± AGL	150°	0°	2°	-	-	-
	RFS APXV18-209014-C-A20	147'± AGL	270°	0°	2°	L1900/G1900	GENERIC TWIN STYLE 1A PCS TMA	(2) 15/8"ø COAX CABLE
GAMMA	RFS APXVAARR24_43-U-NA20	147'± AGL	270°	0°	2°	L600/L700	ERICSSON RADIO 4449 B71+B12	(1) 6x12 HCS CABLE (SHARED)
	ЕМРТҮ	147'± AGL	-	-	-	-	-	-
	RFS APXV18-209014-C-A20	147'± AGL	270°	<i>0</i> °	2°	-	-	_

NOTE: EXISTING (6) 1  $\frac{5}{8}$  "Ø COAX CABLES TO REMAIN UNCONNECTED



RFS APXVAARR24\_43-NA20 PANEL ANTENNA
DIMENSIONS: 95.9"H x 24.0"W x 8.7"D
WEIGHT: 128.0 LBS
1 PER SECTOR, TOTAL OF 3





ERICSSON RADIO 4449 B12+B71
DIMENSIONS: 14.9"H x 13.2"W x 9.3"D
WEIGHT: 74.0 LBS
1 PER SECTOR, TOTAL OF 3

RRU DETAIL
SCALE: N.T.S.



# T-MOBILE NORTHEAST LLC

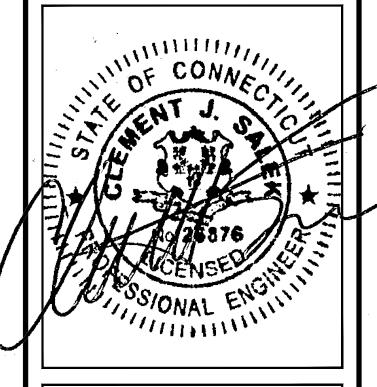
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CHECKED BY: JMT

APPROVED BY: JM

	SUBMITTALS			
REV.	DATE	DESCRIPTION	BY	
1		ISSUED FOR CONSTRUCTION	CMC	
0	05/29/19	ISSUED FOR REVIEW	JRV	

# SITE NUMBER: CT11519D

SITE ADDRESS: 99 KNOWLTON HILL ROAD ASHFORD, CT 06278

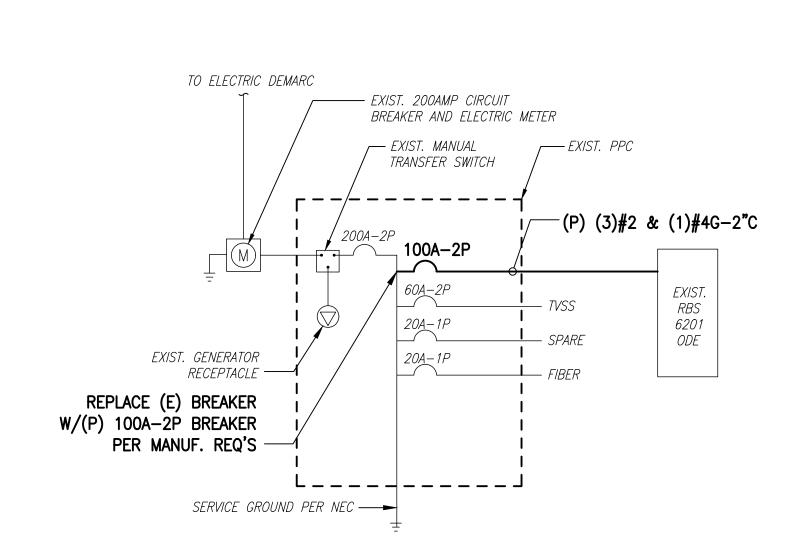
SHEET TITLE

SITE DETAILS

SHEET NUMBER

A-3

1815.0

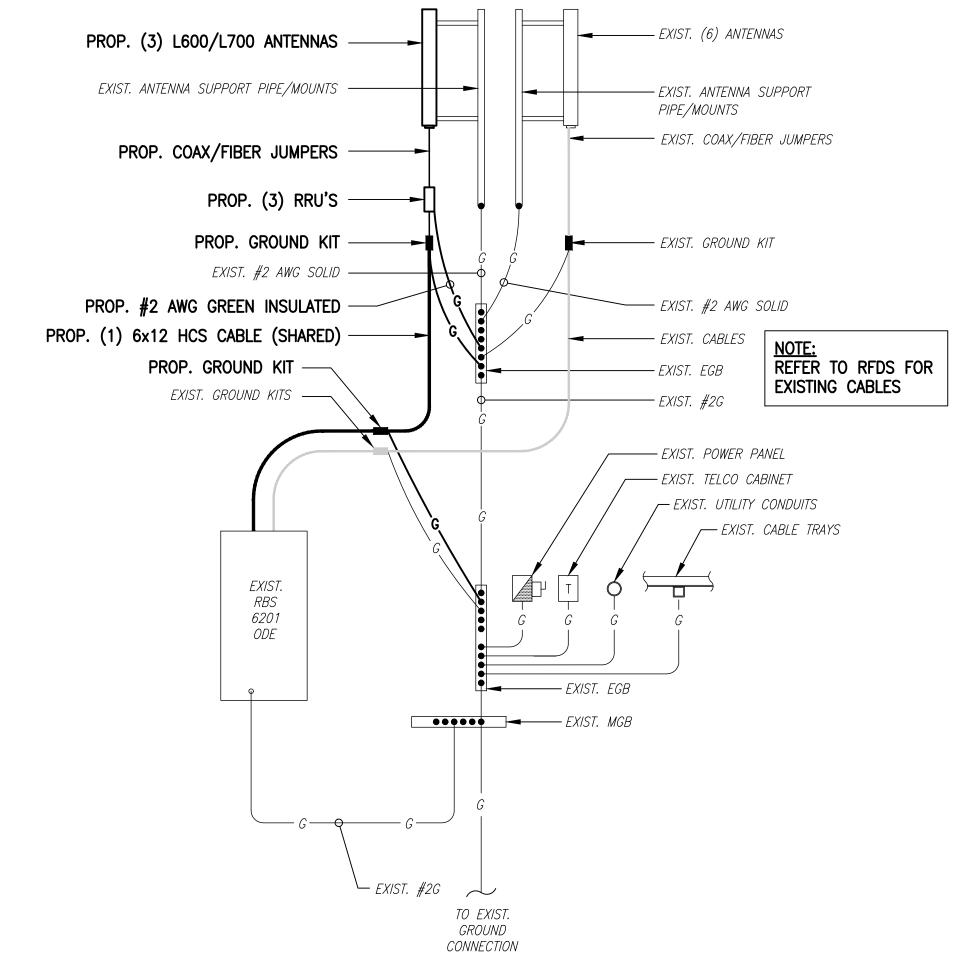


ONE LINE DIAGRAM

E-1

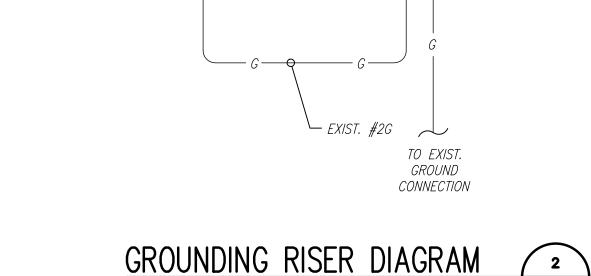
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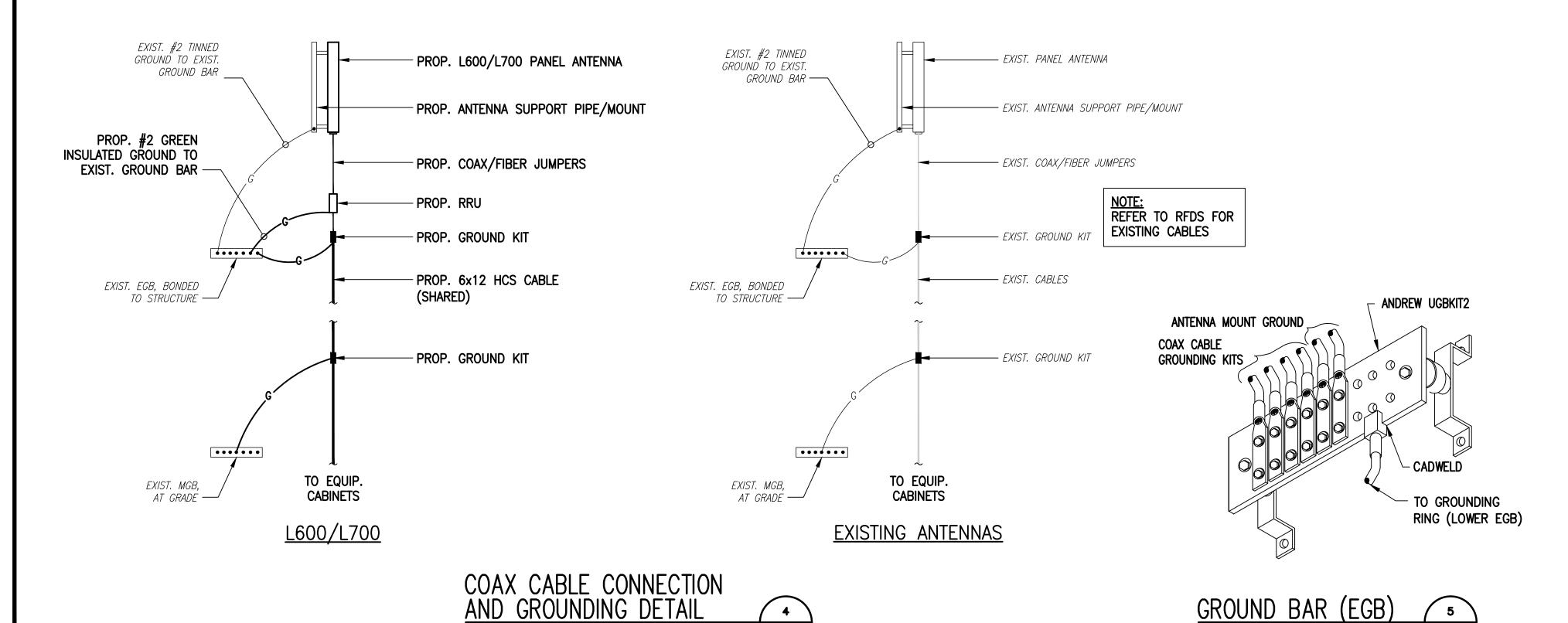


E-1

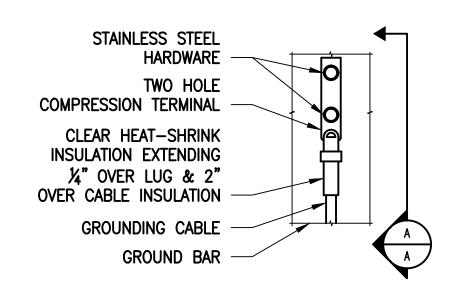
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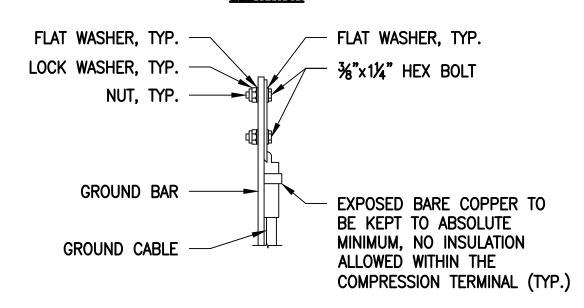
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E-1



# **ELEVATION**



# SECTION A-A

- "DOUBLING UP" OR "STACKING" OF CONNECTION IS NOT PERMITTED. OXIDE INHIBITING COMPOUND TO BE USED AT ALL LOCATIONS.
- 3. CADWELL DOWNLEADS FROM UPPER EGB, LOWER EGB AND MGB.



# ELECTRICAL AND GROUNDING NOTES

- 1. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE NATIONAL ELECTRICAL CODE (NEC) AS WELL AS APPLICABLE STATE AND LOCAL CODES.
- 2. ALL ELECTRICAL ITEMS SHALL BE U.L. APPROVED OR LISTED AND PROCURED PER SPECIFICATION REQUIREMENTS.
- 3. THE ELECTRICAL WORK INCLUDES ALL LABOR AND MATERIAL DESCRIBED BY DRAWINGS AND SPECIFICATION INCLUDING INCIDENTAL WORK TO PROVIDE COMPLETE OPERATING AND APPROVED ELECTRICAL SYSTEM.
- 4. GENERAL CONTRACTOR SHALL PAY FEES FOR PERMITS, AND IS RESPONSIBLE FOR OBTAINING SAID PERMITS AND COORDINATION OF
- 5. ELECTRICAL AND TELCO WIRING OUTSIDE A BUILDING AND EXPOSED TO WEATHER SHALL BE IN WATER TIGHT GALVANIZED RIGID STEEL CONDUITS OR SCHEDULE 80 PVC (AS PERMITTED BY CODE) AND WHERE REQUIRED IN LIQUID TIGHT FLEXIBLE METAL OR NONMETALLIC CONDUITS.
- 6. BURIED CONDUIT SHALL BE SCHEDULE 40 PVC.
- 7. ELECTRICAL WIRING SHALL BE COPPER WITH TYPE XHHW, THWN, OR THININSULATION.
- 8. RUN ELECTRICAL CONDUIT OR CABLE BETWEEN ELECTRICAL UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE PPC AS INDICATED ON THIS DRAWING. PROVIDE FULL LENGTH PULL ROPE. COORDINATE INSTALLATION WITH UTILITY COMPANY.
- 9. RUN TELCO CONDUIT OR CABLE BETWEEN TELEPHONE UTILITY DEMARCATION POINT AND PROJECT OWNER CELL SITE TELCO CABINET AND BTS CABINET AS INDICATED ON THIS DRAWING PROVIDE FULL LENGTH PULL ROPE IN INSTALLED TELCO CONDUIT. PROVIDE GREENLEE CONDUIT MEASURING TAPE AT EACH END.
- 10. WHERE CONDUIT BETWEEN BTS AND PROJECT OWNER CELL SITE PPC AND BETWEEN BTS AND PROJECT OWNER CELL SITE TELCO SERVICE CABINET ARE UNDERGROUND USE PVC, SCHEDULE 40 CONDUIT. ABOVE THE GROUND PORTION OF THESE CONDUITS SHALL BE PVC CONDUIT.
- 11. ALL EQUIPMENT LOCATED OUTSIDE SHALL HAVE NEMA 3R ENCLOSURE.
- 12. PPC SUPPLIED BY PROJECT OWNER.

E-1

- 13. GROUNDING SHALL COMPLY WITH NEC ART. 250. ADDITIONALLY, GROUNDING, BONDING AND LIGHTNING PROTECTION SHALL BE DONE IN ACCORDANCE WITH "T-MOBILE BTS SITE GROUNDING STANDARDS".
- 14. GROUND COAXIAL CABLE SHIELDS MINIMUM AT BOTH ENDS USING MANUFACTURERS COAX CABLE GROUNDING KITS SUPPLIED BY PROJECT
- 15. USE #6 COPPER STRANDED WIRE WITH GREEN COLOR INSULATION FOR ABOVE GRADE GROUNDING (UNLESS OTHERWISE SPECIFIED) AND #2 SOLID TINNED BARE COPPER WIRE FOR BELOW GRADE GROUNDING AS INDICATED ON THE DRAWING.
- 16. ALL GROUND CONNECTIONS TO BE BURNDY HYGROUND COMPRESSION TYPE CONNECTORS OR CADWELD EXOTHERMIC WELD. DO NOT ALLOW BARE COPPER WIRE TO BE IN CONTACT WITH GALVANIZED STEEL.
- 17. ROUTE GROUNDING CONDUCTORS ALONG THE SHORTEST AND STRAIGHTEST PATH POSSIBLE, EXCEPT AS OTHERWISE INDICATED. GROUNDING LEADS SHOULD NEVER BE BENT AT RIGHT ANGLE. ALWAYS MAKE AT LEAST 12" RADIUS BENDS. #6 WIRE CAN BE BENT AT 6" RADIUS WHEN NECESSARY. BOND ANY METAL OBJECTS WITHIN 6 FEET OF PROJECT OWNER EQUIPMENT OR CABINET TO MASTER GROUND BAR OR GROUNDING
- 18. CONNECTIONS TO GROUND BARS SHALL BE MADE WITH TWO HOLE COMPRESSION TYPE COPPER LUGS. APPLY OXIDE INHIBITING COMPOUND TO ALL LOCATIONS.
- 19. APPLY OXIDE INHIBITING COMPOUND TO ALL COMPRESSION TYPE GROUND CONNECTIONS.
- 20. CONTRACTOR SHALL PROVIDE AND INSTALL OMNI DIRECTIONAL ELECTRONIC MARKER SYSTEM (EMS) BALLS OVER EACH GROUND ROD AND BONDING POINT BETWEEN EXIST. TOWER/ MONOPOLE GROUNDING RING AND EQUIPMENT GROUNDING RING.
- 21. CONTRACTOR SHALL TEST COMPLETED GROUND SYSTEM AND RECORD RESULTS FOR PROJECT CLOSE-OUT DOCUMENTATION. 5 OHMNS MINIMUM
- 22. CONTRACTOR SHALL CONDUCT ANTENNA, COAX, AND LNA RETURN-LOSS AND DISTANCE- TO-FAULT MEASUREMENTS (SWEEP TESTS) AND RECORD RESULTS FOR PROJECT CLOSE OUT.

# T-MOBILE NORTHEAST LLC

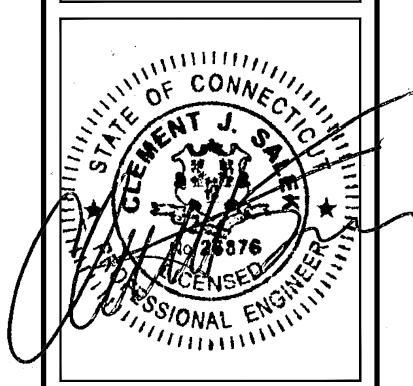
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CHECKED BY:

APPROVED BY:

SUBMITTALS REV. DATE DESCRIPTION

SITE NUMBER:

CT11519D

1 | 11/09/20 | ISSUED FOR CONSTRUCTION | CMC

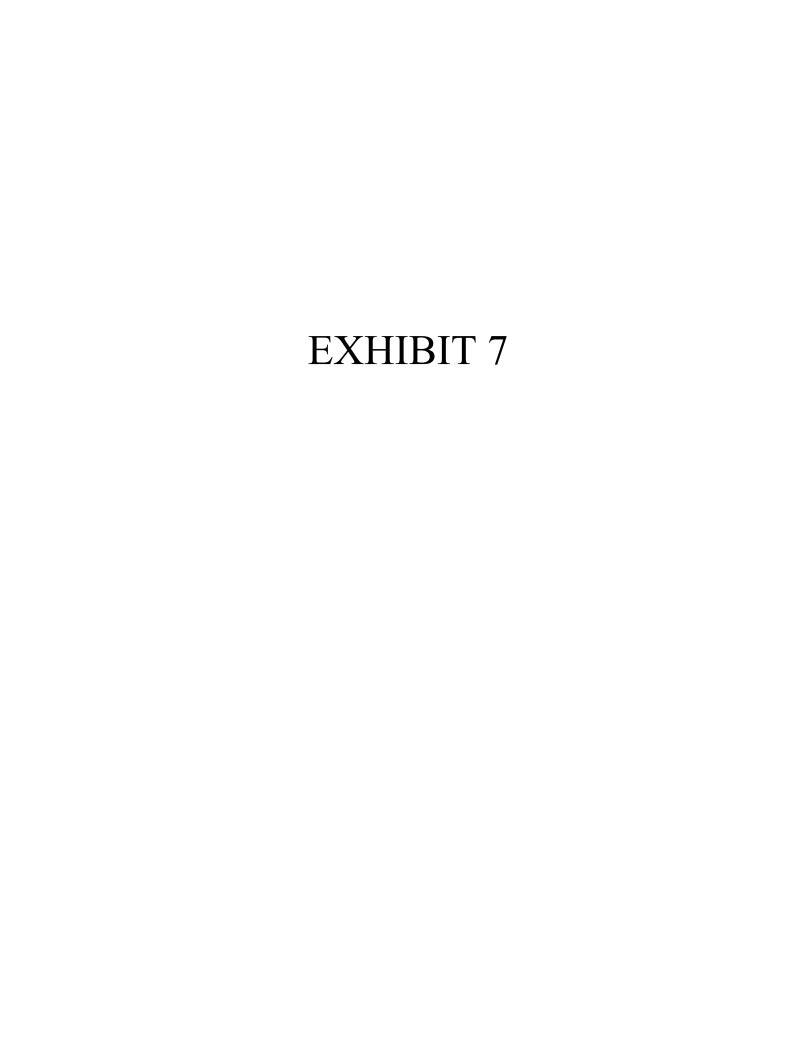
0 | 05/29/19 | ISSUED FOR REVIEW

SITE ADDRESS: 99 KNOWLTON HILL ROAD ASHFORD, CT 06278

SHEET TITLE

**ELECTRIC & GROUNDING** DETAILS

SHEET NUMBER



T-MOBILE: CT11519D SBA: CT13614-A KNOWLTON

# **MOUNT AUGMENTATION @ 147'**

MONOPOLE TOWER

ASHFORD, CT WINDHAM COUNTY

# SITE INFORMATION

STRUCTURE TYPE: MONOPOLE

MOUNT TYPE: PLATFORM

LATITUDE: 41.840773 (NAD 83) LONGITUDE: -72.207521 (NAD 83)

CITY / STATE: ASHFORD, CT

COUNTY: WINDHAM

COORDINATES ARE FOR NAVIGATIONAL PURPOSES ONLY, NOT TO 1A ACCURACY.

# DO NOT SCALE DRAWINGS

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, CONDITIONS ON THE JOB SITE & SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR THE LABOR & MATERIALS FOR THE DISCREPANCIES.

### CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES.

BUILDING CODE AND DESIGN STANDARD: 2015 IBC / TIA-222 / 2018 CT BUILDING CODE

### A&E INFORMATION



DON GEORGE, SE

PO BOX 2621, BOISE, ID 83701 530.539.4787 CONTACT@GEOSTRUCTURAL.COM WWW.GEOSTRUCTURAL.COM

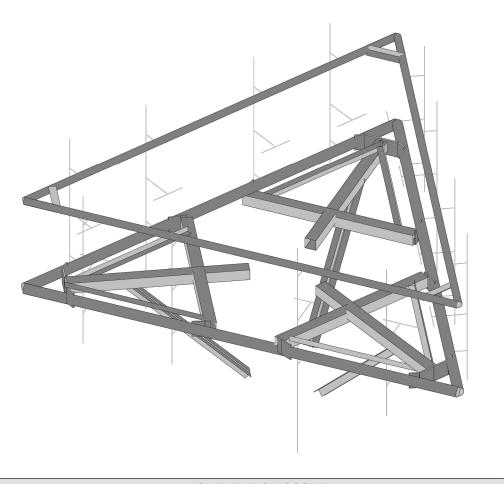
# GENERAL DESIGN NOTES

- 1. THIS PLAN HAS BEEN DESIGNED UTILIZING THE CORRESPONDING MOUNT STRUCTURAL ANALYSIS.
- THESE PLANS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE GOVERNING PROVISIONS OF TIA/EIA-222, ASCE 7, AWS, ACI, AND AISC. MATERIALS AND SERVICES PROVIDED BY THE CONTRACTOR SHALL CONFORM TO THE ABOVE-MENTIONED CODES AND THE CONTRACT SPECIFICATIONS.
- 3. ALL STRUCTURE INFORMATION OBTAINED IN THE FORM OF INFORMATION PROVIDED BY THE CLIENT. CONTRACTOR SHALL OBTAIN AND BECOME FAMILIAR WITH THE REFERENCED DOCUMENTS. CONTRACTOR SHALL ISSUE A REQUEST FOR INFORMATION (RFI) IN THE EVENT ANY DISCREPANCIES ARE DISCOVERED BETWEEN THESE DOCUMENTS AND THE AS-BUILT CONDITIONS IN THE FIELD IN A SITE VISIT THAT SHALL BE PERFORMED PRIOR TO STARTING FABRICATION OR CONSTRUCTION.
- 4. ALL MATERIALS UTILIZED FOR THIS PROJECT MUST BE NEW AND FREE OF ANY DEFECTS.
- 5. ALL PRODUCT OR MATERIAL SUBSTITUTIONS PROPOSED BY THE CONTRACTOR SHALL BE APPROVED IN WRITING BY THE ENGINEER. CONTRACTOR SHALL PROVIDE DOCUMENTATION TO ENGINEER SUITABLE TO DETERMINE IF SUBSTITUTE IS ACCEPTABLE FOR USE AND MEETS THE ORIGINAL DESIGN CRITERIA. DIFFERENCES FROM THE ORIGINAL DESIGN, INCLUDING MAINTENANCE, REPAIR AND REPLACEMENT, SHALL BE NOTED. ESTIMATES OF COSTS/CREDITS ASSOCIATED WITH THE SUBSTITUTION (INCLUDING RE-DESIGN COSTS AND COSTS TO SUB-CONTRACTORS) SHALL BE PROVIDED TO THE ENGINEER. CONTRACTOR SHALL PROVIDE ADDITIONAL DOCUMENTATION AND/OR SPECIFICATIONS TO THE ENGINEER AS REQUESTED.
- PROVIDE STRUCTURAL STEEL SHOP DRAWING(S) TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION (ONLY IF SPECIFICALLY REQUESTED BY ENGINEER).
- 7. UNLESS NOTED OTHERWISE, ALL NEW MEMBERS AND REINFORCING SHALL MAINTAIN THE EXISTING MEMBER WORK LINES AND NOT INTRODUCE ECCENTRICITIES INTO THE STRUCTURE.
- 8. ANY CONTRACTOR-CAUSED DAMAGE TO PROPERTY OF THE LAND OWNER, PROPERTY OF THE STRUCTURE OWNER, PROPERTY OF THE CUSTOMER, SITE FENCING OR GATES, ANY AND ALL UTILITY AND/OR SERVICE LINES, SHOWN OR NOT SHOWN ON THE PLANS, SHALL BE REPAIRED OR REPLACED AT THE SOLE COST OF THE CONTRACTOR AND SHALL BE ACCOMPLISHED BY THE CONTRACTOR OR SUBCONTRACTOR AS APPROVED BY THE ENGINEER OF RECORD AND LAND OWNER. DAMAGE TO EQUIPMENT OR PROPERTY OF ANY KIND BELONGING TO OTHER COMPANIES (BESIDES THE INDICATED CUSTOMER) SHALL BE ADDRESSED BY THE CONTRACTOR WITH THE COMPANIES THAT OWN THE DAMAGED ITEMS.

