



Crown Castle  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

January 10, 2024

Melanie A. Bachman  
Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

RE: Notice of Exempt Modification for T-Mobile: CT11353C  
Crown Site ID# 876345  
33 Janoski Road, Ashford, CT 06238  
Latitude: 41° 57' 7.70" / Longitude: -72° 11' 43.90"

Dear Ms. Bachman:

T-Mobile currently maintains six (6) antennas at the 158-foot mount on the existing 192-foot monopole tower located at 33 Janoski Road, Ashford, CT. The property is owned by Martin Carolyn M L/U, and the tower is owned by Crown Castle. T-Mobile now intends to replace three (3) antennas, three (3) remote radios and ancillary equipment at the 158ft level. This modification/proposal includes hardware that is both 4G (LTE) and 5G capable through remote software configuration and either or both services may be turned on or off at various times.

**Panned Modification:**

**Tower:**

Install New:

- (3) Ericsson – Air 6419 B41
- (3) Ericsson – 4460 B25+B66 Radios
- (3) RF Cellwave – HB158-21U6S24-xxM - Hybrid Cables

Remove:

- (3) RFS – APX16DWV-16DWV-S-E-A20 Antennas
- (3) Ericsson – 4415 B25 Radios
- (3) Ericsson – 4415 B66A Radios
- (3) Ericsson – AIR 6419 B41 Antenna
- (3) HCS 6x12 Hybrid Cables

**Ground:**

Install New:

- (1) Ericsson – 6160 AC V1 Enclosure
- (2) (1) Ericsson- B160 Enclosure

The Foundation for a Wireless World.  
CrownCastle.com

Melanie A. Bachman

Page 2

Remove:

- (1) RBS-6102 MUAC Enclosure
- (1.) Batter Back up Unit

The facility was approved by the Town of Ashford Planning and Zoning Commission on November 12, 1996. The approval was given with conditions which this exempt modification follows.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies §16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.C.S.A. § 16-50j-73, a copy of this letter is being sent to William A. Falletti, First Selectman, Town of Ashford, Michael D'Amato, ZEO, Town of Ashford. Martin Carolyn M L/U, Property Owner and Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.
5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, T-Mobile respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Sincerely,



Jeffrey Barbadora

Site Acquisition Specialist

1800 W. Park Drive

Westborough, MA 01581

(781) 970-0053

Jeff.Barbadora@crowncastle.com

The Foundation for a Wireless World.

CrownCastle.com

Melanie A. Bachman

Page 3

Attachments

cc:

William A. Falletti, First Selectman  
Town of Ashford  
5 Town Hall Road  
Ashford, CT 06278  
(860) 487-4400

Michael D'Amato, ZEO  
Town of Ashford  
5 Town Hall Road  
Ashford, CT 06278  
(860) 487-4415

Martin Carolyn M L/U – Property Owner  
33 Janoski Road  
Ashford, CT 06278

Crown Castle - Tower Owner



FILE SITE # 204

SKY HILL

ZONING

RECEIVED

11-13-96

## MINUTES - ASHFORD PLANNING AND ZONING COMMISSION

Annual Meeting - November 12, 1996

Members present: Organ, Lawrence, Nagy, Levaur, Rossman, McCarthy & White.

Alternates present: Bartok & Specyalski.

The meeting was called to order at 9:55 p.m. after the public hearing (Sprint Spectrum, tower & Moratorium, Lake Chaffee).

Specyalski is the voting alternate for this meeting.

At the Annual Town meeting, Alex Hastillo and Kevin McCarthy were elected to 4 year terms on the Commission ending in the year 2000 and Bartok was elected to a 3 year term as Alternate ending in 1999.

Moved and seconded to consider Old and New Business first. Passed without dissent.

The Commission considered the Sprint Spectrum application for a communications tower to be located on Sky Hill. There were no objections at tonight's public hearing. The tower will be able to hold three sets of antennas. Sprint Spectrum will operate a PCS digital system. It is regulated by the FCC. There will be no lights on the tower. Access will be off Frontage Road to Janowski Road to avoid the wetlands on the east end of Janowski Road. Moved and seconded to approve with conditions the application for a Special Exception under Section 5.2.3 by Sprint Spectrum L.P., Meriden, CT for a 200' communications tower to be located on land leased from David H. Martin off Janowski Road on Sky Hill.

The conditions are:

1. Utilities to the site which is approximately 2500' from Janowski Road will be located underground in the right of way.
2. Space and installation of fire, emergency and municipal communications equipment to meet present and future needs will be provided at no cost.
3. A copy of the liability insurance will be submitted to the Commission.
4. A site plan including driveway design and sedimentation and erosion control measures will be submitted to the Commission before the construction begins.
5. A copy of the lease will be part of the land records.

Motion passed without dissent.

The Commission considered the proposed Moratorium at Lake Chaffee. Tim Backus, Chairman of the Water Pollution Control Authority was the only person to speak at the public hearing. Moved and seconded to approve the following:



Moratorium at Lake Chaffee

WHEREAS, the Department of Environmental Protection has cited the Town of Ashford and the Lake Chaffee Improvement Association, Inc. to study and report upon potential pollution at Lake Chaffee resulting from construction around the lake; and

WHEREAS, the Department of Environmental Protection has found pollution in the tributaries leading to the lake, and

WHEREAS, there is a reasonable expectation that the recommendation of the study may be to limit new construction in that area, or as an alternative to require that homes in the area be connected to an alternative type of sewage disposal system, and

WHEREAS, this Commission does not want to allow any deterioration of the water in the lake or tributaries;

The Planning and Zoning Commission of the Town of Ashford, pursuant to the authority vested in it by Section 9-2 of Connecticut General Statutes, hereby amends the zoning regulations of the Town of Ashford by adoption of the following Moratorium:

"Until December 31, 1997, there shall be no new house construction allowed within the area of Lake Chaffee Improvement Association, Inc. nor any enclosed addition to any existing house in that area. The Zoning Enforcement Officer may not in that period certify that any new construction is in conformity with the zoning regulations of the town."

Motion passed without dissent.

The reasons for reinstating the moratorium include:

1. There is need for more testing of the water and septic systems in the area.
2. There have been minimal applications for construction since the last moratorium was lifted.
3. The WPCA is seeking on-site solutions.
4. There are several sets of vacant lots that may be valuable for sewage disposal systems.

Specyalski stepped down for the next item of business.

Brialee Campground - Brian Specyalski submitted a plan for a six additional campsites at the campground. It was noted that three of these butt onto adjoining property that is owned by the State of Connecticut. The others have a 100' setback that has been the minimum acceptable to the Commission. Moved and seconded to receive the plan and hold a public hearing on December 9th. Passed without dissent. A new map showing only the three sites that meet the setback requirements will be submitted. The Commission will walk the site at 7 a.m. on Saturday November 16th.

The Commission returned to the top of the agenda.

Moved and seconded to approve the minutes of the October 15th meeting. Passed without dissent.

Moved and seconded to send a letter of appreciation to George Quirk Sr., retiring member for his many years of service to the Commission. Passed without dissent.

There were no bills.

A copy of the revised Small Cities Housing Plan was received from the Office of the Selectmen. It will go to a public hearing in December. Copies will be distributed to the Commission members for review.

The revised fee schedule was approved by Town Meeting in October.

Moved and seconded to add to the agenda the election of officers and reappointment of employees. Passed without dissent.

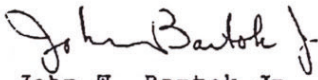
Moved and seconded to reelect the following officers to serve until the next annual meeting of the Commission: Sidney E. Organ, Chairman, Alex Hastillo, Vice Chairman and John Bartok, Secretary. Passed without dissent. The Secretary will cast one ballot for each.

Moved and seconded to reappoint Rudolph Makray, Zoning Enforcement Officer and John Bartok, Recording Secretary for one year or until the next annual meeting. Passed without dissent.

The Commission agreed to hold a Special Meeting on Monday, December 16th at 7 p.m. to review the draft of the revised Plan of Development.

The meeting adjourned at 10:30 p.m.

Respectfully submitted.

  
John W. Bartok, Jr.  
Recording Secretary

## LEGAL NOTICE

### Town of Ashford

The Ashford Planning and Zoning Commission at its meeting on November 12, 1996 took the following actions:

APPROVED with conditions the application of Sprint Spectrum, L.P., Meriden, CT for a 200' communications tower to be built on the David Martin property located off Route 89 on Sky Hill.

APPROVED a request by the Ashford Water Pollution Control Authority to reenstate the moratorium at Lake Chaffee until December 31, 1997 that prohibits construction of new houses or enclosed additions to any existing house.

Dated in Ashford, Connecticut this 14th day of November, 1996.

John W. Bartok, Jr., Sec.  
Ashford Planning and,  
Zoning Commission



The Assessor's office is responsible for the maintenance of records on the ownership of properties. Assessments are computed at 70% of the estimated market value of real property at the time of the last revaluation which was 2021. A plus sign (+) at the end of a Map Block Lot (e.g., 23 52 7+) means three or more lots have been merged.



## Ashford, Connecticut

Information on the Property Records for the Municipality of Ashford was last updated on 1/8/2024.



### Parcel Information

Location:	33 JANOSKI RD	Property Use:	Vacant Land	Primary Use:	Commercial Vacant Land
Unique ID:	00007410	Map Block Lot:	02 F 1.1	Acres:	0.7000
490 Acres:	0.00	Zone:		Volume / Page:	200/ 736
Developers Map / Lot:		Census:			

### Value Information

	Appraised Value	Assessed Value
Land	401,400	280,980
Buildings	0	0
Detached Outbuildings	111,600	78,120

	Appraised Value	Assessed Value
Total	513,000	359,100

### Owner's Information

Owner's Data
MARTIN CAROLYN M L/U MARTIN STEVEN REMAINDERMAN C/O SPRINT SPECTRUM CT-03XC04 PO BOX 8430 KANSAS CITY, MO 64114-8430

### Detached Outbuildings

Type:	Year Built:	Length:	Width:	Area:
6 Ft Chain Fence	2007	0.00	0.00	260
Cell Shed	2007	0.00	0.00	260
Cell Shed	2001	0.00	0.00	360
Cell Shed	2007	0.00	0.00	240
Cell Tower	2001	0.00	0.00	192

### Owner History - Sales

Owner Name	Volume	Page	Sale Date	Deed Type	Sale Price
MARTIN CAROLYN M L/U	0200	0736	12/04/2020	Quit Claim	\$0
MARTIN FAMILY LIV TR DTD 6/20/05	0197	0876	01/31/2020		\$0
MARTIN FAMILY LIV TR DTD 6/20/05	0194	0885	10/15/2018		\$0
MARTIN DAVID H	0109	0811	09/30/1996		\$0

## Building Permits

Permit Number	Permit Type	Date Opened	Reason
23-92E	Commercial	07/31/2023	WIRING OF NEW DISH WIRELESS CELLULAR FACILITY TO INCLUDE 200AMP SERVICE, WIRING OF FIBER CABINET, AN
23-98B	Commercial	06/05/2023	INSTALL 3 ANTENNAS, 6 NEW RRU AND ASSOCIATED EQUIPMENT TO EXISTING TOWER. REAPPLYING FOR EXPIRED PE
23-41E	Commercial	04/27/2023	TALE OVER TENANT LOCATION (METRO PCS) FOR NEW TENANT DISH WIRELESS. 200 AMP FEEDER FROM EXISTING SE
22-108B	Commercial	08/05/2022	INSTALL 3 NEW ANTENNA, REMOTE RADIO UNITS + EQPMT
22-96B	Commercial	07/11/2022	T-MOBILE ADDING NEW MW DISH + ANTENNAS TO EXISTING EQPMT
20-39B	Electrical	04/21/2020	T MOBILE REPLACING 6 EXISTING ANTENNAS
20-16B	Commercial	02/26/2020	ADD 3 ANTENNAS TO EXISTING TOWER.
19-129B	Commercial	12/30/2019	REPLACE SIX EXISTING ANTENNALS + REPLACE 2 RRUS, ADD 6 RRUS.
19-13B	Commercial	03/08/2019	UPGRADE EXISTING T-MOBILE EQPMT
18-54E	Generator	11/19/2018	INSTALL CONCRETE PAD + DIESEL GENERATOR.
18-76	Commercial	09/12/2018	SPRINT TO REPLACE 6 ANTENNAS + ADD 12 REMOTE RADIO HEADS.
18-74B	Commercial	09/07/2018	VERIZON TO SWAP OUT EXISTING EQUIPMENT. ALL NEW EQUIPMENT TO MATCH EXISTING CONDITIONS AND HEIGHTS.
18-29E	Electrical	06/11/2018	15 KW GENERATOR
16-11B	Commercial	11/23/2015	REPLACING ANTENNA PANELS & ADDING REMOTE RADIO HEADS
16130	Commercial	05/05/2015	REMOVE 3 ANTENNAS & REPLACE W/ 4 941-308-5986
15801	Commercial	02/11/2014	50KW DIESEL GENERATOR & 2 ACCESS GATES 508-930-0974
15485	Commercial	12/19/2012	RMV/REPLACE 2 CABINETS GROUNDWORK
15315	Commercial	07/02/2012	ADD 3 ANTENNAS, SWAP 3 ANTENNA, ADD EQUIP TO SHELTER
15158	Commercial	11/16/2011	REPL ANTENNA FOR METRO CELL SITE
15101	Addition	09/27/2011	CELL TOWER CO-LOCATION
11695	Miscellaneous	06/21/2002	

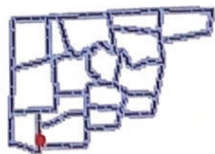


Permit Number	Permit Type	Date Opened	Reason
EXEMPT	Electrical		TMOBILE TO REPLACE 2 ANTENNAS AND ADD 2 MORE. IT IS REPLACING 2 AMPLIFIERS WITH 6 NEW ONES, 4 COAS +

Information Published With Permission From The Assessor



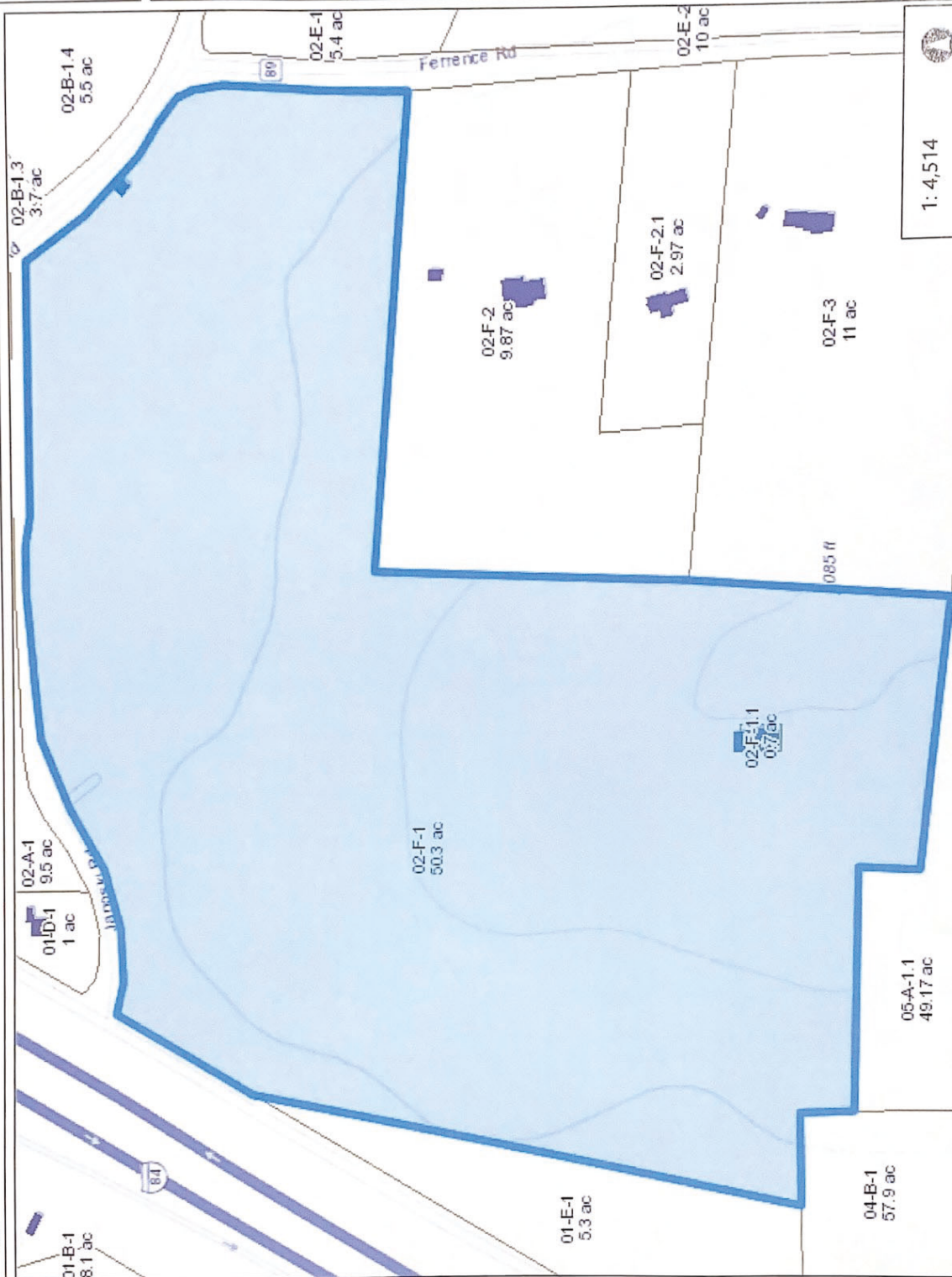
# Necog GIS Site



- Legend
- Town
  - Buildings 2012
  - Parcels

Notes

Enter Map Description



1: 4,514

This map is a user generated static output from an Internet mapping site and is for reference only. Data layers that appear on this map may or may not be accurate, current, or otherwise reliable.

THIS MAP IS NOT TO BE USED FOR NAVIGATION

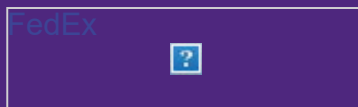
0.1 Miles

WGCS\_1984\_Web\_Mercator\_Auxiliary\_Sphere  
© Latitude Geographics Group Ltd.

**From:** [TrackingUpdates@fedex.com](mailto:TrackingUpdates@fedex.com)  
**To:** [Barbadora, Jeff](#)  
**Subject:** FedEx Shipment 774755759979: Your package has been delivered  
**Date:** Friday, January 12, 2024 1:59:43 PM

---

**CAUTION:** This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.



Hi. Your package was  
delivered Fri, 01/12/2024 at  
1:51pm.



Delivered to 5 TOWN HALL RD, ASHFORD, CT 06278  
Received by S.MUTCH

**OBTAIN PROOF OF DELIVERY**

How was your delivery ?





TRACKING NUMBER	<a href="#">774755759979</a>
FROM	Crown Castle 1800 W. Park Drive WESTBOROUGH, MA, US, 01581
TO	Town of Ashford Willian Falletti, First Selectman 5 Town Hall Road ASHFORD, CT, US, 06278
REFERENCE	799001.7680
SHIPPER REFERENCE	799001.7680
SHIP DATE	Wed 1/10/2024 08:11 PM
DELIVERED TO	Receptionist/Front Desk
PACKAGING TYPE	FedEx Envelope
ORIGIN	WESTBOROUGH, MA, US, 01581
DESTINATION	ASHFORD, CT, US, 06278
NUMBER OF PIECES	1
TOTAL SHIPMENT WEIGHT	0.50 LB
SERVICE TYPE	FedEx Priority Overnight

## Notifications, from start to finish

Get push notifications when you pair FedEx Delivery Manager® with the FedEx® Mobile app. You can activate alerts in the app to track your package. Then listen for the

virtual doorbell chime that lets you know your package was delivered.

**DOWNLOAD THE MOBILE APP**

FOLLOW FEDEX



☐ Please do not respond to this message. This email was sent from an unattended mailbox. This report was generated at approximately 12:59 PM CST 01/12/2024.

All weights are estimated.

To track the latest status of your shipment, click on the tracking number above.

Standard transit is the date and time the package is scheduled to be delivered by, based on the selected service, destination and ship date. Limitations and exceptions may apply. Please see the FedEx Service Guide for terms and conditions of service, including the FedEx Money-Back Guarantee, or contact your FedEx Customer Support representative.

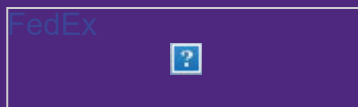
© 2024 Federal Express Corporation. The content of this message is protected by copyright and trademark laws under U.S. and international law. Review our [privacy policy](#). All rights reserved.

Thank you for your business.

**From:** [TrackingUpdates@fedex.com](mailto:TrackingUpdates@fedex.com)  
**To:** [Barbadora, Jeff](#)  
**Subject:** FedEx Shipment 774755812469: Your package has been delivered  
**Date:** Friday, January 12, 2024 1:59:32 PM

---

**CAUTION:** This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.



Hi. Your package was  
delivered Fri, 01/12/2024 at  
1:51pm.



Delivered to 5 TOWN HALL RD, ASHFORD, CT 06278  
Received by S.MUTCH

**OBTAIN PROOF OF DELIVERY**

How was your delivery ?





TRACKING NUMBER	<a href="#">774755812469</a>
FROM	Crown Castle 1800 W. Park Drive WESTBOROUGH, MA, US, 01581
TO	Town of Ashford Michael D' Amato, ZEO 5 Town Hall Road ASHFORD, CT, US, 06278
REFERENCE	799001.7680
SHIPPER REFERENCE	799001.7680
SHIP DATE	Wed 1/10/2024 08:04 PM
DELIVERED TO	Receptionist/Front Desk
PACKAGING TYPE	FedEx Envelope
ORIGIN	WESTBOROUGH, MA, US, 01581
DESTINATION	ASHFORD, CT, US, 06278
NUMBER OF PIECES	1
TOTAL SHIPMENT WEIGHT	1.00 LB
SERVICE TYPE	FedEx Priority Overnight

## Notifications, from start to finish

Get push notifications when you pair FedEx Delivery Manager® with the FedEx® Mobile app. You can activate alerts in the app to track your package. Then listen for the

virtual doorbell chime that lets you know your package was delivered.

**DOWNLOAD THE MOBILE APP**

FOLLOW FEDEX



☐ Please do not respond to this message. This email was sent from an unattended mailbox. This report was generated at approximately 12:59 PM CST 01/12/2024.

All weights are estimated.

To track the latest status of your shipment, click on the tracking number above.

Standard transit is the date and time the package is scheduled to be delivered by, based on the selected service, destination and ship date. Limitations and exceptions may apply. Please see the FedEx Service Guide for terms and conditions of service, including the FedEx Money-Back Guarantee, or contact your FedEx Customer Support representative.

© 2024 Federal Express Corporation. The content of this message is protected by copyright and trademark laws under U.S. and international law. Review our [privacy policy](#). All rights reserved.

Thank you for your business.

Tracking Number:

Remove X

EI919505566US

Copy      Add to Informed Delivery (<https://informedelivery.usps.com/>)

Scheduled Delivery by

THURSDAY

11 January 2024 ⓘ

by 6:00pm ⓘ

Your item was delivered in or at the mailbox at 10:51 am on January 13, 2024 in KANSAS CITY, MO 64114. Waiver of signature was exercised at time of delivery.

Feedback

Get More Out of USPS Tracking:  
USPS Tracking Plus®

**Delivered**  
**Delivered, In/At Mailbox**  
KANSAS CITY, MO 64114  
January 13, 2024, 10:51 am  
  
[See All Tracking History](#)

[What Do USPS Tracking Statuses Mean? \(https://faq.usps.com/s/article/Where-is-my-package\)](https://faq.usps.com/s/article/Where-is-my-package)

- Text & Email Updates
- Proof of Delivery
- USPS Tracking Plus®



## Product Information

**See Less** 

Track Another Package

Enter tracking or barcode numbers

## Need More Help?

Contact USPS Tracking support for further assistance.

**FAQs**

Sent from my iPhone



Date: **September 14, 2023**



Trylon  
1825 W. Walnut Hill Lane,  
Suite 302  
Irving, TX 75038  
214-930-1730

<b>Subject:</b>	<b>Mount Analysis Report</b>	
<b>Carrier Designation:</b>	<b>T-Mobile Equipment Change-Out</b>	
	<b>Carrier Site Number:</b>	CT11353C
	<b>Carrier Site Name:</b>	Ashford/I-84_1
<b>Crown Castle Designation:</b>	<b>BU Number:</b>	876345
	<b>Site Name:</b>	SKY HILL
	<b>JDE Job Number:</b>	752565
	<b>Order Number:</b>	655749 Rev. 0
<b>Engineering Firm Designation:</b>	<b>Trylon Report Designation:</b>	231375
<b>Site Data:</b>	<b>33 Janowski Road, Ashford, Windham County, CT, 06278</b> <b>Latitude 41°57'7.70" Longitude -72°11'43.90"</b>	
<b>Structure Information:</b>	<b>Tower Height &amp; Type:</b>	<b>192.0 ft Self Support Tower</b>
	<b>Mount Elevation:</b>	<b>156.0 ft</b>
	<b>Mount Width &amp; Type:</b>	<b>12.5 ft Sector Frames</b>

Trylon is pleased to submit this "**Mount Analysis Report**" to determine the structural integrity of T-Mobile's antenna mounting system with the proposed appurtenance and equipment addition on the abovementioned supporting tower structure. Analysis of the existing supporting tower structure is to be completed by others and therefore is not part of this analysis. Analysis of the antenna mounting system as a tie-off point for fall protection or rigging is not part of this document.