# SHEET INDEX

SHEET	DESCRIPTION
S-1	TITLE SHEET
S-2	NOTES AND SPECIFICATIONS
S-3	INSPECTION NOTES
S-4	AUGMENTATIONS, SECTIONS & DETAILS

# MOUNT AUGMENTATION CONFIGURATION



# AUGMENTATION SCOPE

MODIFY ALL SECTORS OF CARRIER'S EXISTING MOUNT INSTALLATION AS REQUIRED (UNLESS NOTED OTHERWISE)







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0	06/19/19	ISSUE FOR CONSTRUCTION	RWI

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Į	CHECKED BY:	DWG

THE INFORMATION CONTAINED IN THIS SET OF DOCUMENTS IS PROPRIETARY BY NATURE. ANY USE OR DISCLOSURE OTHER THAN THAT WHICH RELATES TO THE CLIENT NAMES IS STRICTLY PROHIBITED.



SITE INFORMATION:

MOUNT AUGMENTATION

T-MOBILE: CT11519D SBA: CT13614-A KNOWLTON

ASHFORD, CT

LATITUDE: 41.840773 LONGITUDE: -72.207521

SHEET TITLE:

TITLE SHEET

SHEET NUMBER:

S-<sup>-</sup>

### **GENERAL PROJECT NOTES**

- CONTRACTOR IS RESPONSIBLE FOR ERECTING TEMPORARY BARRICADES AND/OR FENCING TO PROTECT THE SAFETY OF THE PUBLIC DURING CONSTRUCTION. THE CONTRACTOR SHALL REMOVE ALL TEMPORARY BARRIERS AND REPAIR ALL DAMAGE TO PROPERTY ON THE SITE CAUSED BY THIS CONSTRUCTION, THE COST OF REPAIR IS THE CONTRACTOR'S RESPONSIBILITY
- ALL WORK SHALL BE IN ACCORDANCE WITH APPLICABLE LOCAL, STATE, AND FEDERAL REQUIREMENTS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL MEASUREMENTS AT THE SITE PRIOR TO ORDERING ANY MATERIALS OR CONDUCTING ANY WORK.
- THESE PLANS DO NOT ADDRESS THE SAFETY AND STABILITY OF THE STRUCTURE DURING ASSEMBLY AND ERECTION, WHICH ARE THE RESPONSIBILITY OF THE ERECTOR, BASED ON THE MEANS AND METHODS CHOSEN BY THE ERECTOR

# CONTRACTOR NOTES

- PRIOR TO BEGINNING CONSTRUCTION, ALL CONTRACTORS AND SUBCONTRACTORS MUST ACKNOWLEDGE IN WRITING TO TOWER OWNER THAT THEY HAVE OBTAINED, UNDERSTAND, AND WILL FOLLOW STRUCTURE OWNER STANDARDS OF PRACTICE, CONSTRUCTION GUIDELINES, ALL SITE AND STRUCTURE/TOWER SAFETY PROCEDURES, ALL PRODUCT LIMITATIONS AND INSTALLATION PROCEDURES USED ON SITE, AND PROPOSED MODIFICATIONS DESCRIBED. RECEIPT OF ACKNOWLEDGEMENT MUST OCCUR PRIOR TO BEGINNING CONSTRUCTION OR CLIMBING. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO PROVIDE THIS DOCUMENTATION FOR STRUCTURE OWNER ON COMPANY LETTERHEAD AND THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO OBTAIN THIS DOCUMENTATION FROM ANY SUBCONTRACTORS (ON SUBCONTRACTOR LETTERHEAD) AND DELIVER IT TO THE STRUCTURE OWNER.
- IF THE CONTRACTOR DISCOVERS ANY EXISTING CONDITIONS THAT ARE NOT REPRESENTED ON THESE DRAWINGS, OR ANY CONDITIONS THAT WOULD INTERFERE WITH THE INSTALLATION OF THE MODIFICATIONS, THE ENGINEER OF RECORD SHALL BE CONTACTED IMMEDIATELY TO EVALUATE THE
- THE CONTRACTOR SHALL SOLICIT AND HIRE THE SERVICES OF A QUALIFIED AUGMENTATION INSPECTOR PRIOR TO BEGINNING CONSTRUCTION. THE AUGMENTATION INSPECTOR MAY BE AN EMPLOYEE OF THE CONTRACTOR'S FIRM, HOWEVER THE INSPECTOR'S ONLY DUTIES SHALL BE INSPECTION, TESTING, AND REPORT CREATION AS REQUIRED ON THE "AUGMENTATION INSPECTION NOTES" SHEET.
- THE CONTRACTOR SHALL NOTIFY THE TOWER OWNER OF THE PLANNED CONSTRUCTION & INSPECTION SCHEDULE, AS WELL AS ANY CHANGES TO THE SCHEDULE, WITHIN TWO BUSINESS DAYS OF THE COMPLETION OF THE SCHEDULE OR SCHEDULE REVISION BOTH PRIOR TO BEGINNING CONSTRUCTION AND DURING CONSTRUCTION AS THE SCHEDULE CHANGES. THE STRUCTURE OWNER WHEN THE WORK HAS BEEN COMPLETED WITHIN 2 BUSINESS DAYS OF THE COMPLETION OF THE WORK AND ASSOCIATED AUGMENTATION INSPECTIONS & TESTING (WHEN APPLICABLE).
- IT IS ASSUMED THAT ANY STRUCTURAL AUGMENTATION WORK SPECIFIED ON THESE PLANS WILL BE ACCOMPLISHED BY KNOWLEDGEABLE WORKMEN WITH TOWER CONSTRUCTION EXPERIENCE. THIS INCLUDES PROVIDING THE NECESSARY CERTIFICATIONS TO THE STRUCTURE OWNER AND ENGINEER INCLUDING BUT NOT LIMITED TO TOWER CLIMBER AND RESCUE CLIMBER CERTIFICATIONS, ET CETERA
- THESE DRAWINGS DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION METHODS, MEANS, TECHNIQUES, SEQUENCES AND PROCEDURES.
- CONTRACTOR SHALL WORK WITHIN THE LIMITS OF THE STRUCTURE OWNER'S PROPERTY OR LEASE AREA AND APPROVED EASEMENTS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY WORK IS WITHIN THESE BOUNDARIES, CONTRACTOR SHALL EMPLOY A SURVEYOR AS REQUIRED, ANY WORK OUTSIDE THESE BOUNDARIES SHALL BE APPROVED IN WRITING BY THE LAND OWNER PRIOR TO MOBILIZATION CONSTRUCTION STAKING AND BOUNDARY MARKING IS THE RESPONSIBILITY OF THE CONTRACTOR.

### STRUCTURAL ERECTION AND BRACING REQUIREMENTS

- THE STRUCTURAL DRAWINGS ILLUSTRATE THE COMPLETED STRUCTURE WITH ALL ELEMENTS IN THEIR FINAL POSITIONS, PROPERLY SUPPORTED AND BRACED.
- THE CONTRACTOR SHALL PROVIDE SHORING AND BRACING AS REQUIRED DURING CONSTRUCTION TO ENSURE STABILITY. DESIGN AND SEQUENCING OF CONSTRUCTION SHORING AND BRACING IS OUTSIDE THE SCOPE OF THIS WORK.
- THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND EXECUTION OF ALL MISCELLANEOUS SHORING, BRACING, TEMPORARY SUPPORTS, GUYING, ETC. NECESSARY TO PROVIDE A COMPLETE AND STABLE STRUCTURE AS SHOWN ON THESE DRAWINGS.

### STRUCTURAL STEEL

- STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE CURRENT EDITION OF THE AISC STEEL CONSTRUCTION MANUAL AND SECTION 4 OF THE TIA CODE
- PRE-QUALIFIED STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING MINIMUM GRADES UNLESS OTHERWISE NOTED:
- ASTM A36, (Fy = 36 KSI) ASTM A36, (Fy = 36 KSI) • PLATES
- PIPES . ASTM A53 GR.B, (Fy = 35 KSI)
- HSS ROUND ASTM A500 GR.B, (Fy = 42 KSI) HSS RECTANGULAR ASTM A500 GR.B. (Fv = 46 KSI)
- W-FLANGE ASTM A992 (Fy = 50 KSI)
- STRUCTURAL BOLTS ASTM A325
- ASTM A307 GR.A • U-BOLTS .
- ASTM A563 (THREADING TO MATCH BOLT) NUTS FOR BOLTS
- WASHERS FOR BOLTS .. ASTM F436
- SEE TABLE 5-1 OF THE TIA CODE FOR ADDITIONAL SHAPES AND STANDARDS THAT ARE NOT LISTED ABOVE.
- NON PRE-QUALIFIED STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING STANDARDS PER THE TIA CODE:
- THE CARBON EQUIVALENT OF STEEL SHALL NOT EXCEED 0.65 PER SECTION 5.4.2 OF THE TIA CODE
- ELONGATION OF STEEL SHALL NOT BE LESS THAN 18%
- TEST REPORTS SHALL BE IN ACCORDANCE WITH ASTM A6 OR A568
- TOLERANCES SHALL BE IN ACCORDANCE WITH ASTM A6
- FIELD CUT EDGES, EXCEPT DRILLED HOLES, SHALL BE GROUND SMOOTH AND COLD GALVANIZED.
- ALL WELDING WORK SHALL CONFORM TO THE AWS D1.1 STRUCTURAL WELDING CODE, ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS ONLY. WELDING ELECTRODES SHALL BE E70XX.
- ALL DETAILING, FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AISC SPECS AND CODES. LATEST EDITION.
- UPON REQUEST, THE CONTRACTOR SHALL SUBMIT DETAILED, ENGINEERED, COORDINATED AND CHECKED SHOP DRAWINGS FOR ALL STRUCTURAL STEEL TO THE ENGINEER OF RECORD TO REVIEW FOR COMPLIANCE WITH DESIGN INTENT PRIOR TO THE START OF FARRICATION AND/OR FRECTION. GEOSTRUCTURAL IS ARSOLVED OF ALL LIABILITY ASSOCIATED WIT THE MISINTERPRETATION OF THE CONSTRUCTION DOCUMENTS IF CONTRACTOR CHOOSES NOT TO SUBMIT SHOP DRAWINGS.
- TORCH-CUTTING OF ANY KIND SHALL NOT BE PERMITTED.
- ALL BOLT HOLES SHALL BE STANDARD SIZE BOLT HOLES PER AISC 360, UNLESS OTHERWISE NOTED, ALL HOLES SHALL BE SHOP DRILLED OR SUB-PUNCHED AND REAMED, BURNING OF HOLES IS NOT PERMITTED. WHERE SLOTTED OR OVERSIZE HOLES ARE SPECIFIED ON THE DRAWINGS, EXTRA-THICK ASTM F436 PLATE WASHERS SHALL BE USED (3/16" MINIMUM THICKNESS) WITH A DIAMETER SUITABLE TO COVER THE EXTENTS OF THE SLOT OR HOLE. BOLTS SHALL BE HEAVY-HEX where available in the size and grade specified, otherwise bolts shall be hex head cap screws.
- ALL STEEL HARDWARE, INCLUDING ADHESIVE OR EMBEDDED ANCHOR BOLTS AND THEIR ACCESSORIES, SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153 (EXCEPT BOLTS SMALLER THAN ½" SHALL CONFORM TO FE/ZN 3 AT PER ASTM F1941 WHERE HOT-DIP GALVANIZED BOLTS ARE NOT AVAILABLE). ALL STEEL MEMBERS, INCLUDING WELDMENTS, SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A 123. REPAIR DAMAGE TO GALVANIZED COATINGS USING ASTM A780 PROCEDURES WITH A ZINC RICH PAINT (SUCH AS ZINC GALVILITE) FOR GALVANIZING DAMAGED BY HANDLING, TRANSPORTING, CUTTING, WELDING, OR BOLTING. DO NOT HEAT SURFACES TO WHICH REPAIR PAINT HAS BEEN APPLIED. CALL OUT HOLES REQUIRED FOR HOT-DIP GALVANIZING ON SHOP DRAWINGS.
- MEMBERS SHALL BE SHOP-FABRICATED AND WELDED TO THE EXTENT PRACTICABLE IN ORDER TO REDUCE FIELD INSTALLATION COSTS

### STRUCTURAL BOLTS

- ALL CONNECTIONS OF STRUCTURAL STEEL MEMBERS SHALL BE MADE USING SPECIFIED GALVANIZED HIGH STRENGTH ASTM A325 OR A490 BOLTS WITH THREADS EXCLUDED FROM SHEAR PLANE.
- FASTENERS SHALL BE INSTALLED IN PROPERLY ALIGNED HOLES, WITH BOLT HEADS FACING DOWN WHERE APPLICABLE.
- ALL BOLTS AT EVERY CONNECTION SHALL BE INSTALLED SNUG-TIGHT UNTIL THE SECTION IS FULLY COMPACTED AND ALL PLIES ARE JOINED, AND THEN TIGHTENED FURTHER BY AISC - "TURN OF THE NUT" METHOD. TIGHTENING SHALL PROGRESS SYSTEMATICALLY
- BOLT LENGTHS UP TO AND INCLUDING 4 DIAMETERS SHALL BE TENSIONED 1/3 TURN BEYOND SNUG-TIGHT. BOLT LENGTHS OVER 4 DIAMETERS SHALL BE 11/2 TURNS BEYOND SNUG-TIGHT.
- ALL BOLTED CONNECTIONS SHALL USE LOCK WASHERS.
- MINIMUM EDGE DISTANCE FOR BOLTS SHALL BE 1/5" CENTER TO EDGE UNLESS OTHERWISE NOTED.

NOMINAL HOLE DIMENSIONS:		
BOLT Ø	STANDARD HOLE Ø	
1/2"Ø	9/16"Ø	
5/8"Ø	11/16"Ø	
3/4"Ø	13/16"Ø	
7/8''Ø	15/16"Ø	
1"Ø	1½6"Ø	







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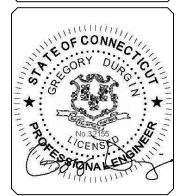
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SITE INFORMATION:

MOUNT AUGMENTATION

T-MOBILE: CT11519D SBA: CT13614-A KNOWLTON

ASHFORD, CT

LATITUDE: 41.840773 LONGITUDE: -72.207521

SHEET TITLE:

**NOTES AND SPECIFICATIONS** 

SHEET NUMBER:

S-2

PRE-CONSTRUCTION INSPECTION CHECKLIST		
CONSTRUCTION AND/OR INSTALLATION INSPECTIONS REQUIRED FOR REPORT? (CHECK=YES, BLANK=NO)	INSPECTION REPORT ITEM	
√	AUGMENTATION INSPECTION CHECKLIST	
√	APPROVED SHOP DRAWINGS (LATEST REVISION)	
√	FABRICATION INSPECTION	
	FABRICATOR'S CERTIFIED WELD INSPECTOR (CWI)	
	FABRICATOR'S QUALIFIED PERSONNEL FOR WELDING	
√	MATERIAL TEST REPORT(S) / MILL CERTIFICATE(S)	
	FABRICATOR'S NON-DESTRUCTIVE TESTING (NDT) TECHNICIAN	
√	PACKING SLIPS FOR STRUCTURAL MATERIALS	

CONSTRUCTION INSPECTION CHECKLIST		
CONSTRUCTION AND/OR INSTALLATION INSPECTIONS REQUIRED FOR REPORT? (CHECK=YES, BLANK=NO)	INSPECTION REPORT ITEM	
√	CONSTRUCTION INSPECTIONS	
	FOUNDATION INSPECTIONS	
	CONCRETE COMPRESSIVE STRENGTH AND SLUMP TESTING RESULTS/CERTIFICATES	
	ADHESIVE ANCHOR ROD(S) INSTALLATION INSPECTION	
	BASE PLATE GROUT INSPECTION	
	THIRD-PARTY CERTIFIED WELD INSPECTION (INCLUDING IBC SPECIAL INSPECTIONS)	
	SOIL EXCAVATION — DENSITY TESTING, COMPACTION INSPECTION/VERIFICATION, USE OF SUITABLE FILL	
√	GALVANIZING REPAIR MATERIAL PREPARATION, INSPECTION, & PAINT APPLICATION	
	GUY WIRE (RE-)TENSION REPORT AND INSPECTION	
√	PRIME CONTRACTOR'S AS-BUILT DOCUMENTS (SIGNED & DATED)	

POST-CONSTRUCTION INSPECTION CHECKLIST		
CONSTRUCTION AND/OR INSTALLATION INSPECTIONS REQUIRED FOR REPORT? (CHECK=YES, BLANK=NO)	INSPECTION REPORT ITEM	
√	AUGMENTATION INSPECTOR'S ISSUE LIST (INCLUDING CORRECTIVE ACTIONS TAKEN) AND/OR REDLINED RECORD DRAWINGS	
	POST-INSTALLED ADHESIVE ANCHOR ROD PULL-OUT TESTING	
V	PHOTOGRAPHS OF AUGMENTATIONS (INCLUDE PHOTOS OF BOTH SIDES OF WELDED OR BOLTED CONNECTIONS, OF OVERALL AND DETAIL VIEWS OF INSTALLED AUGMENTATIONS, AND BEFORE/AFTER PHOTOS OF ANY ISSUES IDENTIFIED BY THE INSPECTOR)	

### **GENERAL NOTES**

- THE POST-AUGMENTATION INSPECTION IS A VISUAL EXAMINATION OF STRUCTURE AUGMENTATIONS AND A REVIEW OF ANY REQUIRED CONSTRUCTION INSPECTIONS, TESTING, AND OTHER DATA TO VERIFY THAT THE AUGMENTATIONS ARE INSTALLED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AS DESIGNED BY THE ENGINEER OF RECORD. THE CONTRACT DOCUMENTS INCLUDE THESE AUGMENTATION DRAWINGS, ANY PROJECT SPECIFICATIONS REFERENCED TO IN THE PROJECT NOTES OR OTHERWISE PROVIDED WITH THE DRAWINGS, AND OTHER DOCUMENTS OR DRAWINGS PROVIDED WITH THE AUGMENTATION DRAWINGS WITH THE INTENT THAT THEY BE USED AS A DESIGN AID OR GUIDELINE FOR CONSTRUCTION.
- 2. THE POST-AUGMENTATION INSPECTION SHALL CONFIRM INSTALLATION CONFIGURATION AND WORKMANSHIP ONLY AND IS NOT A QUALITATIVE REVIEW OF THE ENGINEERING ASPECTS OF THE DESIGN OR THE DESIGN DRAWNINGS. THE AUGMENTATION INSPECTOR IS NOT TAKING OWNERSHIP OF THE AUGMENTATION DESIGN IN THE PERFORMANCE OF THEIR DUTIES. OWNERSHIP OF THE AUGMENTATION DESIGN'S EFFECTIVENESS AND INTENT, LIES WITH THE ENGINEER OF RECORD.
- 3. TO ENSURE THAT THE REQUIREMENTS OF THE POST-AUGMENTATION INSPECTION ARE MET, IT IS ESSENTIAL THAT COORDINATION BETWEEN THE PRIME CONTRACTOR AND THE AUGMENTATION INSPECTOR BEGIN AS SOON AS THE PROJECT IS FUNDED AND WORK ENTERS THE PLANNING STAGE THE PRIME CONTRACTOR AND AUGMENTATION INSPECTOR SHALL BE PROACTIVE IN IDENTIFYING CONSTRUCTION ISSUES AND COMMUNICATING THESE ISSUES TO EACH OTHER AND TO THE ENGINEER OF RECORD AND STRUCTURE OWNER AND/OR CUSTOMER, AS REQUIRED.

### INSPECTION AND REPORT RECOMMENDATIONS

- THE FOLLOWING ARE PROVIDED IN THE INTENT OF ENHANCING THE EFFECTIVENESS OF THE AUGMENTATION INSPECTION AND IMPROVING THE EFFICIENCY OF THE PROCESS OF COLLECTING AND COMPILING THE INFORMATION INTO A USABLE REPORT:
- 1.1. IT IS RECOMMENDED THAT THE PRIME CONTRACTOR PROVIDE THE AUGMENTATION INSPECTOR AT LEAST 5 BUSINESS DAYS NOTICE FOR WHEN THE SITE WILL BE READY FOR THE AUGMENTATION INSPECTION
- 1.2. THE PRIME CONTRACTOR AND THE AUGMENTATION INSPECTOR SHALL COORDINATE CLOSELY THROUGHOUT THE ENTIRE PROJECT.
- 1.3. THE PRIME CONTRACTOR AND AUGMENTATION INSPECTOR SHALL BOTH BE PRESENT DURING THE INITIAL INSPECTION IN ORDER TO ALLOW FOR THE REMEDIATION OF DEFICIENCIES DURING THE INSPECTION, AS PRACTICABLE. IT MAY BE PREFERABLE TO KEEP WORK CREWS AND THEIR EQUIPMENT ON SITE TO REMEDIATE DEFICIENCIES DURING INSPECTIONS.

# INSPECTION RESCHEDULING AND CANCELLATION

1. IF THE PRIME CONTRACTOR AND AUGMENTATION INSPECTOR HAVE AGREED UPON A TIME AND DATE FOR A GIVEN INSPECTION AND EITHER PARTY RESCHEDULES OR CANCELS THE INSPECTION, THE STRUCTURE OWNER SHALL NOT BE RESPONSIBLE FOR COSTS, FEES, LOST DEPOSITS, OR OTHER EXPENSES INCURRED BY THE RIME CONTRACTOR, THEIR SUBCONTRACTOR(S), OR THE AUGMENTATION INSPECTOR DUE TO THESE SCHEDULING CHANGES. EXCEPTIONS MAY BE MADE IN THE EVENT OF UNCONTROLLABLE SITUATIONS SUCH AS NATURAL DISASTERS, SEVERE WEATHER, OR OTHER CONDITIONS THAT COMPROMISE THE SAFETY OF THE PARTIES INVOLVED.

# REMEDIATION OF FAILING INSPECTION

- IN THE EVENT THAT ANY PORTION OF THE AUGMENTATION WORK IS DETERMINED TO BE UNSATISFACTORY BY THE MODIFICATION INSPECTOR, THE PRIME CONTRACTOR SHALL WORK WITH THE AUGMENTATION INSPECTOR TO CREATE A PLAN OF ACTION THAT WILL EITHER:
- 1.1. REPAIR THE DEFICIENT WORK TO SATISFACTORY CONDITION AND INCLUDE A SUBSEQUENT RE-INSPECTION OF THE WORK TO VERIFY THAT IT IS SATISFACTORY.
- 1.2. OR, WITH THE PERMISSION OF THE STRUCTURE OWNER AND/OR CUSTOMER, THE PRIME CONTRACTOR MAY WORK WITH THE ENGINEER OF RECORD TO REVIEW THE AS-BUILT CONDITION OF THE AUGMENTATION TO DETERMINE IF IT IS STRUCTURALLY ACCEPTABLE. IF THIS ACTION IS NOT ACCEPTABLE TO ANY PARTY, THE PRIME CONTRACTOR SHALL PROCEED TO REPAIR THE DEFICIENT WORK TO A SATISFACTORY CONDITION

### AUGMENTATION INSPECTOR'S RESPONSIBILITIES

- 1. THE AUGMENTATION INSPECTOR MAY BE AN EMPLOYEE OF THE CONTRACTOR'S FIRM, HOWEVER THE INSPECTOR'S ONLY DUTIES SHALL BE INSPECTION, TESTING, AND REPORT CREATION.
- 2. THE AUGMENTATION INSPECTOR SHALL CONTACT THE PRIME CONTRACTOR AS SOON AS THEY HAVE RECEIVED A PURCHASE ORDER OR PAYMENT FOR THIS INSPECTION. THE AUGMENTATION INSPECTOR SHALL REVIEW THE REQUIREMENTS OF THE INSPECTION CHECKLIST, SHALL WORK WITH THE PRIME CONTRACTOR TO DEVELOP A SCHEDULE OF NECESSARY ON-SITE INSPECTIONS, AND SHALL DISCUSS ANY SITE-SPECIFIC INSPECTION REQUIREMENTS OR OTHER CONCERNS.
- 3. THE AUGMENTATION INSPECTOR IS RESPONSIBLE FOR COLLECTING ALL PRIME CONTRACTOR INSPECTION AND TEST REPORTS (INCLUDING THOSE OF ASSIGNED SUB-CONTRACTORS), SHALL REVIEW THE REPORTS FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS AND SHALL CONDUCT THE NECESSARY ON-SITE INSPECTIONS.

# PRIME CONTRACTOR'S RESPONSIBILITIES

- 1. THE PRIME CONTRACTOR SHALL CONTACT THE AUGMENTATION INSPECTOR AS SOON AS THEY HAVE RECEIVED A PURCHASE ORDER OR PAYMENT FOR THE AUGMENTATION INSTALLATION OR PROJECT. THE PRIME CONTRACTOR SHALL REVIEW THE REQUIREMENTS OF THE AUGMENTATION INSPECTION CHECKLIST, SHALL WORK WITH THE AUGMENTATION INSPECTOR TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE INSPECTIONS, AND SHALL DISCUSS SPECIFIC INSPECTION AND TESTING REQUIREMENTS WITH THE AUGMENTATION INSPECTOR IN DETAIL TO OBTAIN A FULL UNDERSTANDING OF THE REQUIRED INSPECTIONS AND TESTING.
- THE PRIME CONTRACTOR SHALL PERFORM AND RECORD THE TESTING AND INSPECTION RESULTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE AUGMENTATION INSPECTION CHECKLIST.

# PHOTOGRAPHY REQUIREMENTS

- THE PRIME CONTRACTOR AND AUGMENTATION INSPECTOR SHALL BETWEEN THE EFFORTS OF BOTH PARTIES AND THEIR EMPLOYED PERSONNEL PROVIDE PHOTOGRAPHS WITH THE INSPECTION REPORT TO INCLUDE THE FOIL OWING:
- GENERAL SITE PHOTOGRAPHS PRE-CONSTRUCTION
- AUGMENTATION INSTALLATION PHOTOGRAPHS DURING CONSTRUCTION/ERECTION OPERATIONS AND INSPECTIONS
- o.1. RAW MATERIALS
- b.2. PHOTOS OF DETAILED WORK REQUIRED ON THE DRAWINGS (CONNECTIONS, WELDMENTS, FIELD-FABRICATED MEMBERS, ETC.)
- b.3. BOLT INSTALLATION AND TORQUE/PRETENSION.
- b.4. FINAL INSTALLED CONDITION (AFTER DEFICIENT CONDITIONS, IF ANY, ARE REMEDIATED).
- b.5. REPAIR OF SURFACE COATINGS (INCLUDING GALVANIZING AND/OR PAINT COATING)
- POST-AUGMENTATION PHOTOGRAPHS OF THE SITE & WORK.
- d. PHOTOGRAPHS OF THE FINAL STATE OF THE SITE AT CONCLUSION OF THE WORK BY THE PRIME CONTRACTOR, ASSOCIATED SUBCONTRACTORS, AND THE AUGMENTATION INSPECTOR.
- e. OTHER PHOTOS MAY BE INCLUDED AT PRIME CONTRACTOR & AUGMENTATION INSPECTOR'S DISCRETION.

NOTE: PHOTOS OF AUGMENTATIONS INSTALLED ON THE STRUCTURE ABOVE AN ELEVATION OF 20 FT SHALL REQUIRE PHOTOS TAKEN FROM THE STRUCTURE AS WELL AS OVERALL PHOTOGRAPHS OF THE AUGMENTATIONS TAKEN FROM THE GROUND.

# **OWNER INSPECTIONS**

- THE STRUCTURE OWNER MAY CONDUCT INSPECTIONS TO VERIFY THE QUALITY AND COMPLETENESS
  OF THE PREVIOUSLY COMPLETED AUGMENTATION INSPECTION REPORTS FOR THE AUGMENTATION
  INSTALLATION WORK.
- INSPECTIONS MAY BE COMPLETED BY A 3RD-PARTY FIRM OF THE STRUCTURE OWNER'S CHOOSING AFTER A AUGMENTATION PROJECT IS COMPLETED AND A PASSING AUGMENTATION INSPECTION REPORT IS ISSUED.







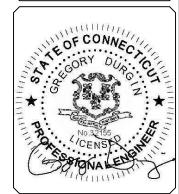
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SITE INFORMATION:

MOUNT AUGMENTATION

T-MOBILE: CT11519D SBA: CT13614-A KNOWLTON

ASHFORD, CT

LATITUDE: 41.840773 LONGITUDE: -72.207521

SHEET TITLE:

INSPECTION NOTES

SHEET NUMBER:

S-3

# NEW MOUNT AUGMENTATIONS

- INSTALL PLATFORM REINFORCEMENT KIT; LOCATED 3.5' BELOW THE EXISTING STANDOFF CENTERLINE TO MONOPOLE SHAFT AND ATTACHING TO THE EXISTING STANDOFF MEMBER APPROXIMATELY 3.5' OUT FROM THE COLLAR ATTACHMENT.

   SITEPRO1 PRK-1245L, (1) TOTAL.
- 2 INSTALL HANDRAIL KIT; LOCATED 3.0' ABOVE THE EXISTING PLATFORM RAIL AND ATTACHING TO THE MOUNT PIPES.
   SITEPRO1 HRK12-U OR 14-U, (1) TOTAL. VERIFY REQUIRED SIZE IN FIELD. ATTACH ALL MOUNT PIPES TO NEW HANDRAIL WITH KIT-PROVIDED CROSS-OVER PLATES.

AUGMENTATIONS SHALL BE COMPLETED PRIOR TO THE INSTALLATION OF ANY NEW EQUIPMENT.

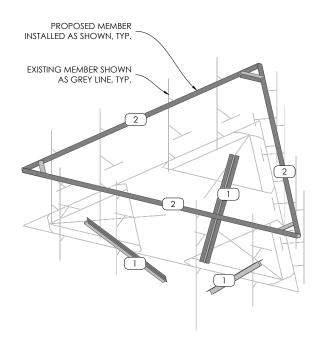
# **CONSTRUCTION NOTES**

- 1. SCOPE OF WORK MUST BE COMPLETED AT WIND SPEEDS < 20 MPH.
- ALL DIMENSIONS ARE APPROXIMATE. CONTRACTOR SHOULD FIELD-VERIFY ALL DIMENSIONS BEFORE FABRICATION OF STEEL AND COMMENCEMENT OF WORK, FIELD CUT MEMBERS AS REQUIRED.
- CONTRACTOR TO COORDINATE THE TEMPORARY
   REMOVAL/RELOCATION/REPLACEMENT OF ELEMENTS (E.G. COAX, CLIPS, TMAS, ETC.)
   CONNECTED TO, OR IN THE DIRECT PATH, OF NEW AUGMENTATION MEMBERS.

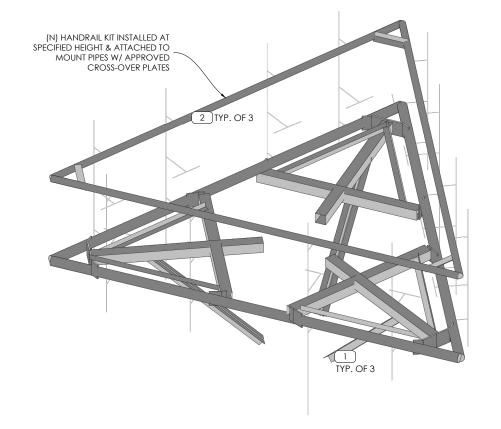


**EXISTING MOUNT** 

# PLATFORM @ 147' AUGMENTATION



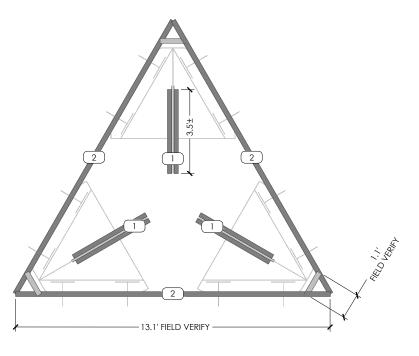
MOUNT AUGMENTATION ISOLATION



AUGMENTED MOUNT ISOMETRIC

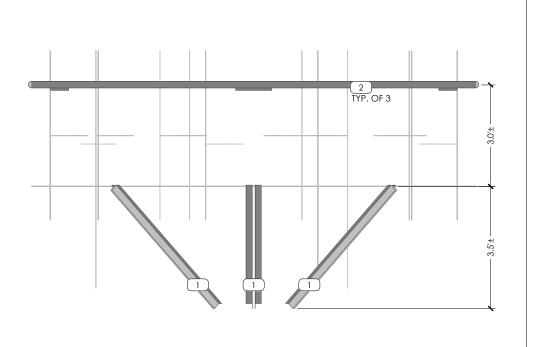
# **INSTALLATION NOTES**

- . AUGMENTATION MEMBER(S) MAY NEED TO BE FIELD-CUT TO LENGTH TO ACCOMMODATE THIS INSTALLATION. CONTRACTOR TO CUT AND DRILL TO SUIT AS REQUIRED AND APPLY (2) COATS OF COLD-GALV. COMPOUND TO CUT MEMBER ENDS.
- CONTRACTOR TO CHECK ALL EXISTING MEMBER CONNECTION BOLTS, PARTICULARLY STANDOFF TO TOWER BOLTS, FOR PROPER INSTALLATION AND TIGHTNESS.
- COORDINATE PLACEMENT OF NEW
   AUGMENTATION MEMBERS WITH EXISTING TOWER
   AND CLIMBING FACILITY ELEMENTS (E.G. STEP PEGS,
   COAX PORTS, ETC.)
- REFER TO CONSTRUCTION DRAWINGS (BY OTHERS)
   AND MOUNT STRUCTURAL ANALYSIS FOR
   APPROVED INSTALLATION LOCATIONS AND
   QUANTITIES OF APPURTENANCES.



AUGMENTED MOUNT PLAN

SCALE: N.T.S.



AUGMENTED MOUNT FRONT ELEVATION SCALE: N.T.S.

T - Mobile



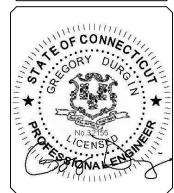


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MOUNT AUGMENTATION

T-MOBILE: CT11519D SBA: CT13614-A KNOWLTON

ASHFORD, CT

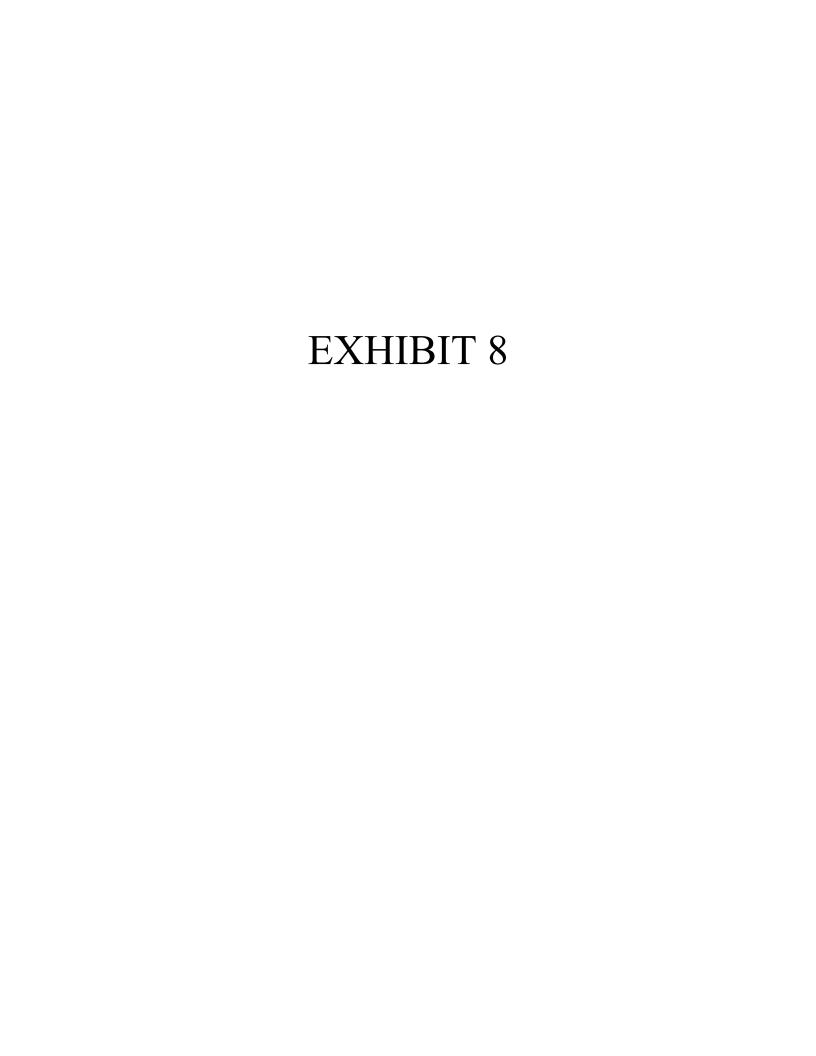
LATITUDE: 41.840773 LONGITUDE: -72.207521

SHEET TITLE:

AUGMENTATIONS SECTIONS & DETAILS

SHEET NUMBER:

S-4





# **Tower Engineering Solutions**

Phone (972) 483-0607, Fax (972) 975-9615 1320 Greenway Drive, Suite 600, Irving, Texas 75038

# **Structural Analysis Report**

**Existing 149 ft Monopole** 

**Customer Name: SBA Communications Corp** 

**Customer Site Number: CT13614-A** 

**Customer Site Name: Knowlton** 

Carrier Name: T-Mobile (App#: 117023, V2)

Carrier Site ID / Name: CT11519D / Knowlton

Site Location: 99 Knowlton Hill Rd

Ashford, Connecticut

**Windham County** 

Latitude: 41.840777

Longitude: -72.207528

# **Analysis Result:**

Max Structural Usage: 48% [Pass]

Max Foundation Usage: 46% [Pass]

Additional Usage Caused by Mount Modification: + 2%

Report Prepared By: Leonardo Klem



# Introduction

The purpose of this report is to summarize the analysis results on the 149 ft Monopole to support the proposed antennas and transmission lines in addition to those currently installed. Any modification listed under Sources of Information was assumed completed and was included in this analysis.

# **Sources of Information**

<b>Tower Drawings</b>	Sabre Job#06-06307 dated 6/29/05.
	FDH TIA Inspection Report.
Foundation Drawing	Sabre Job#06-06307 dated 6/29/19.
Geotechnical Report	JGI Project#05360G dated 6/28/05.
<b>Modification Drawings</b>	N/A

# **Analysis Criteria**

The rigorous analysis was performed in accordance with the requirements and stipulations of the ANSI/TIA-222-G. In accordance with this standard, the structure was analyzed using **TESPoles**, a proprietary analysis software. The program considers the structure as an elastic 3-D model with second-order effects and temperature effects incorporated in the analysis. The analysis was performed using multiple wind directions.

Wind Speed Used in the Analysis: Ultimate Design Wind Speed V<sub>ult</sub> = 130 mph (3-Sec. Gust)/

Nominal Design Wind Speed  $V_{asd} = 101 \text{ mph (3-Sec. Gust)}$ 

Wind Speed with Ice: 50 mph (3-Sec. Gust) with 1" radial ice concurrent

**Operational Wind Speed:** 60 mph + 0" Radial ice

Standard/Codes: ANSI/TIA-222-G / 2015 IBC / 2018 Connecticut State Building

Code

Exposure Category: C
Structure Class: II
Topographic Category: 1
Crest Height: 0 ft

**Seismic Parameters:**  $S_S = 0.174, S_1 = 0.063$ 

This structural analysis is based upon the tower being classified as a Structure Class II; however, if a different classification is required subsequent to the date hereof, the tower classification will be changed to meet such requirement and a new structural analysis will be run.

#### **Existing Antennas, Mounts and Transmission Lines**

The table below summarizes the antennas, mounts and transmission lines that were considered in the analysis as existing on the tower.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner	
-		6	RFS APXV18-209014-02				
-	147	3	RFS LNX-6515DS	12' Low Profile Platform	(10) 1 E /0"	T-Mobile	
-	147	6	TMAs	12 LOW Profile Platform	(18) 1 5/8"	1-Mobile	
-		3	Kathrein 782 11056				
6		6	Powerwave 7770 - Panel				
7		3	KMW AM-X-CD-17-65-00T - Panel		/12\ 1 F /0"		
8	137.5	6	Powerwave LGP21401	14' Low Profile Platform	(12) 1 5/8" (2) 3/4" DC	АТ&Т	
9	157.5	6	Powerwave LGP21903	14 LOW Profile Platform	(1) 7/16" Fiber	AIQI	
10		6	Ericsson RRUS11		(1) //10 Fibel		
11		1	Raycap DC2-48-60-18-8F				
12	127.0	6	Antel LPA-80080/4CF - Panel	10' Low Profile Platform	(12) 1 E /0"	Verizon	
13	127.0	6	Antel LPA 185080-8CF - Panel	TO LOW PROBLE PROBLEM	(12) 1 5/8"	verizori	

### **Proposed Carrier's Final Configuration of Antennas, Mounts and Transmission Lines**

Information pertaining to the proposed carrier's final configuration of antennas and transmission lines was provided by SBA Communications Corp. The proposed antennas and lines are listed below.

Items	Elevation (ft)	Qty.	Antenna Descriptions	Mount Type & Qty.	Transmission Lines	Owner
1		6	RFS APXV18-209014-C-A20 - Panel			
2		3	RFS APXVAARR24_43-U-NA20 - Panel	12' Low Profile Platform	/17\ 1 F /0"	
3	147.0	6	Ericsson KRY 112 489/2 TMA	(1) Sitepro PRK-1245L	(17) 1 5/8" (1) 1 5/8" Fiber	T-Mobile
4		3	Ericsson Radio 4449 B71+B12 RRU	(1) Sitepro HRK12-U	(1) 1 3/8 FIDE	
5		3	Kathrein 782 11056 Bias Ts			

All transmission lines are considered running inside of the pole shafts.

#### **Analysis Results**

The results of the structural analysis, performed for the wind and ice loading and antenna equipment as defined above, are summarized as the following:

	Pole shafts	Anchor Bolts	Base Plate
Max. Usage:	48.1%	47.0%	39.0%
Pass/Fail	Pass	Pass	Pass

#### **Foundations**

	Moment (Kip-Ft)	Shear (Kips)
Original Design Reactions	3739.4	28.2
Analysis Reactions	3254.1	31.1
Factored Reactions*	5048.2	38.1
% of Design Reactions	64.5%	81.6%

<sup>\*</sup> Per section 15.5.1 of the TIA-222-G standard, factored reactions were obtained by multiplying a 1.35 factor to the original design reactions.

The foundation has been investigated using the supplied documents and soils report and was found adequate. Therefore, no modification to the foundation will be required.

### **Operational Condition (Rigidity):**

Operational characteristics of the tower are found to be within the limits prescribed by ANSI/TIA-222-G for the installed antennas. The maximum twist/sway at the elevation of the proposed equipment is 0.6776 degrees under the operational wind speed as specified in the Analysis Criteria.

#### **Conclusions**

Based on the analysis results, the existing structure and its foundation were found to be adequate to safely support the existing and proposed equipment and meet the minimum requirements per the ANSI/TIA-222-G Standard under the design basic wind speed as specified in the Analysis Criteria.

#### **Standard Conditions**

- 1. This analysis was performed based on the information supplied to (TES) Tower Engineering Solutions, LLC. Verification of the information provided was not included in the Scope of Work for TES. The accuracy of the analysis is dependent on the accuracy of the information provided.
- 2. The structural analysis was performance based upon the evidence available at the time of this report. All information provided by the client is considered to be accurate.
- 3. The analyses will be performed based on the codes as specified by the client or based on the best knowledge of the engineering staff of **TES**. In the absence of information to the contrary, all work will be performed in accordance with the latest relevant revision of ANSI/TIA-222. If wind speed and/or ice loads are different from the minimum values recommended by the EIA/TIA-222 standard or other codes, **TES** should be notified in writing and the applicable minimum values provided by the client.
- 4. The configuration of the existing mounts, antennas, coax and other appurtenances were supplied by the customer for the current structural analysis. TES has not visited the tower site to verify the adequacy of the information provided. If there is any discrepancy found in the report regarding the existing conditions, TES should be notified immediately to evaluate the effect of the discrepancy on the analysis results.
- 5. The client will assume responsibility for rework associated with the differences in initially provided information, including tower and foundation information, existing and/or proposed equipment and transmission lines.
- 6. If a feasibility analysis was performed, final acceptance of changed conditions shall be based upon a rigorous structural analysis.

#### Usage Diagram - Max Ratio 48.15% at 0.0ft

Structure: CT13614-A-SBA Code: EIA/TIA-222-G

Site Name:KnowltonExposure:CHeight:149.00 (ft)Gh:1.1

Base Elev: 0.000 (ft)

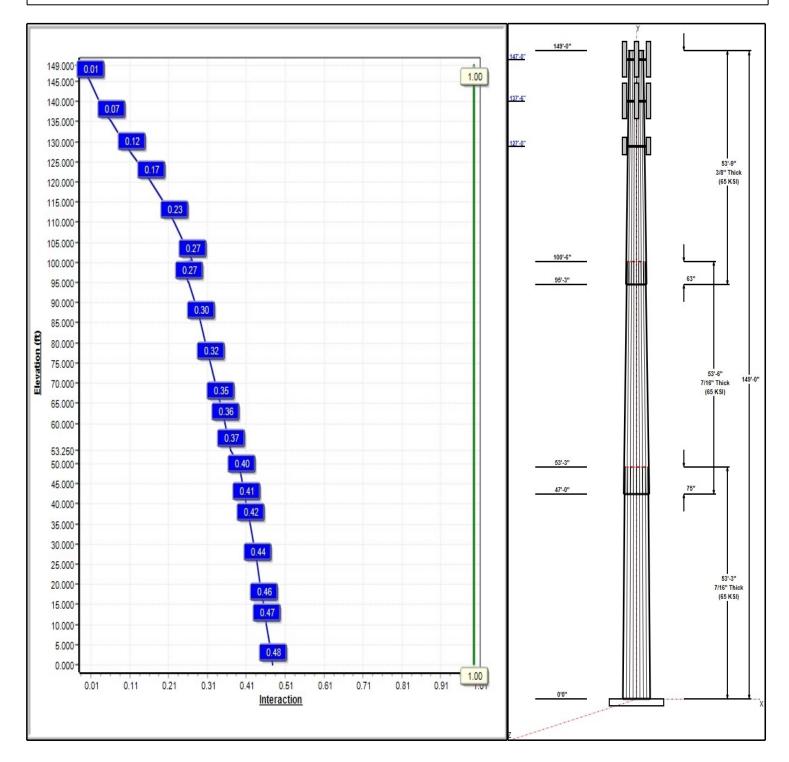
9/6/2019

Page: 1

Tower Engineering Solutions

Dead Load Factor: 1.20
Wind Load Factor: 1.60
Load Case: 1.2D + 1.6W 101 mph Wind

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### Structure: CT13614-A-SBA

Type: Tapered Base Shape: 18 Sided 9/6/2019

Site Name: Knowlton **Taper:** 0.24419

Height: 149.00 (ft) **Base Elev:** 0.00 (ft)

1.2D + 1.0E

0.9D + 1.0E

Page: 2



			Shaft	Proper	ties					Y	7
	Length	Тор	Bottom	Thick	Joint		Grade		149'-0"		Ļ
Seq	(ft)	(in)	(in)	(in)	Туре	Taper	(ksi)	147'-0		1111111111	I
1	53.25	47.46	60.46			0.24419	65				
2	53.50	36.79	49.86	0.438	Slip	0.24419	65	137'-6'			
3	53.75	25.70	38.83	0.375	Slip	0.24419	65	137 -0	-		
		Die	croto	Appurte		2					
Attach	Force		oci ete i	<u> Аррине</u>	ilalice.	<b>.</b>		127'-0'	"		
Elev (ft			Descr	rintion		Carrier				7	53'-9"
147.00				18-209014-	C-A20	T-Mobile					3/8" Thic
147.00				AARR24_4							(65 KSI)
147.00			Low P			T-Mobile					
147.00				245 (kicker	· kit)	T-Mobile					
147.00			HRK1		,	T-Mobile					1
147.00			KRY 1			T-Mobile					, I
147.00			3 4449			T-Mobile			100'-6"		1
147.00			782 10	0662		T-Mobile				MITTE	<u> </u>
137.50						AT&T			95'-3"		63"
137.50				,o CD-17-65-(	OUT DET	AT&T					Ţ [
137.50			AIVI-X-		JUI-KEI	AT&T					
137.50			LGP21			AT&T					
137.50			RRUS			AT&T					
137.50				8-60-8-18F	-n2	AT&T					
137.50				rofile Platfo		AT&T					
127.00				0080/4CF		Verizon					53'-6" 7/16" Thick
127.00				85080/461 85080/8CF		Verizon					(65 KSI)
127.00			Low P			Verizon					
				ppurter	12000						
Elev	Elev	LI	ileai A	ppurter	iances			-			9
From (fl		Place	ment D	escription		Carrier			53'-3"		1 1
0.00	147.00	Insi		5/8" Coax		T-Mobile				пппп	1
0.00	147.00	Insi	de 15	5/8" Fiber		T-Mobile			47'-0"		75"
0.00	137.50	Insi	de 15	5/8" Coax		AT&T				111111111	f
0.00	137.50	Insi	de 3/4	I" DC		AT&T					'
0.00	137.50	Insi	de 7/1	16" Fiber		AT&T					
0.00	127.00	Insi	de 15	5/8" Coax		Verizon					
			And	hor Bo	lts						
			Grade								53'-3" 7/16" Thic
Qty 5	Specification 2.25" 18J		(ksi) 75.0	Arrange Clus				-			(65 KSI)
20	2.23 103										
				se Plate	<del>-</del>			_			
Thickne (in)	ess Spe	cificatio (in)	ns (	Grade (ksi)	Geomet	rv					
3.0000	)	68.0		60.0	Clipped			-			
2.0000			D.						0'0"		
			K	eactions Mom		hear Ax	xial	4	and the second contract of the second contrac		
Load Ca	ase			(FT-K			ips)	7	Company of the Compan		
	.6W 101 mp	h Wind		3254		1.1 53					
	.6W 101 mp			3230		1.0 39					
	.0Di + 1.0Wi		Wind	933		3.9 83					
1.2D + 1				237		2.0 53					