The purpose of the analysis is to determine acceptability of the mount stress level. Based on our analysis we have determined the mount stress level to be:

#### **Sector Frames**

#### **Sufficient**

This analysis utilizes an ultimate 3-second gust wind speed of 118 mph as required by the 2022 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Mount analysis prepared by: Steve Mustaro, P.E.

Respectfully Submitted by:  
Matthew Jamerson, P.E.

*Matthew Jamerson*



## TABLE OF CONTENTS

### 1) INTRODUCTION

### 2) ANALYSIS CRITERIA

Table 1 - Proposed Equipment Configuration

### 3) ANALYSIS PROCEDURE

Table 2 - Documents Provided

3.1) Analysis Method

3.2) Assumptions

### 4) ANALYSIS RESULTS

Table 3 - Mount Component Stresses vs. Capacity

Table 4 - Tieback End Reactions

4.1) Recommendations

### 5) APPENDIX A

Wire Frame and Rendered Models

### 6) APPENDIX B

Software Input Calculations

### 7) APPENDIX C

Software Analysis Output

### 8) APPENDIX D

Additional Calculations

## 1) INTRODUCTION

This is an existing three sector 12.5 ft Sector Frames, designed by Site Pro 1.

## 2) ANALYSIS CRITERIA

<b>Building Code:</b>	2021 IBC / 2022 CTSBC
<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Ultimate Wind Speed:</b>	118 mph
<b>Exposure Category:</b>	B
<b>Topographic Factor at Base:</b>	1.0
<b>Topographic Factor at Mount:</b>	1.0
<b>Ice Thickness:</b>	1.5 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Seismic <math>S_s</math>:</b>	0.178
<b>Seismic <math>S_1</math>:</b>	0.055
<b>Live Loading Wind Speed:</b>	30 mph
<b>Man Live Load at Mid/End-Points:</b>	250 lb
<b>Man Live Load at Mount Pipes:</b>	500 lb

**Table 1 - Proposed Equipment Configuration**

Mount Centerline (ft)	Antenna Centerline (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Mount / Modification Details
156.0	158.0	1	COMMSCOPE	VHLP2-11W/A	12.5 ft Sector Frames
		3	ERICSSON	AIR 6419 B41_TMO_CCIV2	
		3	RFS/CELWAVE	APXVAARR24_43-U-NA20	
		1	CERAGON	FIBEAIR IP-20A_RFU-D	
		3	ERICSSON	RADIO 4449 B71 B85A_T-MOBILE	
		3	ERICSSON	RADIO 4460 B2/B25 B66_TMO	

## 3) ANALYSIS PROCEDURE

**Table 2 - Documents Provided**

Document	Remarks	Reference	Source
Crown Application	T-Mobile Application	655749 Rev. 0	CCI Sites
Mount Manufacturer Drawings	Site Pro 1	VFA12-HD	Trylon

### 3.1) Analysis Method

RISA-3D (Version 17.0.4), a commercially available analysis software package, was used to create a three-dimensional model of the antenna mounting system and calculate member stresses for various loading cases.

A tool internally developed, using Microsoft Excel, by Trylon was used to calculate wind loading on all appurtenances, dishes, and mount members for various load cases. Selected output from the analysis is included in Appendix B.

This analysis was performed in accordance with Crown Castle's ENG-SOW-10208 *Tower Mount Analysis* (Revision E).

### 3.2) Assumptions

- 1) The antenna mounting system was properly fabricated, installed and maintained in good condition in accordance with its original design and manufacturer's specifications.
- 2) The configuration of antennas, mounts, and other appurtenances are as specified in Table 1 and the referenced drawings.
- 3) All member connections are assumed to have been designed to meet or exceed the load carrying capacity of the connected member unless otherwise specified in this report.
- 4) The analysis will be required to be revised if the existing conditions in the field differ from those shown in the above-referenced documents or assumed in this analysis. No allowance was made for any damaged, missing, or rusted members.
- 5) Prior structural modifications to the tower mounting system are assumed to be installed as shown per available data.
- 6) Steel grades have been assumed as follows, unless noted otherwise:
 

Channel, Solid Round, Angle, Plate	ASTM A36 (GR 36)
HSS (Rectangular)	ASTM A500 (GR B-46)
Pipe	ASTM A53 (GR 35)
Connection Bolts	ASTM A325

This analysis may be affected if any assumptions are not valid or have been made in error. Tylon should be notified to determine the effect on the structural integrity of the antenna mounting system.

### 4) ANALYSIS RESULTS

**Table 3 - Mount Component Stresses vs. Capacity (Sector Frames, Worst Case Sector)**

Notes	Component	Critical Member	Centerline (ft)	% Capacity	Pass / Fail
1, 2, 3	Mount Pipe(s)	MP3	156.0	32.3	Pass
	Horizontal(s)	M19		65.7	Pass
	Standoff(s)	M21		27.1	Pass
	Bracing(s)	M43		20.9	Pass
	Tieback(s)	M54		19.0	Pass
	Mount Connection(s)	-		30.9	Pass

<b>Structure Rating (max from all components) =</b>	<b>65.7%</b>
---	--------------

Notes:

- 1) See additional documentation in "Appendix C - Software Analysis Output" for calculations supporting the % capacity consumed.
- 2) See additional documentation in "Appendix D – Additional Calculations" for detailed mount connection calculations.
- 3) Rating per TIA-222-H, Section 15.5

**Table 4 - Tieback Connection Data Table**

<b>Tower Connection Node No.</b>	<b>Existing / Proposed</b>	<b>Resultant End Reaction (lb)</b>	<b>Connected Member Type</b>	<b>Connected Member Size</b>	<b>Member Compressive Capacity (lb)<sup>3</sup></b>	<b>Notes</b>
N80C	Existing	982.1	Leg	ROHN 3 EH	4,716.9	1
N79B	Existing	765.8	Leg	ROHN 3 EH	4,716.9	1

Notes:

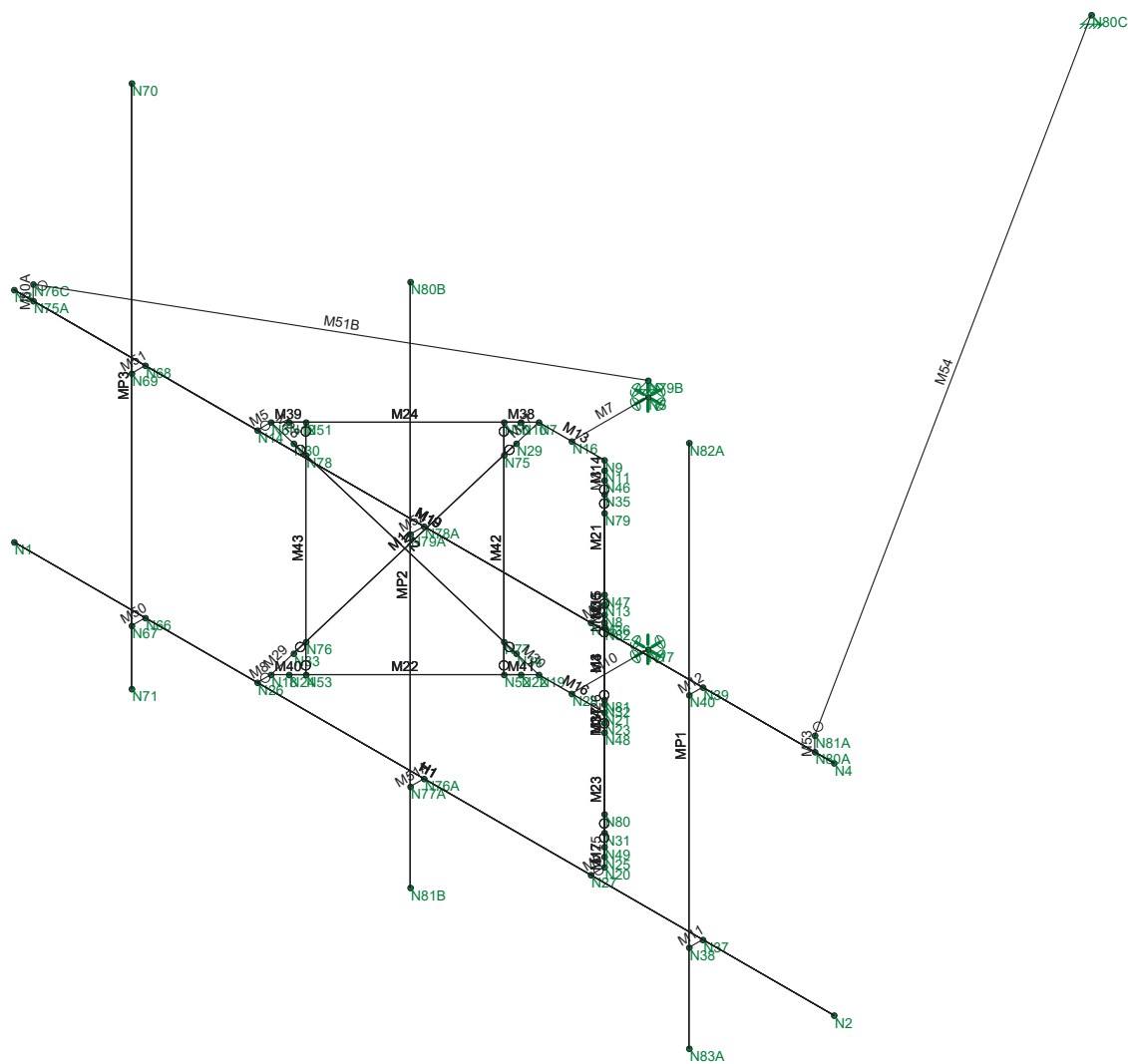
- 1) Tieback connection point is within 25% of either end of the connected tower member
- 2) Tieback connection point is NOT within 25% of either end of the connected tower member
- 3) Reduced member compressive capacity according to CED-STD-10294 *Standard for Installation of Mounts and Appurtenances*

#### **4.1) Recommendations**

The mount has sufficient capacity to carry the proposed loading configuration. No modifications are required at this time.

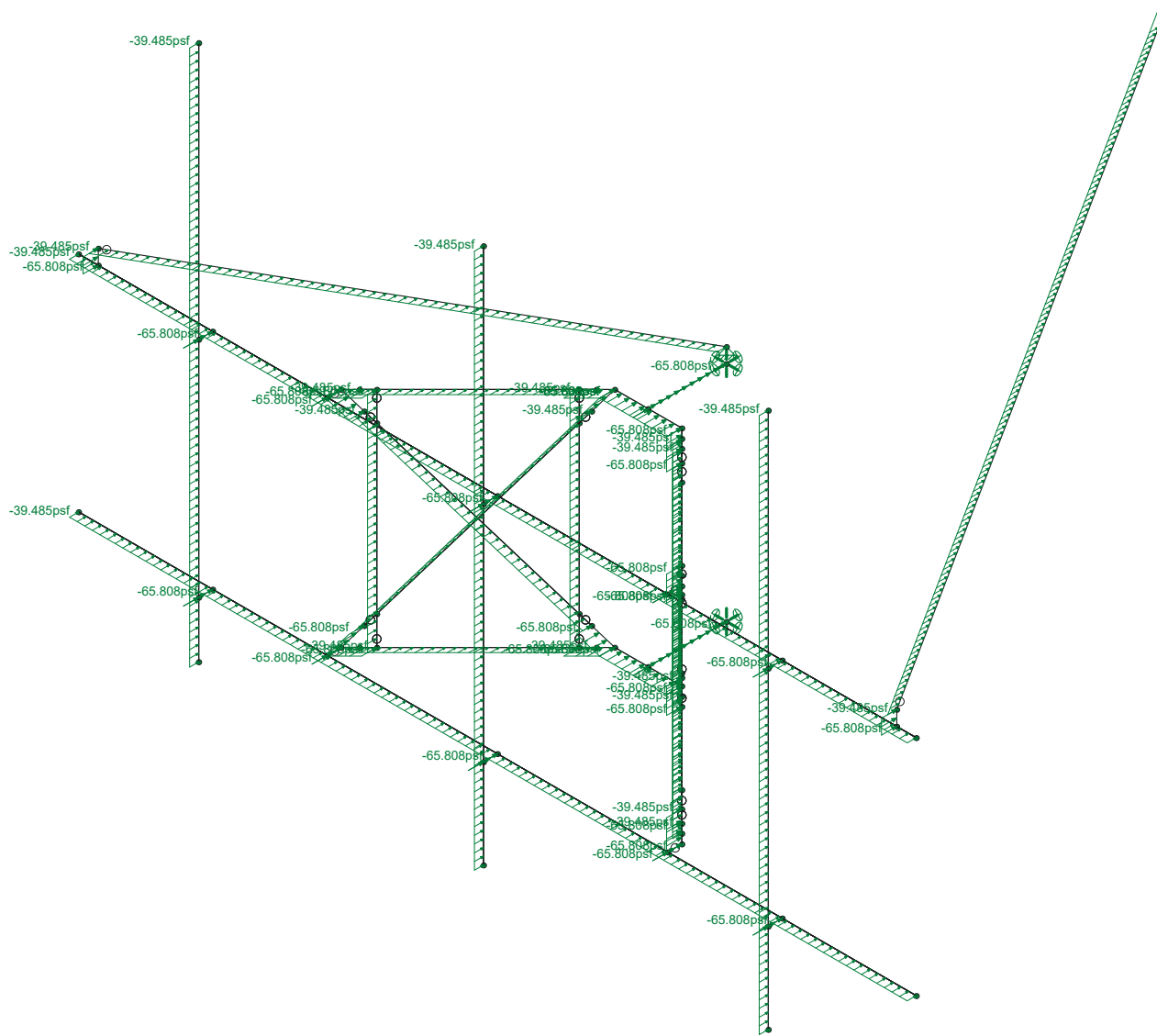


**APPENDIX A**  
**WIRE FRAME AND RENDERED MODELS**

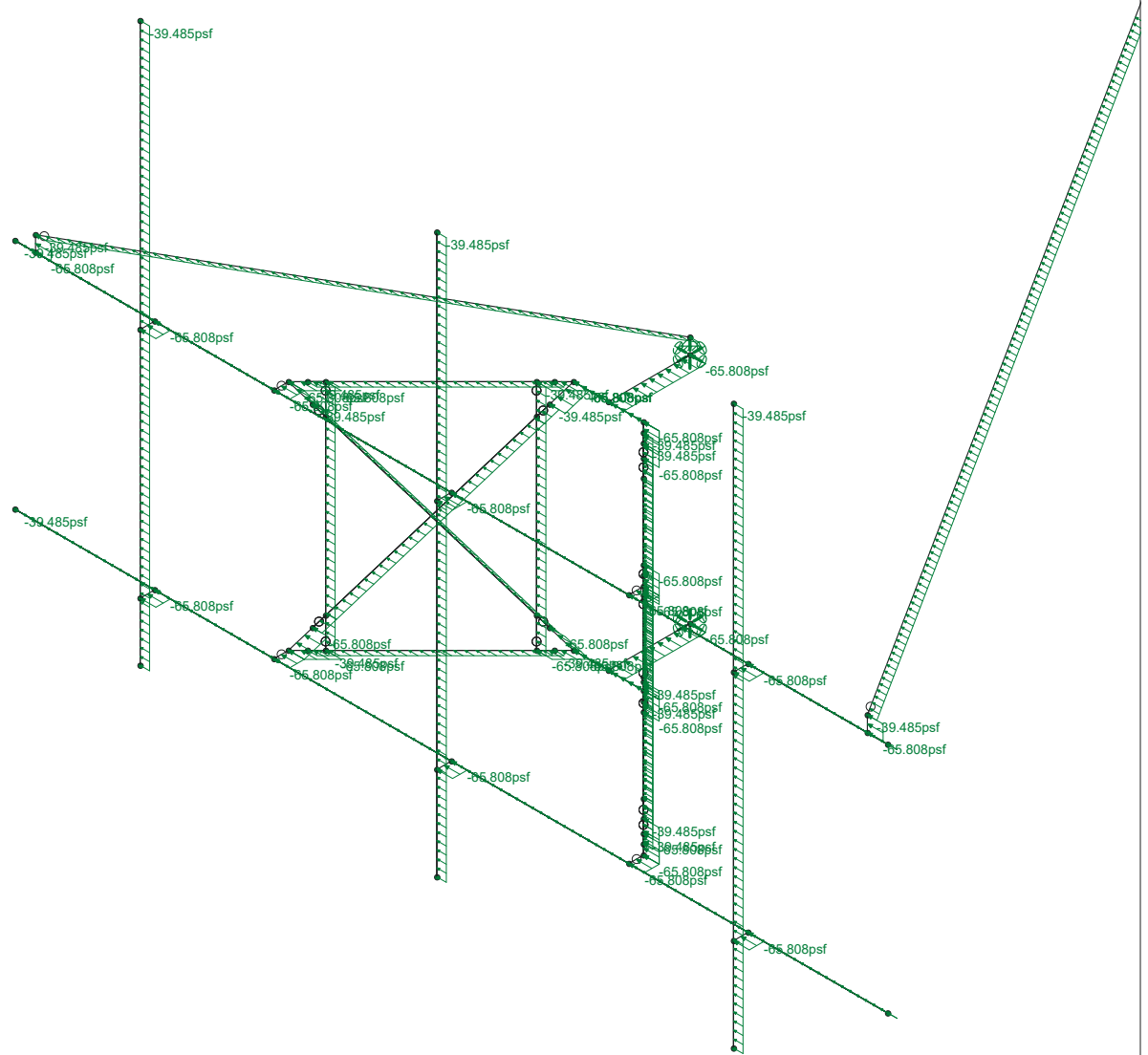


Envelope Only Solution

Trylon	876345	Wireframe
SMM		Sept 8, 2023 at 9:23 AM
231375		876345_loaded.r3d

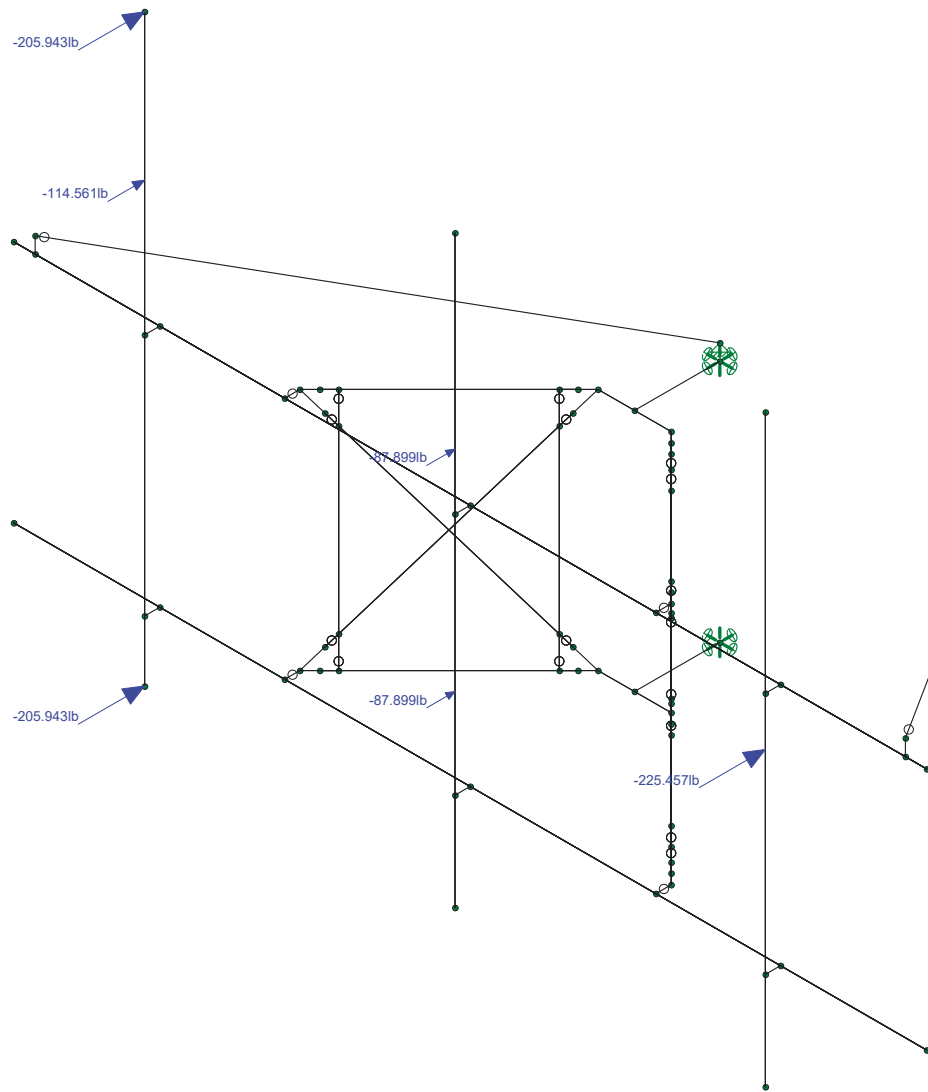


Loads: BLC 2, Structure Wind Z Envelope Only Solution		876345	Wind Loads
Trylon			Sept 8, 2023 at 9:23 AM
SMM			876345_loaded.r3d
231375			



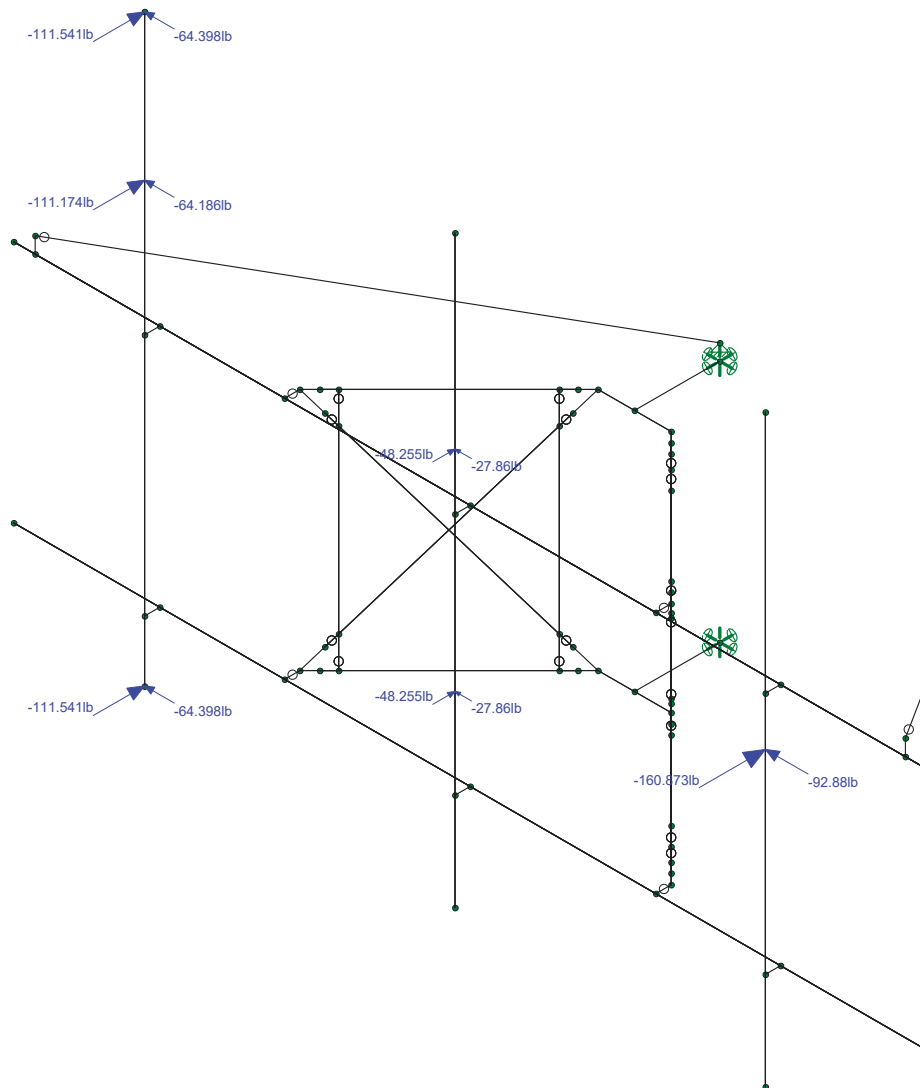
Trylon	876345	Wind Loads
SMM		Sept 8, 2023 at 9:23 AM
231375		876345_loaded.r3d





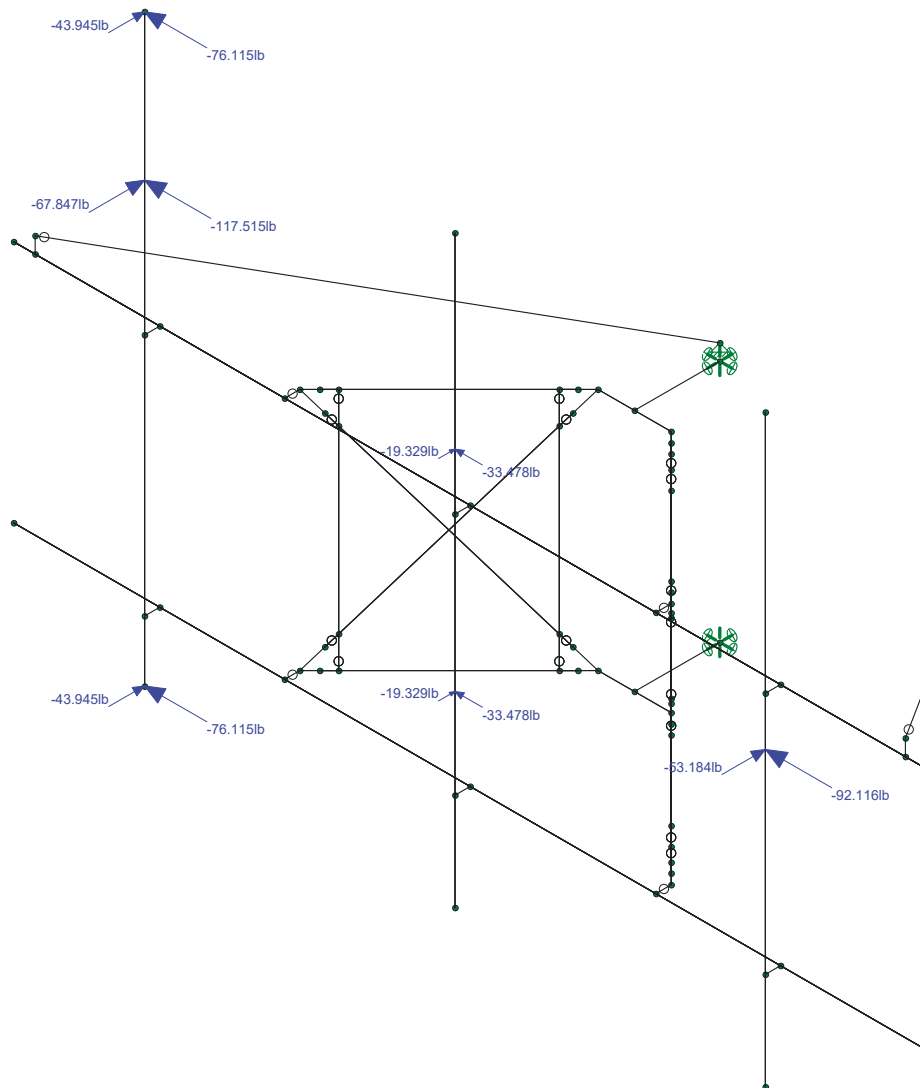
Loads: BLC 4, Wind Load 0 AZI  
Envelope Only Solution

Trylon	876345	Wind Loads
SMM		Sept 8, 2023 at 9:23 AM
231375		876345_loaded.r3d



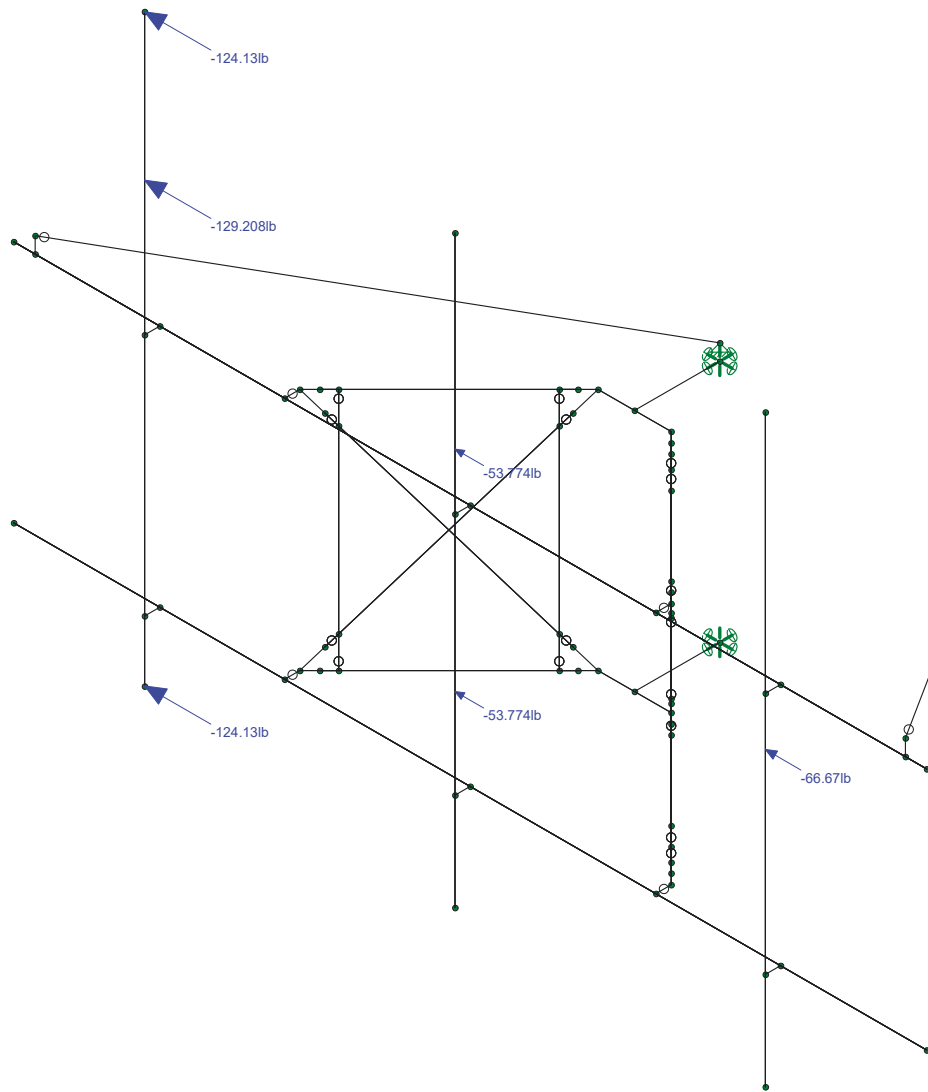
Loads: BLC 5, Wind Load 30 AZI  
Envelope Only Solution

Trylon	876345	Wind Loads
SMM		Sept 8, 2023 at 9:23 AM
231375		876345_loaded.r3d



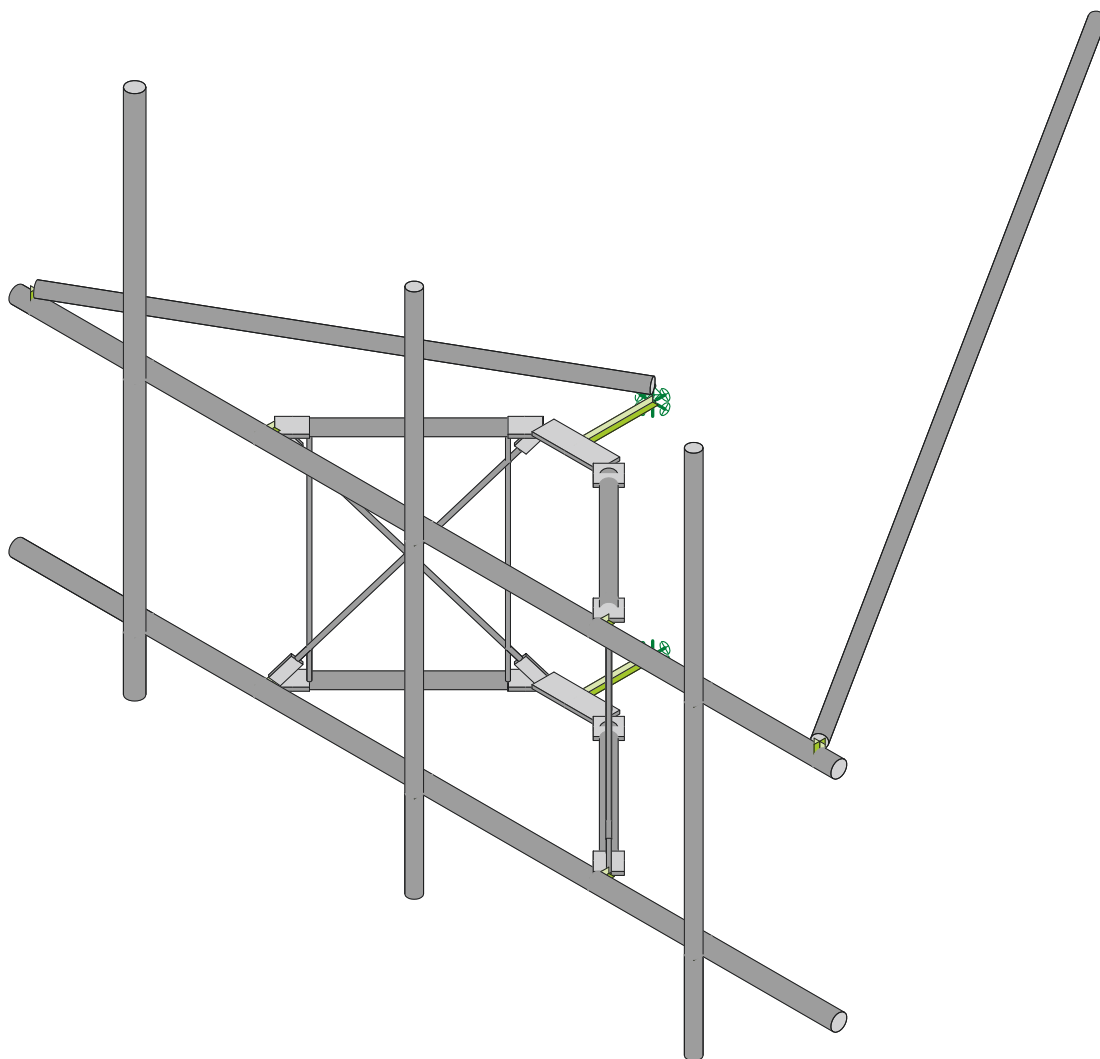
Loads: BLC 7, Wind Load 60 AZI  
Envelope Only Solution

Trylon	876345	Wind Loads
SMM		Sept 8, 2023 at 9:24 AM
231375		876345_loaded.r3d



Loads: BLC 8, Wind Load 90 AZI  
Envelope Only Solution

Trylon	876345	Wind Loads
SMM		Sept 8, 2023 at 9:24 AM
231375		876345_loaded.r3d



Envelope Only Solution

Trylon

SMM

231375

876345

Render

Sept 8, 2023 at 9:24 AM

876345\_loaded.r3d



**APPENDIX B**  
**SOFTWARE INPUT CALCULATIONS**

# ASCE 7 Hazards Report

**Address:**

No Address at This Location

**Standard:**

ASCE/SEI 7-16

**Risk Category:** II**Soil Class:**

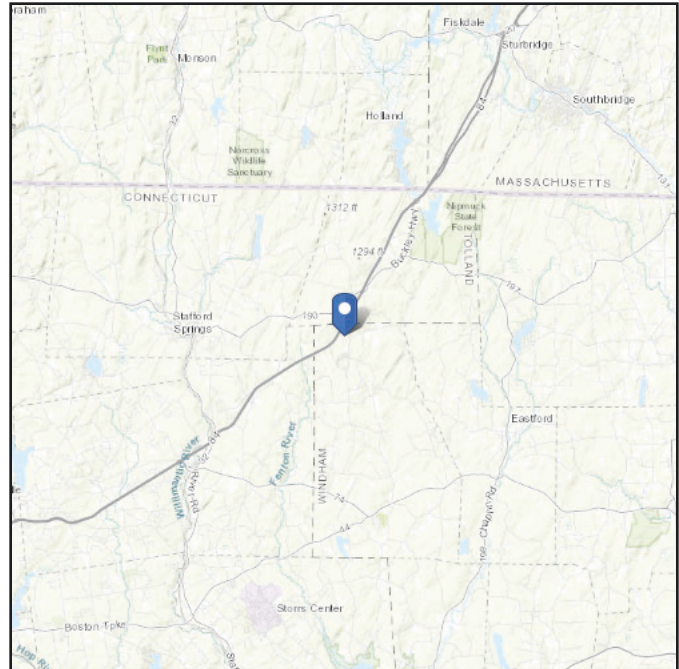
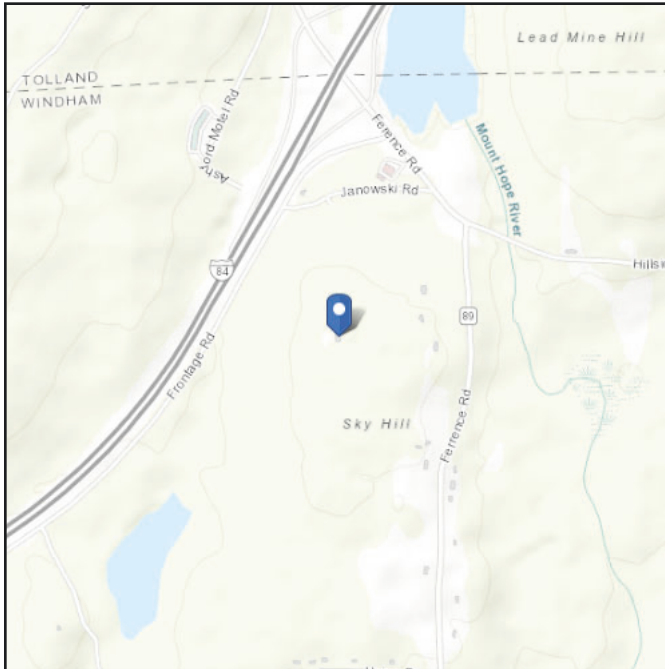
D - Default (see  
Section 11.4.3)

**Latitude:**

41.952139

**Longitude:** -72.195528**Elevation:**

1066.1276209828807 ft  
(NAVD 88)



## Wind

**Results:**

Wind Speed	118 Vmph
10-year MRI	75 Vmph
25-year MRI	84 Vmph
50-year MRI	90 Vmph
100-year MRI	98 Vmph

**Data Source:**

ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2

**Date Accessed:**

Fri Sep 08 2023

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

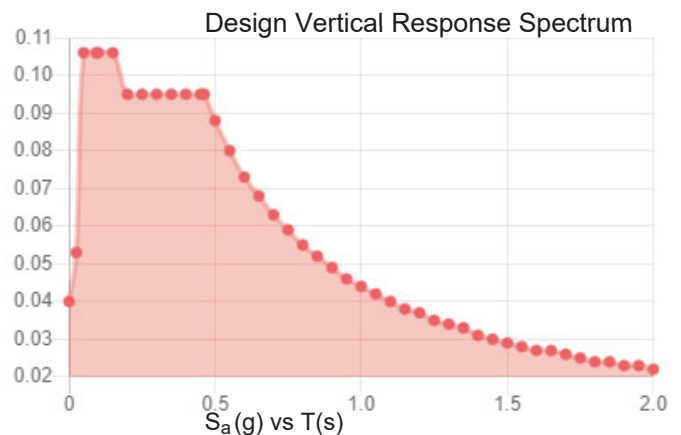
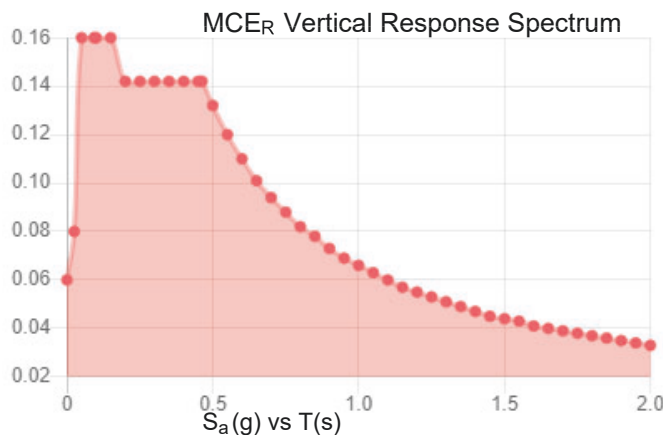
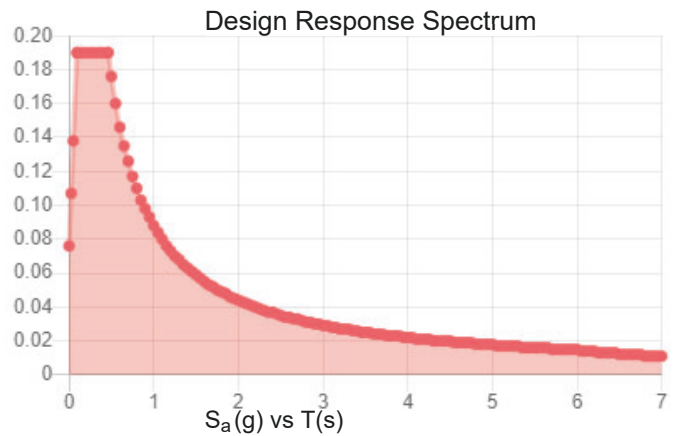
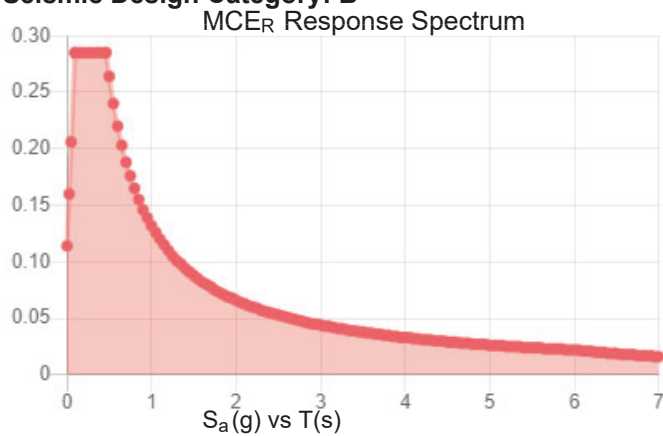
Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

**Site Soil Class:**

**Results:**

$S_S$ :	0.178	$S_{D1}$ :	0.088
$S_1$ :	0.055	$T_L$ :	6
$F_a$ :	1.6	PGA :	0.095
$F_v$ :	2.4	PGA <sub>M</sub> :	0.151
$S_{MS}$ :	0.285	$F_{PGA}$ :	1.6
$S_{M1}$ :	0.132	$I_e$ :	1
$S_{DS}$ :	0.19	$C_v$ :	0.7

**Seismic Design Category: B**



**Data Accessed:**

**Fri Sep 08 2023**

**Date Source:**

**USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.**

**Results:**

Ice Thickness: 1.50 in.  
Concurrent Temperature: 5 F  
Gust Speed 50 mph

**Data Source:** Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

**Date Accessed:** Fri Sep 08 2023

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

---

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.



# Trylon

1825 W. Walnut Hill Lane, Suite 120  
Irving, Texas 75038

## TIA LOAD CALCULATOR 2.2

PROJECT DATA	
Job Code:	231375
Carrier Site ID:	BU 876345
Carrier Site Name:	SKY HILL

CODES AND STANDARDS	
Building Code:	2021 IBC
Local Building Code:	2022 CTSCB
Design Standard:	TIA-222-H

STRUCTURE DETAILS		
Mount Type:	Sector Frame	--
Mount Elevation:	156.0	ft.
Number of Sectors:	3	--
Structure Type:	Self Support Tower	--
Structure Height:	192.0	ft.

ANALYSIS CRITERIA		
Structure Risk Category:	II	--
Exposure Category:	B	--
Site Class:	D - Default	--
Ground Elevation:	1066.1	ft.

TOPOGRAPHIC DATA		
Topographic Category:	1.00	--
Topographic Feature:	N/A	--
Crest Point Elevation:	0.00	ft.
Base Point Elevation:	0.00	ft.
Crest to Mid-Height (L/2):	0.00	ft.
Distance from Crest (x):	0.00	ft.
Base Topo Factor ( $K_{zt}$ ):	1.00	--
Mount Topo Factor ( $K_{zt}$ ):	1.00	--

WIND PARAMETERS		
Design Wind Speed:	118	mph
Wind Escalation Factor ( $K_s$ ):	1.00	--
Velocity Coefficient ( $K_z$ ):	1.12	--
Directionality Factor ( $K_d$ ):	0.95	--
Gust Effect Factor ( $G_h$ ):	1.00	--
Shielding Factor ( $K_a$ ):	0.90	--
Velocity Pressure ( $q_z$ ):	36.56	psf
Ground Elevation Factor ( $K_e$ ):	0.96	--

ICE PARAMETERS		
Design Ice Wind Speed:	50	mph
Design Ice Thickness ( $t_i$ ):	1.50	in
Importance Factor ( $I_i$ ):	1.00	--
Ice Velocity Pressure ( $q_{zi}$ ):	6.83	psf
Mount Ice Thickness ( $t_{iz}$ ):	1.75	in

WIND STRUCTURE CALCULATIONS		
Flat Member Pressure:	65.81	psf
Round Member Pressure:	39.48	psf
Ice Wind Pressure:	7.38	psf

SEISMIC PARAMETERS		
Importance Factor ( $I_e$ ):	1.00	--
Short Period Accel. ( $S_s$ ):	0.18	g
1 Second Accel. ( $S_1$ ):	0.06	g
Short Period Des. ( $S_{DS}$ ):	0.19	g
1 Second Des. ( $S_{D1}$ ):	0.09	g
Short Period Coeff. ( $F_a$ ):	1.60	--
1 Second Coeff. ( $F_v$ ):	2.40	--
Response Coefficient ( $C_s$ ):	0.09	--
Amplification Factor ( $A_S$ ):	1.20	--



## LOAD COMBINATIONS [LRFD]

#	Description
1	1.4DL
2	1.2DL + 1WL 0 AZI
3	1.2DL + 1WL 30 AZI
4	1.2DL + 1WL 45 AZI
5	1.2DL + 1WL 60 AZI
6	1.2DL + 1WL 90 AZI
7	1.2DL + 1WL 120 AZI
8	1.2DL + 1WL 135 AZI
9	1.2DL + 1WL 150 AZI
10	1.2DL + 1WL 180 AZI
11	1.2DL + 1WL 210 AZI
12	1.2DL + 1WL 225 AZI
13	1.2DL + 1WL 240 AZI
14	1.2DL + 1WL 270 AZI
15	1.2DL + 1WL 300 AZI
16	1.2DL + 1WL 315 AZI
17	1.2DL + 1WL 330 AZI
18	0.9DL + 1WL 0 AZI
19	0.9DL + 1WL 30 AZI
20	0.9DL + 1WL 45 AZI
21	0.9DL + 1WL 60 AZI
22	0.9DL + 1WL 90 AZI
23	0.9DL + 1WL 120 AZI
24	0.9DL + 1WL 135 AZI
25	0.9DL + 1WL 150 AZI
26	0.9DL + 1WL 180 AZI
27	0.9DL + 1WL 210 AZI
28	0.9DL + 1WL 225 AZI
29	0.9DL + 1WL 240 AZI
30	0.9DL + 1WL 270 AZI
31	0.9DL + 1WL 300 AZI
32	0.9DL + 1WL 315 AZI
33	0.9DL + 1WL 330 AZI
34	1.2DL + 1DLi + 1WLi 0 AZI
35	1.2DL + 1DLi + 1WLi 30 AZI
36	1.2DL + 1DLi + 1WLi 45 AZI
37	1.2DL + 1DLi + 1WLi 60 AZI
38	1.2DL + 1DLi + 1WLi 90 AZI
39	1.2DL + 1DLi + 1WLi 120 AZI
40	1.2DL + 1DLi + 1WLi 135 AZI
41	1.2DL + 1DLi + 1WLi 150 AZI

#	Description
42	1.2DL + 1DLi + 1WLi 180 AZI
43	1.2DL + 1DLi + 1WLi 210 AZI
44	1.2DL + 1DLi + 1WLi 225 AZI
45	1.2DL + 1DLi + 1WLi 240 AZI
46	1.2DL + 1DLi + 1WLi 270 AZI
47	1.2DL + 1DLi + 1WLi 300 AZI
48	1.2DL + 1DLi + 1WLi 315 AZI
49	1.2DL + 1DLi + 1WLi 330 AZI
50	(1.2+0.2Sds) + 1.0E 0 AZI
51	(1.2+0.2Sds) + 1.0E 30 AZI
52	(1.2+0.2Sds) + 1.0E 45 AZI
53	(1.2+0.2Sds) + 1.0E 60 AZI
54	(1.2+0.2Sds) + 1.0E 90 AZI
55	(1.2+0.2Sds) + 1.0E 120 AZI
56	(1.2+0.2Sds) + 1.0E 135 AZI
57	(1.2+0.2Sds) + 1.0E 150 AZI
58	(1.2+0.2Sds) + 1.0E 180 AZI
59	(1.2+0.2Sds) + 1.0E 210 AZI
60	(1.2+0.2Sds) + 1.0E 225 AZI
61	(1.2+0.2Sds) + 1.0E 240 AZI
62	(1.2+0.2Sds) + 1.0E 270 AZI
63	(1.2+0.2Sds) + 1.0E 300 AZI
64	(1.2+0.2Sds) + 1.0E 315 AZI
65	(1.2+0.2Sds) + 1.0E 330 AZI
66	(0.9-0.2Sds) + 1.0E 0 AZI
67	(0.9-0.2Sds) + 1.0E 30 AZI
68	(0.9-0.2Sds) + 1.0E 45 AZI
69	(0.9-0.2Sds) + 1.0E 60 AZI
70	(0.9-0.2Sds) + 1.0E 90 AZI
71	(0.9-0.2Sds) + 1.0E 120 AZI
72	(0.9-0.2Sds) + 1.0E 135 AZI
73	(0.9-0.2Sds) + 1.0E 150 AZI
74	(0.9-0.2Sds) + 1.0E 180 AZI
75	(0.9-0.2Sds) + 1.0E 210 AZI
76	(0.9-0.2Sds) + 1.0E 225 AZI
77	(0.9-0.2Sds) + 1.0E 240 AZI
78	(0.9-0.2Sds) + 1.0E 270 AZI
79	(0.9-0.2Sds) + 1.0E 300 AZI
80	(0.9-0.2Sds) + 1.0E 315 AZI
81	(0.9-0.2Sds) + 1.0E 330 AZI
82-88	1.2D + 1.5 Lv1

#	Description
89	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP1
90	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP1
91	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP1
92	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP1
93	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP1
94	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP1
95	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP1
96	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP1
97	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP1
98	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP1
99	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP1
100	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP1
101	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP1
102	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP1
103	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP1
104	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP1
105	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP2
106	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP2
107	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP2
108	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP2
109	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP2
110	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP2
111	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP2
112	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP2
113	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP2
114	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP2
115	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP2
116	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP2
117	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP2
118	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP2
119	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP2
120	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP2