53.0

39.8

237.0

235.1

2.0

2.0

### Structure: CT13614-A-SBA

Type: Tapered Base Shape: 18 Sided 9/6/2019

Site Name: Knowlton **Taper:** 0.24419

149.00 (ft) Height: 0.00 (ft) Base Elev:

Page: 3



1.0D + 1.0W 60 mph Wind

714.5

6.8

44.2

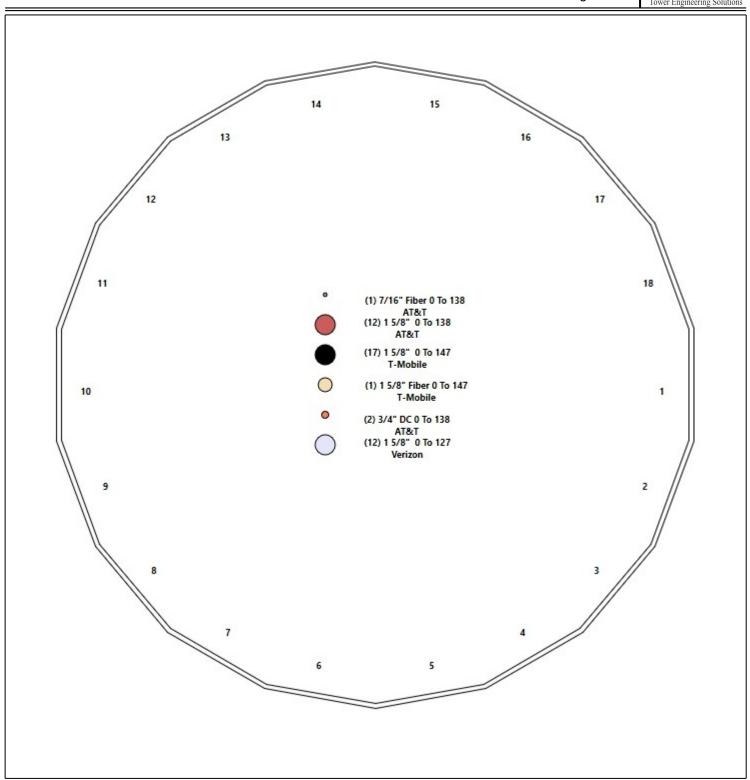
### Structure: CT13614-A-SBA - Coax Line Placement

Type: Monopole 9/6/2019

Site Name: Knowlton
Height: 149.00 (ft)

Tower Engineering Solutions

Page: 4



### **Shaft Properties**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: II Page: 5



Sec. No.	Shape	Length (ft)	Thick (in)	Fy (ksi)	Joint Type	Overlap (in)	Weight (lb)					
1	18	53.250	0.4375	65		0.00	13,466					
2	18	53.500	0.4375	65	Slip	75.00	10,842					
3	18	53.750	0.3750	65	Slip	63.00	6,942					
					Total Sha	aft Weight:	31 249	•				

			Bo	ottom										
Sec. No.	Dia (in)	Elev (ft)	Area (sqin)	lx (in^4)	W/t Ratio	D/t Ratio	Dia (in)	Elev (ft)	Area (sqin)	lx (in^4)	W/t Ratio	D/t Ratio	Taper	
1	60.46	0.00	83.35	37937.15	22.96	138.19	47.46	53.25	65.29	18236.7	17.72	108.4	0.244195	
2	49.86	47.00	68.62	21175.81	18.68	113.96	36.79	100.50	50.48	8430.41	13.42	84.10	0.244195	
3	38.83	95.25	45.76	8548.31	16.85	103.53	25.70	149.00	30.14	2442.44	10.67	68.53	0.244195	

#### **Load Summary**

**Structure**: CT13614-A-SBA **Code**: EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: II Page: 6



## **Discrete Appurtenances**

			1	No Ice			Ice			
No.	Elev (ft) Description	Qty	Weight (lb)	CaAa (sf)	CaAa Factor	Weight (lb)	CaAa (sf)	CaAa Factor	Hor. Ecc. (ft)	Vert Ecc (ft)
1	147.00 APXV18-209014-C-A20	6	18.70	3.53	0.96	109.79	5.943	0.96	0.00	0.00
2	147.00 APXVAARR24_43-U-NA	.20 3	128.00	20.24	0.70	707.71	22.800	0.70	0.00	0.00
3	147.00 Low Profile Platform-Roเ	und 1	1500.00	22.00	1.00	3241.69	45.501	1.00	0.00	0.00
4	147.00 PRK-1245 (kicker kit)	1	464.91	9.50	1.00	896.77	22.737	1.00	0.00	0.00
5	147.00 HRK12-HD	1	406.61	9.75	1.00	1048.70	22.430	1.00	0.00	0.00
6	147.00 KRY 112 89/4	6	15.40	0.65	0.67	38.85	1.464	0.67	0.00	0.00
7	147.00 4449	3	70.00	1.65	0.67	168.74	2.392	0.67	0.00	0.00
8	147.00 782 10662	3	2.60	0.28	0.67	11.29	0.814	0.67	0.00	0.00
9	137.50 7770.00	6	35.00	5.50	0.73	226.95	6.938	0.73	0.00	0.00
10	137.50 AM-X-CD-17-65-00T-RE	T (96") 3	59.50	11.31	0.80	407.26	13.521	0.80	0.00	0.00
11	137.50 LGP21401	6	14.10	1.29	0.67	47.15	2.395	0.67	0.00	0.00
12	137.50 LGP21903	6	5.50	0.27	0.67	16.64	0.796	0.67	0.00	0.00
13	137.50 RRUS-11	6	51.00	2.52	0.67	146.53	3.357	0.67	0.00	0.00
14	137.50 DC2-48-60-8-18F-02	1	14.50	2.92	1.00	98.08	4.526	1.00	0.00	0.00
15	137.50 Low Profile Platform-flat	1	1200.00	25.00	1.00	2584.08	52.681	1.00	0.00	0.00
16	127.00 LPA-80080/4CF	6	12.00	2.61	1.70	201.54	3.756	1.70	0.00	0.00
17	127.00 LPA-185080/8CF	6	7.00	2.09	1.22	122.98	3.215	1.22	0.00	0.00
18	127.00 Low Profile Platform-Rou	ınd 1	1500.00	22.00	1.00	3216.41	45.160	1.00	0.00	0.00

Totals: 66 6,818.52 20,433.26

### **Linear Appurtenances**

<b>Bottom</b>	Тор				
Elev.	Elev.		Exposed		
(ft)	(ft)	Description	Width	Exposed	
0.00	147.00	(17) 1 5/8" Coax	0.00	Inside	
0.00	147.00	(1) 1 5/8" Fiber	0.00	Inside	
0.00	137.50	(12) 1 5/8" Coax	0.00	Inside	
0.00	137.50	(2) 3/4" DC	0.00	Inside	
0.00	137.50	(1) 7/16" Fiber	0.00	Inside	
0.00	127.00	(12) 1 5/8" Coax	0.00	Inside	

### **Shaft Section Properties**

9/6/2019 **Structure**: CT13614-A-SBA Code: EIA/TIA-222-G

Site Name: Knowlton Exposure: С Height: 149.00 (ft) Crest Height: 0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Topography: 1 Gh: 1.1 Struct Class: || Page: 7



Increment Length: 5 (ft)

Elev		Thick	Dia	Area	lx	W/t	D/t	Fpy	S	Weight
<u>(ft)</u>	Description	(in)	(in)	(in^2)	(in^4)	Ratio	Ratio		(in^3)	(lb)
0.00		0.4375	60.460	83.346	37937.1	22.96	138.19		1235.	0.0
5.00		0.4375	59.239	81.650	35668.8	22.46	135.40	75.0	1185.	1403.6
10.00		0.4375	58.018	79.955	33492.7	21.97	132.61		1137.	1374.8
15.00		0.4375	56.797	78.260	31406.9	21.48	129.82	76.1	1089.	1345.9
20.00		0.4375	55.576	76.564		20.99	127.03		1042.	1317.1
25.00		0.4375	54.355		27498.9	20.50	124.24		996.5	1288.2
30.00		0.4375	53.134		25672.7	20.00	121.45		951.7	1259.4
35.00		0.4375	51.913		23929.2	19.51	118.66		907.9	1230.5
40.00		0.4375	50.692		22266.5	19.02	115.87		865.2	1201.7
45.00		0.4375	49.471	68.087		18.53	113.08		823.4	1172.8
47.00	Bot - Section 2	0.4375	48.983	67.409	20070.8	18.33	111.96		807.1	461.1
50.00		0.4375	48.250	66.392	19175.8	18.04	110.29		782.8	1378.3
53.25	Top - Section 1	0.4375	48.332		19273.9	18.07	110.47	0.0	0.0	1469.7
55.00		0.4375	47.904		18762.6	17.90	109.50		771.4	394.3
60.00		0.4375	46.683	64.216	17351.6	17.40	106.70		732.1	1107.0
65.00		0.4375	45.462	62.520	16013.2	16.91	103.91	81.5	693.8	1078.1
70.00		0.4375	44.241	60.825	14745.5	16.42	101.12		656.5	1049.3
75.00		0.4375	43.020	59.130	13546.5	15.93	98.33	82.5	620.2	1020.4
80.00		0.4375	41.799	57.434	12414.4	15.44	95.54	82.5	585.0	991.6
85.00		0.4375	40.578	55.739	11347.1	14.94	92.75	82.5	550.8	962.8
90.00		0.4375	39.357	54.043	10342.9	14.45	89.96	82.5	517.6	933.9
95.00		0.4375	38.137	52.348	9399.7	13.96	87.17	82.5	485.5	905.1
95.25	Bot - Section 3	0.4375	38.075	52.263	9354.1	13.94	87.03	82.5	483.9	44.5
100.00		0.4375	36.916	50.652	8515.6	13.47	84.38	82.5	454.3	1560.3
100.50	Top - Section 2	0.3750	37.543	44.238	7721.4	16.24	100.12	0.0	0.0	161.4
105.00		0.3750	36.445	42.930	7056.6	15.73	97.19	82.5	381.4	667.4
110.00		0.3750	35.224	41.477	6364.0	15.15	93.93	82.5	355.9	718.0
115.00		0.3750	34.003	40.024	5718.3	14.58	90.67	82.5	331.2	693.3
120.00		0.3750	32.782	38.571	5117.7	14.00	87.42	82.5	307.5	668.6
125.00		0.3750	31.561	37.117	4560.8	13.43	84.16	82.5	284.6	643.9
127.00		0.3750	31.072	36.536	4349.9	13.20	82.86	82.5	275.7	250.6
130.00		0.3750	30.340	35.664	4045.8	12.86	80.91		262.6	368.5
135.00		0.3750	29.119	34.211	3571.1	12.28	77.65		241.6	594.4
137.50		0.3750	28.508	33.484	3348.4	11.99	76.02		231.3	287.9
140.00		0.3750	27.898	32.758	3135.1	11.71	74.39		221.3	281.8
145.00		0.3750	26.677	31.305	2736.1	11.13	71.14		202.0	545.0
147.00		0.3750	26.188	30.723	2586.5	10.90	69.84		194.5	211.1
149.00		0.3750	25.700	30.142	2442.4	10.67	68.53		187.2	207.1
1-40.00		0.07.00	20.700	50.172	2772.4	10.07	00.00	02.0	101.2	

31249.4

### Wind Loading - Shaft

**Structure**: CT13614-A-SBA **Code**: EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



21

Page: 8 Tow

Load Case: 1.2D + 1.6W 101 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.60



Iterations

Elev (ft) Descripti	ion Kzt	Kz	qz (psf)	qzGh (psf)	C (mph-ft)	Cf	lce Thick (in)	Tributary (ft)	Aa (sf)	CfAa (sf)	Wind Force X (lb)	Dead Load Ice (lb)	Tot Dead Load (lb)
0.00	1.00	0.85	21.088	23.20	476.39	0.650	0.000	0.00	0.000	0.00	0.0	0.0	0.0
5.00	1.00	0.85	21.088	23.20	466.77	0.650	0.000	5.00	25.322	16.46	610.9	0.0	1684.3
10.00	1.00	0.85	21.088	23.20	457.15	0.650	0.000	5.00	24.805	16.12	598.4	0.0	1649.7
15.00	1.00	0.85	21.088	23.20	447.53	0.650	0.000	5.00	24.289	15.79	585.9	0.0	1615.1
20.00	1.00	0.90	22.375	24.61	451.08	0.650	0.000	5.00	23.772	15.45	608.5	0.0	1580.5
25.00	1.00	0.95	23.451	25.80	451.65	0.650	0.000	5.00	23.256	15.12	623.9	0.0	1545.9
30.00	1.00	0.98	24.369	26.81	450.06	0.650	0.000		22.739	14.78	633.9	0.0	1511.3
35.00	1.00	1.01	25.172	27.69	446.92	0.650	0.000	5.00	22.222	14.44	639.9	0.0	1476.6
40.00	1.00	1.04	25.890	28.48	442.58	0.650	0.000	5.00	21.706	14.11	642.9	0.0	1442.0
45.00	1.00	1.07	26.540	29.19	437.31	0.650	0.000	5.00	21.189	13.77	643.3	0.0	1407.4
47.00 Bot - Section 2	1.00	1.08	26.784	29.46	434.98	0.650	0.000	2.00	8.331	5.42	255.3	0.0	553.3
50.00	1.00	1.09	27.135	29.85	431.27	0.650	0.000	3.00	12.564	8.17	390.0	0.0	1653.9
53.25 Top - Section 1		1.11	27.497	30.25	427.00	0.650	0.000	3.25	13.401	8.71	421.6	0.0	1763.6
55.00	1.00	1.12	27.685	30.45	432.50	0.650	0.000	1.75	7.125	4.63	225.7	0.0	473.1
60.00	1.00	1.14	28.197	31.02	425.35	0.650	0.000	5.00	20.010	13.01	645.5	0.0	1328.4
65.00	1.00	1.16	28.676	31.54	417.73	0.650	0.000	5.00	19.493	12.67	639.5	0.0	1293.8
70.00	1.00	1.17	29.127	32.04	409.70	0.650	0.000	5.00	18.977	12.33	632.3	0.0	1259.2
75.00	1.00	1.19	29.553	32.51	401.29	0.650	0.000	5.00	18.460	12.00	624.1	0.0	1224.5
80.00	1.00	1.21	29.958	32.95	392.56	0.650	0.000	5.00	17.943	11.66	614.9	0.0	1189.9
85.00	1.00	1.22	30.342	33.38	383.54	0.650	0.000	5.00	17.427	11.33	604.9	0.0	1155.3
90.00	1.00	1.24	30.710	33.78	374.24	0.650	0.000	5.00	16.910	10.99	594.1	0.0	1120.7
95.00	1.00	1.25	31.061	34.17	364.70	0.650	0.000	5.00	16.394	10.66	582.5	0.0	1086.1
95.25 Bot - Section 3	1.00	1.25	31.078	34.19	364.22	0.650	0.000	0.25	0.806	0.52	28.7	0.0	53.4
00.00	1.00	1.27	31.399	34.54	354.94	0.650	0.000	4.75	15.372	9.99	552.2	0.0	1872.3
00.50 Top - Section 2		1.27	31.432	34.57	353.95	0.650	0.000	0.50	1.591	1.03	57.2	0.0	193.7
05.00	1.00	1.28	31.723	34.89	352.21	0.650	0.000	4.50	14.087	9.16	511.2	0.0	800.9
110.00	1.00		32.035	35.24	342.08	0.650	0.000	5.00	15.161	9.85	555.6	0.0	861.7
115.00	1.00		32.336	35.57	331.77	0.650	0.000		14.645	9.52	541.7	0.0	832.0
20.00	1.00		32.627	35.89	321.30	0.650	0.000		14.128	9.18	527.3	0.0	802.3
25.00	1.00		32.909	36.20	310.66	0.650	0.000		13.611	8.85	512.4	0.0	772.6
27.00 Appurtenance(	,		33.019	36.32	306.37	0.650	0.000	2.00	5.300	3.44	200.2	0.0	300.8
30.00	1.00		33.182	36.50	299.88	0.650	0.000	3.00	7.795	5.07	295.9	0.0	442.2
35.00	1.00		33.446	36.79	288.96	0.650	0.000		12.578	8.18	481.3	0.0	713.3
37.50 Appurtenance(	,		33.576	36.93	283.44	0.650	0.000	2.50	6.095	3.96	234.1	0.0	345.5
40.00	1.00		33.703	37.07	277.90	0.650	0.000	2.50	5.966	3.88	230.0	0.0	338.1
45.00	1.00		33.953	37.35	266.72	0.650	0.000		11.545	7.50	448.4	0.0	654.0
147.00 Appurtenance(	,		34.051	37.46	262.22	0.650	0.000	2.00	4.473	2.91	174.3	0.0	253.3
49.00	1.00	1.38	34.148	37.56	257.69	0.650	0.000	2.00	4.391	2.85	171.5	0.0	248.5
							Totals:	149.00			17,340.3		37,499.3

#### **Discrete Appurtenance Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



**Load Case:** 1.2D + 1.6W 101 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.60



Page: 9

Iterations 21

	Elev			~-Ch	Orient		Total	Dead	Horiz	Vert	Wind FX	Mom Y	Mom
No.	Elev (ft) Description	Qty	qz (psf)	qzGh (psf)	Factor x Ka	Ka	CaAa (sf)	Load (lb)	Ecc (ft)	Ecc (ft)	(lb)	t (lb-ft)	Z (lb-ft)
1	147.00 4449	3	34.051	37.456	0.50	0.75	2.49	252.00	0.000	0.000	149.07	0.00	0.00
2	147.00 KRY 112 89/4	6	34.051	37.456	0.50	0.75	1.96	110.88	0.000	0.000	117.45	0.00	0.00
3	147.00 HRK12-HD	1	34.051	37.456	1.00	1.00	9.75	487.93	0.000	0.000	584.32	0.00	0.00
4	147.00 PRK-1245 (kicker kit)	1	34.051	37.456	1.00	1.00	9.50	557.89	0.000	0.000	569.34	0.00	0.00
5	147.00 Low Profile	1	34.051	37.456	1.00	1.00	22.00	1800.00	0.000	0.000	1318.47	0.00	0.00
6	147.00 APXVAARR24 43-U-NA	2 3	34.051	37.456	0.52	0.75	31.88	460.80	0.000	0.000	1910.46	0.00	0.00
7	147.00 APXV18-209014-C-A20	6	34.051	37.456	0.72	0.75	15.25	134.64	0.000	0.000	913.91	0.00	0.00
8	147.00 782 10662	3	34.051	37.456	0.50	0.75	0.42	9.36	0.000	0.000	25.30	0.00	0.00
9	137.50 Low Profile Platform-flat	1	33.576	36.933	1.00	1.00	25.00	1440.00	0.000	0.000	1477.33	0.00	0.00
10	137.50 DC2-48-60-8-18F-02	1	33.576	36.933	1.00	1.00	2.92	17.40	0.000	0.000	172.55	0.00	0.00
11	137.50 RRUS-11	6	33.576	36.933	0.54	0.80	8.10	367.20	0.000	0.000	478.91	0.00	0.00
12	137.50 LGP21903	6	33.576	36.933	0.54	0.80	0.87	39.60	0.000	0.000	51.31	0.00	0.00
13	137.50 LGP21401	6	33.576	36.933	0.54	0.80	4.15	101.52	0.000	0.000	245.16	0.00	0.00
14	137.50 AM-X-CD-17-65-00T-RE	т 3	33.576	36.933	0.64	0.80	21.72	214.20	0.000	0.000	1283.22	0.00	0.00
15	137.50 7770.00	6	33.576	36.933	0.58	0.80	19.27	252.00	0.000	0.000	1138.85	0.00	0.00
16	127.00 Low Profile	1	33.019	36.321	1.00	1.00	22.00	1800.00	0.000	0.000	1278.49	0.00	0.00
17	127.00 LPA-185080/8CF	6	33.019	36.321	0.98	0.80	12.24	50.40	0.000	0.000	711.25	0.00	0.00
18	127.00 LPA-80080/4CF	6	33.019	36.321	1.36	0.80	21.30	86.40	0.000	0.000	1237.67	0.00	0.00

Totals: 8,182.22 13,663.06

# **Total Applied Force Summary**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Iterations

Page: 10

21

**Load Case:** 1.2D + 1.6W 101 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.60

Elev	Decembrish	Lateral FX (-) (lb)	Axial FY (-)	Torsion MY	Moment MZ	
(ft)	Description		(lb)	(lb-ft)	(lb-ft)	
0.00		0.00	0.00	0.00	0.00	
5.00		610.87	1949.17	0.00	0.00	
10.00		598.41	1914.56	0.00	0.00	
15.00		585.95	1879.95	0.00	0.00	
20.00		608.49	1845.33	0.00	0.00	
25.00		623.90	1810.72	0.00	0.00	
30.00		633.91	1776.10	0.00	0.00	
35.00		639.94	1741.49	0.00	0.00	
40.00		642.89	1706.87	0.00	0.00	
45.00		643.35	1672.26	0.00	0.00	
47.00		255.27	659.21	0.00	0.00	
50.00		390.01	1812.84	0.00	0.00	
53.25		421.55	1935.79	0.00	0.00	
55.00		225.68	565.80	0.00	0.00	
60.00		645.46	1593.22	0.00	0.00	
65.00		639.49	1558.61	0.00	0.00	
70.00		632.33	1523.99	0.00	0.00	
75.00		624.11	1489.38	0.00	0.00	
80.00		614.95	1454.76	0.00	0.00	
85.00		604.91	1420.15	0.00	0.00	
90.00		594.09	1385.53	0.00	0.00	
95.00		582.53	1350.92	0.00	0.00	
95.25		28.66	66.64	0.00	0.00	
100.00		552.17	2123.91	0.00	0.00	
100.50		57.21	220.19	0.00	0.00	
105.00		511.22	1039.22	0.00	0.00	
110.00		555.63	1126.50	0.00	0.00	
115.00		541.74	1096.83	0.00	0.00	
120.00		527.33	1067.16	0.00	0.00	
125.00		512.44	1037.49	0.00	0.00	
127.00	(13) attachments	3427.61	2343.49	0.00	0.00	
130.00	(12) 2112331110	295.89	556.20	0.00	0.00	
135.00		481.28	903.27	0.00	0.00	
137.50	(29) attachments	5081.46	2872.43	0.00	0.00	
140.00	(20) attachments	230.04	392.65	0.00	0.00	
145.00		448.44	763.05	0.00	0.00	
147.00	(24) attachments	5762.57	4110.42	0.00	0.00	
149.00	(24) attachments	171.53	248.53	0.00	0.00	
143.00						
	Totals:	31,003.32	53,014.61	0.00	0.00	

#### **Calculated Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



21

Load Case: 1.2D + 1.6W 101 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.60