#	Description
121	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP3
122	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP3
123	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP3
124	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP3
125	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP3
126	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP3
127	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP3
128	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP3
129	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP3
130	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP3
131	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP3
132	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP3
133	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP3
134	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP3
135	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP3
136	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP3
137	1.2D + 1.5Lm + 1.0Wm 0 AZI - MP4
138	1.2D + 1.5Lm + 1.0Wm 30 AZI - MP4
139	1.2D + 1.5Lm + 1.0Wm 45 AZI - MP4
140	1.2D + 1.5Lm + 1.0Wm 60 AZI - MP4
141	1.2D + 1.5Lm + 1.0Wm 90 AZI - MP4
142	1.2D + 1.5Lm + 1.0Wm 120 AZI - MP4
143	1.2D + 1.5Lm + 1.0Wm 135 AZI - MP4
144	1.2D + 1.5Lm + 1.0Wm 150 AZI - MP4
145	1.2D + 1.5Lm + 1.0Wm 180 AZI - MP4
146	1.2D + 1.5Lm + 1.0Wm 210 AZI - MP4
147	1.2D + 1.5Lm + 1.0Wm 225 AZI - MP4
148	1.2D + 1.5Lm + 1.0Wm 240 AZI - MP4
149	1.2D + 1.5Lm + 1.0Wm 270 AZI - MP4
150	1.2D + 1.5Lm + 1.0Wm 300 AZI - MP4
151	1.2D + 1.5Lm + 1.0Wm 315 AZI - MP4
152	1.2D + 1.5Lm + 1.0Wm 330 AZI - MP4

\*This page shows an example of maintenance loads for (4) pipes, the number of mount pipe LCs may vary per site

## EQUIPMENT LOADING

[illegible]

## EQUIPMENT LOADING [CONT.]

<i>Appurtenance Name</i>	<i>Qty.</i>	<i>Elevation [ft]</i>	<i>--</i>	<i>EPA<sub>N</sub> (ft2)</i>	<i>EPA<sub>T</sub> (ft2)</i>	<i>Weight (lbs)</i>
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			
			No Ice			
--	--	--	w/ Ice			

## EQUIPMENT WIND CALCULATIONS

[illegible]

## EQUIPMENT LATERAL WIND FORCE CALCULATIONS

[illegible]



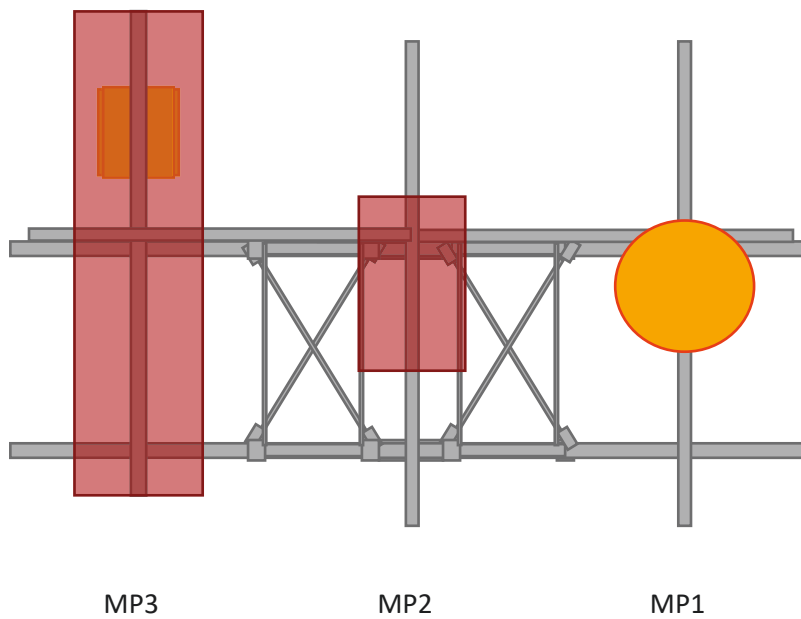
## EQUIPMENT LATERAL WIND FORCE CALCULATIONS [CONT.]

[illegible]

## EQUIPMENT SEISMIC FORCE CALCULATIONS

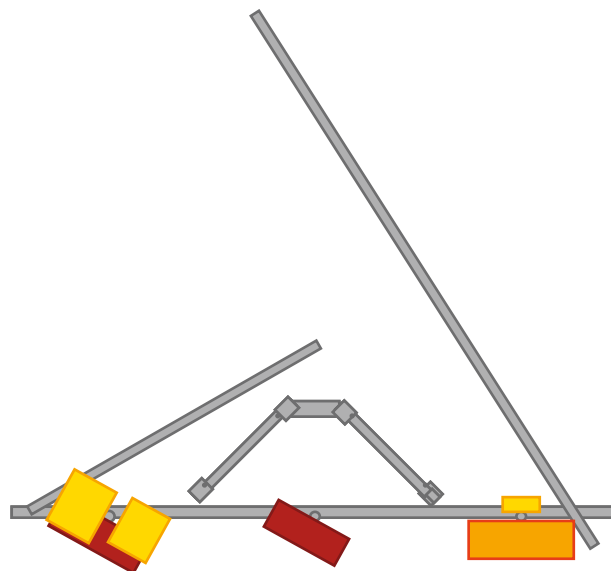
[illegible]

### ELEVATION VIEW



\*these drawings are intended to show approximate locations of equipment on the mount and should not be used to determine exact placement of equipment or additional hardware

### PLAN VIEW





**APPENDIX C**  
**SOFTWARE ANALYSIS OUTPUT**

### **(Global) Model Settings**

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	24
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver

Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISACONNECTION CODE	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR_SET_ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

### (Global) Model Settings, Continued

Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

### Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/1...	Density[k/ft^3]	Yield[ksi]	Ry	Fu[ksi]	Rt
1	A992	29000	11154	.3	.65	.49	50	1.1	65	1.1
2	A36 Gr.36	29000	11154	.3	.65	.49	36	1.5	58	1.2
3	A572 Gr.50	29000	11154	.3	.65	.49	50	1.1	65	1.1
4	A500 Gr.B RND	29000	11154	.3	.65	.527	42	1.4	58	1.3
5	A500 Gr.B Rect	29000	11154	.3	.65	.527	46	1.4	58	1.3
6	A53 Gr.B	29000	11154	.3	.65	.49	35	1.6	60	1.2
7	A1085	29000	11154	.3	.65	.49	50	1.4	65	1.3

### Cold Formed Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (/1E5 F)	Density[k/ft^3]	Yield[ksi]	Fu[ksi]
1	A653 SS Gr33	29500	11346	.3	.65	.49	33	45
2	A653 SS Gr50/1	29500	11346	.3	.65	.49	50	65

### Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rul...	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	PIPE 2.0	PIPE 2.0	Beam	Pipe	A53 Gr.B	Typical	1.02	.627	.627	1.25
2	PIPE 2.5	PIPE 2.5	Beam	Pipe	A53 Gr.B	Typical	1.61	1.45	1.45	2.89
3	SR 3/4	SR 3/4	Beam	BAR	A36 Gr.36	Typical	.442	.016	.016	.031
4	Plate	PL 4"x0.625"	Beam	RECT	A36 Gr.36	Typical	2.5	.081	3.333	.293
5	SR 5/8	SR 5/8	Beam	BAR	A36 Gr.36	Typical	.307	.007	.007	.015
6	D Plate	PL 2"x0.625"	Beam	RECT	A36 Gr.36	Typical	1.25	.041	.417	.131



## Cold Formed Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rules	A [in <sup>2</sup> ]	I <sub>yy</sub> [in <sup>4</sup> ]	I <sub>zz</sub> [in <sup>4</sup> ]	J [in <sup>4</sup> ]
1	CF1	162T 125-18	Beam	None	A653 SS Gr33	Typical	.078	.013	.042	9e-6

## Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot. [k-ft/rad]	Y Rot. [k-ft/rad]	Z Rot. [k-ft/rad]
1	N5	Reaction	Reaction	Reaction	Reaction		Reaction
2	N17	Reaction	Reaction	Reaction	Reaction		Reaction
3	N80C	Reaction	Reaction	Reaction			
4	N79B	Reaction	Reaction	Reaction			

## Basic Load Cases

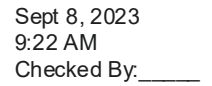
	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...Surface...
1	Self Weight	DL		-1			8		
2	Structure Wind Z	WLZ						51	
3	Structure Wind X	WLX						51	
4	Wind Load 0 AZI	WLZ					16		
5	Wind Load 30 AZI	None					16		
6	Wind Load 45 AZI	None					16		
7	Wind Load 60 AZI	None					16		
8	Wind Load 90 AZI	WLX					16		
9	Wind Load 120 AZI	None					16		
10	Wind Load 135 AZI	None					16		
11	Wind Load 150 AZI	None					16		
12	Ice Weight	OL1					8	51	
13	Ice Structure Wind Z	OL2						51	
14	Ice Structure Wind X	OL3						51	
15	Ice Wind Load 0 AZI	OL2					16		
16	Ice Wind Load 30 AZI	None					16		
17	Ice Wind Load 45 AZI	None					16		
18	Ice Wind Load 60 AZI	None					16		
19	Ice Wind Load 90 AZI	OL3					16		
20	Ice Wind Load 120 AZI	None					16		
21	Ice Wind Load 135 AZI	None					16		
22	Ice Wind Load 150 AZI	None					16		
23	Seismic Load Z	ELZ			-.114		8		
24	Seismic Load X	ELX	-.114				8		
25	Live Load 1 (Lv)	None					1		
26	Live Load 2 (Lv)	None					1		
27	Live Load 3 (Lv)	None					1		
28	Maintenance Load 1 (Lm)	None					1		
29	Maintenance Load 2 (Lm)	None					1		
30	Maintenance Load 3 (Lm)	None					1		

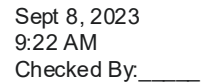
## Load Combinations

	Description	So..P...	S...	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..
1	1.4DL	Yes	Y		DL 1.4															
2	1.2DL + 1WL 0 AZI	Yes	Y		DL 1.2	2	1	3		4	1									

### Load Combinations (Continued)

	Description	So..P...	S...	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..	BLCFac..
3	1.2DL + 1WL 30 AZI	Yes	Y	DL	1.2	2	.866	3	.5	5	1									
4	1.2DL + 1WL 45 AZI	Yes	Y	DL	1.2	2	.707	3	.707	6	1									
5	1.2DL + 1WL 60 AZI	Yes	Y	DL	1.2	2	.5	3	.866	7	1									
6	1.2DL + 1WL 90 AZI	Yes	Y	DL	1.2	2		3	1	8	1									
7	1.2DL + 1WL 120 AZI	Yes	Y	DL	1.2	2	-.5	3	.866	9	1									
8	1.2DL + 1WL 135 AZI	Yes	Y	DL	1.2	2	-.707	3	.707	10	1									
9	1.2DL + 1WL 150 AZI	Yes	Y	DL	1.2	2	-.866	3	.5	11	1									
10	1.2DL + 1WL 180 AZI	Yes	Y	DL	1.2	2	-1	3		4	-1									
11	1.2DL + 1WL 210 AZI	Yes	Y	DL	1.2	2	-.866	3	-.5	5	-1									
12	1.2DL + 1WL 225 AZI	Yes	Y	DL	1.2	2	-.707	3	-.707	6	-1									
13	1.2DL + 1WL 240 AZI	Yes	Y	DL	1.2	2	-.5	3	-.866	7	-1									
14	1.2DL + 1WL 270 AZI	Yes	Y	DL	1.2	2		3	-1	8	-1									
15	1.2DL + 1WL 300 AZI	Yes	Y	DL	1.2	2	.5	3	-.866	9	-1									
16	1.2DL + 1WL 315 AZI	Yes	Y	DL	1.2	2	.707	3	-.707	10	-1									
17	1.2DL + 1WL 330 AZI	Yes	Y	DL	1.2	2	.866	3	-.5	11	-1									
18	0.9DL + 1WL 0 AZI	Yes	Y	DL	.9	2	1	3		4	1									
19	0.9DL + 1WL 30 AZI	Yes	Y	DL	.9	2	.866	3	.5	5	1									
20	0.9DL + 1WL 45 AZI	Yes	Y	DL	.9	2	.707	3	.707	6	1									
21	0.9DL + 1WL 60 AZI	Yes	Y	DL	.9	2	.5	3	.866	7	1									
22	0.9DL + 1WL 90 AZI	Yes	Y	DL	.9	2		3	1	8	1									
23	0.9DL + 1WL 120 AZI	Yes	Y	DL	.9	2	-.5	3	.866	9	1									
24	0.9DL + 1WL 135 AZI	Yes	Y	DL	.9	2	-.707	3	.707	10	1									
25	0.9DL + 1WL 150 AZI	Yes	Y	DL	.9	2	-.866	3	.5	11	1									
26	0.9DL + 1WL 180 AZI	Yes	Y	DL	.9	2	-1	3		4	-1									
27	0.9DL + 1WL 210 AZI	Yes	Y	DL	.9	2	-.866	3	-.5	5	-1									
28	0.9DL + 1WL 225 AZI	Yes	Y	DL	.9	2	-.707	3	-.707	6	-1									
29	0.9DL + 1WL 240 AZI	Yes	Y	DL	.9	2	-.5	3	-.866	7	-1									
30	0.9DL + 1WL 270 AZI	Yes	Y	DL	.9	2		3	-1	8	-1									
31	0.9DL + 1WL 300 AZI	Yes	Y	DL	.9	2	.5	3	-.866	9	-1									
32	0.9DL + 1WL 315 AZI	Yes	Y	DL	.9	2	.707	3	-.707	10	-1									
33	0.9DL + 1WL 330 AZI	Yes	Y	DL	.9	2	.866	3	-.5	11	-1									
34	1.2DL + 1DLi + 1WL 0 ...	Yes	Y	DL	1.2	OL1	1	13	1	14	15	1								
35	1.2DL + 1DLi + 1WL 3...	Yes	Y	DL	1.2	OL1	1	13	.866	14	.5	16	1							
36	1.2DL + 1DLi + 1WL 4...	Yes	Y	DL	1.2	OL1	1	13	.707	14	.707	17	1							
37	1.2DL + 1DLi + 1WL 6...	Yes	Y	DL	1.2	OL1	1	13	.5	14	.866	18	1							
38	1.2DL + 1DLi + 1WL 9...	Yes	Y	DL	1.2	OL1	1	13		14	1	19	1							
39	1.2DL + 1DLi + 1WL 1...	Yes	Y	DL	1.2	OL1	1	13	-.5	14	.866	20	1							
40	1.2DL + 1DLi + 1WL 1...	Yes	Y	DL	1.2	OL1	1	13	-.707	14	.707	21	1							
41	1.2DL + 1DLi + 1WL 1...	Yes	Y	DL	1.2	OL1	1	13	-.866	14	.5	22	1							
42	1.2DL + 1DLi + 1WL 1...	Yes	Y	DL	1.2	OL1	1	13	-1	14		15	-1							
43	1.2DL + 1DLi + 1WL 2...	Yes	Y	DL	1.2	OL1	1	13	-.866	14	-.5	16	-1							
44	1.2DL + 1DLi + 1WL 2...	Yes	Y	DL	1.2	OL1	1	13	-.707	14	-.707	17	-1							
45	1.2DL + 1DLi + 1WL 2...	Yes	Y	DL	1.2	OL1	1	13	-.5	14	-.866	18	-1							
46	1.2DL + 1DLi + 1WL 2...	Yes	Y	DL	1.2	OL1	1	13		14	-1	19	-1							
47	1.2DL + 1DLi + 1WL 3...	Yes	Y	DL	1.2	OL1	1	13	.5	14	-.866	20	-1							
48	1.2DL + 1DLi + 1WL 3...	Yes	Y	DL	1.2	OL1	1	13	.707	14	-.707	21	-1							
49	1.2DL + 1DLi + 1WL 3...	Yes	Y	DL	1.2	OL1	1	13	.866	14	-.5	22	-1							
50	(1.2+0.2Sds)DL + 1E 0 ...	Yes	Y	DL	1.2...	23	1	24												
51	(1.2+0.2Sds)DL + 1E 3...	Yes	Y	DL	1.2...	23	.866	24	.5											
52	(1.2+0.2Sds)DL + 1E 4...	Yes	Y	DL	1.2...	23	.707	24	.707											
53	(1.2+0.2Sds)DL + 1E 6...	Yes	Y	DL	1.2...	23	.5	24	.866											
54	(1.2+0.2Sds)DL + 1E 9...	Yes	Y	DL	1.2...	23		24	1											





## Page 6

### Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)

	Member	Shape	Code...	Loc[in]	LC	Shear ...	Loc[in]	Dir	LC	phi*Pnc [...]	phi*Pnt [...]	phi*Mn y...	phi*Mn z...	Cb	Eqn
8	M24	PIPE 2.0	.236	27.813	6	.052	2.188		37	29810.292	32130	1871.625	1871.625	1	H1-1b
9	M43	SR 5/8	.219	35	100	.055	5		4	2339.328	9946.8	96.768	96.768	1	H1-1b
10	H1	PIPE 2.5	.207	126....	84	.084	75		14	14558.792	50715	3596.25	3596.25	1	H1-1b
11	M54	PIPE 2.0	.199	82.692	46	.009	165....		47	5178.703	32130	1871.625	1871.625	1...	H1-1b
12	M22	PIPE 2.0	.163	2.188	100	.053	27.813		40	29810.292	32130	1871.625	1871.625	1	H1-1b
13	M40	PL 4"x0.625"	.161	4.512	39	.086	2.209	y	123	79311.075	81000	1054.688	6750	2...	H1-1b
14	M39	PL 4"x0.625"	.160	4.512	49	.087	2.209	y	130	79311.075	81000	1054.688	6750	1...	H1-1b
15	M37	SR 5/8	.157	35	84	.074	35		34	2339.328	9946.8	96.768	96.768	4...	H1-1b
16	M23	PIPE 2.0	.153	2.188	98	.035	27.813		127	29810.292	32130	1871.625	1871.625	1	H1-1b
17	M42	SR 5/8	.141	35	41	.048	35		3	2339.328	9946.8	96.768	96.768	3...	H1-1b
18	M38	PL 4"x0.625"	.135	1.081	38	.087	0	y	130	79311.075	81000	1054.688	6750	1...	H1-1b
19	M36	SR 5/8	.128	35	40	.097	5		34	2339.328	9946.8	96.768	96.768	1	H1-1b
20	M41	PL 4"x0.625"	.126	4.512	48	.086	2.209	y	124	79311.075	81000	1054.688	6750	1...	H1-1b
21	M14	PL 4"x0.625"	.112	2.303	4	.066	0	y	131	79311.075	81000	1054.688	6750	1...	H1-1b
22	M15	PL 4"x0.625"	.110	4.512	110	.066	2.209	y	131	79311.075	81000	1054.688	6750	1...	H1-1b
23	M17	PL 4"x0.625"	.110	4.512	112	.067	2.209	y	127	79311.075	81000	1054.688	6750	1	H1-1b
24	M1	SR 3/4	.102	2.75	39	.048	2.292		35	4289.781	14313.866	178.929	178.929	3...	H1-1b*
25	M34	PL 4"x0.625"	.091	2.209	44	.067	2.209	y	127	79311.075	81000	1054.688	6750	1...	H1-1b
26	M3	SR 3/4	.063	41.25	111	.030	2.292		4	4289.781	14313.866	178.929	178.929	3...	H1-1b*
27	M29	PL 2"x0.625"	.056	4.46	36	.014	4.46	y	36	39953.263	40500	527.345	1687.5	1...	H1-1b
28	M30	PL 2"x0.625"	.049	4.46	43	.018	0	y	43	39953.263	40500	527.345	1687.5	1	H1-1b
29	M27	PL 2"x0.625"	.049	0	35	.019	4.46	y	35	39953.263	40500	527.345	1687.5	1	H1-1b
30	M28	PL 2"x0.625"	.048	0	45	.014	0	y	82	39953.263	40500	527.345	1687.5	1...	H1-1b
31	M51B	PIPE 2.0	.047	41.211	43	.005	82.421		43	18250.01	32130	1871.625	1871.625	1...	H1-1b
32	M31	PL 2"x0.625"	.041	4.46	34	.011	0	y	113	39953.263	40500	527.345	1687.5	1	H1-1b
33	M26	PL 2"x0.625"	.037	0	40	.011	4.46	y	106	39953.263	40500	527.345	1687.5	1	H1-1b
34	M25	PL 2"x0.625"	.035	0	41	.011	4.46	y	4	39953.263	40500	527.345	1687.5	1...	H1-1b
35	M32	PL 2"x0.625"	.032	4.46	34	.011	0	y	3	39953.263	40500	527.345	1687.5	1...	H1-1b
36	M4	SR 3/4	.003	2.292	82	.030	44		3	4289.781	14313.866	178.929	178.929	2...	H1-1b*
37	M2	SR 3/4	.000	0	132	.046	41.708		43	4289.781	14313.866	178.929	178.929	1	H1-1a

### Envelope AISI S100-16: LRFD Cold Formed Steel Code Checks

Member	Shape	Code ...	Loc[in]	LC	Shear ...	Loc[in]	Dir	LC	phi*Pn[lb]	phi*Tn[lb]	phi*Mny...	phi*Mnz...	phi*V...	phi*V...	Cb	Eqn
No Data to Print ...																

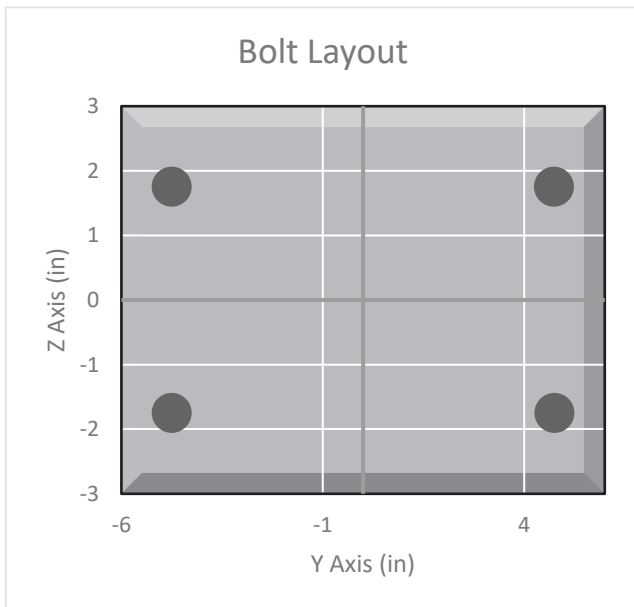
**APPENDIX D**  
**ADDITIONAL CALCULATIONS**

**BOLT TOOL 1.5.3**

Project Data	
Job Code:	231375
Carrier Site ID:	BU 876345
Carrier Site Name:	SKY HILL

Code	
Design Standard:	TIA-222-H
Slip Check:	Yes
Pretension Standard:	TIA-222-H

Bolt Properties		
Connection Type:	Threaded Rod	
Diameter:	0.625	in
Grade:	A307	--
Yield Strength (Fy):	36	ksi
Ultimate Strength (Fu):	60	ksi
Number of Bolts:	4	--
Threads Included:	Yes	--
Double Shear:	No	--
Connection Pipe Size:	9.5	in



Connection Description
Standoff to Tower Leg

Bolt Check*		
Tensile Capacity ( $\phi T_n$ ):	10170.1	lbs
Shear Capacity ( $\phi V_n$ ):	6902.9	lbs
Tension Force ( $T_u$ ):	3304.6	lbs
Shear Force ( $V_u$ ):	461.9	lbs
Tension Usage:	30.9%	--
Shear Usage:	6.4%	--
Interaction:	30.9%	Pass
Controlling Member:	M7	--
Controlling LC:	43	--

\*Rating per TIA-222-H Section 15.5

Slip Check*		
Sliding Capacity ( $\phi R_{ns}$ ):	13417.9	lbs
Torsion Capacity ( $\phi R_{nr}$ ):	5311.3	lb-ft
Sliding Force ( $V_{us}$ ):	1432.8	lbs
Torsional Force ( $T_{ur}$ ):	0.0	lb-ft
Sliding Usage:	10.2%	--
Torsion Usage:	0.0%	--
Interaction:	10.2%	Pass
Controlling Member:	M7	--
Controlling LC:	42	--

\*Rating per TIA-222-H Section 15.5





Date: **September 13, 2023**

MTS Engineering, P.L.L.C.  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
(918) 587-4630

**Subject:** **Structural Analysis Report**

**Carrier Designation:** **T-Mobile Co-Locate**  
**Site Number:** CT11353C  
**Site Name:** Ashford/I-84\_1

**Crown Castle Designation:** **BU Number:** 876345  
**Site Name:** Sky Hill  
**JDE Job Number:** 752565  
**Work Order Number:** 2256449  
**Order Number:** 655749 Rev. 0

**Engineering Firm Designation:** **Project Number:** 77921.018.01.0001

**Site Data:** **33 Janowski Road, Ashford, Windham County, CT**  
**Latitude 41° 57' 7.7", Longitude -72° 11' 43.9"**  
**192 Foot - Self Support Tower**

We are pleased to submit this "**Structural Analysis Report**" to determine the structural integrity of the above-mentioned tower.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Proposed Equipment Configuration

**Sufficient Capacity-69.0%**

This analysis utilizes an ultimate 3-second gust wind speed of 118 mph as required by the 2022 Connecticut State Building Code. Applicable Standard references and design criteria are listed in Section 2 - Analysis Criteria.

Structural analysis prepared by: Jennifer Tillson, E.I.