Page: 11

Seg Elev	Pu FY (-)	Vu FX (-)	Tu MY (-)	Mu MZ	Mu MX	Resultant Moment	phi Pn	phi Vn	phi Tn	phi Mn	Total Deflect	Rotation Sway	Rotation Twist	Stress
(ft)	(kips)	(kips)	(ft-kips)	(ft-kips)	(ft-kips)	(ft-kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(in)	(deg)	(deg)	Ratio
0.00	-52.98	-31.06	0.00	-3254.1	0.00	3254.12	5580.79	2790.40	13771.9	6896.19	0.00	0.000	0.000	0.481
5.00	-50.97	-30.55	0.00	-3098.8	0.00	3098.83	5509.80	2754.90	13318.1	6668.96	0.06	-0.119	0.000	0.474
10.00	-48.99	-30.05	0.00	-2946.0	0.00	2946.07	5437.04	2718.52	12867.3	6443.24	0.25	-0.240	0.000	0.466
15.00	-47.05	-29.56	0.00	-2795.8	0.00	2795.81	5362.51	2681.26	12419.8	6219.15	0.57	-0.362	0.000	0.458
20.00	-45.15	-29.03	0.00	-2648.0	0.00	2648.03	5286.22	2643.11	11975.8	5996.84	1.02	-0.485	0.000	0.450
25.00	-43.28	-28.49	0.00	-2502.8	0.00	2502.87	5208.16	2604.08	11535.7	5776.42	1.59	-0.609	0.000	0.442
30.00	-41.45	-27.92	0.00	-2360.4	0.00	2360.44	5128.34	2564.17	11099.6	5558.05	2.30	-0.735	0.000	0.433
35.00	-39.66	-27.35	0.00	-2220.8	0.00	2220.83	5046.75	2523.38	10667.8	5341.85	3.14	-0.862	0.000	0.424
40.00	-37.90	-26.76	0.00	-2084.1	0.00	2084.10	4963.39	2481.70	10240.6	5127.96	4.11	-0.990	0.000	0.414
45.00	-36.20	-26.14	0.00	-1950.2	0.00	1950.29	4878.27	2439.14	9818.41	4916.50	5.22	-1.119	0.000	0.404
47.00	-35.52	-25.92	0.00	-1898.0	0.00	1898.00	4843.73	2421.86	9650.93	4832.64	5.70	-1.172	0.000	0.400
50.00	-33.68	-25.54	0.00	-1820.2	0.00	1820.25	4791.38	2395.69	9401.28	4707.63	6.46	-1.251	0.000	0.394
53.25	-31.72	-25.11	0.00	-1737.2	0.00	1737.26	4797.23	2398.61	9428.91	4721.46	7.34	-1.337	0.000	0.375
55.00	-31.13	-24.92	0.00	-1693.3	0.00	1693.32	4766.44	2383.22	9284.05	4648.93	7.84	-1.384	0.000	0.371
60.00	-29.50	-24.29	0.00	-1568.7	0.00	1568.74	4677.28	2338.64	8873.91	4443.55	9.35	-1.507	0.000	0.359
65.00	-27.90	-23.67	0.00	-1447.2	0.00	1447.27	4586.36	2293.18	8469.53	4241.06	11.00	-1.631	0.000	0.347
70.00	-26.35	-23.05	0.00	-1328.9	0.00	1328.91	4493.67	2246.84	8071.16	4041.58	12.77	-1.754	0.000	0.335
75.00	-24.83	-22.43	0.00	-1213.6	0.00	1213.65	4393.03	2196.52	7668.29	3839.84	14.68	-1.877	0.000	0.322
80.00	-23.36	-21.82	0.00	-1101.4	0.00	1101.48	4267.07	2133.53	7232.68	3621.71	16.71	-1.998	0.000	0.310
85.00	-21.91	-21.21	0.00	-992.38	0.00	992.38	4141.11	2070.55	6809.81	3409.96	18.87	-2.118	0.000	0.296
90.00	-20.51	-20.61	0.00	-886.33	0.00	886.33	4015.15	2007.57	6399.67	3204.59	21.15	-2.236	0.000	0.282
95.00	-19.16	-19.99	0.00	-783.31	0.00	783.31	3889.19	1944.59	6002.28	3005.60	23.55	-2.351	0.000	0.266
95.25	-19.08	-19.98	0.00	-778.31	0.00	778.31	3882.89	1941.44	5982.74	2995.82	23.68	-2.357	0.000	0.265
100.00	-16.96	-19.35	0.00	-683.42	0.00	683.42	3763.23	1881.61	5617.62	2812.98	26.08	-2.462	0.000	0.248
100.50	-16.72	-19.30	0.00	-673.75	0.00	673.75	3276.58	1638.29	4993.13	2500.28	26.33	-2.474	0.000	0.275
105.00	-15.67	-18.77	0.00	-586.89	0.00	586.89	3189.50	1594.75	4715.31	2361.16	28.71	-2.569	0.000	0.254
110.00	-14.54	-18.19	0.00	-493.02	0.00	493.02	3081.54	1540.77	4399.90	2203.22	31.46	-2.678	0.000	0.229
115.00	-13.44	-17.62	0.00	-402.06	0.00	402.06	2973.57	1486.79	4095.40	2050.74	34.32	-2.778	0.000	0.201
120.00	-12.38	-17.06	0.00	-313.95	0.00	313.95	2865.60	1432.80	3801.82	1903.74	37.28	-2.867	0.000	0.169
125.00	-11.35	-16.51	0.00	-228.65	0.00	228.65	2757.64	1378.82	3519.16	1762.20	40.33	-2.943	0.000	0.134
127.00	-9.18	-12.97	0.00	-195.63	0.00	195.63	2714.45	1357.23	3409.16	1707.11	41.56	-2.969	0.000	0.118
130.00	-8.63	-12.65	0.00	-156.73	0.00	156.73	2649.67	1324.84	3247.42	1626.12	43.44	-3.004	0.000	0.100
135.00	-7.75	-12.12	0.00	-93.49	0.00	93.49	2541.71	1270.85	2986.60	1495.52	46.61	-3.048	0.000	0.066
137.50	-5.15	-6.90	0.00	-63.18	0.00	63.18	2487.72	1243.86	2860.28	1432.27	48.21	-3.063	0.000	0.046
140.00	-4.77	-6.65	0.00	-45.93	0.00	45.93	2433.74	1216.87	2736.70	1370.38	49.82	-3.075	0.000	0.036
145.00	-4.03	-6.16	0.00	-12.69	0.00	12.69	2325.77	1162.89	2497.71	1250.71	53.05	-3.088	0.000	0.012
147.00	-0.24	-0.18	0.00	-0.37	0.00	0.37	2282.59	1141.29	2405.17	1204.38	54.34	-3.090	0.000	0.000
149.00	0.00	-0.17	0.00	0.00	0.00	0.00	2239.40	1119.70	2314.38	1158.91	55.63	-3.090	0.000	0.000

#### Wind Loading - Shaft

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



21

**Load Case:** 0.9D + 1.6W 101 mph Wind

Dead Load Factor 0.90 Wind Load Factor 1.60



Iterations

Elev (ft) De	escription	Kzt	Kz	qz (psf)	qzGh (psf)	C (mph-ft)	Cf	lce Thick (in)	Tributary (ft)	Aa (sf)	CfAa (sf)	Wind Force X (lb)	Dead Load Ice (Ib)	Tot Dead Load (lb)
0.00		1.00	0.85	21.088	23.20	476.39	0.650	0.000	0.00	0.000	0.00	0.0	0.0	0.0
5.00		1.00	0.85	21.088	23.20	466.77	0.650	0.000	5.00	25.322	16.46	610.9	0.0	1263.3
10.00		1.00	0.85	21.088	23.20	457.15	0.650	0.000	5.00	24.805	16.12	598.4	0.0	1237.3
15.00		1.00	0.85	21.088	23.20	447.53	0.650	0.000	5.00	24.289	15.79	585.9	0.0	1211.3
20.00		1.00	0.90	22.375	24.61	451.08	0.650	0.000	5.00	23.772	15.45	608.5	0.0	1185.4
25.00		1.00	0.95	23.451	25.80	451.65	0.650	0.000	5.00	23.256	15.12	623.9	0.0	1159.4
30.00		1.00	0.98	24.369	26.81	450.06	0.650	0.000	5.00	22.739	14.78	633.9	0.0	1133.4
35.00		1.00	1.01	25.172	27.69	446.92	0.650	0.000	5.00	22.222	14.44	639.9	0.0	1107.5
40.00		1.00	1.04	25.890	28.48	442.58	0.650	0.000	5.00	21.706	14.11	642.9	0.0	1081.5
45.00		1.00	1.07	26.540	29.19	437.31	0.650	0.000	5.00	21.189	13.77	643.3	0.0	1055.6
47.00 Bot - Se	ection 2	1.00	1.08	26.784	29.46	434.98	0.650	0.000	2.00	8.331	5.42	255.3	0.0	415.0
50.00		1.00	1.09	27.135	29.85	431.27	0.650	0.000	3.00	12.564	8.17	390.0	0.0	1240.5
53.25 Top - Se	ection 1	1.00	1.11	27.497	30.25	427.00	0.650	0.000	3.25	13.401	8.71	421.6	0.0	1322.7
55.00		1.00	1.12	27.685	30.45	432.50	0.650	0.000	1.75	7.125	4.63	225.7	0.0	354.8
60.00		1.00	1.14	28.197	31.02	425.35	0.650	0.000	5.00	20.010	13.01	645.5	0.0	996.3
65.00		1.00	1.16	28.676	31.54	417.73	0.650	0.000	5.00	19.493	12.67	639.5	0.0	970.3
70.00		1.00	1.17	29.127	32.04	409.70	0.650	0.000	5.00	18.977	12.33	632.3	0.0	944.4
75.00		1.00	1.19	29.553	32.51	401.29	0.650	0.000	5.00	18.460	12.00	624.1	0.0	918.4
80.00		1.00	1.21	29.958	32.95	392.56	0.650	0.000	5.00	17.943	11.66	614.9	0.0	892.4
85.00		1.00	1.22	30.342	33.38	383.54	0.650	0.000	5.00	17.427	11.33	604.9	0.0	866.5
90.00		1.00	1.24	30.710	33.78	374.24	0.650	0.000	5.00	16.910	10.99	594.1	0.0	840.5
95.00		1.00	1.25	31.061	34.17	364.70	0.650	0.000	5.00	16.394	10.66	582.5	0.0	814.6
95.25 Bot - Se	ection 3	1.00	1.25	31.078	34.19	364.22	0.650	0.000	0.25	0.806	0.52	28.7	0.0	40.0
00.00		1.00	1.27	31.399	34.54	354.94	0.650	0.000	4.75	15.372	9.99	552.2	0.0	1404.2
00.50 Top - Se	ection 2	1.00	1.27	31.432	34.57	353.95	0.650	0.000	0.50	1.591	1.03	57.2	0.0	145.3
05.00		1.00	1.28	31.723	34.89	352.21	0.650	0.000	4.50	14.087	9.16	511.2	0.0	600.6
10.00		1.00	1.29	32.035	35.24	342.08	0.650	0.000	5.00	15.161	9.85	555.6	0.0	646.2
15.00		1.00	1.30	32.336	35.57	331.77	0.650	0.000	5.00	14.645	9.52	541.7	0.0	624.0
20.00		1.00	1.32	32.627	35.89	321.30	0.650	0.000	5.00	14.128	9.18	527.3	0.0	601.7
25.00		1.00	1.33	32.909	36.20	310.66	0.650	0.000	5.00	13.611	8.85	512.4	0.0	579.5
27.00 Appurte	nance(s)	1.00	1.33	33.019	36.32	306.37	0.650	0.000	2.00	5.300	3.44	200.2	0.0	225.6
30.00		1.00	1.34	33.182	36.50	299.88	0.650	0.000	3.00	7.795	5.07	295.9	0.0	331.7
35.00		1.00	1.35	33.446	36.79	288.96	0.650	0.000	5.00	12.578	8.18	481.3	0.0	535.0
37.50 Appurte	nance(s)	1.00		33.576	36.93	283.44	0.650	0.000	2.50	6.095	3.96	234.1	0.0	259.1
40.00		1.00		33.703	37.07	277.90	0.650	0.000	2.50	5.966	3.88	230.0	0.0	253.6
45.00		1.00	1.37	33.953	37.35	266.72	0.650	0.000	5.00	11.545	7.50	448.4	0.0	490.5
47.00 Appurte	nance(s)	1.00	1.37	34.051	37.46	262.22	0.650	0.000	2.00	4.473	2.91	174.3	0.0	190.0
49.00		1.00	1.38	34.148	37.56	257.69	0.650	0.000	2.00	4.391	2.85	171.5	0.0	186.4
								Totals:	149.00	_		17,340.3	 !	28,124.5

#### **Discrete Appurtenance Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



**Load Case:** 0.9D + 1.6W 101 mph Wind

Dead Load Factor 0.90 Wind Load Factor 1.60



Page: 13

Iterations 21

	Elev			qz	qzGh	Orient Factor		Total CaAa	Dead Load	Horiz Ecc	Vert Ecc	Wind FX	Mom Y	Mom Z
No.	(ft)	Description	Qty	qz (psf)	(psf)	x Ka	Ka	(sf)	(lb)	(ft)	(ft)	(lb)	(lb-ft)	(lb-ft)
1	147.00 4	4449	3	34.051	37.456	0.50	0.75	2.49	189.00	0.000	0.000	149.07	0.00	0.00
2	147.00 H	KRY 112 89/4	6	34.051	37.456	0.50	0.75	1.96	83.16	0.000	0.000	117.45	0.00	0.00
3	147.00 l	HRK12-HD	1	34.051	37.456	1.00	1.00	9.75	365.95	0.000	0.000	584.32	0.00	0.00
4	147.00 F	PRK-1245 (kicker kit)	1	34.051	37.456	1.00	1.00	9.50	418.42	0.000	0.000	569.34	0.00	0.00
5	147.00 լ	Low Profile	1	34.051	37.456	1.00	1.00	22.00	1350.00	0.000	0.000	1318.47	0.00	0.00
6	147.00	APXVAARR24 43-U-NA2	3	34.051	37.456	0.52	0.75	31.88	345.60	0.000	0.000	1910.46	0.00	0.00
7	147.00 /	APXV18-209014-C-A20	6	34.051	37.456	0.72	0.75	15.25	100.98	0.000	0.000	913.91	0.00	0.00
8	147.00	782 10662	3	34.051	37.456	0.50	0.75	0.42	7.02	0.000	0.000	25.30	0.00	0.00
9	137.50 l	Low Profile Platform-flat	1	33.576	36.933	1.00	1.00	25.00	1080.00	0.000	0.000	1477.33	0.00	0.00
10	137.50 I	DC2-48-60-8-18F-02	1	33.576	36.933	1.00	1.00	2.92	13.05	0.000	0.000	172.55	0.00	0.00
11	137.50 F	RRUS-11	6	33.576	36.933	0.54	0.80	8.10	275.40	0.000	0.000	478.91	0.00	0.00
12	137.50 l	LGP21903	6	33.576	36.933	0.54	0.80	0.87	29.70	0.000	0.000	51.31	0.00	0.00
13	137.50 l	LGP21401	6	33.576	36.933	0.54	0.80	4.15	76.14	0.000	0.000	245.16	0.00	0.00
14	137.50	AM-X-CD-17-65-00T-RET	3	33.576	36.933	0.64	0.80	21.72	160.65	0.000	0.000	1283.22	0.00	0.00
15	137.50	7770.00	6	33.576	36.933	0.58	0.80	19.27	189.00	0.000	0.000	1138.85	0.00	0.00
16	127.00 լ	Low Profile	1	33.019	36.321	1.00	1.00	22.00	1350.00	0.000	0.000	1278.49	0.00	0.00
17	127.00 l	LPA-185080/8CF	6	33.019	36.321	0.98	0.80	12.24	37.80	0.000	0.000	711.25	0.00	0.00
18	127.00 L	LPA-80080/4CF	6	33.019	36.321	1.36	0.80	21.30	64.80	0.000	0.000	1237.67	0.00	0.00

Totals: 6,136.67 13,663.06

# **Total Applied Force Summary**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



**Load Case:** 0.9D + 1.6W 101 mph Wind

Dead Load Factor 0.90 Wind Load Factor 1.60



Page: 14

	Iterations	21
X		

Elev		Lateral FX (-)	Axial FY (-)	Torsion MY	Moment MZ
(ft)	Description	(lb)	(lb)	(lb-ft)	(lb-ft)
0.00		0.00	0.00	0.00	0.00
5.00		610.87	1461.88	0.00	0.00
10.00		598.41	1435.92	0.00	0.00
15.00		585.95	1409.96	0.00	0.00
20.00		608.49	1384.00	0.00	0.00
25.00		623.90	1358.04	0.00	0.00
30.00		633.91	1332.08	0.00	0.00
35.00		639.94	1306.12	0.00	0.00
40.00		642.89	1280.15	0.00	0.00
45.00		643.35	1254.19	0.00	0.00
47.00		255.27	494.41	0.00	0.00
50.00		390.01	1359.63	0.00	0.00
53.25		421.55	1451.84	0.00	0.00
55.00		225.68	424.35	0.00	0.00
60.00		645.46	1194.91	0.00	0.00
65.00		639.49	1168.95	0.00	0.00
70.00		632.33	1142.99	0.00	0.00
75.00		624.11	1117.03	0.00	0.00
80.00		614.95	1091.07	0.00	0.00
85.00		604.91	1065.11	0.00	0.00
90.00		594.09	1039.15	0.00	0.00
95.00		582.53	1013.19	0.00	0.00
95.25		28.66	49.98	0.00	0.00
100.00		552.17	1592.93	0.00	0.00
100.50		57.21	165.15	0.00	0.00
105.00		511.22	779.41	0.00	0.00
110.00		555.63	844.87	0.00	0.00
115.00		541.74	822.62	0.00	0.00
120.00		527.33	800.37	0.00	0.00
125.00		512.44	778.12	0.00	0.00
123.00	(12) attachments	3427.61			0.00
	(13) attachments		1757.62	0.00	
130.00		295.89	417.15	0.00	0.00
135.00	(00) - #   + -	481.28	677.45	0.00	0.00
137.50	(29) attachments	5081.46	2154.32	0.00	0.00
140.00		230.04	294.49	0.00	0.00
145.00	(0.4)	448.44	572.29	0.00	0.00
147.00	(24) attachments	5762.57	3082.81	0.00	0.00
149.00		171.53	186.40	0.00	0.00
	Totals:	31,003.32	39,760.96	0.00	0.00

#### **Calculated Forces**

phi

Pn

(kips)

5580.79

5509.80

5437.04

5362.51

5286.22

5208.16

5128.34

5046.75

4963.39

4878.27

4843.73

4791.38

4797.23

4766.44

4677.28

4586.36

4493.67

4393.03

4267.07

4141.11

4015.15

3889.19

3882.89

3763.23

3276.58

3189.50

3081.54

2973.57

2865.60

2757.64

2714.45

2649.67

2541.71

2487.72

2433.74

2325.77

2282.59

2239.40

phi

Vn

(kips)

2790.40

2754.90

2718.52

2681.26

2643.11

2604.08

2564.17

2523.38

2481.70

2439.14

2421.86

2395.69

2398.61

2383.22

2338.64

2293.18

2246.84

2196.52

2133.53

2070.55

2007.57

1944.59

1941.44

1881.61

1638.29

1594.75

1540.77

1486.79

1432.80

1378.82

1357.23

1324.84

1270.85

1243.86

1216.87

1162.89

1141.29

1119.70

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Mu

MX

(ft-kips)

0.00

0.00

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0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||

Resultant

Moment

(ft-kips)

3230.67

3075.45

2922.90

2772.97

2625.64

2481.03

2339.26

2200.40

2064.50

1931.60

1879.69

1802.52

1720.17

1676.58

1553.03

1432.61

1315.34

1201.18

1090.12

982.13

877.19

775.26

770.32

676 45

666.87

580.91

488.02

397.99

310.78

226.34

193.65

155.15

92.53

62.51

45.45

12.56

0.36

0.00



21

Load Case: 0.9D + 1.6W 101 mph Wind

**Dead Load Factor** 0.90 **Wind Load Factor** 1.60

Tu

MY (-)

(ft-kips)

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

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Мп

ΜZ

(ft-kips)

-3230.6

-3075.4

-2922.9

-2772.9

-2625.6

-2481.0

-2339.2

-2200.4

-2064.5

-1931.6

-1879.6

-1802.5

-1720.1

-1676.5

-1553.0

-1432.6

-1315.3

-1201.1

-1090.1

-982.13

-877.19

-775.26

-770.32

-676.45

-666.87

-580.91

-488.02

-397.99

-310.78

-226.34

-193.65

-155.15

-92.53

-62.51

-45.45

-12.56

-0.36

0.00

Vıı

FX (-)

(kips)

-31.04

-30.51

-29.99

-29.47

-28.92

-28.35

-27.77

-27.18

-26.58

-25.96

-25.72

-25.34

-24.91

-24.71

-24.08

-23.46

-22.83

-22.21

-21.60

-20.99

-20.39

-19.78

-19.76

-19.16

-19.10

-18.58

-18.01

-17.44

-16.89

-16.34

-12.84

-12.52

-12.01

-6.82

-6.58

-6 10

-0.18

-0.17

Seg

Elev

(ft)

0.00

5.00

10.00

15.00

20.00

25.00

30.00

35.00

40.00

45.00

47.00

50.00

53.25

55.00

60.00

65.00

70.00

75.00

80.00

85.00

90.00

95.00

95.25

100.00

100.50

105.00

110.00

115.00

120.00

125.00

127.00

130.00

135.00

137.50

140.00

145.00

147.00

149.00

Pu

FY (-)

(kips)

-39.73

-38.20

-36.71

-35.24

-33.80

-32.38

-31.00

-29.64

-28.31

-27.03

-26.51

-25.12

-23.65

-23.20

-21.97

-20.77

-19.59

-18.45

-17.33

-16.25

-15.19

-14.18

-14.11

-12.53

-12.35

-11.56

-10.71

-9.88

-9.08

-8.32

-6.73

-6.32

-5.66

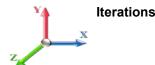
-3.78

-3.50

-2 95

-0.18

0.00



phi

Tn

(ft-kips)

13771.9

13318.1

12867.3

12419.8

11975.8

11535.7

11099.6

10667.8

10240.6

9818.41

9650.93

9401.28

9428.91

9284.05

8873.91

8469.53

8071.16

7668.29

7232.68

6809.81

6399.67

6002.28

5982.74

5617.62

4993.13

4715.31

4399.90

4095.40

3801.82

3519.16

3409.16

3247 42

2986.60

2860.28

2736.70

2497.71

2405.17

2314.38

phi

Mn

(ft-kips)

6896.19

6668.96

6443.24

6219.15

5996.84

5776.42

5558.05

5341.85

5127.96

4916.50

4832.64

4707.63

4721.46

4648.93

4443.55

4241.06

4041.58

3839.84

3621.71

3409.96

3204.59

3005.60

2995.82

2812.98

2500.28

2361.16

2203.22

2050.74

1903.74

1762.20

1707.11

1626.12

1495.52

1432.27

1370.38

1250 71

1204.38

1158.91

Page: 15

Total Deflect (in)	Rotation Sway (deg)	Rotation Twist (deg)	Stress Ratio
0.00	0.000	0.000	0.476
0.06	-0.118	0.000	0.468
0.25	-0.238	0.000	0.461
0.57	-0.359	0.000	0.453
1.01	-0.481	0.000	0.444
1.58	-0.605	0.000	0.436
2.28	-0.729	0.000	0.427
3.11	-0.855	0.000	0.418
4.08	-0.982	0.000	0.408
5.17	-1.109	0.000	0.399
5.65	-1.162	0.000	0.395
6.40	-1.240	0.000	0.388
7.28	-1.325	0.000	0.369
7.77	-1.371	0.000	0.366
9.28	-1.494	0.000	0.354
10.91	-1.616	0.000	0.342
12.67	-1.738	0.000	0.330
14.55	-1.860	0.000	0.317
16.56	-1.980	0.000	0.305
18.70	-2.099	0.000	0.292
20.96	-2.215	0.000	0.278

-2.329

-2.335

-2.439

-2.450

-2.545

-2.653

-2.752

-2.840

-2.915

-2.941

-2.975

-3.019

-3.034

-3.046

-3.059

-3.060

-3.060

23.34

23.47

25.84

26.10

28.46

31.18

34.01

36.94

39.96

41.18

43.04

46.18

47.77

49.36

52.56

53.84

55.12

0.000

0.000

0.000

0.000

0.000

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0.000

0.000

0.000

0.000

0.000

0.000

0.000

0.000

0.000

0.262

0.261

0.244

0.271

0.250

0.225

0.198

0.167

0.132

0 116

0.098

0.064

0.045

0.035

0.011

0.000

0.000

### Wind Loading - Shaft

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: II Page: 16



Load Case: 1.2D + 1.0Di + 1.0Wi 50 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.00