Respectfully submitted by: MTS Engineering, P.L.L.C.  
COA: PEC.0001564; Expires: 02/01/2024



Chad E. Tuttle, P.E.

tnxTower Report - version 8.1.1.0

## **TABLE OF CONTENTS**

### **1) INTRODUCTION**

### **2) ANALYSIS CRITERIA**

Table 1 - Proposed Equipment Configuration

Table 2 - Other Considered Equipment

### **3) ANALYSIS PROCEDURE**

Table 3 - Documents Provided

3.1) Analysis Method

3.2) Assumptions

### **4) ANALYSIS RESULTS**

Table 4 - Section Capacity (Summary)

Table 5 – Tower Component Stresses vs. Capacity – LC7

4.1) Recommendations

Table 6 - Proposed Loading Tilt-Sway Results – LC7

### **5) APPENDIX A**

tnxTower Output

### **6) APPENDIX B**

Base Level Drawing

### **7) APPENDIX C**

Additional Calculations

## 1) INTRODUCTION

This tower is a 192 ft. Self-Support tower designed by Rohn.

## 2) ANALYSIS CRITERIA

<b>TIA-222 Revision:</b>	TIA-222-H
<b>Risk Category:</b>	II
<b>Wind Speed:</b>	118 mph
<b>Exposure Category:</b>	B
<b>Topographic Factor:</b>	1
<b>Ice Thickness:</b>	1.5 in
<b>Wind Speed with Ice:</b>	50 mph
<b>Service Wind Speed:</b>	60 mph

**Table 1 - Proposed Equipment Configuration**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
156.0	158.0	1	Ceragon	FIBEAIR IP-20A_RFU-D	3 2 2	1-5/8 21/64 7/32
		1	Commscope	VHLP2-11W/A		
		3	Ericsson	AIR 6419 B41_TMO_CCIV2		
		3	Ericsson	RADIO 4449 B71 B85A_T-MOBILE		
		3	Ericsson	RADIO 4460 B2/B25 B66_TMO		
		3	RFS Celwave	APXVAARR24_43-U-NA20		
	156.0	1	--	Sector Mount [SM 503-3]		

**Table 2 - Other Considered Equipment**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
190.0	192.0	3	Alcatel Lucent	PCS 1900MHz 4x45W-65MHz	4 1	1-1/4 1/2
		6	Alcatel Lucent	RRH2X50-800		
		3	Commscope	NNVV-65B-R4		
		3	Nokia	FZHN		
		3	RFS Celwave	APXVTM14-ALU-I20		
	190.0	1	--	Sector Mount [SM 504-3]		
180.0	184.0	1	Symmetricon	58532A	8 1	1-5/8 1/2
	181.0	3	Samsung Telecom.	MT6407-77A		
		6	Antel	LPA-80080/4CF		
		6	Commscope	JAHH-65B-R3B		
		2	Commscope	RC3DC-3315-PF-48		
		3	Samsung Telecom.	RFV01U-D1A		
		3	Samsung Telecom.	RFV01U-D2A		
	180.0	1	--	Sector Mount [SM 304-3]		
171.0	179.0	3	Samsung Telecom.	CBRS	9	1-5/8
	172.0	9	Allgon	7130.16.33.00		
	171.0	1	--	Sector Mount [SM 504-3]		

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
162.0	162.0	3	Andrew	HBX-6516DS-VTM	6	1-5/8
		1	--	Sector Mount [SM 104-3]		
140.0	142.0	1	Raycap	DC6-48-60-0-8C-EV	14 2 2	7/8 3/4 3/8
		1	Raycap	DC6-48-60-18-8F		
	140.0	4	CCI Antennas	TPA65R-BU4D		
		2	Commscope	NNHH-65B-R4		
		3	Ericsson	RRUS 4449 B5/B12		
		3	Ericsson	RRUS 4478 B14		
		3	Ericsson	RRUS 8843 B2/B66A		
		3	Powerwave Tech.	7770.00		
		3	Powerwave Tech.	TT19-08BP111-001		
		1	--	Sector Mount [SM 502-3]		
130.0	130.0	3	Fujitsu	TA08025-B604	1	1-1/2
		3	Fujitsu	TA08025-B605		
		3	JMA Wireless	MX08FRO665-21		
		1	Raycap	RDIDC-9181-PF-48		
		1	--	Commscope MTC3975083 (3)		
98.0	102.0	1	Symmetricom	58532A	1	1/2
	98.0	1	--	Side Arm Mount [SO 305-1]		

### 3) ANALYSIS PROCEDURE

**Table 3 - Documents Provided**

Document	Reference	Source
Tower Manufacturer Drawing	1631630	CCI Sites
Foundation Drawing	1631622	CCI Sites
Geotech Report	2189896	CCI Sites
Crown CAD Package	Date: 09/04/2023	CCI Sites

#### 3.1) Analysis Method

tnxTower (version 8.1.1.0), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A. When applicable, Crown Castle has calculated and provided the effective area for panel antennas using approved methods following the intent of the TIA-222 standard.

#### 3.2) Assumptions

- 1) The tower and structures were maintained in accordance with the - TIA-222 standard.
- 2) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.

This analysis may be affected if any assumptions are not valid or have been made in error. We should be notified to determine the effect on the structural integrity of the tower.

#### 4) ANALYSIS RESULTS

**Table 4 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	192 - 180	Leg	ROHN 2.5 STD	1	-5.741	66.738	8.6	Pass
T2	180 - 160	Leg	ROHN 2.5 STD	25	-29.959	59.996	49.9	Pass
T3	160 - 140	Leg	ROHN 3 EH	55	-59.379	99.054	59.9	Pass
T4	140 - 120	Leg	ROHN 4 EH	76	-94.442	167.894	56.3	Pass
T5	120 - 100	Leg	ROHN 5 EH	97	-128.236	251.347	51.0	Pass
T6	100 - 80	Leg	ROHN 6 EHS	118	-157.572	256.249	61.5	Pass
T7	80 - 60	Leg	ROHN 6 EH	133	-189.468	318.945	59.4	Pass
T8	60 - 40	Leg	ROHN 8 EHS	148	-219.603	405.672	54.1	Pass
T9	40 - 20	Leg	ROHN 8 EHS	163	-249.733	405.729	61.6	Pass
T10	20 - 0	Leg	ROHN 8 EHS	178	-279.877	405.717	69.0	Pass
T1	192 - 180	Diagonal	L1 3/4x1 3/4x3/16	7	-1.215	11.895	10.2	Pass
T2	180 - 160	Diagonal	L2x2x3/16	36	-3.299	10.392	31.7	Pass
T3	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-4.976	16.480	30.2	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	81	-6.527	12.587	51.9	Pass
T5	120 - 100	Diagonal	L3x3x1/4	102	-6.866	17.432	39.4	Pass
T6	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	123	-8.013	19.016	42.1	Pass
T7	80 - 60	Diagonal	L4x4x1/4	138	-8.807	24.136	36.5	Pass
T8	60 - 40	Diagonal	L4x4x5/16	153	-8.446	24.922	33.9	Pass
T9	40 - 20	Diagonal	L4x4x5/16	168	-10.043	21.484	46.7	Pass
T10	20 - 0	Diagonal	L4x4x3/8	183	-10.583	21.926	48.3	Pass
T1	192 - 180	Top Girt	L1 3/4x1 3/4x3/16	5	-0.069	4.122	1.7	Pass
T2	180 - 160	Top Girt	L2x2x3/16	29	-0.643	6.245	10.3	Pass
							Summary	
						Leg (T10)	69.0	Pass
						Diagonal (T4)	51.9	Pass
						Top Girt (T2)	10.3	Pass
						Bolt Checks	58.7	Pass
						Rating =	69.0	Pass

**Table 5 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1,2	Anchor Rods	Base	39.0	Pass
1,2	Base Foundation (Structure)	Base	12.2	Pass
1,2	Base Foundation (Soil Interaction)	Base	40.5	Pass

<b>Structure Rating (max from all components) =</b>	<b>69.0%</b>
---	--------------

Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.
- 2) Rating per TIA-222-H Section 15.5.

#### 4.1) Recommendations

The tower and its foundations have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

The results of the tilt and twist values for a 60 mph 3-second gust service wind speed per the TIA-222-H standard are given below:

**Table 6 – Proposed Equipment Tilt-Sway Results for 60 mph Service Wind – LC7**

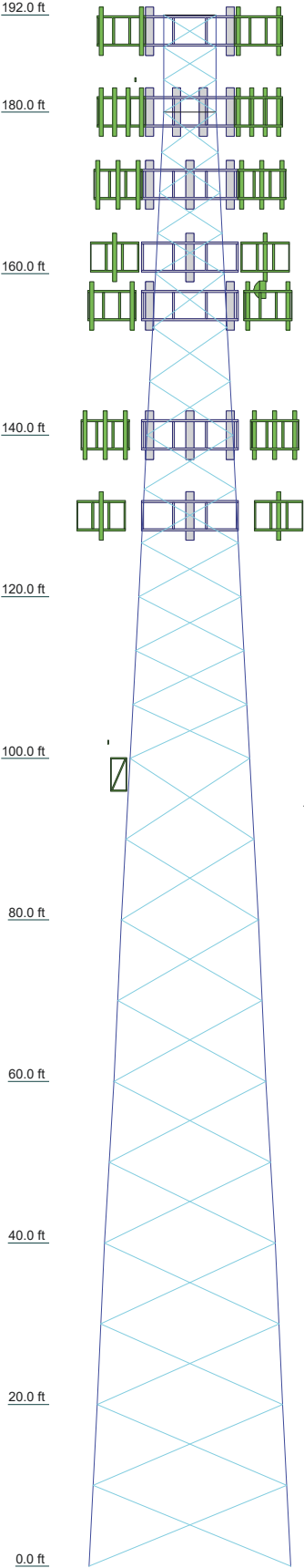
<i><b>Elevation (ft)</b></i>	<i><b>Dish Model</b></i>	<i><b>Diameter (ft)</b></i>	<i><b>Tilt (°)</b></i>	<i><b>Twist (°)</b></i>
158.0	VHLP2-11W/A	2.167	0.171	0.019

## **APPENDIX A**

### **TNXTOWER OUTPUT**



Section	T10	T9	T8	T7	T6	T5	T4	T3	T2	T1		
Legs	ROHN 8 EHS			ROHN 6 EH	ROHN 6 EHS	ROHN 5 EH	ROHN 4 EH	ROHN 3 EH	ROHN 2.5 STD			
Leg Grade	A572-50											
Diagonals	L4x4x3/8	L4x4x5/16		L4x4x1/4	L3 1/2x3 1/2x1/4	L3x3x1/4	L2 1/2x2 1/2x1/4				A	
Diagonal Grade	A572-50					A36						
Top Girts	N.A.									L2x2x3/16	A	
Face Width (ft)	23.05	21.13	18.88	16.92	14.83	12.74	10.61	8.54	6.58			
# Panels @ (ft)	10 @ 10			9 @ 6.66667							4 @ 5	3 @ 4
Weight (K)	5.3	4.6	4.4	3.5	2.8	2.7	2.0	1.5	1.0	0.6		



SYMBOL LIST			
MARK	SIZE		MARK
A	L1 3/4x1 3/4x3/16		

MATERIAL STRENGTH					
GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	65 ksi	A36	36 ksi	58 ksi

### TOWER DESIGN NOTES

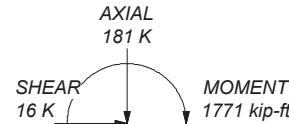
1. Tower is located in Windham County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-H Standard.
3. Tower designed for a 118 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 50 mph basic wind with 1.50 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.000 ft
8. TIA-222-H Annex S
9. TOWER RATING: 69%

ALL REACTIONS  
ARE FACTORED

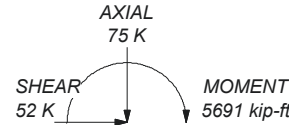
MAX. CORNER REACTIONS AT BASE:

DOWN: 287 K  
SHEAR: 33 K


UPLIFT: -233 K  
SHEAR: 27 K



TORQUE 11 kip-ft  
50 mph WIND - 1.500 in ICE



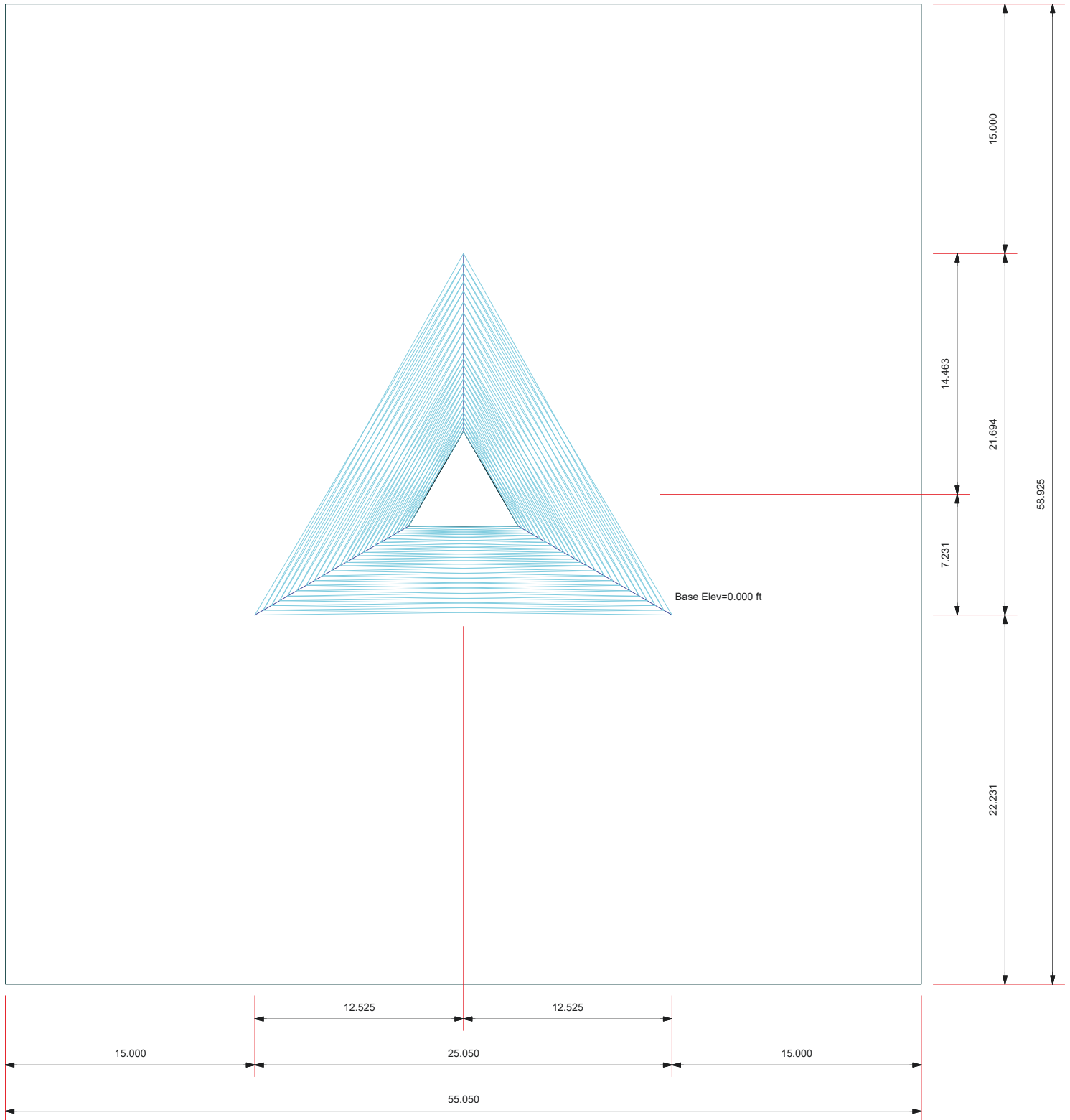
TORQUE 43 kip-ft  
REACTIONS - 118 mph WIND



**MTS Engineering, P.L.L.C.**  
 1717 S. Boulder, Suite 300  
 Tulsa, OK 74119  
 Phone: (918) 587-4630  
 FAX: (918) 587-4630

Job: 77921.018.01.0001 - SKY HILL, CT (BU# 87634)		
Project:		
Client: Crown Castle	Drawn by: R AITHAL	App'd:
Code: TIA-222-H	Date: 09/13/23	Scale: NTS
Path:	Dwg No: E-1	

**Plot Plan**  
**Total Area - 0.07 Acres**



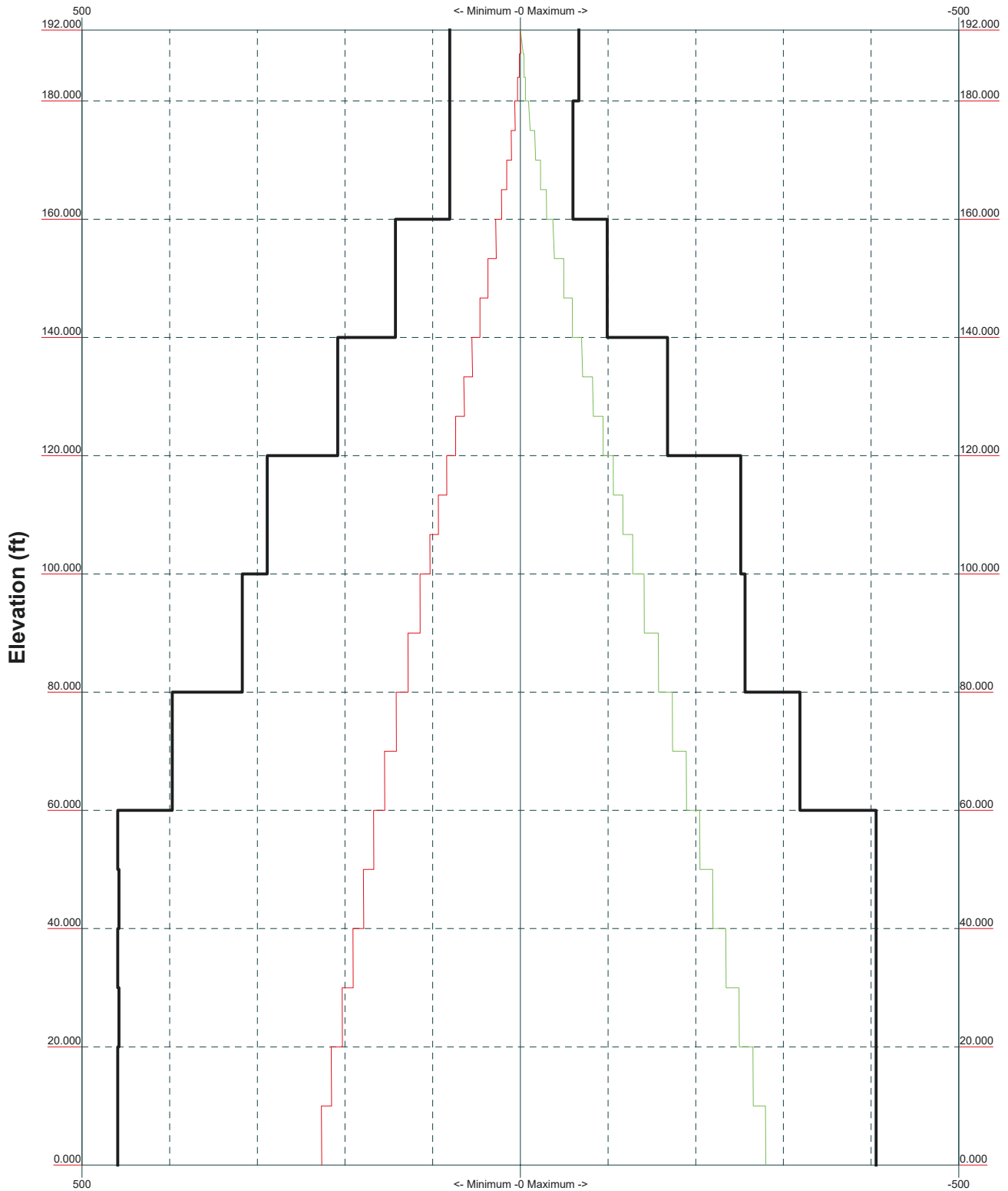
**MTS Engineering, P.L.L.C.**  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
Phone: (918) 587-4630  
FAX: (918) 587-4630

Job: <b>77921.018.01.0001 - SKY HILL, CT (BU# 87634)</b>		
Project:		
Client: Crown Castle	Drawn by: R AITHAL	App'd:
Code: TIA-222-H	Date: 09/13/23	Scale: NTS
Path:	Dwg No: E-2	

# TIA-222-H - 118 mph/50 mph 1.500 in Ice Exposure B

Leg Capacity ———

Leg Compression (K)



**MTS Engineering, P.L.L.C.**

1717 S. Boulder, Suite 300

Tulsa, OK 74119

Phone: (918) 587-4630

FAX: (918) 587-4630

Job: **77921.018.01.0001 - SKY HILL, CT (BU# 87634)**

Project:

Client: Crown Castle

Drawn by: R AITHAL

App'd:

Code: TIA-222-H

Date: 09/13/23

Scale: NTS

Path:

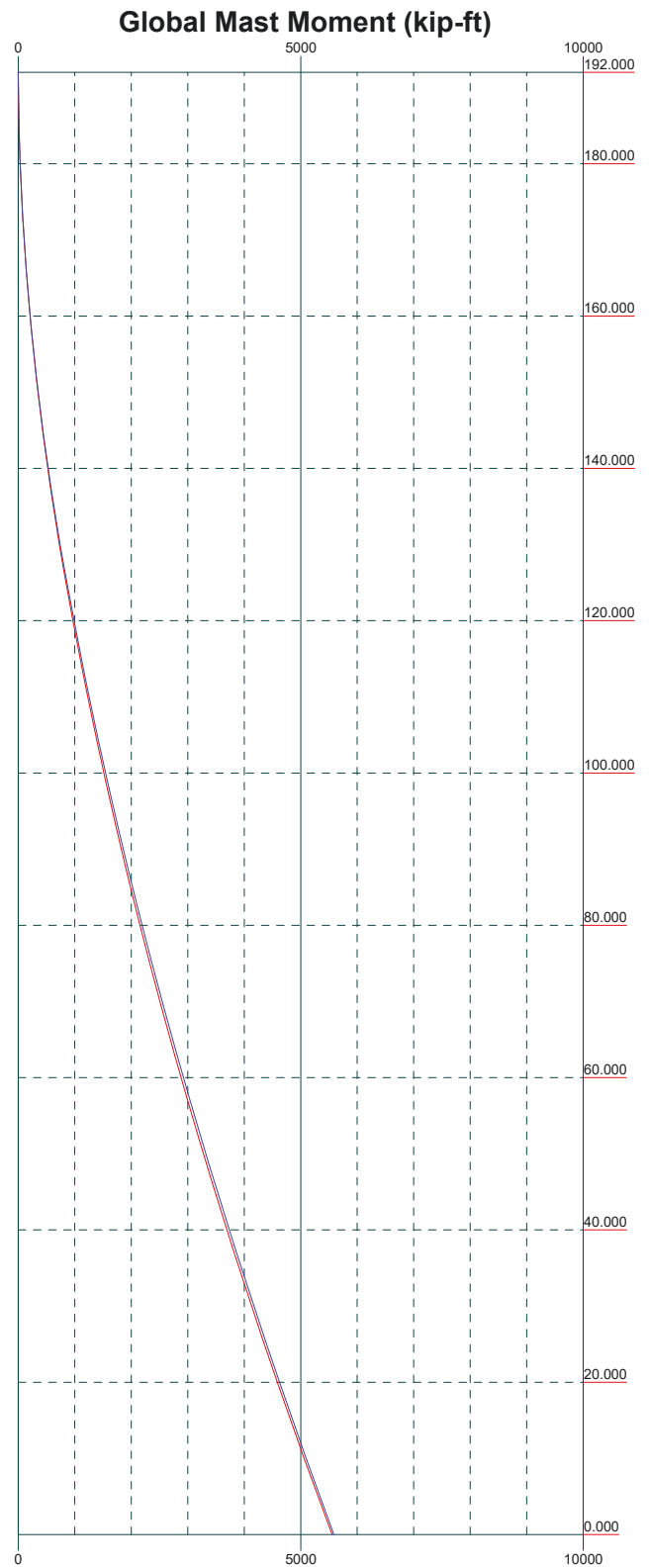
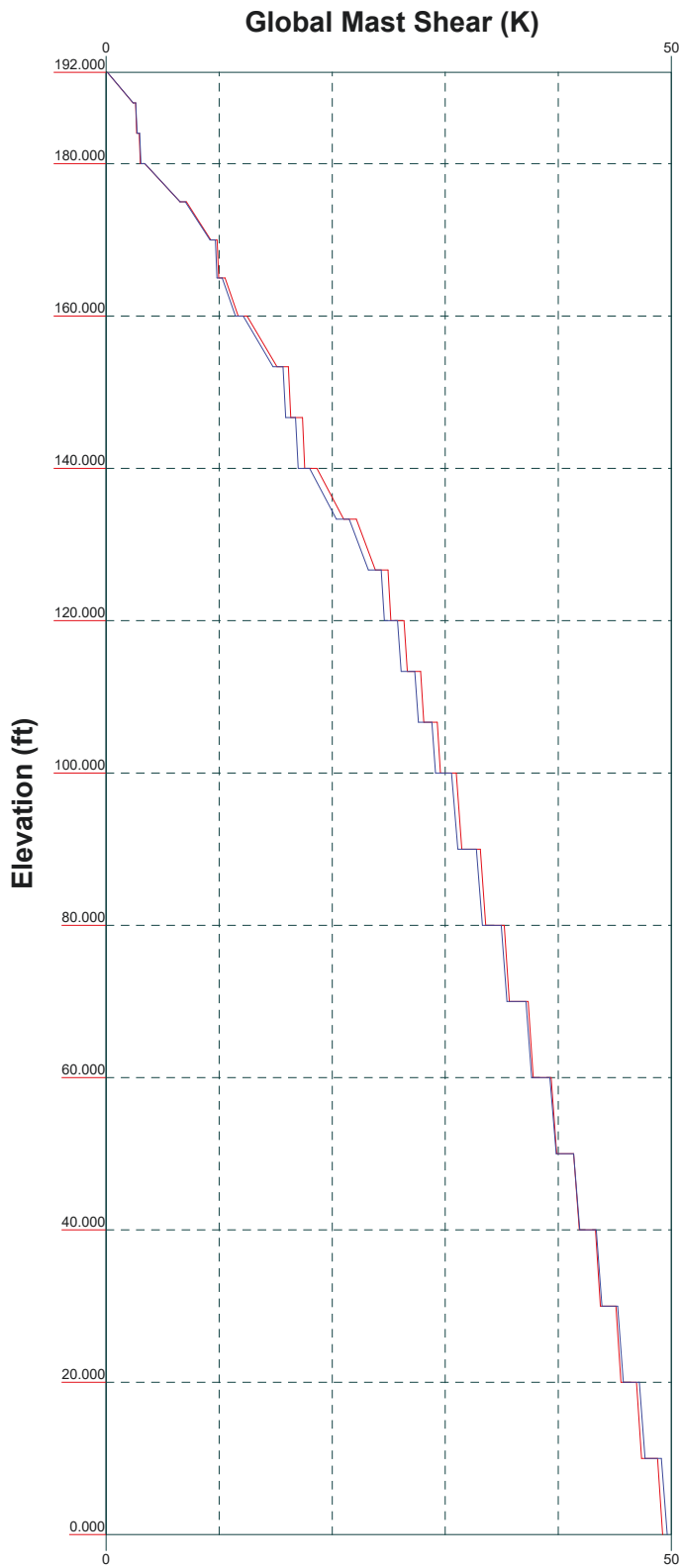
Dwg No: E-3