Iterations 21

							1				Wind	Dand	Tot Dead
Elev (ft) Description	Kzt	Kz	qz (psf)	qzGh (psf)	C (mph-ft)	Cf	lce Thick (in)	Tributary (ft)	Aa (sf)	CfAa (sf)		Dead Load Ice (lb)	Load (lb)
0.00	1.00	0.85	5.168	5.68	0.00	1.200	0.000	0.00	0.000	0.00	0.0	0.0	0.0
5.00	1.00	0.85	5.168	5.68	0.00	1.200	1.656	5.00	26.702	32.04	182.2	631.7	2316.0
10.00	1.00	0.85	5.168	5.68	0.00	1.200	1.775	5.00	26.285	31.54	179.3	664.8	2314.5
15.00	1.00	0.85	5.168	5.68	0.00	1.200	1.848	5.00	25.829	30.99	176.2	679.0	2294.1
20.00	1.00	0.90	5.483	6.03	0.00	1.200	1.902	5.00	25.357	30.43	183.5	684.9	2265.4
25.00	1.00	0.95	5.747	6.32	0.00	1.200	1.945	5.00	24.877	29.85	188.7	685.9	2231.8
30.00	1.00	0.98	5.972	6.57	0.00	1.200	1.981	5.00	24.390	29.27	192.3	683.8	2195.1
35.00	1.00	1.01	6.169	6.79	0.00	1.200	2.012	5.00	23.899	28.68	194.6	679.5	2156.1
40.00	1.00	1.04	6.345	6.98	0.00	1.200	2.039	5.00	23.405	28.09	196.0	673.3	2115.4
45.00	1.00	1.07	6.504	7.15	0.00	1.200	2.063	5.00	22.908	27.49	196.7	665.8	2073.3
47.00 Bot - Section 2	1.00	1.08	6.564	7.22	0.00	1.200	2.072	2.00	9.022	10.83	78.2	265.0	818.3
50.00	1.00	1.09	6.650	7.32	0.00	1.200	2.085	3.00	13.606	16.33	119.4	401.2	2055.1
53.25 Top - Section 1	1.00	1.11	6.739	7.41	0.00	1.200	2.098	3.25	14.537	17.44	129.3	430.7	2194.3
55.00	1.00	1.12	6.785	7.46	0.00	1.200	2.105	1.75	7.739	9.29	69.3	230.7	703.8
60.00	1.00	1.14	6.910	7.60	0.00	1.200	2.123	5.00	21.779	26.13	198.7	649.0	1977.3
65.00	1.00	1.16	7.028	7.73	0.00	1.200	2.140	5.00	21.277	25.53	197.4	638.0	1931.8
70.00	1.00	1.17	7.138	7.85	0.00	1.200	2.156		20.773	24.93	195.7	626.5	1885.6
75.00	1.00	1.19	7.243	7.97	0.00	1.200	2.171	5.00	20.269	24.32	193.8	614.4	1838.9
80.00	1.00	1.21	7.342	8.08	0.00	1.200	2.185	5.00	19.764	23.72	191.5	601.9	1791.8
85.00	1.00	1.22	7.436	8.18	0.00	1.200	2.198	5.00	19.259	23.11	189.0	588.9	1744.2
90.00	1.00	1.24	7.526	8.28	0.00	1.200	2.211	5.00	18.753	22.50	186.3	575.5	1696.2
95.00	1.00	1.25	7.612	8.37	0.00	1.200	2.223	5.00	18.246	21.90	183.3	561.8	1647.9
95.25 Bot - Section 3	1.00	1.25	7.617	8.38	0.00	1.200	2.224	0.25	0.899	1.08	9.0	28.1	81.4
100.00	1.00	1.27	7.695	8.46	0.00	1.200	2.234	4.75	17.141	20.57	174.1	530.3	2402.6
100.50 Top - Section 2	1.00	1.27	7.703	8.47	0.00	1.200	2.236	0.50	1.777	2.13	18.1	55.7	249.4
105.00	1.00	1.28	7.774	8.55	0.00	1.200	2.245	4.50	15.771	18.92	161.8	489.5	1290.4
110.00	1.00	1.29	7.851	8.64	0.00	1.200	2.256	5.00	17.041	20.45	176.6	529.4	1391.0
115.00	1.00	1.30	7.925	8.72	0.00	1.200	2.266	5.00	16.533	19.84	172.9	514.5	1346.5
120.00	1.00	1.32	7.996	8.80	0.00	1.200	2.276	5.00	16.024	19.23	169.1	499.4	1301.8
125.00	1.00	1.33	8.065	8.87	0.00	1.200	2.285	5.00	15.516	18.62	165.2	484.1	1256.8
127.00 Appurtenance(s)	1.00	1.33	8.092	8.90	0.00	1.200	2.289	2.00	6.063	7.28	64.8	191.2	491.9
130.00	1.00	1.34	8.132	8.95	0.00	1.200	2.294	3.00	8.942	10.73	96.0	281.2	723.4
135.00	1.00	1.35	8.197	9.02	0.00	1.200	2.303	5.00	14.497	17.40	156.9	452.9	1166.2
137.50 Appurtenance(s)	1.00	1.35	8.229	9.05	0.00	1.200	2.307	2.50	7.057	8.47	76.6	222.5	568.0
140.00	1.00	1.36	8.260	9.09	0.00	1.200	2.311	2.50	6.929	8.31	75.5	218.5	556.6
145.00	1.00	1.37	8.321	9.15	0.00	1.200	2.319	5.00	13.478	16.17	148.0	420.9	1074.9
147.00 Appurtenance(s)	1.00	1.37	8.345	9.18	0.00	1.200	2.322	2.00	5.247	6.30	57.8	165.8	419.0
149.00	1.00	1.38	8.369	9.21	0.00	1.200	2.325	2.00	5.166	6.20	57.1	163.2	411.7
							Totala	140.00	-		E 404 2		E4 079 6

Totals: 149.00 5,401.2 54,978.6

#### **Discrete Appurtenance Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Load Case: 1.2D + 1.0Di + 1.0Wi 50 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.00



Page: 17

Iterations 21

	Elev			qz	qzGh	Orient Factor		Total CaAa	Dead Load	Horiz Ecc	Vert Ecc	Wind FX	Mom Y	Mom Z
No.	(ft)	Description	Qty	(psf)	(psf)	x Ka	Ka	(sf)	(lb)	(ft)	(ft)	(lb)	(lb-ft)	(lb-ft)
1	147.00 44	449	3	8.345	9.180	0.50	0.75	3.61	548.22	0.000	0.000	33.09	0.00	0.00
2	147.00 K	RY 112 89/4	6	8.345	9.180	0.50	0.75	4.42	221.56	0.000	0.000	40.53	0.00	0.00
3	147.00 H	RK12-HD	1	8.345	9.180	1.00	1.00	22.43	1536.63	0.000	0.000	205.89	0.00	0.00
4	147.00 P	RK-1245 (kicker kit)	1	8.345	9.180	1.00	1.00	22.74	894.66	0.000	0.000	208.72	0.00	0.00
5	147.00 Lo	ow Profile	1	8.345	9.180	1.00	1.00	45.50	3241.69	0.000	0.000	417.68	0.00	0.00
6	147.00 A	PXVAARR24 43-U-NA2	3	8.345	9.180	0.52	0.75	35.91	2199.94	0.000	0.000	329.63	0.00	0.00
7	147.00 A	PXV18-209014-C-A20	6	8.345	9.180	0.72	0.75	25.68	563.58	0.000	0.000	235.69	0.00	0.00
8	147.00 78	82 10662	3	8.345	9.180	0.50	0.75	1.23	29.74	0.000	0.000	11.26	0.00	0.00
9	137.50 Lo	ow Profile Platform-flat	1	8.229	9.051	1.00	1.00	52.68	2524.08	0.000	0.000	476.84	0.00	0.00
10	137.50 D	C2-48-60-8-18F-02	1	8.229	9.051	1.00	1.00	4.53	82.88	0.000	0.000	40.97	0.00	0.00
11	137.50 R	RUS-11	6	8.229	9.051	0.54	0.80	10.80	844.37	0.000	0.000	97.72	0.00	0.00
12	137.50 LO	GP21903	6	8.229	9.051	0.54	0.80	2.56	92.04	0.000	0.000	23.16	0.00	0.00
13	137.50 LO	GP21401	6	8.229	9.051	0.54	0.80	7.70	257.20	0.000	0.000	69.71	0.00	0.00
14	137.50 A	M-X-CD-17-65-00T-RET	3	8.229	9.051	0.64	0.80	25.96	1257.47	0.000	0.000	234.97	0.00	0.00
15	137.50 77	770.00	6	8.229	9.051	0.58	0.80	24.31	1403.72	0.000	0.000	220.04	0.00	0.00
16	127.00 Lo	ow Profile	1	8.092	8.901	1.00	1.00	45.16	3216.41	0.000	0.000	401.98	0.00	0.00
17	127.00 LF	PA-185080/8CF	6	8.092	8.901	0.98	0.80	18.83	746.25	0.000	0.000	167.57	0.00	0.00
18	127.00 LF	PA-80080/4CF	6	8.092	8.901	1.36	0.80	30.65	1223.65	0.000	0.000	272.80	0.00	0.00

Totals: 20,884.09 3,488.27

# **Total Applied Force Summary**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Iterations

Page: 18

21

Load Case: 1.2D + 1.0Di + 1.0Wi 50 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.00

Elev		Lateral FX (-)	Axial FY (-)	Torsion MY	Moment MZ
(ft)	Description	(lb)	(lb)	(lb-ft)	(lb-ft)
0.00		0.00	0.00	0.00	0.00
5.00		182.15	2580.88	0.00	0.00
10.00		179.31	2579.33	0.00	0.00
15.00		176.20	2558.91	0.00	0.00
20.00		183.54	2530.19	0.00	0.00
25.00		188.72	2496.66	0.00	0.00
30.00		192.27	2459.95	0.00	0.00
35.00		194.61	2420.95	0.00	0.00
40.00		196.02	2380.21	0.00	0.00
45.00		196.68	2338.09	0.00	0.00
47.00		78.17	924.22	0.00	0.00
50.00		119.44	2214.03	0.00	0.00
53.25		129.31	2366.49	0.00	0.00
55.00		69.31	796.53	0.00	0.00
60.00		198.66	2242.18	0.00	0.00
65.00		197.38	2196.63	0.00	0.00
70.00		195.74	2150.48	0.00	0.00
75.00		193.78	2103.79	0.00	0.00
80.00		191.54	2056.62	0.00	0.00
85.00		189.04	2009.03	0.00	0.00
90.00		186.30	1961.04	0.00	0.00
95.00		183.34	1912.70	0.00	0.00
95.25		9.04	94.69	0.00	0.00
100.00		174.11	2654.23	0.00	0.00
100.50		18.07	275.88	0.00	0.00
105.00		161.84	1528.76	0.00	0.00
110.00		176.60	1655.86	0.00	0.00
115.00		172.94	1611.35	0.00	0.00
120.00		169.13	1566.60	0.00	0.00
125.00		165.18	1521.62	0.00	0.00
127.00	(13) attachments	907.12	5784.17	0.00	0.00
130.00	(10) attacriments	95.98	837.37	0.00	0.00
135.00		156.86	1356.15	0.00	0.00
	(20) attachments				0.00
137.50 140.00	(29) attachments	1240.05	7124.75	0.00	
		75.55	611.14	0.00	0.00
145.00	(0.4) - #   + -	148.04	1183.93	0.00	0.00
147.00	(24) attachments	1540.31	9698.69	0.00	0.00
149.00		57.07	411.69	0.00	0.00
	Totals:	8,889.43	83,195.76	0.00	0.00

#### **Calculated Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

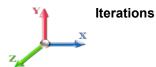
Gh: 1.1 Topography: 1 Struct Class: ||



21

**Load Case:** 1.2D + 1.0Di + 1.0Wi 50 mph Wind

Dead Load Factor 1.20 Wind Load Factor 1.00



Page: 19

Seg Elev	Pu FY (-)	Vu FX (-)	Tu MY (-)	Mu MZ	Mu MX	Resultant Moment	phi Pn	phi Vn	phi Tn	phi Mn	Total Deflect	Rotation Sway	Rotation Twist	Stress
(ft)	(kips)				(ft-kips)	(ft-kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(in)	(deg)	(deg)	Ratio
0.00	-83.19	-8.91	0.00	-933.78	0.00	933.78	5580.79	2790.40	13771.9	6896.19	0.00	0.000	0.000	0.150
5.00	-80.61	-8.78	0.00	-889.21	0.00	889.21	5509.80	2754.90	13318.1	6668.96	0.02	-0.034	0.000	0.148
10.00	-78.02	-8.65	0.00	-845.31	0.00	845.31	5437.04	2718.52	12867.3	6443.24	0.07	-0.069	0.000	0.146
15.00	-75.46	-8.51	0.00	-802.08	0.00	802.08	5362.51	2681.26	12419.8	6219.15	0.16	-0.104	0.000	0.143
20.00	-72.92	-8.37	0.00	-759.52	0.00	759.52	5286.22	2643.11	11975.8	5996.84	0.29	-0.139	0.000	0.140
25.00	-70.42	-8.22	0.00	-717.68	0.00	717.68	5208.16	2604.08	11535.7	5776.42	0.46	-0.175	0.000	0.138
30.00	-67.96	-8.06	0.00	-676.60	0.00	676.60	5128.34	2564.17	11099.6	5558.05	0.66	-0.211	0.000	0.135
35.00	-65.53	-7.90	0.00	-636.30	0.00	636.30	5046.75	2523.38	10667.8	5341.85	0.90	-0.247	0.000	0.132
40.00	-63.15	-7.73	0.00	-596.82	0.00	596.82	4963.39	2481.70	10240.6	5127.96	1.18	-0.284	0.000	0.129
45.00	-60.81	-7.55	0.00	-558.17	0.00	558.17	4878.27	2439.14	9818.41	4916.50	1.50	-0.321	0.000	0.126
47.00	-59.88	-7.49	0.00	-543.07	0.00	543.07	4843.73	2421.86	9650.93	4832.64	1.63	-0.336	0.000	0.125
50.00	-57.67	-7.38	0.00	-520.62	0.00	520.62	4791.38	2395.69	9401.28	4707.63	1.85	-0.359	0.000	0.123
53.25	-55.30	-7.25	0.00	-496.64	0.00	496.64	4797.23	2398.61	9428.91	4721.46	2.11	-0.383	0.000	0.117
55.00	-54.50	-7.20	0.00	-483.96	0.00	483.96	4766.44	2383.22	9284.05	4648.93	2.25	-0.396	0.000	0.116
60.00	-52.25	-7.02	0.00	-447.97	0.00	447.97	4677.28	2338.64	8873.91	4443.55	2.68	-0.432	0.000	0.112
65.00	-50.05	-6.83	0.00	-412.89	0.00	412.89	4586.36	2293.18	8469.53	4241.06	3.15	-0.467	0.000	0.108
70.00	-47.90	-6.65	0.00	-378.74	0.00	378.74	4493.67	2246.84	8071.16	4041.58	3.66	-0.502	0.000	0.104
75.00	-45.80	-6.46	0.00	-345.50	0.00	345.50	4393.03	2196.52	7668.29	3839.84	4.21	-0.537	0.000	0.100
80.00	-43.74	-6.28	0.00	-313.19	0.00	313.19	4267.07	2133.53	7232.68	3621.71	4.79	-0.572	0.000	0.097
85.00	-41.73	-6.09	0.00	-281.81	0.00	281.81	4141.11	2070.55	6809.81	3409.96	5.41	-0.606	0.000	0.093
90.00	-39.76	-5.91	0.00	-251.35	0.00	251.35	4015.15	2007.57	6399.67	3204.59	6.06	-0.639	0.000	0.088
95.00	-37.85	-5.71	0.00	-221.82	0.00	221.82	3889.19	1944.59	6002.28	3005.60	6.75	-0.672	0.000	0.084
95.25	-37.76	-5.71	0.00	-220.39	0.00	220.39	3882.89	1941.44	5982.74	2995.82	6.78	-0.674	0.000	0.083
100.00	-35.10	-5.52	0.00	-193.26	0.00	193.26	3763.23	1881.61	5617.62	2812.98	7.47	-0.703	0.000	0.078
100.50	-34.83	-5.51	0.00	-190.50	0.00	190.50	3276.58	1638.29	4993.13	2500.28	7.54	-0.707	0.000	0.087
105.00	-33.30	-5.34	0.00	-165.73	0.00	165.73	3189.50	1594.75	4715.31	2361.16	8.22	-0.734	0.000	0.081
110.00	-31.64	-5.16	0.00	-139.02	0.00	139.02	3081.54	1540.77	4399.90	2203.22	9.01	-0.764	0.000	0.073
115.00	-30.03	-4.98	0.00	-113.23	0.00	113.23	2973.57	1486.79	4095.40	2050.74	9.82	-0.793	0.000	0.065
120.00	-28.46	-4.80	0.00	-88.34	0.00	88.34	2865.60	1432.80	3801.82	1903.74	10.67	-0.818	0.000	0.056
125.00	-26.94	-4.62	0.00	-64.35	0.00	64.35	2757.64	1378.82	3519.16	1762.20	11.53	-0.839	0.000	0.046
127.00	-21.17	-3.63	0.00	-55.11	0.00	55.11	2714.45	1357.23	3409.16	1707.11	11.89	-0.846	0.000	0.040
130.00	-20.34	-3.52	0.00	-44.23	0.00	44.23	2649.67	1324.84	3247.42	1626.12	12.42	-0.856	0.000	0.035
135.00	-18.98	-3.35	0.00	-26.60	0.00	26.60	2541.71	1270.85	2986.60	1495.52	13.33	-0.869	0.000	0.025
137.50	-11.88	-2.00	0.00	-18.23	0.00	18.23	2487.72	1243.86	2860.28	1432.27	13.78	-0.873	0.000	0.018
140.00	-11.27	-1.92	0.00	-13.22	0.00	13.22	2433.74	1216.87	2736.70	1370.38	14.24	-0.876	0.000	0.014
145.00	-10.08	-1.75	0.00	-3.63	0.00	3.63	2325.77	1162.89	2497.71	1250.71	15.16	-0.880	0.000	0.007
147.00	-0.41	-0.06	0.00	-0.13	0.00	0.13	2282.59	1141.29	2405.17	1204.38	15.53	-0.881	0.000	0.000
149.00	0.00	-0.06	0.00	0.00	0.00	0.00	2239.40	1119.70	2314.38	1158.91	15.90	-0.881	0.000	0.000

#### **Seismic Segment Forces (Factored)**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Page: 20

Load Case: 1.2D + 1.0E **Iterations** 19 0.19 **Gust Response Factor** 1.10 0.17 Sds Ss **Dead Load Factor** 1.20 Seismic Load Factor 1.00 Sd1 0.10 **S1** 0.06 Wind Load Factor 0.00 Structure Frequency (f1) 0.46 SA 0.05 Seismic Importance Factor 1.00

Top			<b>10/</b> -				Lateral		
Elev (ft)	Description		Wz (lb)	а	b	С	Fs (lb)		R: 1.50
0.00			0.00	0.00	0.00	0.00	0.00		
5.00			1403.6	0.00	0.03	0.02	20.16		
10.00			1374.7	0.01	0.05	0.03	29.69		
15.00			1345.9	0.02	0.06	0.04	34.16		
20.00			1317.0	0.03	0.07	0.04	36.13		
25.00			1288.2	0.05	0.07	0.04	36.93		
30.00			1259.3	0.08	0.07	0.04	37.26		
35.00			1239.5	0.10	0.07	0.04	37.44		
40.00			1201.6	0.14	0.07	0.03	37.49		
45.00			1172.8	0.17	0.07	0.03	37.24		
47.00	Bot - Section 2		461.06	0.19	0.06	0.02	14.69		
50.00	201 00010112		1378.2	0.21	0.06	0.02	43.82		
53.25	Top - Section 1		1469.7	0.24	0.06	0.02	46.08		
55.00	rop cocacii i		394.26	0.26	0.05	0.02	12.19		
60.00			1106.9	0.31	0.04	0.01	31.68		
65.00			1078.1	0.36	0.03	0.01	26.40		
70.00			1076.1	0.42	0.01	0.01	19.15		
75.00			1020.4	0.48	-0.01	0.01	10.24		
80.00			991.60	0.54	-0.03	0.01	0.55		
85.00			962.76	0.62	-0.06	0.02	-8.59		
90.00			933.91	0.69	-0.08	0.03	-15.72		
95.00			905.06	0.77	-0.11	0.05	-19.69		
95.25	Bot - Section 3		44.50	0.77	-0.11	0.05	-0.97		
100.00	Bot Godion o		1560.2	0.85	-0.12	0.07	-35.29		
100.50	Top - Section 2		161.43	0.86	-0.12	0.07	-3.62		
105.00	TOP COOLON 2		667.38	0.94	-0.12	0.10	-12.56		
110.00			718.05	1.03	-0.10	0.15	-7.22		
115.00			693.32	1.13	-0.05	0.20	2.62		
120.00			668.60	1.23	0.03	0.27	15.21		
125.00			643.87	1.33	0.16	0.36	30.18		
127.00	Appurtenance(s)		1864.6	1.37	0.23	0.40	108.12		
130.00	Apparteriarioc(3)		368.52	1.44	0.36	0.47	28.09		
135.00			594.42	1.55	0.64	0.61	65.90		
137.50	Appurtenance(s)		2314.5	1.61	0.81	0.68	301.31		
140.00	/ ipparteriarioc(3)		2314.5	1.67	1.01	0.77	42.51		
145.00			544.97	1.79	1.49	0.77	106.99		
147.00	Appurtenance(s)			1.79	1.72	1.05	732.23		
147.00	Apparterialice(s)		3388.9 207.11	1.89	1.72	1.14	49.02		
145.00				1.09	1.90	1.14			
		Totals:	38,067.9				1,889.8	Total Wind:	31,003.3

Seismic Base Shear is Less Than 50% of Wind Force - An Analysis is NOT Required

#### **Calculated Forces**

**Structure**: CT13614-A-SBA **Code**: EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Page: 21

<b>Load Case:</b> 1.2D + 1.0E					* Iterations	19
<b>Gust Response Factor</b>	1.10		Sds	0.19	Ss	0.17
<b>Dead Load Factor</b>	1.20 Seismic Load Factor	1.00	Sd1	0.10	Z S1	0.06
Wind Load Factor	0.00 Structure Frequency (f1)	0.46	SA	0.05	Seismic Importance Factor	1.00

Seg Elev	Pu FY (-)	Vu FX (-)	Tu MY (-)	Mu MZ	Mu MX	Resultant Moment	phi Pn	phi Vn	phi Tn	phi Mn	Total Deflect	Rotation Sway	Rotation Twist	Stress
(ft)	(kips)				(ft-kips)	(ft-kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(in)	(deg)	(deg)	Ratio
0.00	-53.01	-2.00	0.00	-236.96	0.00	236.96	5580.79	2790.40	13771.9	6896.19	,	0.00	0.00	0.044
5.00	-51.06	-1.98	0.00	-226.98	0.00	226.98	5509.80	2754.90	13318.1	6668.96		0.00	-0.01	0.043
10.00	-49.15	-1.96	0.00	-217.06	0.00	217.06	5437.04	2718.52	12867.3	6443.24		0.02	-0.02	0.043
15.00	-47.27	-1.93	0.00	-207.25	0.00	207.25	5362.51	2681.26	12419.8	6219.15		0.04	-0.03	0.042
20.00	-45.42	-1.90	0.00	-197.58	0.00	197.58	5286.22	2643.11	11975.8	5996.84		0.07	-0.04	0.042
25.00	-43.61	-1.87	0.00	-188.06	0.00	188.06	5208.16	2604.08	11535.7	5776.42		0.12	-0.05	0.041
30.00	-41.84	-1.84	0.00	-178.69	0.00	178.69	5128.34	2564.17	11099.6	5558.05		0.17	-0.05	0.040
35.00	-40.10	-1.81	0.00	-169.49	0.00	169.49	5046.75	2523.38	10667.8	5341.85		0.23	-0.06	0.040
40.00	-38.39	-1.78	0.00	-160.44	0.00	160.44	4963.39	2481.70	10240.6	5127.96		0.30	-0.07	0.039
45.00	-36.72	-1.74	0.00	-151.56	0.00	151.56	4878.27	2439.14	9818.41	4916.50		0.39	-0.08	0.038
47.00	-36.06	-1.73	0.00	-148.08	0.00	148.08	4843.73	2421.86	9650.93	4832.64		0.42	-0.09	0.038
50.00	-34.24	-1.69	0.00	-142.90	0.00	142.90	4791.38	2395.69	9401.28	4707.63		0.48	-0.09	0.038
53.25	-32.31	-1.64	0.00	-137.42	0.00	137.42	4797.23	2398.61	9428.91	4721.46		0.55	-0.10	0.036
55.00	-31.74	-1.63	0.00	-134.55	0.00	134.55	4766.44	2383.22	9284.05	4648.93		0.58	-0.10	0.036
60.00	-30.15	-1.60	0.00	-126.40	0.00	126.40	4677.28	2338.64	8873.91	4443.55		0.70	-0.11	0.035
65.00	-28.59	-1.58	0.00	-118.40	0.00	118.40	4586.36	2293.18	8469.53	4241.06		0.83	-0.12	0.034
70.00	-27.06	-1.56	0.00	-110.52	0.00	110.52	4493.67	2246.84	8071.16	4041.58		0.96	-0.13	0.033
75.00	-25.58	-1.55	0.00	-102.73	0.00	102.73	4393.03	2196.52	7668.29	3839.84		1.11	-0.15	0.033
80.00	-24.12	-1.55	0.00	-94.98	0.00	94.98	4267.07	2133.53	7232.68	3621.71		1.27	-0.16	0.032
85.00	-22.70	-1.55	0.00	-87.24	0.00	87.24	4141.11	2070.55	6809.81	3409.96		1.43	-0.17	0.031
90.00	-21.31	-1.55	0.00	-79.50	0.00	79.50	4015.15	2007.57	6399.67	3204.59		1.61	-0.18	0.030
95.00	-19.96	-1.55	0.00	-71.75	0.00	71.75	3889.19	1944.59	6002.28	3005.60		1.80	-0.19	0.029
95.25	-19.90	-1.55	0.00	-71.37	0.00	71.37	3882.89	1941.44	5982.74	2995.82		1.81	-0.19	0.029
100.00	-17.77	-1.54	0.00	-64.01	0.00	64.01	3763.23	1881.61	5617.62	2812.98		2.01	-0.20	0.027
100.50	-17.55	-1.54	0.00	-63.24	0.00	63.24	3276.58	1638.29	4993.13	2500.28		2.03	-0.20	0.031
105.00	-16.51	-1.54	0.00	-56.30	0.00	56.30	3189.50	1594.75	4715.31	2361.16		2.22	-0.21	0.029
110.00	-15.39	-1.54	0.00	-48.58	0.00	48.58	3081.54	1540.77	4399.90	2203.22		2.44	-0.22	0.027
115.00	-14.29	-1.54	0.00	-40.88	0.00	40.88	2973.57	1486.79	4095.40	2050.74		2.67	-0.23	0.025
120.00	-13.22	-1.52	0.00	-33.19	0.00	33.19	2865.60	1432.80	3801.82	1903.74		2.92	-0.24	0.022
125.00	-12.18	-1.49	0.00	-25.59	0.00	25.59	2757.64	1378.82	3519.16	1762.20		3.17	-0.25	0.019
127.00	-9.84	-1.37	0.00	-22.62	0.00	22.62	2714.45	1357.23	3409.16	1707.11		3.27	-0.25	0.017
130.00	-9.28	-1.34	0.00	-18.52	0.00	18.52	2649.67	1324.84	3247.42	1626.12		3.43	-0.25	0.015
135.00	-8.38	-1.27	0.00	-11.82	0.00	11.82	2541.71	1270.85	2986.60	1495.52		3.70	-0.26	0.011
137.50	-5.51	-0.96	0.00	-8.65	0.00	8.65	2487.72	1243.86	2860.28	1432.27		3.83	-0.26	0.008
140.00	<b>-</b> 5.12	-0.91	0.00	-6.26	0.00	6.26	2433.74	1216.87	2736.70	1370.38		3.97	-0.26	0.007
145.00	-4.36	-0.80	0.00	-1.70	0.00	1.70	2325.77	1162.89	2497.71	1250.71		4.24	-0.26	0.003
147.00	-0.25	-0.05	0.00	-0.10	0.00	0.10	2282.59	1141.29	2405.17	1204.38		4.36	-0.26	0.000
149.00	0.00	-0.05	0.00	0.00	0.00	0.00	2239.40	1119.70	2314.38	1158.91		4.47	-0.26	0.000

#### **Seismic Segment Forces (Factored)**

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Page: 22

**Load Case:** 0.9D + 1.0E **Iterations** 19 **Gust Response Factor** 1.10 0.19 0.17 Sds Ss **Dead Load Factor** 0.90 Seismic Load Factor 1.00 Sd1 0.10 **S1** 0.06 **Wind Load Factor** 0.00 Structure Frequency (f1) 0.46 SA 0.05 Seismic Importance Factor 1.00

Top Elev			Wz				Lateral Fs		
(ft)	Description		(lb)	а	b	С	(lb)		R: 1.50
0.00	•		0.00	0.00	0.00	0.00	0.00		
5.00			1403.6	0.00	0.03	0.02	20.16		
10.00			1374.7	0.01	0.05	0.03	29.69		
15.00			1345.9	0.02	0.06	0.04	34.16		
20.00			1317.0	0.03	0.07	0.04	36.13		
25.00			1288.2	0.05	0.07	0.04	36.93		
30.00			1259.3	0.08	0.07	0.04	37.26		
35.00			1230.5	0.10	0.07	0.04	37.44		
40.00			1201.6	0.14	0.07	0.03	37.49		
45.00			1172.8	0.17	0.07	0.03	37.24		
47.00	Bot - Section 2		461.06	0.19	0.06	0.02	14.69		
50.00			1378.2	0.21	0.06	0.02	43.82		
53.25	Top - Section 1		1469.7	0.24	0.06	0.02	46.08		
55.00			394.26	0.26	0.05	0.02	12.19		
60.00			1106.9	0.31	0.04	0.01	31.68		
65.00			1078.1	0.36	0.03	0.01	26.40		
70.00			1049.2	0.42	0.01	0.01	19.15		
75.00			1020.4	0.48	-0.01	0.01	10.24		
80.00			991.60	0.54	-0.03	0.01	0.55		
85.00			962.76	0.62	-0.06	0.02	-8.59		
90.00			933.91	0.69	-0.08	0.03	-15.72		
95.00			905.06	0.77	-0.11	0.05	-19.69		
95.25	Bot - Section 3		44.50	0.77	-0.11	0.05	-0.97		
100.00			1560.2	0.85	-0.12	0.07	-35.29		
100.50	Top - Section 2		161.43	0.86	-0.12	0.07	-3.62		
105.00			667.38	0.94	-0.12	0.10	-12.56		
110.00			718.05	1.03	-0.10	0.15	-7.22		
115.00			693.32	1.13	-0.05	0.20	2.62		
120.00			668.60	1.23	0.03	0.27	15.21		
125.00			643.87	1.33	0.16	0.36	30.18		
127.00	Appurtenance(s)		1864.6	1.37	0.23	0.40	108.12		
130.00			368.52	1.44	0.36	0.47	28.09		
135.00			594.42	1.55	0.64	0.61	65.90		
137.50	Appurtenance(s)		2314.5	1.61	0.81	0.68	301.31		
140.00			281.76	1.67	1.01	0.77	42.51		
145.00			544.97	1.79	1.49	0.96	106.99		
147.00	Appurtenance(s)		3388.9	1.84	1.72	1.05	732.23		
149.00			207.11	1.89	1.98	1.14	49.02		
		Totals:	38,067.9				1,889.8	Total Wind:	31,003.3

Seismic Base Shear is Less Than 50% of Wind Force - An Analysis is NOT Required

#### **Calculated Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Page: 23

Load Case: 0.9D + 1.0E **Iterations** 19 **Gust Response Factor** 1.10 Sds 0.19 Ss 0.17 **S1 Dead Load Factor** 0.90 Seismic Load Factor 1.00 Sd1 0.10 0.06 **Wind Load Factor** 0.00 Structure Frequency (f1) 0.46 SA 0.05 Seismic Importance Factor 1.00

Seg Elev	Pu FY (-)	Vu FX (-)	Tu MY (-)	Mu MZ	Mu MX	Resultant Moment	phi Pn	phi Vn	phi Tn	phi Mn	Total Deflect	Rotation Sway	Rotation Twist	Stress
(ft)	(kips)				(ft-kips)	(ft-kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(in)	(deg)	(deg)	Ratio
0.00	-39.76	-2.00	0.00	-235.13	0.00	235.13	5580.79	2790.40	13771.9	6896.19	,	0.00	0.00	0.041
5.00	-38.30	-1.98	0.00	-225.15	0.00	225.15	5509.80	2754.90	13318.1	6668.96		0.00	-0.01	0.041
10.00	-36.86	-1.96	0.00	-215.25	0.00	215.25	5437.04	2718.52	12867.3	6443.24		0.02	-0.02	0.040
15.00	-35.45	-1.93	0.00	-205.46	0.00	205.46	5362.51	2681.26	12419.8	6219.15		0.04	-0.03	0.040
20.00	-34.07	-1.90	0.00	-195.83	0.00	195.83	5286.22	2643.11	11975.8	5996.84		0.07	-0.04	0.039
25.00	-32.71	-1.86	0.00	-186.35	0.00	186.35	5208.16	2604.08	11535.7	5776.42		0.12	-0.04	0.039
30.00	-31.38	-1.83	0.00	-177.03	0.00	177.03	5128.34	2564.17	11099.6	5558.05		0.17	-0.05	0.038
35.00	-30.07	-1.80	0.00	-167.88	0.00	167.88	5046.75	2523.38	10667.8	5341.85		0.23	-0.06	0.037
40.00	-28.79	-1.76	0.00	-158.89	0.00	158.89	4963.39	2481.70	10240.6	5127.96		0.30	-0.07	0.037
45.00	-27.54	-1.73	0.00	-150.08	0.00	150.08	4878.27	2439.14	9818.41	4916.50		0.38	-0.08	0.036
47.00	-27.04	-1.71	0.00	-146.63	0.00	146.63	4843.73	2421.86	9650.93	4832.64		0.42	-0.09	0.036
50.00	-25.68	-1.67	0.00	-141.49	0.00	141.49	4791.38	2395.69	9401.28	4707.63		0.48	-0.09	0.035
53.25	-24.23	-1.62	0.00	-136.06	0.00	136.06	4797.23	2398.61	9428.91	4721.46		0.54	-0.10	0.034
55.00	-23.81	-1.61	0.00	-133.22	0.00	133.22	4766.44	2383.22	9284.05	4648.93		0.58	-0.10	0.034
60.00	-22.61	-1.58	0.00	-125.14	0.00	125.14	4677.28	2338.64	8873.91	4443.55		0.69	-0.11	0.033
65.00	-21.44	-1.56	0.00	-117.22	0.00	117.22	4586.36	2293.18	8469.53	4241.06		0.82	-0.12	0.032
70.00	-20.30	-1.54	0.00	-109.43	0.00	109.43	4493.67	2246.84	8071.16	4041.58		0.95	-0.13	0.032
75.00	-19.18	-1.53	0.00	-101.72	0.00	101.72	4393.03	2196.52	7668.29	3839.84		1.10	-0.14	0.031
80.00	-18.09	-1.53	0.00	-94.06	0.00	94.06	4267.07	2133.53	7232.68	3621.71		1.25	-0.15	0.030
85.00	-17.02	-1.53	0.00	-86.41	0.00	86.41	4141.11	2070.55	6809.81	3409.96		1.42	-0.16	0.029
90.00	-15.98	-1.53	0.00	-78.75	0.00	78.75	4015.15	2007.57	6399.67	3204.59		1.60	-0.17	0.029
95.00	-14.97	-1.53	0.00	-71.09	0.00	71.09	3889.19	1944.59	6002.28	3005.60		1.79	-0.19	0.028
95.25	-14.92	-1.53	0.00	-70.71	0.00	70.71	3882.89	1941.44	5982.74	2995.82		1.80	-0.19	0.027
100.00	-13.33	-1.53	0.00	-63.44	0.00	63.44	3763.23	1881.61	5617.62	2812.98		1.99	-0.20	0.026
100.50	-13.16	-1.53	0.00	-62.67	0.00	62.67	3276.58	1638.29	4993.13	2500.28		2.01	-0.20	0.029
105.00	-12.38	-1.53	0.00	-55.80	0.00	55.80	3189.50	1594.75	4715.31	2361.16		2.20	-0.21	0.028
110.00	-11.54	-1.53	0.00	-48.16	0.00	48.16	3081.54	1540.77	4399.90	2203.22		2.42	-0.22	0.026
115.00	-10.71	-1.52	0.00	-40.54	0.00	40.54	2973.57	1486.79	4095.40	2050.74		2.65	-0.23	0.023
120.00	-9.91	-1.51	0.00	-32.93	0.00	32.93	2865.60	1432.80	3801.82	1903.74		2.89	-0.23	0.021
125.00	-9.14	-1.47	0.00	-25.40	0.00	25.40	2757.64	1378.82	3519.16	1762.20		3.14	-0.24	0.018
127.00	-7.38	-1.36	0.00	-22.45	0.00	22.45	2714.45	1357.23	3409.16	1707.11		3.24	-0.25	0.016
130.00	-6.96	-1.33	0.00	-18.38	0.00	18.38	2649.67	1324.84	3247.42	1626.12		3.40	-0.25	0.014
135.00	-6.28	-1.26	0.00	-11.74	0.00	11.74	2541.71	1270.85	2986.60	1495.52		3.66	-0.26	0.010
137.50	-4.13	-0.95	0.00	-8.59	0.00	8.59	2487.72	1243.86	2860.28	1432.27		3.80	-0.26	0.008
140.00	-3.84	-0.91	0.00	-6.22	0.00	6.22	2433.74	1216.87	2736.70	1370.38		3.93	-0.26	0.006
145.00	-3.27	-0.80	0.00	-1.69	0.00	1.69	2325.77	1162.89	2497.71	1250.71		4.21	-0.26	0.003
147.00	-0.19	-0.05	0.00	-0.10	0.00	0.10	2282.59	1141.29	2405.17	1204.38		4.32	-0.26	0.000
149.00	0.00	-0.05	0.00	0.00	0.00	0.00	2239.40	1119.70	2314.38	1158.91		4.42	-0.26	0.000

### Wind Loading - Shaft

**Structure**: CT13614-A-SBA **Code**: EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Load Case: 1.0D + 1.0W 60 mph Wind

Dead Load Factor 1.00 Wind Load Factor 1.00



Page: 24

Iterations

20

Elev				qz	qzGh	С		lce Thick	Tributary	Aa	CfAa		Dead Load Ice	Tot Dead Load
(ft)	Description	Kzt	Kz	(psf)	(psf)	(mph-ft)	Cf	(in)	(ft)	(sf)	(sf)	(lb)	(lb)	(lb)
0.00		1.00	0.85	7.442	8.19	283.01	0.650	0.000	0.00	0.000	0.00	0.0	0.0	0.0
5.00		1.00	0.85	7.442	8.19	277.29	0.650	0.000	5.00	25.322	16.46	134.7	0.0	1403.6
10.00		1.00	0.85	7.442	8.19	271.58	0.650	0.000	5.00	24.805	16.12	132.0	0.0	1374.8
15.00		1.00	0.85	7.442	8.19	265.86	0.650	0.000	5.00	24.289	15.79	129.2	0.0	1345.9
20.00		1.00	0.90	7.896	8.69	267.97	0.650	0.000	5.00	23.772	15.45	134.2	0.0	1317.1
25.00		1.00	0.95	8.276	9.10	268.31	0.650	0.000	5.00	23.256	15.12	137.6	0.0	1288.2
30.00		1.00	0.98	8.600	9.46	267.36	0.650	0.000	5.00	22.739	14.78	139.8	0.0	1259.4
35.00		1.00	1.01	8.883	9.77	265.49	0.650	0.000	5.00	22.222	14.44	141.2	0.0	1230.5
40.00		1.00	1.04	9.137	10.05	262.92	0.650	0.000	5.00	21.706	14.11	141.8	0.0	1201.7
45.00		1.00	1.07	9.366	10.30	259.79	0.650	0.000	5.00	21.189	13.77	141.9	0.0	1172.8
47.00 Bot	- Section 2	1.00	1.08	9.452	10.40	258.40	0.650	0.000	2.00	8.331	5.42	56.3	0.0	461.1
50.00		1.00	1.09	9.576	10.53	256.20	0.650	0.000	3.00	12.564	8.17	86.0	0.0	1378.3
53.25 Top	- Section 1	1.00	1.11	9.704	10.67	253.66	0.650	0.000	3.25	13.401	8.71	93.0	0.0	1469.7
55.00		1.00	1.12	9.770	10.75	256.93	0.650	0.000	1.75	7.125	4.63	49.8	0.0	394.3
60.00		1.00	1.14	9.951	10.95	252.68	0.650	0.000	5.00	20.010	13.01	142.4	0.0	1107.0
65.00		1.00	1.16	10.120	11.13	248.16	0.650	0.000	5.00	19.493	12.67	141.0	0.0	1078.1
70.00		1.00	1.17	10.279	11.31	243.38	0.650	0.000	5.00	18.977	12.33	139.5	0.0	1049.3
75.00		1.00	1.19	10.430	11.47	238.39	0.650	0.000	5.00	18.460	12.00	137.7	0.0	1020.4
80.00		1.00	1.21	10.572	11.63	233.21	0.650	0.000	5.00	17.943	11.66	135.6	0.0	991.6
85.00		1.00	1.22	10.708	11.78	227.84	0.650	0.000	5.00	17.427	11.33	133.4	0.0	962.8
90.00		1.00	1.24	10.838	11.92	222.32	0.650	0.000	5.00	16.910	10.99	131.0	0.0	933.9
95.00		1.00	1.25	10.962	12.06	216.65	0.650	0.000	5.00	16.394	10.66	128.5	0.0	905.1
95.25 Bot	- Section 3	1.00	1.25	10.968	12.06	216.37	0.650	0.000	0.25	0.806	0.52	6.3	0.0	44.5
100.00		1.00	1.27	11.081	12.19	210.85	0.650	0.000	4.75	15.372	9.99	121.8		1560.3
	- Section 2	1.00		11.092	12.20	210.27	0.650	0.000	0.50	1.591	1.03	12.6		161.4
105.00		1.00	1.28	11.195	12.31	209.23	0.650	0.000	4.50	14.087	9.16	112.8	0.0	667.4
110.00		1.00	1.29	11.305	12.44	203.22		0.000	5.00	15.161	9.85	122.6		718.0
115.00		1.00		11.412	12.55	197.09	0.650	0.000		14.645	9.52	119.5		693.3
120.00		1.00		11.514	12.67	190.87	0.650	0.000		14.128	9.18	116.3		668.6
125.00		1.00		11.614	12.78	184.55	0.650	0.000		13.611	8.85	113.0		643.9
	ourtenance(s)	1.00		11.653	12.82	182.00	0.650	0.000	2.00	5.300	3.44	44.2		250.6
130.00		1.00		11.710	12.88	178.15		0.000	3.00	7.795	5.07	65.3		368.5
135.00		1.00		11.803	12.98	171.66	0.650	0.000		12.578	8.18	106.2		594.4
	ourtenance(s)	1.00		11.849	13.03	168.38	0.650	0.000	2.50	6.095	3.96	51.6		287.9
140.00		1.00		11.894	13.08	165.09	0.650	0.000	2.50	5.966	3.88	50.7		281.8
145.00		1.00		11.982	13.18	158.45		0.000		11.545	7.50	98.9		545.0
	ourtenance(s)	1.00		12.017	13.22	155.77	0.650	0.000	2.00	4.473	2.91	38.4		211.1
149.00 App		1.00		12.051	13.26	153.77	0.650	0.000	2.00	4.391	2.85	37.8		207.1
		1.00	1.00		10.20	100.00		Totals:	149.00	-	2.00	3.824.7		31,249.4

#### **Discrete Appurtenance Forces**

**Structure:** CT13614-A-SBA **Code:** EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: II Page: 25



Load Case: 1.0D + 1.0W 60 mph Wind

Dead Load Factor 1.00 Wind Load Factor 1.00



Iterations 20

						Orient		Total	Dead	Horiz	Vert	Wind	Mom	Mom
N.	Elev	Danamintian	<b>0</b> 4	qz (maf)	qzGh	Factor	<b>1</b> /-	CaAa	Load	Ecc	Ecc	FX	Y (Ib. <del>fr</del> )	Z
No.	(ft)	Description	Qty	(psf)	(psf)	х Ка	Ka	(sf)	(lb)	(ft)	(ft)	(lb)	(lb-ft)	(lb-ft)
1	147.00 44	49	3	12.017	13.219	0.50	0.75	2.49	210.00	0.000	0.000	32.88	0.00	0.00
2	147.00 KF	RY 112 89/4	6	12.017	13.219	0.50	0.75	1.96	92.40	0.000	0.000	25.91	0.00	0.00
3	147.00 HF	RK12-HD	1	12.017	13.219	1.00	1.00	9.75	406.61	0.000	0.000	128.88	0.00	0.00
4	147.00 PF	RK-1245 (kicker kit)	1	12.017	13.219	1.00	1.00	9.50	464.91	0.000	0.000	125.58	0.00	0.00
5	147.00 Lo	w Profile	1	12.017	13.219	1.00	1.00	22.00	1500.00	0.000	0.000	290.81	0.00	0.00
6	147.00 AF	PXVAARR24 43-U-NA2	3	12.017	13.219	0.52	0.75	31.88	384.00	0.000	0.000	421.38	0.00	0.00
7	147.00 AF	PXV18-209014-C-A20	6	12.017	13.219	0.72	0.75	15.25	112.20	0.000	0.000	201.58	0.00	0.00
8	147.00 78	32 10662	3	12.017	13.219	0.50	0.75	0.42	7.80	0.000	0.000	5.58	0.00	0.00
9	137.50 Lo	w Profile Platform-flat	1	11.849	13.034	1.00	1.00	25.00	1200.00	0.000	0.000	325.85	0.00	0.00
10	137.50 DC	C2-48-60-8-18F-02	1	11.849	13.034	1.00	1.00	2.92	14.50	0.000	0.000	38.06	0.00	0.00
11	137.50 RF	RUS-11	6	11.849	13.034	0.54	0.80	8.10	306.00	0.000	0.000	105.63	0.00	0.00
12	137.50 LG	GP21903	6	11.849	13.034	0.54	0.80	0.87	33.00	0.000	0.000	11.32	0.00	0.00
13	137.50 LG	GP21401	6	11.849	13.034	0.54	0.80	4.15	84.60	0.000	0.000	54.07	0.00	0.00
14	137.50 AN	M-X-CD-17-65-00T-RET	3	11.849	13.034	0.64	0.80	21.72	178.50	0.000	0.000	283.04	0.00	0.00
15	137.50 77		6	11.849	13.034	0.58	0.80	19.27	210.00	0.000	0.000	251.19	0.00	0.00
16	127.00 Lo	w Profile	1	11.653	12.818	1.00	1.00	22.00	1500.00	0.000	0.000	281.99	0.00	0.00
17		PA-185080/8CF	6	11.653	12.818	0.98	0.80	12.24	42.00	0.000	0.000	156.88	0.00	0.00
18	127.00 LP	PA-80080/4CF	6	11.653	12.818	1.36	0.80	21.30	72.00	0.000	0.000	272.99	0.00	0.00
		•												

Totals: 6,818.52 3,013.62

# **Total Applied Force Summary**

**Structure**: CT13614-A-SBA **Code**: EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



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**Iterations** 

Page: 26

20

Load Case: 1.0D + 1.0W 60 mph Wind

Dead Load Factor 1.00 Wind Load Factor 1.00

Elev		Lateral FX (-)	Axial FY (-)	Torsion MY	Moment MZ
(ft)	Description	(lb)	(lb)	(lb-ft)	(lb-ft)
0.00		0.00	0.00	0.00	0.00
5.00		134.74	1624.31	0.00	0.00
10.00		131.99	1595.47	0.00	0.00
15.00		129.24	1566.62	0.00	0.00
20.00		134.21	1537.78	0.00	0.00
25.00		137.61	1508.93	0.00	0.00
30.00		139.82	1480.08	0.00	0.00
35.00		141.15	1451.24	0.00	0.00
40.00		141.80	1422.39	0.00	0.00
45.00		141.90	1393.55	0.00	0.00
47.00		56.30	549.34	0.00	0.00
50.00		86.02	1510.70	0.00	0.00
53.25		92.98	1613.16	0.00	0.00
55.00		49.78	471.50	0.00	0.00
60.00		142.37	1327.68	0.00	0.00
65.00		141.05	1298.84	0.00	0.00
70.00		139.47	1269.99	0.00	0.00
75.00		137.66	1241.15	0.00	0.00
80.00		135.64	1212.30	0.00	0.00
85.00		133.42	1183.46	0.00	0.00
90.00		131.04	1154.61	0.00	0.00
95.00		128.49	1125.76	0.00	0.00
95.25		6.32	55.53	0.00	0.00
100.00		121.79	1769.92	0.00	0.00
100.50		12.62	183.50	0.00	0.00
105.00		112.76	866.01	0.00	0.00
110.00		122.55	938.75	0.00	0.00
115.00		119.49	914.02	0.00	0.00
120.00		116.31	889.30	0.00	0.00
125.00		113.03	864.57	0.00	0.00
	(12) attachments				0.00
127.00	(13) attachments	756.02	1952.91	0.00	
130.00		65.26	463.50	0.00	0.00
135.00	(00) # 1 1	106.15	752.72	0.00	0.00
137.50	(29) attachments	1120.80	2393.69	0.00	0.00
140.00		50.74	327.21	0.00	0.00
145.00		98.91	635.87	0.00	0.00
147.00	(24) attachments	1271.03	3425.35	0.00	0.00
149.00		37.83	207.11	0.00	0.00
	Totals:	6,838.30	44,178.84	0.00	0.00

#### **Calculated Forces**

**Structure:** CT13614-A-SBA Code: EIA/TIA-222-G 9/6/2019

Site Name: Knowlton Exposure: С Height: 149.00 (ft) Crest Height: 0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: ||