Vx

Vz

Mx

Mz



**MTS Engineering, P.L.L.C.**

1717 S. Boulder, Suite 300

Tulsa, OK 74119

Phone: (918) 587-4630

FAX: (918) 587-4630

Job: **77921.018.01.0001 - SKY HILL, CT (BU# 87634)**

Project:

Client: Crown Castle

Drawn by: R AITHAL

App'd:

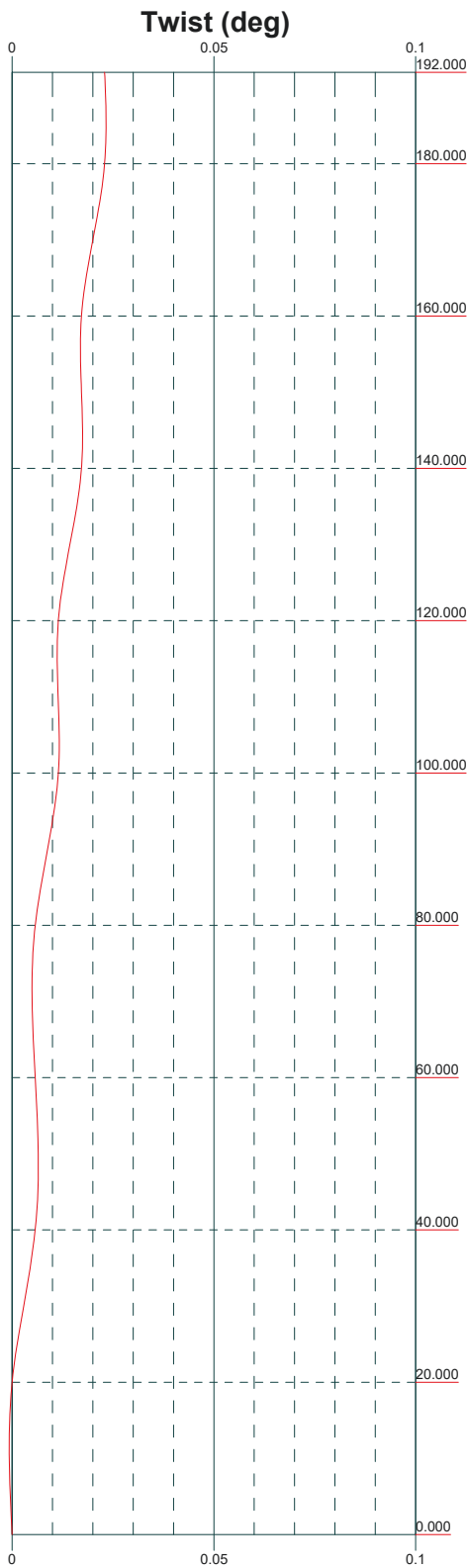
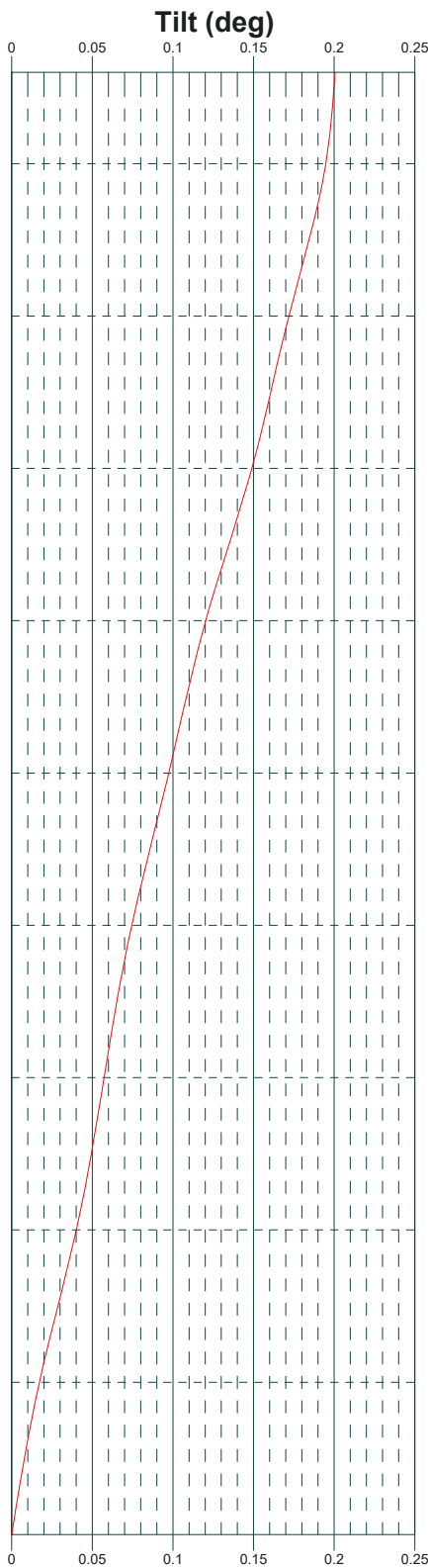
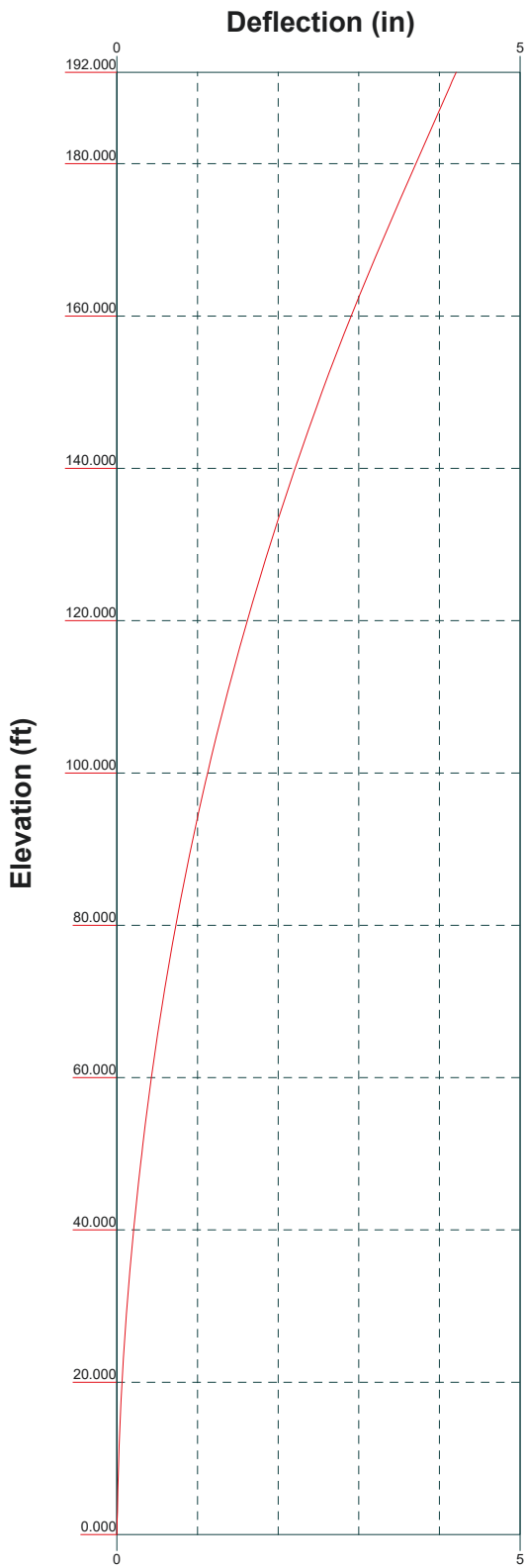
Code: TIA-222-H


Date: 09/13/23

Scale: NTS

Path:

Dwg No: E-4



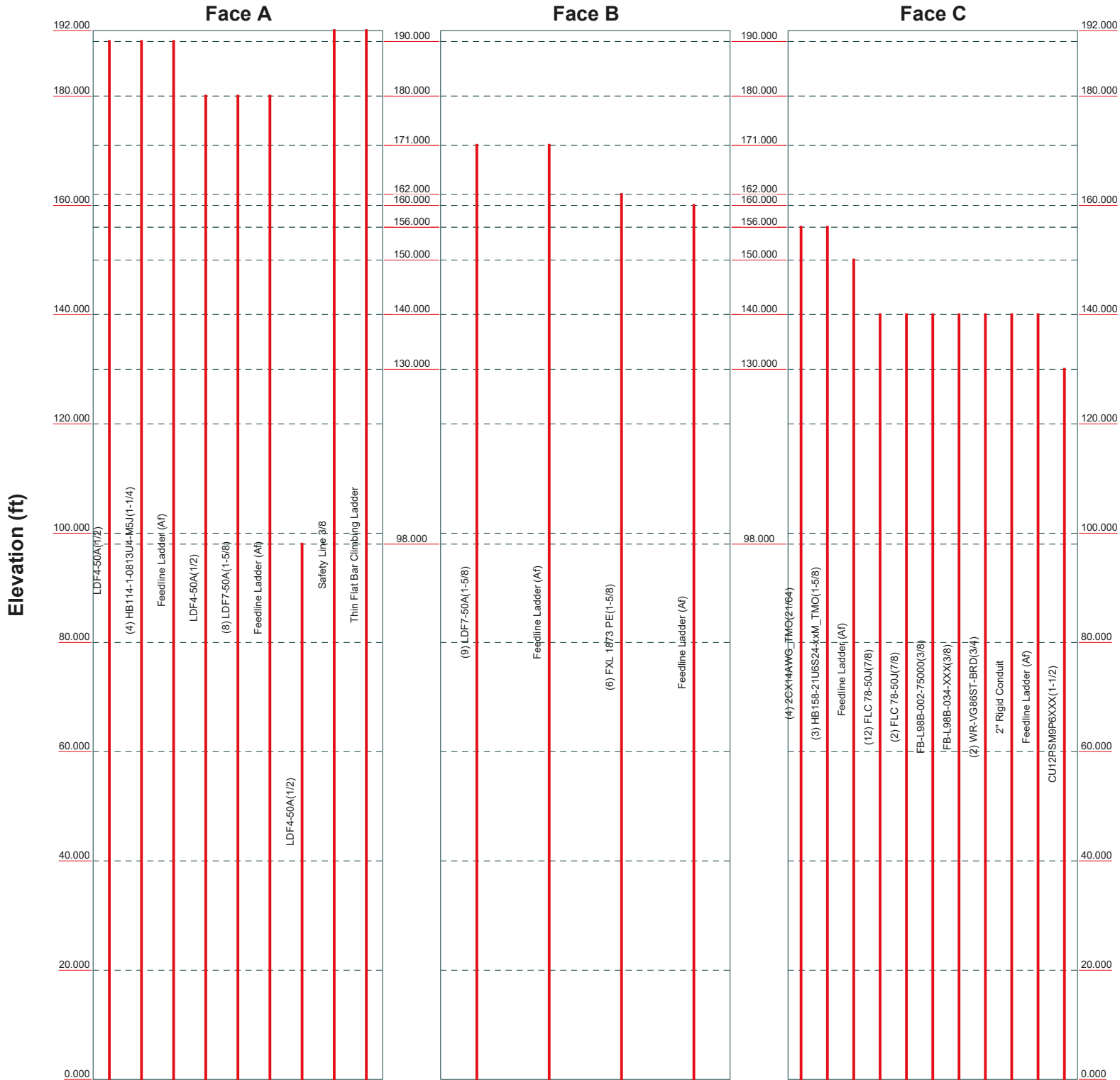


**MTS Engineering, P.L.L.C.**  
1717 S. Boulder, Suite 300  
Tulsa, OK 74119  
Phone: (918) 587-4630  
FAX: (918) 587-4630

Job: 77921.018.01.0001 - SKY HILL, CT (BU# 87634)		
Project:		
Client: Crown Castle	Drawn by: R AITHAL	App'd:
Code: TIA-222-H	Date: 09/13/23	Scale: NTS
Path:	Dwg No: E-5	

Feed Line Distribution Chart  
0' - 192'

Round Flat App In Face App Out Face Truss Leg



<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b> 1 of 35
	<b>Project</b>	<b>Date</b> 15:20:18 09/13/23
	<b>Client</b> Crown Castle	<b>Designed by</b> R AITHAL

## Tower Input Data

The main tower is a 3x free standing tower with an overall height of 192.000 ft above the ground line.

The base of the tower is set at an elevation of 0.000 ft above the ground line.

The face width of the tower is 6.580 ft at the top and 25.050 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Windham County, Connecticut.

Tower base elevation above sea level: 1066.000 ft.

Basic wind speed of 118 mph.

Risk Category II.

Exposure Category B.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.000 ft.

Nominal ice thickness of 1.500 in.

Ice thickness is considered to increase with height.

Ice density of 56.000 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50.000 °F.

Deflections calculated using a wind speed of 60 mph.

TIA-222-H Annex S.

Pressures are calculated at each section.

Stress ratio used in tower member design is 1.

Tower analysis based on target reliabilities in accordance with Annex S.

Load Modification Factors used:  $K_{es}(F_w) = 0.95$ ,  $K_{es}(t_i) = 0.85$ .

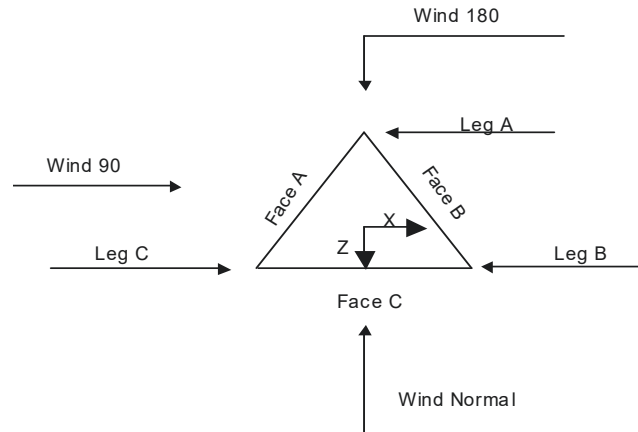
Maximum demand-capacity ratio is: 1.05.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	√ Calculate Redundant Bracing Forces
Consider Moments - Diagonals	√ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	√ Use Clear Spans For Wind Area	√ SR Leg Bolts Resist Compression
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	All Leg Panels Have Same Allowable
√ Use Code Safety Factors - Guys	Retension Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	√ Bypass Mast Stability Checks	√ Consider Feed Line Torque
Always Use Max Kz	√ Use Azimuth Dish Coefficients	√ Include Angle Block Shear Check
Use Special Wind Profile	√ Project Wind Area of Appurt.	Use TIA-222-H Bracing Resist. Exemption
Include Bolts In Member Capacity	Autocalc Torque Arm Areas	Use TIA-222-H Tension Splice Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Poles
√ Secondary Horizontal Braces Leg	√ Sort Capacity Reports By Component	Include Shear-Torsion Interaction
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Always Use Sub-Critical Flow
SR Members Have Cut Ends	Treat Feed Line Bundles As Cylinder	Use Top Mounted Sockets
SR Members Are Concentric	Ignore KL/ry For 60 Deg. Angle Legs	Pole Without Linear Attachments
		Pole With Shroud Or No Appurtenances
		Outside and Inside Corner Radii Are
		Known

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	2 of 35
	<b>Project</b>	<b>Date</b>
		15:20:18 09/13/23
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL



**Triangular Tower**

## Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	192.000-180.000			6.580	1	12.000
T2	180.000-160.000			6.580	1	20.000
T3	160.000-140.000			8.540	1	20.000
T4	140.000-120.000			10.610	1	20.000
T5	120.000-100.000			12.740	1	20.000
T6	100.000-80.000			14.830	1	20.000
T7	80.000-60.000			16.920	1	20.000
T8	60.000-40.000			18.880	1	20.000
T9	40.000-20.000			21.130	1	20.000
T10	20.000-0.000			23.050	1	20.000

## Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	192.000-180.000	4.000	X Brace	No	No	0.000	0.000
T2	180.000-160.000	5.000	X Brace	No	No	0.000	0.000
T3	160.000-140.000	6.667	X Brace	No	No	0.000	0.000



<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	3 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

<i>Tower Section</i>	<i>Tower Elevation</i>	<i>Diagonal Spacing</i>	<i>Bracing Type</i>	<i>Has K Brace End Panels</i>	<i>Has Horizontals</i>	<i>Top Girt Offset</i>	<i>Bottom Girt Offset</i>
	<i>ft</i>	<i>ft</i>				<i>in</i>	<i>in</i>
T4	140.000-120.000	6.667	X Brace	No	No	0.000	0.000
T5	120.000-100.000	6.667	X Brace	No	No	0.000	0.000
T6	100.000-80.000	10.000	X Brace	No	No	0.000	0.000
T7	80.000-60.000	10.000	X Brace	No	No	0.000	0.000
T8	60.000-40.000	10.000	X Brace	No	No	0.000	0.000
T9	40.000-20.000	10.000	X Brace	No	No	0.000	0.000
T10	20.000-0.000	10.000	X Brace	No	No	0.000	0.000

### Tower Section Geometry (cont'd)

<i>Tower Elevation</i> <i>ft</i>	<i>Leg Type</i>	<i>Leg Size</i>	<i>Leg Grade</i>	<i>Diagonal Type</i>	<i>Diagonal Size</i>	<i>Diagonal Grade</i>
T1 192.000-180.000	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 180.000-160.000	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L2x2x3/16	A36 (36 ksi)
T3 160.000-140.000	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T4 140.000-120.000	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T5 120.000-100.000	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L3x3x1/4	A572-50 (50 ksi)
T6 100.000-80.000	Pipe	ROHN 6 EHS	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T7 80.000-60.000	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L4x4x1/4	A572-50 (50 ksi)
T8 60.000-40.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x5/16	A572-50 (50 ksi)
T9 40.000-20.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x5/16	A572-50 (50 ksi)
T10 20.000-0.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

<i>Tower Elevation</i> <i>ft</i>	<i>Top Girt Type</i>	<i>Top Girt Size</i>	<i>Top Girt Grade</i>	<i>Bottom Girt Type</i>	<i>Bottom Girt Size</i>	<i>Bottom Girt Grade</i>
T1 192.000-180.000	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T2 180.000-160.000	Equal Angle	L2x2x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)

### Tower Section Geometry (cont'd)





<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b> 6 of 35
	<b>Project</b>	<b>Date</b> 15:20:18 09/13/23
	<b>Client</b> Crown Castle	<b>Designed by</b> R AITHAL

Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 192.000-180.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T2 180.000-160.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T3 160.000-140.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 140.000-120.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 120.000-100.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 100.000-80.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 80.000-60.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T8 60.000-40.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T9 40.000-20.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T10 20.000-0.000	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 192.000-180.000	Flange	0.625 A325N	4	0.625 A325N	1	0.625 A325N	1	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T2 180.000-160.000	Flange	0.625 A325N	4	0.625 A325N	1	0.625 A325N	1	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T3 160.000-140.000	Flange	0.875 A325N	4	0.625 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T4 140.000-120.000	Flange	1.000 A325N	4	0.625 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T5 120.000-100.000	Flange	1.000 A325N	6	0.750 A325N	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T6 100.000-80.000	Flange	1.000 A325N	6	0.750 A325N	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	7 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T7 80.000-60.000	Flange	1.000 A325N	8	0.750 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T8 60.000-40.000	Flange	1.000 A325N	8	0.750 A325X	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T9 40.000-20.000	Flange	1.000 A325N	8	0.750 A325X	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T10 20.000-0.000	Flange	0.000 A354-BC	0	0.750 A325X	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
LDF4-50A(1/2)	A	No	No	Ar (CaAa)	190.000 - 0.000	0.000	-0.385	1	1	0.500	0.630		0.000
HB114-1-081 3U4-M5J(1-1/4)	A	No	No	Ar (CaAa)	190.000 - 0.000	0.000	-0.405	4	4	0.850 0.750	1.540		0.001
Feedline Ladder (Af) *	A	No	No	Af (CaAa)	190.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		0.008
LDF4-50A(1/2)	A	No	No	Ar (CaAa)	180.000 - 0.000	0.000	0.445	1	1	0.500	0.630		0.000
LDF7-50A(1-5/8)	A	No	No	Ar (CaAa)	180.000 - 0.000	0.000	0.4	8	8	0.850 0.750	1.980		0.001
Feedline Ladder (Af) *	A	No	No	Af (CaAa)	180.000 - 0.000	0.000	0.4	1	1	3.000	3.000		0.008
LDF7-50A(1-5/8)	B	No	No	Ar (CaAa)	171.000 - 0.000	0.000	-0.4	9	9	0.850 0.750	1.980		0.001
Feedline Ladder (Af) *	B	No	No	Af (CaAa)	171.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		0.008
FXL 1873 PE(1-5/8)	B	No	No	Ar (CaAa)	162.000 - 0.000	-3.500	0.4	6	3	0.850 0.750	1.980		0.001
Feedline Ladder (Af) *	B	No	No	Af (CaAa)	160.000 - 0.000	-0.500	0.4	1	1	3.000	3.000		0.008
2CX14AWG_ TMO(21/64)	C	No	No	Ar (CaAa)	156.000 - 0.000	0.000	0.42	4	2	0.500	0.320		0.000
HB158-21U6S 24-xxM TMO (1-5/8)	C	No	No	Ar (CaAa)	156.000 - 0.000	0.000	0.4	3	3	0.850 0.750	1.996		0.003
Feedline Ladder (Af) *	C	No	No	Af (CaAa)	150.000 - 0.000	0.000	0.4	1	1	3.000	3.000		0.008
FLC 78-50J(7/8)	C	No	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.41	12	12	1.000 0.750	1.112		0.000
FLC 78-50J(7/8)	C	No	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.355	2	2	0.750	1.112		0.000
FB-L98B-002- 75000(3/8)	C	No	No	Ar (CaAa)	140.000 - 0.000	1.750	-0.355	1	1	0.400	0.394		0.000

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	8 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight klf
FB-L98B-034-XXX(3/8)	C	No	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.37	1	1	0.500	0.394		0.000
WR-VG86ST-BRD(3/4)	C	No	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.37	2	2	0.500	0.795		0.001
2" Rigid Conduit	C	No	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.37	1	1	2.000	2.000		0.003
Feedline Ladder (Af)	C	No	No	Af (CaAa)	140.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		0.008
* CU12PSM9P6 XXX(1-1/2)	C	No	No	Ar (CaAa)	130.000 - 0.000	0.000	0.38	1	1	0.850 0.750	1.600		0.002
* LDF4-50A(1/2)	A	No	No	Ar (CaAa)	98.000 - 0.000	0.000	-0.375	1	1	0.500	0.630		0.000
* Safety Line 3/8	A	No	No	Ar (CaAa)	192.000 - 0.000	-6.000	0.45	1	1	0.375	0.375		0.000
* Thin Flat Bar Climbing Ladder	A	No	No	Af (CaAa)	192.000 - 0.000	-6.000	0.45	1	1	2.000	2.000		0.004
* *													

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number	C <sub>A</sub> A <sub>A</sub> ft <sup>2</sup> /ft	Weight klf
* *								

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	192.000-180.000	A	0.000	0.000	16.240	0.000	0.184
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	0.000
T2	180.000-160.000	A	0.000	0.000	73.937	0.000	0.654
		B	0.000	0.000	27.478	0.000	0.182
		C	0.000	0.000	0.000	0.000	0.000
T3	160.000-140.000	A	0.000	0.000	73.937	0.000	0.654
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	16.629	0.000	0.208
T4	140.000-120.000	A	0.000	0.000	73.937	0.000	0.654
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	76.027	0.000	0.709
T5	120.000-100.000	A	0.000	0.000	73.937	0.000	0.654

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	9 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	15:20:18 09/13/23
	Crown Castle	<b>Designed by</b>
		R AITHAL

<i>Tower Section</i>	<i>Tower Elevation ft</i>	<i>Face</i>	<i>A<sub>R</sub></i> <i>ft<sup>2</sup></i>	<i>A<sub>F</sub></i> <i>ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub></i> <i>In Face ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub></i> <i>Out Face ft<sup>2</sup></i>	<i>Weight</i> <i>K</i>
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	77.627	0.000	0.732
T6	100.000-80.000	A	0.000	0.000	75.071	0.000	0.656
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	77.627	0.000	0.732
T7	80.000-60.000	A	0.000	0.000	75.197	0.000	0.657
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	77.627	0.000	0.732
T8	60.000-40.000	A	0.000	0.000	75.197	0.000	0.657
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	77.627	0.000	0.732
T9	40.000-20.000	A	0.000	0.000	75.197	0.000	0.657
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	77.627	0.000	0.732
T10	20.000-0.000	A	0.000	0.000	75.197	0.000	0.657
		B	0.000	0.000	79.400	0.000	0.564
		C	0.000	0.000	77.627	0.000	0.732

## Feed Line/Linear Appurtenances Section Areas - With Ice

<i>Tower Section</i>	<i>Tower Elevation ft</i>	<i>Face or Leg</i>	<i>Ice Thickness in</i>	<i>A<sub>R</sub></i> <i>ft<sup>2</sup></i>	<i>A<sub>F</sub></i> <i>ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub></i> <i>In Face ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub></i> <i>Out Face ft<sup>2</sup></i>	<i>Weight</i> <i>K</i>
T1	192.000-180.000	A	1.516	0.000	0.000	39.168	0.000	0.630
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	0.000	0.000	0.000
T2	180.000-160.000	A	1.502	0.000	0.000	160.625	0.000	2.502
		B		0.000	0.000	50.688	0.000	0.788
		C		0.000	0.000	0.000	0.000	0.000
T3	160.000-140.000	A	1.483	0.000	0.000	159.946	0.000	2.475
		B		0.000	0.000	134.668	0.000	2.224
		C		0.000	0.000	43.091	0.000	0.636
T4	140.000-120.000	A	1.462	0.000	0.000	159.180	0.000	2.446
		B		0.000	0.000	134.230	0.000	2.201
		C		0.000	0.000	205.356	0.000	2.815
T5	120.000-100.000	A	1.438	0.000	0.000	158.300	0.000	2.411
		B		0.000	0.000	133.727	0.000	2.175
		C		0.000	0.000	208.492	0.000	2.851
T6	100.000-80.000	A	1.410	0.000	0.000	163.470	0.000	2.438
		B		0.000	0.000	133.134	0.000	2.145
		C		0.000	0.000	206.854	0.000	2.801
T7	80.000-60.000	A	1.375	0.000	0.000	162.750	0.000	2.394
		B		0.000	0.000	132.407	0.000	2.107
		C		0.000	0.000	204.850	0.000	2.741
T8	60.000-40.000	A	1.329	0.000	0.000	160.918	0.000	2.328
		B		0.000	0.000	131.464	0.000	2.059
		C		0.000	0.000	202.246	0.000	2.663
T9	40.000-20.000	A	1.263	0.000	0.000	158.254	0.000	2.234
		B		0.000	0.000	130.091	0.000	1.990
		C		0.000	0.000	198.458	0.000	2.553
T10	20.000-0.000	A	1.132	0.000	0.000	152.973	0.000	2.054
		B		0.000	0.000	127.371	0.000	1.856
		C		0.000	0.000	190.949	0.000	2.340

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	10 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

### Feed Line Center of Pressure

Section	Elevation	CP <sub>x</sub>	CP <sub>z</sub>	CP <sub>x</sub> Ice	CP <sub>z</sub> Ice
	ft	in	in	in	in
T1	192.000-180.000	-4.804	0.523	-5.800	-0.754
T2	180.000-160.000	-4.537	-16.388	-5.753	-16.388
T3	160.000-140.000	-2.656	-15.515	-5.263	-15.112
T4	140.000-120.000	4.591	-12.443	5.635	-9.557
T5	120.000-100.000	4.548	-13.026	5.707	-10.176
T6	100.000-80.000	4.905	-14.724	5.605	-11.255
T7	80.000-60.000	5.072	-15.434	5.989	-12.215
T8	60.000-40.000	5.413	-16.493	6.490	-13.192
T9	40.000-20.000	5.723	-17.488	7.140	-14.450
T10	20.000-0.000	6.027	-18.473	7.963	-15.928

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T1	1	LDF4-50A(1/2)	180.00 - 190.00	0.6000	0.6000
T1	2	HB114-1-0813U4-M5J(1-1/4)	180.00 - 190.00	0.6000	0.6000
T1	4	Feedline Ladder (Af)	180.00 - 190.00	0.6000	0.6000
T1	36	Safety Line 3/8	180.00 - 192.00	0.6000	0.6000
T1	37	Thin Flat Bar Climbing Ladder	180.00 - 192.00	0.6000	0.6000
T2	1	LDF4-50A(1/2)	160.00 - 180.00	0.6000	0.6000
T2	2	HB114-1-0813U4-M5J(1-1/4)	160.00 - 180.00	0.6000	0.6000
T2	4	Feedline Ladder (Af)	160.00 - 180.00	0.6000	0.6000
T2	6	LDF4-50A(1/2)	160.00 - 180.00	0.6000	0.6000
T2	7	LDF7-50A(1-5/8)	160.00 - 180.00	0.6000	0.6000
T2	10	Feedline Ladder (Af)	160.00 - 180.00	0.6000	0.6000
T2	12	LDF7-50A(1-5/8)	160.00 - 171.00	0.6000	0.6000
T2	13	Feedline Ladder (Af)	160.00 - 171.00	0.6000	0.6000
T2	15	FXL 1873 PE(1-5/8)	160.00 - 162.00	0.6000	0.6000
T2	36	Safety Line 3/8	160.00 - 180.00	0.6000	0.6000
T2	37	Thin Flat Bar Climbing Ladder	160.00 - 180.00	0.6000	0.6000
T3	1	LDF4-50A(1/2)	140.00 - 160.00	0.6000	0.6000
T3	2	HB114-1-0813U4-M5J(1-1/4)	140.00 - 160.00	0.6000	0.6000
T3	4	Feedline Ladder (Af)	140.00 -	0.6000	0.6000



<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b> 11 of 35
	<b>Project</b>	<b>Date</b> 15:20:18 09/13/23
	<b>Client</b> Crown Castle	<b>Designed by</b> R AITHAL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T3	6	LDF4-50A(1/2)	160.00 140.00 -	0.6000	0.6000
T3	7	LDF7-50A(1-5/8)	160.00 140.00 -	0.6000	0.6000
T3	10	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
T3	12	LDF7-50A(1-5/8)	160.00 140.00 -	0.6000	0.6000
T3	13	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
T3	15	FXL 1873 PE(1-5/8)	160.00 140.00 -	0.6000	0.6000
T3	16	Feedline Ladder (Af)	160.00 140.00 -	0.6000	0.6000
T3	19	2CX14AWG_TMO(21/64)	160.00 140.00 -	0.6000	0.6000
T3	21	HB158-21U6S24-xxM_TMO (1-5/8)	156.00 140.00 -	0.6000	0.6000
T3	22	Feedline Ladder (Af)	156.00 140.00 -	0.6000	0.6000
T3	36	Safety Line 3/8	150.00 140.00 -	0.6000	0.6000
T3	37	Thin Flat Bar Climbing Ladder	160.00 140.00 -	0.6000	0.6000
T4	1	LDF4-50A(1/2)	160.00 120.00 -	0.6000	0.6000
T4	2	HB114-1-0813U4-M5J(1-1/4 )	140.00 120.00 -	0.6000	0.6000
T4	4	Feedline Ladder (Af)	140.00 120.00 -	0.6000	0.6000
T4	6	LDF4-50A(1/2)	140.00 120.00 -	0.6000	0.6000
T4	7	LDF7-50A(1-5/8)	140.00 120.00 -	0.6000	0.6000
T4	10	Feedline Ladder (Af)	140.00 120.00 -	0.6000	0.6000
T4	12	LDF7-50A(1-5/8)	140.00 120.00 -	0.6000	0.6000
T4	13	Feedline Ladder (Af)	140.00 120.00 -	0.6000	0.6000
T4	15	FXL 1873 PE(1-5/8)	140.00 120.00 -	0.6000	0.6000
T4	16	Feedline Ladder (Af)	140.00 120.00 -	0.6000	0.6000
T4	19	2CX14AWG_TMO(21/64)	140.00 120.00 -	0.6000	0.6000
T4	21	HB158-21U6S24-xxM_TMO (1-5/8)	140.00 120.00 -	0.6000	0.6000
T4	22	Feedline Ladder (Af)	140.00 120.00 -	0.6000	0.6000
T4	24	FLC 78-50J(7/8)	140.00 120.00 -	0.6000	0.6000
T4	25	FLC 78-50J(7/8)	140.00 120.00 -	0.6000	0.6000
T4	26	FB-L98B-002-75000(3/8)	140.00 120.00 -	0.6000	0.6000
T4	27	FB-L98B-034-XXX(3/8)	140.00 120.00 -	0.0000	0.0000
T4	28	WR-VG86ST-BRD(3/4)	140.00 120.00 -	0.0000	0.0000
T4	29	2" Rigid Conduit	120.00 -	0.6000	0.6000

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b> 77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b> 12 of 35
	<b>Project</b>	<b>Date</b> 15:20:18 09/13/23
	<b>Client</b> Crown Castle	<b>Designed by</b> R AITHAL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
			140.00		
T4	30	Feedline Ladder (Af)	120.00 -	0.6000	0.6000
			140.00		
T4	32	CU12PSM9P6XXX(1-1/2)	120.00 -	0.6000	0.6000
			130.00		
T4	36	Safety Line 3/8	120.00 -	0.6000	0.6000
			140.00		
T4	37	Thin Flat Bar Climbing Ladder	120.00 -	0.6000	0.6000
			140.00		
T5	1	LDF4-50A(1/2)	100.00 -	0.6000	0.6000
			120.00		
T5	2	HB114-1-0813U4-M5J(1-1/4 )	100.00 -	0.6000	0.6000
			120.00		
T5	4	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	6	LDF4-50A(1/2)	100.00 -	0.6000	0.6000
			120.00		
T5	7	LDF7-50A(1-5/8)	100.00 -	0.6000	0.6000
			120.00		
T5	10	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	12	LDF7-50A(1-5/8)	100.00 -	0.6000	0.6000
			120.00		
T5	13	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	15	FXL 1873 PE(1-5/8)	100.00 -	0.6000	0.6000
			120.00		
T5	16	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	19	2CX14AWG_TMO(21/64)	100.00 -	0.6000	0.6000
			120.00		
T5	21	HB158-21U6S24-xxM_TMO (1-5/8)	100.00 -	0.6000	0.6000
			120.00		
T5	22	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	24	FLC 78-50J(7/8)	100.00 -	0.6000	0.6000
			120.00		
T5	25	FLC 78-50J(7/8)	100.00 -	0.6000	0.6000
			120.00		
T5	26	FB-L98B-002-75000(3/8)	100.00 -	0.6000	0.6000
			120.00		
T5	27	FB-L98B-034-XXX(3/8)	100.00 -	0.0000	0.0000
			120.00		
T5	28	WR-VG86ST-BRD(3/4)	100.00 -	0.0000	0.0000
			120.00		
T5	29	2" Rigid Conduit	100.00 -	0.6000	0.6000
			120.00		
T5	30	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	32	CU12PSM9P6XXX(1-1/2)	100.00 -	0.6000	0.6000
			120.00		
T5	36	Safety Line 3/8	100.00 -	0.6000	0.6000
			120.00		
T5	37	Thin Flat Bar Climbing Ladder	100.00 -	0.6000	0.6000
			120.00		
T6	1	LDF4-50A(1/2)	80.00 - 100.00	0.6000	0.6000
T6	2	HB114-1-0813U4-M5J(1-1/4 )	80.00 - 100.00	0.6000	0.6000
T6	4	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	6	LDF4-50A(1/2)	80.00 - 100.00	0.6000	0.6000
T6	7	LDF7-50A(1-5/8)	80.00 - 100.00	0.6000	0.6000
T6	10	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b>	13 of 35
	<b>Project</b>		<b>Date</b>	15:20:18 09/13/23
	<b>Client</b>	Crown Castle	<b>Designed by</b>	R AITHAL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T6	12	LDF7-50A(1-5/8)	80.00 - 100.00	0.6000	0.6000
T6	13	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	15	FXL 1873 PE(1-5/8)	80.00 - 100.00	0.6000	0.6000
T6	16	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	19	2CX14AWG_TMO(21/64)	80.00 - 100.00	0.6000	0.6000
T6	21	HB158-21U6S24-xxM_TMO (1-5/8)	80.00 - 100.00	0.6000	0.6000
T6	22	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	24	FLC 78-50J(7/8)	80.00 - 100.00	0.6000	0.6000
T6	25	FLC 78-50J(7/8)	80.00 - 100.00	0.6000	0.6000
T6	26	FB-L98B-002-75000(3/8)	80.00 - 100.00	0.6000	0.6000
T6	27	FB-L98B-034-XXX(3/8)	80.00 - 100.00	0.0000	0.0000
T6	28	WR-VG86ST-BRD(3/4)	80.00 - 100.00	0.0000	0.0000
T6	29	2" Rigid Conduit	80.00 - 100.00	0.6000	0.6000
T6	30	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	32	CU12PSM9P6XXX(1-1/2)	80.00 - 100.00	0.6000	0.6000
T6	34	LDF4-50A(1/2)	80.00 - 98.00	0.6000	0.6000
T6	36	Safety Line 3/8	80.00 - 100.00	0.6000	0.6000
T6	37	Thin Flat Bar Climbing Ladder	80.00 - 100.00	0.6000	0.6000
T7	1	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
T7	2	HB114-1-0813U4-M5J(1-1/4 )	60.00 - 80.00	0.6000	0.6000
T7	4	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	6	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
T7	7	LDF7-50A(1-5/8)	60.00 - 80.00	0.6000	0.6000
T7	10	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	12	LDF7-50A(1-5/8)	60.00 - 80.00	0.6000	0.6000
T7	13	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	15	FXL 1873 PE(1-5/8)	60.00 - 80.00	0.6000	0.6000
T7	16	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	19	2CX14AWG_TMO(21/64)	60.00 - 80.00	0.6000	0.6000
T7	21	HB158-21U6S24-xxM_TMO (1-5/8)	60.00 - 80.00	0.6000	0.6000
T7	22	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	24	FLC 78-50J(7/8)	60.00 - 80.00	0.6000	0.6000
T7	25	FLC 78-50J(7/8)	60.00 - 80.00	0.6000	0.6000
T7	26	FB-L98B-002-75000(3/8)	60.00 - 80.00	0.6000	0.6000
T7	27	FB-L98B-034-XXX(3/8)	60.00 - 80.00	0.0000	0.0000
T7	28	WR-VG86ST-BRD(3/4)	60.00 - 80.00	0.0000	0.0000
T7	29	2" Rigid Conduit	60.00 - 80.00	0.6000	0.6000
T7	30	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	32	CU12PSM9P6XXX(1-1/2)	60.00 - 80.00	0.6000	0.6000
T7	34	LDF4-50A(1/2)	60.00 - 80.00	0.6000	0.6000
T7	36	Safety Line 3/8	60.00 - 80.00	0.6000	0.6000
T7	37	Thin Flat Bar Climbing Ladder	60.00 - 80.00	0.6000	0.6000
T8	1	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T8	2	HB114-1-0813U4-M5J(1-1/4 )	40.00 - 60.00	0.6000	0.6000
T8	4	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	6	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T8	7	LDF7-50A(1-5/8)	40.00 - 60.00	0.6000	0.6000
T8	10	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	12	LDF7-50A(1-5/8)	40.00 - 60.00	0.6000	0.6000
T8	13	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	15	FXL 1873 PE(1-5/8)	40.00 - 60.00	0.6000	0.6000
T8	16	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	19	2CX14AWG_TMO(21/64)	40.00 - 60.00	0.6000	0.6000
T8	21	HB158-21U6S24-xxM_TMO (1-5/8)	40.00 - 60.00	0.6000	0.6000
T8	22	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	14 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	15:20:18 09/13/23
	Crown Castle	<b>Designed by</b>
		R AITHAL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T8	24	FLC 78-50J(7/8)	40.00 - 60.00	0.6000	0.6000
T8	25	FLC 78-50J(7/8)	40.00 - 60.00	0.6000	0.6000
T8	26	FB-L98B-002-75000(3/8)	40.00 - 60.00	0.6000	0.6000
T8	27	FB-L98B-034-XXX(3/8)	40.00 - 60.00	0.0000	0.0000
T8	28	WR-VG86ST-BRD(3/4)	40.00 - 60.00	0.0000	0.0000
T8	29	2" Rigid Conduit	40.00 - 60.00	0.6000	0.6000
T8	30	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	32	CU12PSM9P6XXX(1-1/2)	40.00 - 60.00	0.6000	0.6000
T8	34	LDF4-50A(1/2)	40.00 - 60.00	0.6000	0.6000
T8	36	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T8	37	Thin Flat Bar Climbing Ladder	40.00 - 60.00	0.6000	0.6000
T9	1	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T9	2	HB114-1-0813U4-M5J(1-1/4 )	20.00 - 40.00	0.6000	0.6000
T9	4	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	6	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T9	7	LDF7-50A(1-5/8)	20.00 - 40.00	0.6000	0.6000
T9	10	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	12	LDF7-50A(1-5/8)	20.00 - 40.00	0.6000	0.6000
T9	13	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	15	FXL 1873 PE(1-5/8)	20.00 - 40.00	0.6000	0.6000
T9	16	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	19	2CX14AWG_TMO(21/64)	20.00 - 40.00	0.6000	0.6000
T9	21	HB158-21U6S24-xxM_TMO (1-5/8)	20.00 - 40.00	0.6000	0.6000
T9	22	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	24	FLC 78-50J(7/8)	20.00 - 40.00	0.6000	0.6000
T9	25	FLC 78-50J(7/8)	20.00 - 40.00	0.6000	0.6000
T9	26	FB-L98B-002-75000(3/8)	20.00 - 40.00	0.6000	0.6000
T9	27	FB-L98B-034-XXX(3/8)	20.00 - 40.00	0.0000	0.0000
T9	28	WR-VG86ST-BRD(3/4)	20.00 - 40.00	0.0000	0.0000
T9	29	2" Rigid Conduit	20.00 - 40.00	0.6000	0.6000
T9	30	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	32	CU12PSM9P6XXX(1-1/2)	20.00 - 40.00	0.6000	0.6000
T9	34	LDF4-50A(1/2)	20.00 - 40.00	0.6000	0.6000
T9	36	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T9	37	Thin Flat Bar Climbing Ladder	20.00 - 40.00	0.6000	0.6000
T10	1	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T10	2	HB114-1-0813U4-M5J(1-1/4 )	0.00 - 20.00	0.6000	0.6000
T10	4	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	6	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T10	7	LDF7-50A(1-5/8)	0.00 - 20.00	0.6000	0.6000
T10	10	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	12	LDF7-50A(1-5/8)	0.00 - 20.00	0.6000	0.6000
T10	13	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	15	FXL 1873 PE(1-5/8)	0.00 - 20.00	0.6000	0.6000
T10	16	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	19	2CX14AWG_TMO(21/64)	0.00 - 20.00	0.6000	0.6000
T10	21	HB158-21U6S24-xxM_TMO (1-5/8)	0.00 - 20.00	0.6000	0.6000
T10	22	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	24	FLC 78-50J(7/8)	0.00 - 20.00	0.6000	0.6000
T10	25	FLC 78-50J(7/8)	0.00 - 20.00	0.6000	0.6000
T10	26	FB-L98B-002-75000(3/8)	0.00 - 20.00	0.6000	0.6000
T10	27	FB-L98B-034-XXX(3/8)	0.00 - 20.00	0.0000	0.0000
T10	28	WR-VG86ST-BRD(3/4)	0.00 - 20.00	0.0000	0.0000
T10	29	2" Rigid Conduit	0.00 - 20.00	0.6000	0.6000
T10	30	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	32	CU12PSM9P6XXX(1-1/2)	0.00 - 20.00	0.6000	0.6000

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b>	15 of 35
	<b>Project</b>		<b>Date</b>	15:20:18 09/13/23
	<b>Client</b>	Crown Castle	<b>Designed by</b>	R AITHAL

<i>Tower Section</i>	<i>Feed Line Record No.</i>	<i>Description</i>	<i>Feed Line Segment Elev.</i>	<i>K<sub>a</sub> No Ice</i>	<i>K<sub>a</sub> Ice</i>
T10	34	LDF4-50A(1/2)	0.00 - 20.00	0.6000	0.6000
T10	36	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000
T10	37	Thin Flat Bar Climbing Ladder	0.00 - 20.00	0.6000	0.6000

## Discrete Tower Loads

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert</i> <i>ft ft ft</i>	<i>Azimuth Adjustment</i> <i>°</i>	<i>Placement</i> <i>ft</i>		<i>C<sub>A</sub>A<sub>A</sub> Front</i> <i>ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side</i> <i>ft<sup>2</sup></i>	<i>Weight</i> <i>K</i>
APXVTM14-ALU-I20 w/ Mount Pipe	A	From Leg	4.000	0.000	190.000	No Ice	4.091	2.862	0.077
			0.000			1/2" Ice	4.480	3.229	0.127
			2.000			1" Ice	4.880	3.607	0.185
						2" Ice	5.712	4.396	0.331
APXVTM14-ALU-I20 w/ Mount Pipe	B	From Leg	4.000	0.000	190.000	No Ice	4.091	2.862	0.077
			0.000			1/2" Ice	4.480	3.229	0.127
			2.000			1" Ice	4.880	3.607	0.185
						2" Ice	5.712	4.396	0.331
APXVTM14-ALU-I20 w/ Mount Pipe	C	From Leg	4.000	0.000	190.000	No Ice	4.091	2.862	0.077
			0.000			1/2" Ice	4.480	3.229	0.127
			2.000			1" Ice	4.880	3.607	0.185
						2" Ice	5.712	4.396	0.331
NNVV-65B-R4 w/ Mount Pipe	A	From Leg	4.000	0.000	190.000	No Ice	7.550	4.230	0.110
			0.000			1/2" Ice	8.040	4.670	0.197
			2.000			1" Ice	8.530	5.120	0.296
						2" Ice	9.560	6.050	0.529
NNVV-65B-R4 w/ Mount Pipe	B	From Leg	4.000	0.000	190.000	No Ice	7.550	4.230	0.110
			0.000			1/2" Ice	8.040	4.670	0.197
			2.000			1" Ice	8.530	5.120	0.296
						2" Ice	9.560	6.050	0.529
NNVV-65B-R4 w/ Mount Pipe	C	From Leg	4.000	0.000	190.000	No Ice	7.550	4.230	0.110
			0.000			1/2" Ice	8.040	4.670	0.197
			2.000			1" Ice	8.530	5.120	0.296
						2" Ice	9.560	6.050	0.529
FZHN	A	From Leg	4.000	0.000	190.000	No Ice	2.020	0.607	0.044
			0.000			1/2" Ice	2.197	0.715	0.058
			2.000			1" Ice	2.381	0.829	0.075
						2" Ice	2.772	1.089	0.116
FZHN	B	From Leg	4.000	0.000	190.000	No Ice	2.020	0.607	0.044
			0.000			1/2" Ice	2.197	0.715	0.058
			2.000			1" Ice	2.381	0.829	0.075
						2" Ice	2.772	1.089	0.116
FZHN	C	From Leg	4.000	0.000	190.000	No Ice	2.020	0.607	0.044
			0.000			1/2" Ice	2.197	0.715	0.058
			2.000			1" Ice	2.381	0.829	0.075
						2" Ice	2.772	1.089	0.116
PCS 1900MHz 4x45W-65MHz	A	From Leg	4.000	0.000	190.000	No Ice	2.322	2.238	0.060
			0.000			1/2" Ice	2.527	2.441	0.083
			2.000			1" Ice	2.739	2.651	0.110
						2" Ice	3.185	3.093	0.173
PCS 1900MHz	B	From Leg	4.000	0.000	190.000	No Ice	2.322	2.238	0.060