Page: 27

**Iterations** 

20

Load Case: 1.0D + 1.0W 60 mph Wind

**Dead Load Factor** 1.00 **Wind Load Factor** 1.00

Seg Elev	Pu FY (-)	Vu FX (-)	Tu MY (-)	Mu MZ	Mu MX	Resultant Moment	phi Pn	phi Vn	phi Tn	phi Mn	Total Deflect	Rotation Sway	Rotation Twist	Stress
(ft)	(kips)	٠,	(ft-kips)		(ft-kips)	(ft-kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(in)	(deg)	(deg)	Ratio
0.00	-44.18	-6.85	0.00	-714.53	0.00	714.53	5580.79	2790.40	13771.9	6896.19	0.00	0.000	0.000	0.112
5.00	-42.55	-6.73	0.00	-680.29	0.00	680.29	5509.80	2754.90	13318.1	6668.96	0.01	-0.026	0.000	0.110
10.00	-40.95	-6.62	0.00	-646.63	0.00	646.63	5437.04	2718.52	12867.3	6443.24	0.06	-0.053	0.000	0.108
15.00	-39.38	-6.51	0.00	-613.54	0.00	613.54	5362.51	2681.26	12419.8	6219.15	0.13	-0.079	0.000	0.106
20.00	-37.84	-6.39	0.00	-581.02	0.00	581.02	5286.22	2643.11	11975.8	5996.84	0.22	-0.106	0.000	0.104
25.00	-36.33	-6.26	0.00	-549.08	0.00	549.08	5208.16	2604.08	11535.7	5776.42	0.35	-0.134	0.000	0.102
30.00	-34.85	-6.14	0.00	-517.77	0.00	517.77	5128.34	2564.17	11099.6	5558.05	0.50	-0.161	0.000	0.100
35.00	-33.39	-6.01	0.00	-487.08	0.00	487.08	5046.75	2523.38	10667.8	5341.85	0.69	-0.189	0.000	0.098
40.00	-31.97	-5.88	0.00	-457.05	0.00	457.05	4963.39	2481.70	10240.6	5127.96	0.90	-0.217	0.000	0.096
45.00	-30.57	-5.74	0.00	-427.67	0.00	427.67	4878.27	2439.14	9818.41	4916.50	1.14	-0.246	0.000	0.093
47.00	-30.02	-5.69	0.00	-416.19	0.00	416.19	4843.73	2421.86	9650.93	4832.64	1.25	-0.257	0.000	0.092
50.00	-28.51	-5.60	0.00	-399.12	0.00	399.12	4791.38	2395.69	9401.28	4707.63	1.42	-0.274	0.000	0.091
53.25	-26.90	-5.51	0.00	-380.91	0.00	380.91	4797.23	2398.61	9428.91	4721.46	1.61	-0.293	0.000	0.086
55.00	-26.42	-5.47	0.00	-371.27	0.00	371.27	4766.44	2383.22	9284.05	4648.93	1.72	-0.304	0.000	0.085
60.00	-25.09	-5.33	0.00	-343.94	0.00	343.94	4677.28	2338.64	8873.91	4443.55	2.05	-0.331	0.000	0.083
65.00	-23.79	-5.19	0.00	-317.29	0.00	317.29	4586.36	2293.18	8469.53	4241.06	2.41	-0.358	0.000	0.080
70.00	-22.52	-5.05	0.00	-291.34	0.00	291.34	4493.67	2246.84	8071.16	4041.58	2.80	-0.385	0.000	0.077
75.00	-21.28	-4.92	0.00	-266.07	0.00	266.07	4393.03	2196.52	7668.29	3839.84	3.22	-0.412	0.000	0.074
80.00	-20.07	-4.78	0.00	-241.48	0.00	241.48	4267.07	2133.53	7232.68	3621.71	3.67	-0.438	0.000	0.071
85.00	-18.88	-4.65	0.00	-217.57	0.00	217.57	4141.11	2070.55	6809.81	3409.96	4.14	-0.465	0.000	0.068
90.00	-17.73	-4.52	0.00	-194.33	0.00	194.33	4015.15	2007.57	6399.67	3204.59	4.64	-0.490	0.000	0.065
95.00	-16.60	-4.38	0.00	-171.76	0.00	171.76	3889.19	1944.59	6002.28	3005.60	5.17	-0.516	0.000	0.061
95.25	-16.54	-4.38	0.00	-170.66	0.00	170.66	3882.89	1941.44	5982.74	2995.82	5.19	-0.517	0.000	0.061
100.00	-14.78	-4.24	0.00	-149.87	0.00	149.87	3763.23	1881.61	5617.62	2812.98	5.72	-0.540	0.000	0.057
100.50	-14.59	-4.23	0.00	-147.75	0.00	147.75	3276.58	1638.29	4993.13	2500.28	5.78	-0.543	0.000	0.064
105.00	-13.72	-4.12	0.00	-128.70	0.00	128.70	3189.50	1594.75	4715.31	2361.16	6.30	-0.564	0.000	0.059
110.00	-12.79	-3.99	0.00	-108.13	0.00	108.13	3081.54	1540.77	4399.90	2203.22	6.90	-0.587	0.000	0.053
115.00	-11.87	-3.86	0.00	-88.18	0.00	88.18	2973.57	1486.79	4095.40	2050.74	7.53	-0.609	0.000	0.047
120.00	-10.98	-3.74	0.00	-68.86	0.00	68.86	2865.60	1432.80	3801.82	1903.74	8.18	-0.629	0.000	0.040
125.00	-10.12	-3.62	0.00	-50.15	0.00	50.15	2757.64	1378.82	3519.16	1762.20	8.85	-0.645	0.000	0.032
127.00	-8.17	-2.84	0.00	-42.91	0.00	42.91	2714.45	1357.23	3409.16	1707.11	9.12	-0.651	0.000	0.028
130.00	-7.71	-2.77	0.00	-34.38	0.00	34.38	2649.67	1324.84	3247.42	1626.12	9.53	-0.659	0.000	0.024
135.00	-6.96	-2.66	0.00	-20.50	0.00	20.50	2541.71	1270.85	2986.60	1495.52	10.23	-0.668	0.000	0.016
137.50	-4.58	-1.51	0.00	-13.85	0.00	13.85	2487.72	1243.86	2860.28	1432.27	10.58	-0.672	0.000	0.012
140.00	-4.25	-1.46	0.00	-10.07	0.00	10.07	2433.74	1216.87	2736.70	1370.38	10.93	-0.674	0.000	0.009
145.00	-3.62	-1.35	0.00	-2.78	0.00	2.78	2325.77	1162.89	2497.71	1250.71	11.64	-0.677	0.000	0.004
147.00	-0.21	-0.04	0.00	-0.08	0.00	0.08	2282.59	1141.29	2405.17	1204.38	11.92	-0.678	0.000	0.000
149.00	0.00	-0.04	0.00	0.00	0.00	0.00	2239.40	1119.70	2314.38	1158.91	12.21	-0.678	0.000	0.000

### **Final Analysis Summary**

**Structure**: CT13614-A-SBA **Code**: EIA/TIA-222-G 9/6/2019

Site Name:KnowltonExposure:CHeight:149.00 (ft)Crest Height:0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: II Page: 28



#### **Reactions**

	Shear FX	Shear FZ	Axial FY	Moment MX	Moment MY	Moment MZ
Load Case	(kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(ft-kips)
1.2D + 1.6W 101 mph Wind	31.1	0.00	52.98	0.00	0.00	3254.12
0.9D + 1.6W 101 mph Wind	31.0	0.00	39.73	0.00	0.00	3230.67
1.2D + 1.0Di + 1.0Wi 50 mph Wind	8.9	0.00	83.19	0.00	0.00	933.78
1.2D + 1.0E	2.0	0.00	53.01	0.00	0.00	236.96
0.9D + 1.0E	2.0	0.00	39.76	0.00	0.00	235.13
1.0D + 1.0W 60 mph Wind	6.8	0.00	44.18	0.00	0.00	714.53

#### **Max Stresses**

	Pu FY (-)	Vu FX (-)	Tu MY (-)	Mu MZ	Mu MX	Resultant Moment	phi Pn	phi Vn	phi Tn	phi Mn	Elev	Stress
Load Case	(kips)	(kips)	(ft-kips)	(ft-kips)	(ft-kips)	(ft-kips)	(kips)	(kips)	(ft-kips)	(ft-kips)	(ft)	Ratio
1.2D + 1.6W 101 mph Wind	-52.98	-31.06	0.00	-3254.1	0.00	-3254.1	5580.79	2790.4	13771.9	6896.19	0.00	0.481
0.9D + 1.6W 101 mph Wind	-39.73	-31.04	0.00	-3230.6	0.00	-3230.6	5580.79	2790.4	13771.9	6896.19	0.00	0.476
1.2D + 1.0Di + 1.0Wi 50 mph Wind	-83.19	-8.91	0.00	-933.78	0.00	-933.78	5580.79	2790.4	13771.9	6896.19	0.00	0.150
1.2D + 1.0E	-53.01	-2.00	0.00	-236.96	0.00	-236.96	5580.79	2790.4	13771.9	6896.19	0.00	0.044
0.9D + 1.0E	-39.76	-2.00	0.00	-235.13	0.00	-235.13	5580.79	2790.4	13771.9	6896.19	0.00	0.041
1.0D + 1.0W 60 mph Wind	-44.18	-6.85	0.00	-714.53	0.00	-714.53	5580.79	2790.4	13771.9	6896.19	0.00	0.112

#### **Base Plate Summary**

Structure: Code: EIA/TIA-222-G 9/6/2019 CT13614-A-SB

Site Name: Knowlton С **Exposure:** Height: 149.00 (ft) Crest Height: 0.00

Base Elev: 0.000 (ft) Site Class: D - Stiff Soil

Gh: 1.1 Topography: 1 Struct Class: || Page: 29



Reaction	S	Base Pla	ate	Anchor Bolts		
Original Des	sign	Yield (ksi):	60.00	<b>Bolt Circle:</b>	68.00	
Moment (kip-ft):	3739.40	Width (in):	68.00	Number Bolts:	20.00	
Axial (kip):	55.50	Style:	Clipped	Bolt Type:	2.25" 18J	
Shear (kip):	28.20	Polygon Sides:	4.00	Bolt Diameter (in):	2.25	
Analygia		Clip Length (in):	14.00	Yield (ksi):	75.00	
Analysis  Moment (kip-ft):	3254.12	Effective Len (in):	9.47	Ultimate (ksi):	100.00	
Axial (kip):	83.19	Moment (kip-in):	448.67	Arrangement:	Clustered	
Shear (kip):	31.06	Allow Stress (ksi):	81.00	Cluster Dist (in):	6.00	
Sileai (Kip).	31.00	Applied Stress (ksi):	0.00	Start Angle (deg):	45.00	
Moment Design %:	87.02	Stress Ratio:	0.39	Compres	ssion	
3				Force (kip):	119.01	

Force (kip): Allowable (kip): 260.00 0.47

Ratio:

Tension Force (kip): 110.69 Allowable (kip): 260.00

> Ratio: 0.44



Mono	pole Mat Foundation	Docian	Date
IVIOITO	pole Mat Foundation	Design	9/6/2019
Customer Name:	T-Mobile	EIA/TIA Standard:	EIA-222-G
Site Name:		Structure Height (Ft.):	149
Site Number:	CT13614-A-SBA	Engineer Name:	L. Klem
Engr. Number:	80544	Engineer Login ID:	

Foundation Info Obtained from:	Drawings/Calculations					
Structure Type:	Monopole					
Analysis or Design?	Analysis				1.00	0.00
Base Reactions (Factored):					*	
Axial Load (Kips):	53.0	Shear Force (Kips):	31.1			7 # 4
Uplift Force (Kips):	0.0	Moment (Kips-ft):	3254.1			99.0 47 # 10
Allowable overstress %: 5.0%						47 # 10
Foundation Geometries:					5.5	//47 # 10
		Mods required -Yes/No ?:	No			<u> </u>
Diameter of Pier (ft.):	8.0	Depth of Base BG (ft.):	5.5			• • • • • • • • • • • • • • • • • • •
Pier Height A. G. (ft.):	1.00	Thickness of Pad (ft):	2.00			2.00
Length of Pad (ft.):	27.5	Width of Pad (ft.):	27.5		<u> </u>	
		. ,				27.5
Final Length of pad (ft)	27.5	Final width of pad (ft):	27.5		$\overline{\uparrow}$	0.0
,		. , ,				
Material Properties and Reabr Info	<u>:</u>					8.0
Concrete Strength (psi):	4000	Steel Elastic Modulus:	29000	ksi		
Vertical bar yield (ksi)	60	Tie steel yield (ksi):	60			27.5
Vertical Rebar Size #:	9	Tie / Stirrup Size #:	4		27.5	w
Qty. of Vertical Rebars:	38	Tie Spacing (in):	12.0			
Pad Rebar Yield (Ksi):	60	Pad Steel Rebar Size (#):	10			38 # 9
Concrete Cover (in.):	3	Unit Weight of Concrete:	150.0	pcf		
Rebar at the bottom of the concrete	pad:					0.0
Qty. of Rebar in Pad (L):	47	Qty. of Rebar in Pad (W):	47		<u> </u>	0.0
Rebar at the top of the concrete page	d:					27.5 L
Oty. of Rebar in Pad (L):	47	Qty. of Rebar in Pad (W):	47			
	1.35					
Soil Design Parameters:						
Soil Unit Weight (pcf):	120.0	Soil Buoyant Weight:	50.0	Pct	F	
Water Table B.G.S. (ft):	99.0	Unit Weight of Water:	62.4	pct	f Angle	from Top of Pad: 30
Ultimate Bearing Pressure (psf):	21000	Ultimate Skin Friction:	425	Psf	_	from Bottm of Pad: 25
Consider Friction for O.T.M. (Y/N):	No	Consider Friction for bearing		Yes	U	from Bottm of Pad: 25
Consider soil hor. resist. for OTM.:	Yes	Reduction factor on the m	axımum soil	bearın	g pressur	re: 1.00
Foundation Analysis and Design:	Unlift Str	ength Reduction Factor:	0.75	Com	nression	Strength Reduction Factor: 0.75
Total Dry Soil Volume (cu. Ft.):	орсос.	engar neadation raction			•	Weight (Kips): 296.51
Total Buoyant Soil Volume (cu. I			0.00		-	t Soil Weight (Kips): 0.00
Total Effective Soil Weight (Kips):		296.51	Weig	ght from	the Concrete Block at Top (K): 0.00	
Total Dry Concrete Volume (cu. Ft.):		1738.69		•	ncrete Weight (Kips): 260.80	
·	Total Buoyant Concrete Volume (cu. Ft.):		0.00		•	t Concrete Weight (Kips): 0.00
Total Effective Concrete Weight	(Kips):		260.80	Tota	I Vertical	Load on Base (Kips): 610.30
Check Soil Capacities:						Capacity Ratio
Calculated Maxium Net Soil Pressur		· · ·	1546	<		able Factored Soil Bearing (psf): 15750 0.10 OK!
Allowable Foundation Overturning I			7625.3	>		n Factored Momont (kips-ft): 3354 0.44 OK!
Factor of Safety Against Overturning (O. R. Moment/Design Moment):			2.27	OK	!	

TES Engr. Number: 80544 9/6/2019 Page 2/2 Date:

Check the capacities of Reinforceing Concrete:						
Strength reduction factor (Flexure and axial tension):		Strength reduction factor (Shear):		0.75		
Strength reduction factor (Axial compresion):	0.65	Wind	Load Factor on Concrete Design:	1.00		
					Load/ Capacity	
(1) Concrete Pier:					Ratio	
Vertical Steel Rebar Area (sq. in./each):	1.00		Tie / Stirrup Area (sq. in./each):	0.20		
Calculated Moment Capacity (Mn,Kips-Ft):	7455.7	>	Design Factored Moment (Mu, Kips-F	3394.1	0.46	OK!
Calculated Shear Capacity (Kips):	840.3	>	Design Factored Shear (Kips):	31.1	0.04	OK!
Calculated Tension Capacity (Tn, Kips):	2052.0	>	Design Factored Tension (Tu Kips):	0.0	0.00	OK!
Calculated Compression Capacity (Pn, Kips):	12730.0	>	Design Factored Axial Load (Pu Kips):	53.0	0.00	OK!
Moment & Axial Strength Combination:	0.46	OK!	Check Tie Spacing (Design/Required):		1	OK!
Pier Reinforcement Ratio:	0.005		Reinforcement Ratio is satisfied per A	CI		
(2).Concrete Pad:						
One-Way Design Shear Capacity (L-Direction, Kips):	637.9	>	One-Way Factored Shear (L-D. Kips):	221.6	0.35	OK!
One-Way Design Shear Capacity (W-Direction, Kips):	637.9	>	One-Way Factored Shear (W-D., Kips)	221.6	0.35	OK!
One-Way Design Shear Capacity (Corner-Corner. Kips):	637.8	>	One-Way Factored Shear (C-C, Kips):	213.1	0.33	OK!
Lower Steel Pad Reinforcement Ratio (L-Direct. ):	0.0089	OK!	Lower Steel Pad Reinf. Ratio (W-Direc	0.0089		
Lower Steel Pad Moment Capacity (L-Direction. Kips-ft):	5044.1	>	Moment at Bottom (L-Dir. K-Ft):	1204.4	0.24	OK!
Lower Steel Pad Moment Capacity (W-Direction. Kips-ft):	5044.1	>	Moment at Bottom ( W-Dir. K-Ft):	1204.4	0.24	OK!
Lower Steel Pad Moment Capacity (Corner-Corner, K-ft):	6976.5	>	Moment at Bottom ( C-C Dir. K-Ft):	1703.3	0.24	OK!
Upper Steel Pad Reinforcement Ratio (L-Direct. ):	0.0089	OK!	Upper Steel Reinf. Ratio (W-Dir. ):	0.0089		
Upper Steel Pad Moment Capacity (L-Direc. Kips-ft):	5044.1	>	Moment at the top (L-Dir K-Ft):	496.8	0.10	OK!
Upper Steel Pad Moment Capacity (W-Direc. Kips-ft):	5044.1	>	Moment at the top (W-Dir K-Ft):	496.8	0.10	OK!
Upper Steel Pad Moment Capacity (Corner-Corner. K-ft):	6976.5	>	Moment at the top (C-C Dir. K-Ft):	466.8	0.07	OK!
(3). Check Punching Shear Capacity due to Moment in the Pier:						
Moment transferred by punching shear:	1301.6	k-ft.	Max. factored shear stress v <sub>u CD</sub> :		0.5	Psi
Max. factored shear stress $v_{IIAB}$ :	12.6	Psi	Factored shear Strength φv <sub>n</sub> :		189.7	Psi
20			<b>5</b>			

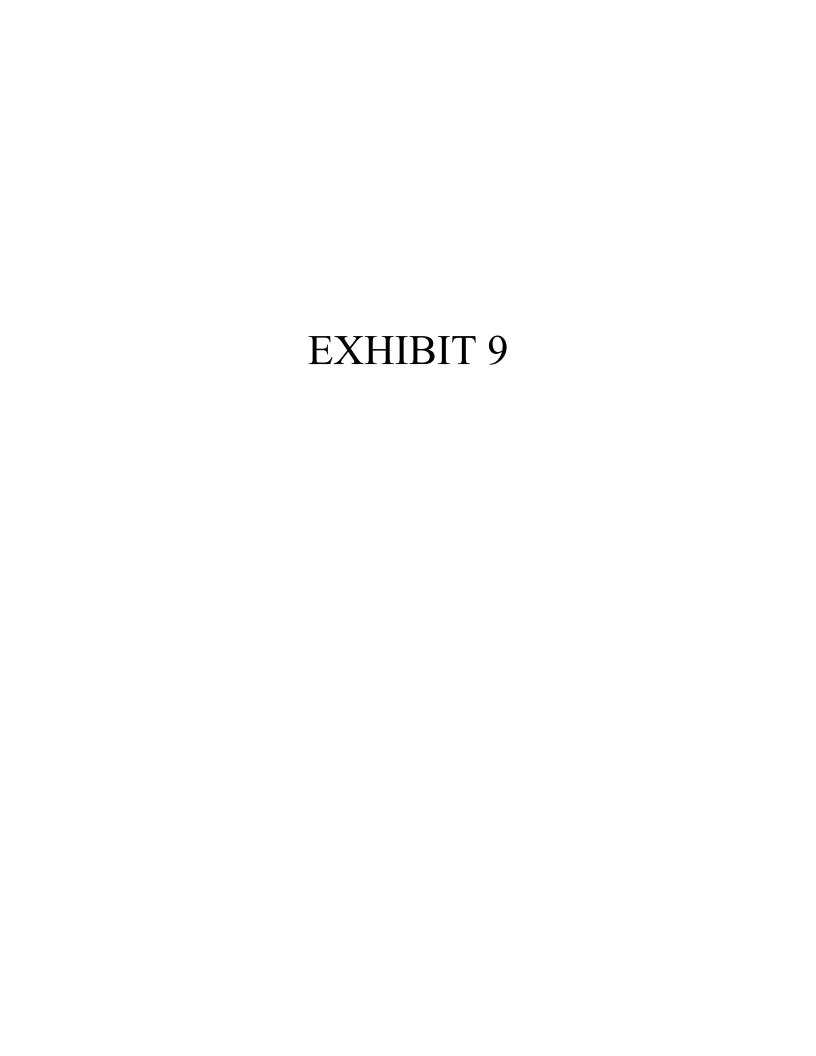
12.6

Psi

Check Usage of Punching Shear Capacity:

0.07 OK!

Max. factored shear stress  $v_u$ :



Wireless Network Design and Deployment

# Radio Frequency Emissions Analysis Report

**T-MOBILE** Existing Facility

**Site ID: CT11519D** 

CT519/TVI Ashford - Prime 99 Knowlton Hill Rd Ashford, CT 06278

June 10, 2019

Transcom Engineering Project Number: 737001-0084

Site Compliance Summary					
Compliance Status:	COMPLIANT				
Site total MPE% of FCC general population allowable limit:	5.54 %				

Wireless Network Design and Deployment

June 10, 2019

T-MOBILE Attn: Jason Overbey, RF Manager 35 Griffin Road South Bloomfield, CT 6009

Emissions Analysis for Site: CT11519D – CT519/TVI Ashford - Prime

Transcom Engineering, Inc ("Transcom") was directed to analyze the proposed upgrades to the T-MOBILE facility located at **99 Knowlton Hill Rd**, **Ashford**, **CT**, for the purpose of determining whether the emissions from the Proposed T-MOBILE Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu$ W/cm2). The number of  $\mu$ W/cm² calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) - (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu$ W/cm²). The general population exposure limits for the 600 MHz & 700 MHz bands are approximately 400  $\mu$ W/cm² and 467  $\mu$ W/cm² respectively. The general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is 1000  $\mu$ W/cm². Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.

Wireless Network Design and Deployment

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

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#### **CALCULATIONS**

Calculations were performed for the proposed upgrades to the T-MOBILE antenna facility located at **99 Knowlton Hill Rd**, **Ashford**, **CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-MOBILE is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. All power values expressed and analyzed are maximum power levels expected to be used on all radios.

All emissions values for additional carriers were taken from the Connecticut Siting Council (CSC) active MPE database. Values in this database are provided by the individual carriers themselves

For each sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
LTE	1900 MHz (PCS)	4	40
GSM	1900 MHz (PCS)	1	15
LTE / 5G NR	600 MHz	2	40
LTE	700 MHz	2	20

Table 1: Channel Data Table

Wireless Network Design and Deployment

The following antennas listed in *Table 2* were used in the modeling for transmission in the 600 MHz, 700 MHz, 1900 MHz (PCS) and 2100 MHz (AWS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.

			Antenna
	Antenna		Centerline
Sector	Number	Antenna Make / Model	(ft)
A	1	RFS APXV18-209014-C-A20	167
A	2	RFS APXVAARR24_43-U-NA20	167
A	3	RFS APXV18-209014-C-A20 (Dormant)	167
В	1	RFS APXV18-209014-C-A20	167
В	2	RFS APXVAARR24_43-U-NA20	167
В	3	RFS APXV18-209014-C-A20 (Dormant)	167
C	1	RFS APXV18-209014-C-A20	167
С	2	RFS APXVAARR24_43-U-NA20	167
С	3	RFS APXV18-209014-C-A20 (Dormant)	167

Table 2: Antenna Data

All calculations were done with respect to uncontrolled / general population threshold limits.

Cable losses were factored in the calculations for this site. Since all 1900 MHz (PCS) radios are ground mounted the following cable loss values were used. For each ground mounted 1900 MHz (PCS) radio there was 2.06 dB of cable loss calculated into the system gains / losses for this site. These values were calculated based upon the manufacturers specifications for 200 feet of 1-5/8" coax.

Wireless Network Design and Deployment

#### **RESULTS**

Per the calculations completed for the proposed T-MOBILE configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

A			A	Channel	Total TX Power		
Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBd)	Count	(W)	ERP (W)	MPE %
Antenna	RFS	1 requeriey Bands	Gain (dDd)	Count	(**)	ERI (W)	WII L /0
A1	APXV18-209014-C-A20	1900 MHz (PCS)	14.35	5	175	2,965.09	0.42
Antenna	RFS						
A2	APXVAARR24_43-U-NA20	600 MHz / 700 MHz	12.95 / 13.35	4	120	2,443.03	0.81
Antenna	RFS						
A3	APXV18-209014-C-A20	Dormant	N/A	0	0	0.00	0.00
				Sec	tor A Comp	osite MPE%	1.23
Antenna	RFS						
B1	APXV18-209014-C-A20	1900 MHz (PCS)	14.35	5	175	2,965.09	0.42
Antenna	RFS						
B2	APXVAARR24_43-U-NA20	600 MHz / 700 MHz	12.95 / 13.35	4	120	2,443.03	0.81
Antenna	RFS						
В3	APXV18-209014-C-A20	Dormant	N/A	0	0	0.00	0.00
				Sec	tor B Comp	osite MPE%	1.23
Antenna	RFS						
C1	APXV18-209014-C-A20	1900 MHz (PCS)	14.35	5	175	2,965.09	0.42
Antenna	RFS						
C2	APXVAARR24_43-U-NA20	600 MHz / 700 MHz	12.95 / 13.35	4	120	2,443.03	0.81
Antenna	RFS						
C3	APXV18-209014-C-A20	Dormant	N/A	0	0	0.00	0.00
Sector C Composite MPE%							1.23

Table 3: T-MOBILE Emissions Levels

Wireless Network Design and Deployment

The Following table (*table 4*) shows all additional carriers on site and their MPE% as recorded in the CSC active MPE database for this facility along with the newly calculated maximum T-MOBILE MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, all three sectors have the same configuration yielding the same results on all three sectors. *Table 5* below shows a summary for each T-MOBILE Sector as well as the composite MPE value for the site.

Site Composite MPE%					
Carrier	MPE%				
T-MOBILE – Max Per Sector Value	1.23 %				
AT&T	2.07 %				
Verizon Wireless	2.24 %				
Site Total MPE %:	5.54 %				

Table 4: All Carrier MPE Contributions

T-MOBILE Sector A Total:	1.23 %
T-MOBILE Sector B Total:	1.23 %
T-MOBILE Sector C Total:	1.23 %
Site Total:	5.54 %

Table 5: Site MPE Summary

Wireless Network Design and Deployment

FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. *Table 6* below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated T-MOBILE sector(s). For this site, all three sectors have the same configuration yielding the same results on all three sectors.

T-MOBILE _ Frequency Band / Technology Max Power Values (Per Sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density (µW/cm²)	Frequency (MHz)	Allowable MPE (µW/cm²)	Calculated % MPE
T-Mobile 1900 MHz (PCS) LTE	4	677.74	167	3.76	1900 MHz (PCS)	1000	0.38%
T-Mobile 1900 MHz (PCS) GSM	1	254.15	167	0.35	1900 MHz (PCS)	1000	0.04%
T-Mobile 600 MHz LTE / 5G NR	2	788.97	167	2.19	600 MHz	400	0.55%
T-Mobile 700 MHz LTE	2	432.54	167	1.20	700 MHz	467	0.26%
						Total:	1.23%

Table 6: T-MOBILE Maximum Sector MPE Power Values

Wireless Network Design and Deployment

#### **Summary**

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-MOBILE facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-MOBILE Sector	Power Density Value (%)			
Sector A:	1.23 %			
Sector B:	1.23 %			
Sector C:	1.23 %			
T-MOBILE Maximum	1 22 0/			
Total (per sector):	1.23 %			
Site Total:	5.54 %			
Site Compliance Status:	COMPLIANT			

The anticipated composite MPE value for this site assuming all carriers present is **5.54** % of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan

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