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	16 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>A</sub>A<sub>A</sub> Front ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
4x45W-65MHz			0.000 2.000			1/2" Ice 2.527 1" Ice 2.739 2" Ice 3.185	2.441 2.651 3.093	0.083 0.110 0.173
PCS 1900MHz 4x45W-65MHz	C	From Leg	4.000 0.000 2.000	0.000	190.000	No Ice 2.322 1/2" Ice 2.527 1" Ice 2.739 2" Ice 3.185	2.238 2.441 2.651 3.093	0.060 0.083 0.110 0.173
(2) RRH2X50-800	A	From Leg	4.000 0.000 2.000	0.000	190.000	No Ice 1.701 1/2" Ice 1.864 1" Ice 2.035 2" Ice 2.398	1.282 1.428 1.580 1.908	0.053 0.070 0.090 0.138
(2) RRH2X50-800	B	From Leg	4.000 0.000 2.000	0.000	190.000	No Ice 1.701 1/2" Ice 1.864 1" Ice 2.035 2" Ice 2.398	1.282 1.428 1.580 1.908	0.053 0.070 0.090 0.138
(2) RRH2X50-800	C	From Leg	4.000 0.000 2.000	0.000	190.000	No Ice 1.701 1/2" Ice 1.864 1" Ice 2.035 2" Ice 2.398	1.282 1.428 1.580 1.908	0.053 0.070 0.090 0.138
5' x 2" Pipe Mount	A	From Leg	4.000 0.000 0.000	0.000	190.000	No Ice 1.188 1/2" Ice 1.496 1" Ice 1.807 2" Ice 2.458	1.188 1.496 1.807 2.458	0.018 0.027 0.040 0.076
5' x 2" Pipe Mount	B	From Leg	4.000 0.000 0.000	0.000	190.000	No Ice 1.188 1/2" Ice 1.496 1" Ice 1.807 2" Ice 2.458	1.188 1.496 1.807 2.458	0.018 0.027 0.040 0.076
5' x 2" Pipe Mount	C	From Leg	4.000 0.000 0.000	0.000	190.000	No Ice 1.188 1/2" Ice 1.496 1" Ice 1.807 2" Ice 2.458	1.188 1.496 1.807 2.458	0.018 0.027 0.040 0.076
Sector Mount [SM 504-3]	C	None		0.000	190.000	No Ice 31.050 1/2" Ice 43.830 1" Ice 56.440 2" Ice 81.280	31.050 43.830 56.440 81.280	1.708 2.326 3.143 5.358
*								
(2) LPA-80080/4CF	A	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 2.139 1/2" Ice 2.575 1" Ice 3.028 2" Ice 3.983	4.917 5.413 5.925 6.997	0.024 0.057 0.095 0.184
(2) LPA-80080/4CF	B	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 2.139 1/2" Ice 2.575 1" Ice 3.028 2" Ice 3.983	4.917 5.413 5.925 6.997	0.024 0.057 0.095 0.184
(2) LPA-80080/4CF	C	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 2.139 1/2" Ice 2.575 1" Ice 3.028 2" Ice 3.983	4.917 5.413 5.925 6.997	0.024 0.057 0.095 0.184
CBRS w/ Mount Pipe	A	From Leg	4.000 0.000 -1.000	0.000	180.000	No Ice 1.452 1/2" Ice 1.671 1" Ice 1.905 2" Ice 2.418	0.994 1.185 1.391 1.847	0.032 0.048 0.068 0.123
CBRS w/ Mount Pipe	B	From Leg	4.000 0.000 -1.000	0.000	180.000	No Ice 1.452 1/2" Ice 1.671 1" Ice 1.905 2" Ice 2.418	0.994 1.185 1.391 1.847	0.032 0.048 0.068 0.123
CBRS w/ Mount Pipe	C	From Leg	4.000	0.000	180.000	No Ice 1.452	0.994	0.032

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>		<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)		17 of 35
	<b>Project</b>		<b>Date</b>
	Crown Castle		15:20:18 09/13/23
			<b>Designed by</b>
			R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>A</sub>A<sub>A</sub> Front ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
			0.000			1/2" Ice	1.671	1.185
			-1.000			1" Ice	1.905	1.391
						2" Ice	2.418	1.847
(2) JAHH-65B-R3B	A	From Leg	4.000	0.000	180.000	No Ice	5.286	3.053
			0.000			1/2" Ice	5.750	3.485
			1.000			1" Ice	6.223	3.927
						2" Ice	7.203	4.845
(2) JAHH-65B-R3B	B	From Leg	4.000	0.000	180.000	No Ice	5.286	3.053
			0.000			1/2" Ice	5.750	3.485
			1.000			1" Ice	6.223	3.927
						2" Ice	7.203	4.845
(2) JAHH-65B-R3B	C	From Leg	4.000	0.000	180.000	No Ice	5.286	3.053
			0.000			1/2" Ice	5.750	3.485
			1.000			1" Ice	6.223	3.927
						2" Ice	7.203	4.845
58532A	C	From Leg	4.000	0.000	180.000	No Ice	0.189	0.189
			0.000			1/2" Ice	0.248	0.248
			4.000			1" Ice	0.315	0.315
						2" Ice	0.470	0.470
RFV01U-D1A	A	From Leg	4.000	0.000	180.000	No Ice	1.875	1.250
			0.000			1/2" Ice	2.045	1.393
			1.000			1" Ice	2.223	1.543
						2" Ice	2.601	1.865
RFV01U-D1A	B	From Leg	4.000	0.000	180.000	No Ice	1.875	1.250
			0.000			1/2" Ice	2.045	1.393
			1.000			1" Ice	2.223	1.543
						2" Ice	2.601	1.865
RFV01U-D1A	C	From Leg	4.000	0.000	180.000	No Ice	1.875	1.250
			0.000			1/2" Ice	2.045	1.393
			1.000			1" Ice	2.223	1.543
						2" Ice	2.601	1.865
RFV01U-D2A	A	From Leg	4.000	0.000	180.000	No Ice	1.875	1.013
			0.000			1/2" Ice	2.045	1.145
			1.000			1" Ice	2.223	1.284
						2" Ice	2.601	1.585
RFV01U-D2A	B	From Leg	4.000	0.000	180.000	No Ice	1.875	1.013
			0.000			1/2" Ice	2.045	1.145
			1.000			1" Ice	2.223	1.284
						2" Ice	2.601	1.585
RFV01U-D2A	C	From Leg	4.000	0.000	180.000	No Ice	1.875	1.013
			0.000			1/2" Ice	2.045	1.145
			1.000			1" Ice	2.223	1.284
						2" Ice	2.601	1.585
(2) RC3DC-3315-PF-48	C	From Leg	4.000	0.000	180.000	No Ice	3.792	2.512
			0.000			1/2" Ice	4.044	2.725
			1.000			1" Ice	4.303	2.945
						2" Ice	4.844	3.414
MT6407-77A w/ Mount Pipe	A	From Leg	4.000	0.000	180.000	No Ice	5.940	3.100
			0.000			1/2" Ice	6.470	3.550
			3.000			1" Ice	7.020	4.020
						2" Ice	8.170	5.010
MT6407-77A w/ Mount Pipe	B	From Leg	4.000	0.000	180.000	No Ice	5.940	3.100
			0.000			1/2" Ice	6.470	3.550
			3.000			1" Ice	7.020	4.020
						2" Ice	8.170	5.010
MT6407-77A w/ Mount Pipe	C	From Leg	4.000	0.000	180.000	No Ice	5.940	3.100
			0.000			1/2" Ice	6.470	3.550





<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>		<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)		19 of 35
	<b>Project</b>		<b>Date</b>
	Crown Castle		15:20:18 09/13/23
	<b>Client</b>		<b>Designed by</b>
			R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment  °</i>	<i>Placement  ft</i>		<i>C<sub>A</sub>A<sub>A</sub> Front  ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side  ft<sup>2</sup></i>	<i>Weight  K</i>
APXVAARR24_43-U-NA20 w/ Mount Pipe	A	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	14.694 15.455 16.230 17.816	6.873 7.554 8.247 9.670	0.186 0.315 0.458 0.788
APXVAARR24_43-U-NA20 w/ Mount Pipe	B	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	14.694 15.455 16.230 17.816	6.873 7.554 8.247 9.670	0.186 0.315 0.458 0.788
APXVAARR24_43-U-NA20 w/ Mount Pipe	C	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	14.694 15.455 16.230 17.816	6.873 7.554 8.247 9.670	0.186 0.315 0.458 0.788
AIR 6419 B41_TMO_CCIV2	A	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	6.240 6.740 7.260 8.360	2.340 2.730 3.140 4.020	0.082 0.120 0.163 0.262
AIR 6419 B41_TMO_CCIV2	B	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	6.240 6.740 7.260 8.360	2.340 2.730 3.140 4.020	0.082 0.120 0.163 0.262
AIR 6419 B41_TMO_CCIV2	C	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	6.240 6.740 7.260 8.360	2.340 2.730 3.140 4.020	0.082 0.120 0.163 0.262
FIBEAIR IP-20A_RFU-D	A	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	0.692 0.796 0.909 1.156	0.290 0.366 0.449 0.636	0.014 0.020 0.027 0.048
RADIO 4449 B71 B85A_T-MOBILE	A	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.970 2.147 2.331 2.721	1.587 1.749 1.918 2.280	0.073 0.093 0.116 0.170
RADIO 4449 B71 B85A_T-MOBILE	B	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.970 2.147 2.331 2.721	1.587 1.749 1.918 2.280	0.073 0.093 0.116 0.170
RADIO 4449 B71 B85A_T-MOBILE	C	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.970 2.147 2.331 2.721	1.587 1.749 1.918 2.280	0.073 0.093 0.116 0.170
RADIO 4460 B2/B25 B66_TMO	A	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.139 2.321 2.511 2.912	1.686 1.850 2.022 2.387	0.109 0.131 0.156 0.217
RADIO 4460 B2/B25 B66_TMO	B	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.139 2.321 2.511 2.912	1.686 1.850 2.022 2.387	0.109 0.131 0.156 0.217
RADIO 4460 B2/B25 B66_TMO	C	From Leg	4.000 0.000 2.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	2.139 2.321 2.511 2.912	1.686 1.850 2.022 2.387	0.109 0.131 0.156 0.217
(2) 8' x 2" Mount Pipe	A	From Leg	4.000 0.000 0.000	0.000	156.000	No Ice 1/2" Ice 1" Ice 2" Ice	1.900 2.728 3.401 4.396	1.900 2.728 3.401 4.396	0.029 0.044 0.063 0.119
(2) 8' x 2" Mount Pipe	B	From Leg	4.000	0.000	156.000	No Ice	1.900	1.900	0.029

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	20 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>		<i>C<sub>A</sub>A<sub>A</sub> Front ft²</i>	<i>C<sub>A</sub>A<sub>A</sub> Side ft²</i>	<i>Weight K</i>
			0.000			1/2" Ice	2.728	2.728	0.044
			0.000			1" Ice	3.401	3.401	0.063
						2" Ice	4.396	4.396	0.119
(2) 8' x 2" Mount Pipe	C	From Leg	4.000	0.000	156.000	No Ice	1.900	1.900	0.029
			0.000			1/2" Ice	2.728	2.728	0.044
			0.000			1" Ice	3.401	3.401	0.063
						2" Ice	4.396	4.396	0.119
8' x 2.375" Horizontal Mount Pipe	A	From Leg	2.000	0.000	156.000	No Ice	2.380	0.010	0.037
			0.000			1/2" Ice	3.410	0.050	0.054
			0.000			1" Ice	4.450	0.100	0.079
						2" Ice	5.910	0.240	0.147
8' x 2.375" Horizontal Mount Pipe	B	From Leg	2.000	0.000	156.000	No Ice	2.380	0.010	0.037
			0.000			1/2" Ice	3.410	0.050	0.054
			0.000			1" Ice	4.450	0.100	0.079
						2" Ice	5.910	0.240	0.147
8' x 2.375" Horizontal Mount Pipe	C	From Leg	2.000	0.000	156.000	No Ice	2.380	0.010	0.037
			0.000			1/2" Ice	3.410	0.050	0.054
			0.000			1" Ice	4.450	0.100	0.079
						2" Ice	5.910	0.240	0.147
Sector Mount [SM 503-3]	C	None		0.000	156.000	No Ice	30.430	30.430	1.690
						1/2" Ice	43.020	43.020	2.296
						1" Ice	55.430	55.430	3.097
						2" Ice	79.890	79.890	5.269
*									
7770.00 w/ Mount Pipe	A	From Leg	4.000	0.000	140.000	No Ice	3.385	2.323	0.055
			0.000			1/2" Ice	3.746	2.664	0.098
			0.000			1" Ice	4.117	3.016	0.149
						2" Ice	4.891	3.751	0.279
7770.00 w/ Mount Pipe	B	From Leg	4.000	0.000	140.000	No Ice	3.385	2.323	0.055
			0.000			1/2" Ice	3.746	2.664	0.098
			0.000			1" Ice	4.117	3.016	0.149
						2" Ice	4.891	3.751	0.279
7770.00 w/ Mount Pipe	C	From Leg	4.000	0.000	140.000	No Ice	3.385	2.323	0.055
			0.000			1/2" Ice	3.746	2.664	0.098
			0.000			1" Ice	4.117	3.016	0.149
						2" Ice	4.891	3.751	0.279
(2) NNHH-65B-R4 w/ Mount Pipe	A	From Leg	4.000	0.000	140.000	No Ice	7.551	4.228	0.110
			0.000			1/2" Ice	8.037	4.668	0.197
			0.000			1" Ice	8.533	5.118	0.296
						2" Ice	9.557	6.049	0.529
(2) TPA65R-BU4D w/ Mount Pipe	B	From Leg	4.000	0.000	140.000	No Ice	8.100	4.033	0.080
			0.000			1/2" Ice	8.650	4.498	0.141
			0.000			1" Ice	9.214	4.978	0.212
						2" Ice	10.390	5.984	0.379
(2) TPA65R-BU4D w/ Mount Pipe	C	From Leg	4.000	0.000	140.000	No Ice	8.100	4.033	0.080
			0.000			1/2" Ice	8.650	4.498	0.141
			0.000			1" Ice	9.214	4.978	0.212
						2" Ice	10.390	5.984	0.379
TT19-08BP1111-001	A	From Leg	4.000	0.000	140.000	No Ice	0.545	0.442	0.016
			0.000			1/2" Ice	0.641	0.530	0.022
			0.000			1" Ice	0.743	0.626	0.029
						2" Ice	0.971	0.840	0.049
TT19-08BP1111-001	B	From Leg	4.000	0.000	140.000	No Ice	0.545	0.442	0.016
			0.000			1/2" Ice	0.641	0.530	0.022
			0.000			1" Ice	0.743	0.626	0.029
						2" Ice	0.971	0.840	0.049
TT19-08BP1111-001	C	From Leg	4.000	0.000	140.000	No Ice	0.545	0.442	0.016

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>		<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)		21 of 35
	<b>Project</b>		<b>Date</b>
	Crown Castle		15:20:18 09/13/23
			<b>Designed by</b>
			R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment  °</i>	<i>Placement  ft</i>	<i>C<sub>A</sub>A<sub>A</sub> Front  ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side  ft<sup>2</sup></i>	<i>Weight  K</i>
			0.000			1/2" Ice	0.641	0.022
			0.000			1" Ice	0.743	0.029
						2" Ice	0.971	0.049
RRUS 4478 B14	A	From Leg	4.000	0.000	140.000	No Ice	1.843	0.060
			0.000			1/2" Ice	2.012	0.076
			0.000			1" Ice	2.190	0.094
						2" Ice	2.566	0.140
RRUS 4478 B14	B	From Leg	4.000	0.000	140.000	No Ice	1.843	0.060
			0.000			1/2" Ice	2.012	0.076
			0.000			1" Ice	2.190	0.094
						2" Ice	2.566	0.140
RRUS 4478 B14	C	From Leg	4.000	0.000	140.000	No Ice	1.843	0.060
			0.000			1/2" Ice	2.012	0.076
			0.000			1" Ice	2.190	0.094
						2" Ice	2.566	0.140
RRUS 4449 B5/B12	A	From Leg	4.000	0.000	140.000	No Ice	1.968	0.071
			0.000			1/2" Ice	2.144	0.090
			0.000			1" Ice	2.328	0.111
						2" Ice	2.718	0.163
RRUS 4449 B5/B12	B	From Leg	4.000	0.000	140.000	No Ice	1.968	0.071
			0.000			1/2" Ice	2.144	0.090
			0.000			1" Ice	2.328	0.111
						2" Ice	2.718	0.163
RRUS 4449 B5/B12	C	From Leg	4.000	0.000	140.000	No Ice	1.968	0.071
			0.000			1/2" Ice	2.144	0.090
			0.000			1" Ice	2.328	0.111
						2" Ice	2.718	0.163
RRUS 8843 B2/B66A	A	From Leg	4.000	0.000	140.000	No Ice	1.639	0.072
			0.000			1/2" Ice	1.799	0.090
			0.000			1" Ice	1.966	0.110
						2" Ice	2.323	0.159
RRUS 8843 B2/B66A	C	From Leg	4.000	0.000	140.000	No Ice	1.639	0.072
			0.000			1/2" Ice	1.799	0.090
			0.000			1" Ice	1.966	0.110
						2" Ice	2.323	0.159
RRUS 8843 B2/B66A	B	From Leg	4.000	0.000	140.000	No Ice	1.639	0.072
			0.000			1/2" Ice	1.799	0.090
			0.000			1" Ice	1.966	0.110
						2" Ice	2.323	0.159
DC6-48-60-18-8F	A	From Leg	1.000	0.000	140.000	No Ice	1.212	0.033
			0.000			1/2" Ice	1.892	0.055
			2.000			1" Ice	2.105	0.080
						2" Ice	2.570	0.138
DC6-48-60-0-8C-EV	B	From Leg	1.000	0.000	140.000	No Ice	2.736	0.026
			0.000			1/2" Ice	2.962	0.063
			2.000			1" Ice	3.195	0.104
						2" Ice	3.683	0.200
Sector Mount [SM 502-3]	C	None		0.000	140.000	No Ice	29.820	1.673
						1/2" Ice	42.210	2.266
						1" Ice	54.430	3.052
						2" Ice	78.490	5.180
*								
MX08FRO665-21 w/ Mount	A	From Leg	4.000	0.000	130.000	No Ice	8.009	0.108
Pipe			0.000			1/2" Ice	8.518	0.194
			0.000			1" Ice	9.038	0.292
						2" Ice	10.109	0.522
MX08FRO665-21 w/ Mount	B	From Leg	4.000	0.000	130.000	No Ice	8.009	0.108

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>		<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)		22 of 35
	<b>Project</b>		<b>Date</b>
	Crown Castle		15:20:18 09/13/23
	<b>Client</b>		<b>Designed by</b>
			R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>A</sub>A<sub>A</sub> Front ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
Pipe			0.000			1/2" Ice 8.518	4.689	0.194
			0.000			1" Ice 9.038	5.156	0.292
						2" Ice 10.109	6.122	0.522
MX08FRO665-21 w/ Mount Pipe	C	From Leg	4.000	0.000	130.000	No Ice 8.009	4.233	0.108
			0.000			1/2" Ice 8.518	4.689	0.194
			0.000			1" Ice 9.038	5.156	0.292
						2" Ice 10.109	6.122	0.522
TA08025-B605	A	From Leg	4.000	0.000	130.000	No Ice 1.964	1.129	0.075
			0.000			1/2" Ice 2.138	1.267	0.093
			0.000			1" Ice 2.320	1.411	0.114
						2" Ice 2.705	1.723	0.164
TA08025-B605	B	From Leg	4.000	0.000	130.000	No Ice 1.964	1.129	0.075
			0.000			1/2" Ice 2.138	1.267	0.093
			0.000			1" Ice 2.320	1.411	0.114
						2" Ice 2.705	1.723	0.164
TA08025-B605	C	From Leg	4.000	0.000	130.000	No Ice 1.964	1.129	0.075
			0.000			1/2" Ice 2.138	1.267	0.093
			0.000			1" Ice 2.320	1.411	0.114
						2" Ice 2.705	1.723	0.164
TA08025-B604	A	From Leg	4.000	0.000	130.000	No Ice 1.964	0.981	0.064
			0.000			1/2" Ice 2.138	1.112	0.081
			0.000			1" Ice 2.320	1.250	0.100
						2" Ice 2.705	1.548	0.148
TA08025-B604	B	From Leg	4.000	0.000	130.000	No Ice 1.964	0.981	0.064
			0.000			1/2" Ice 2.138	1.112	0.081
			0.000			1" Ice 2.320	1.250	0.100
						2" Ice 2.705	1.548	0.148
TA08025-B604	C	From Leg	4.000	0.000	130.000	No Ice 1.964	0.981	0.064
			0.000			1/2" Ice 2.138	1.112	0.081
			0.000			1" Ice 2.320	1.250	0.100
						2" Ice 2.705	1.548	0.148
RDIDC-9181-PF-48	B	From Leg	4.000	0.000	130.000	No Ice 2.012	1.168	0.022
			0.000			1/2" Ice 2.189	1.311	0.040
			0.000			1" Ice 2.373	1.461	0.060
						2" Ice 2.763	1.784	0.110
(2) 8' x 2" Mount Pipe	A	From Leg	4.000	0.000	130.000	No Ice 1.900	1.900	0.029
			0.000			1/2" Ice 2.728	2.728	0.044
			0.000			1" Ice 3.401	3.401	0.063
						2" Ice 4.396	4.396	0.119
(2) 8' x 2" Mount Pipe	B	From Leg	4.000	0.000	130.000	No Ice 1.900	1.900	0.029
			0.000			1/2" Ice 2.728	2.728	0.044
			0.000			1" Ice 3.401	3.401	0.063
						2" Ice 4.396	4.396	0.119
(2) 8' x 2" Mount Pipe	C	From Leg	4.000	0.000	130.000	No Ice 1.900	1.900	0.029
			0.000			1/2" Ice 2.728	2.728	0.044
			0.000			1" Ice 3.401	3.401	0.063
						2" Ice 4.396	4.396	0.119
Commscope MTC3975083 (3)	C	None		0.000	130.000	No Ice 23.850	23.850	1.260
						1/2" Ice 34.120	34.120	1.803
						1" Ice 44.390	44.390	2.345
						2" Ice 64.930	64.930	3.431
* 58532A	C	From Leg	3.000	0.000	98.000	No Ice 0.189	0.189	0.000
			0.000			1/2" Ice 0.248	0.248	0.003
			4.000			1" Ice 0.315	0.315	0.006
						2" Ice 0.470	0.470	0.017
Side Arm Mount [SO 305-1]	C	From Leg	1.500	0.000	98.000	No Ice 0.530	1.520	0.030

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b>	23 of 35
	<b>Project</b>		<b>Date</b>	15:20:18 09/13/23
	<b>Client</b>	Crown Castle	<b>Designed by</b>	R AITHAL

<i>Description</i>	<i>Face or Leg</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft ft ft</i>	<i>Azimuth Adjustment °</i>	<i>Placement ft</i>	<i>C<sub>A</sub>A<sub>A</sub> Front ft<sup>2</sup></i>	<i>C<sub>A</sub>A<sub>A</sub> Side ft<sup>2</sup></i>	<i>Weight K</i>
			0.000		1/2" Ice	0.780	2.070	0.044
			0.000		1" Ice	1.060	2.660	0.064
					2" Ice	1.730	3.910	0.125
*								
*								

## Dishes

<i>Description</i>	<i>Face or Leg</i>	<i>Dish Type</i>	<i>Offset Type</i>	<i>Offsets: Horz Lateral Vert ft</i>	<i>Azimuth Adjustment °</i>	<i>3 dB Beam Width °</i>	<i>Elevation ft</i>	<i>Outside Diameter ft</i>	<i>Aperture Area ft²</i>	<i>Weight K</i>	
*											
VHLP2-11W/A	B	Paraboloid w/Shroud (HP)	From Leg	4.000 0.000 2.000	-65.000		156.000	2.167	No Ice 1/2" Ice 1" Ice 2" Ice	3.687 3.976 4.265 4.843	0.018 0.038 0.058 0.099
*											

## Load Combinations

<i>Comb. No.</i>	<i>Description</i>
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice

<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b>	24 of 35
	<b>Project</b>		<b>Date</b>	15:20:18 09/13/23
	<b>Client</b>	Crown Castle	<b>Designed by</b>	R AITHAL

<i>Comb. No.</i>	<i>Description</i>
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

## Maximum Member Forces

<i>Section No.</i>	<i>Elevation ft</i>	<i>Component Type</i>	<i>Condition</i>	<i>Gov. Load Comb.</i>	<i>Axial K</i>	<i>Major Axis Moment kip-ft</i>	<i>Minor Axis Moment kip-ft</i>
T1	192 - 180	Leg	Max Tension	23	3.178	-0.050	-0.037
			Max. Compression	18	-5.741	0.023	-0.033
			Max. Mx	20	-1.240	-0.612	0.002
			Max. My	2	-0.955	-0.019	-0.611
			Max. Vy	20	-0.452	0.295	-0.026
			Max. Vx	2	-0.458	-0.005	0.311
		Diagonal	Max Tension	16	1.164	0.000	0.000
			Max. Compression	20	-1.215	0.000	0.000
			Max. Mx	36	0.136	0.019	0.000
			Max. My	16	1.159	0.005	-0.001
			Max. Vy	36	-0.021	0.019	0.000
			Max. Vx	16	0.000	0.000	0.000
		Top Girt	Max Tension	22	0.080	0.000	0.000
			Max. Compression	19	-0.069	0.000	0.000
			Max. Mx	26	0.003	-0.054	0.000
			Max. Vy	26	-0.033	0.000	0.000
T2	180 - 160	Leg	Max Tension	23	21.593	-0.007	-0.025
			Max. Compression	18	-29.959	0.519	0.004
			Max. Mx	14	19.156	-0.575	0.030
			Max. My	17	-3.309	-0.029	0.550
			Max. Vy	6	-1.053	-0.004	0.017
			Max. Vx	12	-1.029	0.015	0.054
		Diagonal	Max Tension	16	3.367	0.000	0.000
			Max. Compression	4	-3.407	0.000	0.000
			Max. Mx	27	1.065	0.037	-0.004
			Max. My	34	-1.165	0.023	-0.004
			Max. Vy	27	-0.030	0.037	-0.004



<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b>	26 of 35
	<b>Project</b>		<b>Date</b>	15:20:18 09/13/23
	<b>Client</b>	Crown Castle	<b>Designed by</b>	R AITHAL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft		
T8	60 - 40	Diagonal	Max Tension	12	8.662	0.000	0.000		
			Max. Compression	12	-8.807	0.000	0.000		
		Leg	Max. Mx	27	2.421	0.263	-0.032		
			Max. My	30	2.697	0.259	0.033		
			Max. Vy	29	0.119	0.254	-0.030		
			Max. Vx	30	-0.006	0.000	0.000		
			Max Tension	23	179.030	-1.159	-0.019		
			Max. Compression	18	-219.603	0.999	-0.009		
			Max. Mx	37	12.975	-1.986	-0.015		
			Max. My	20	-17.738	-0.076	-1.125		
			Max. Vy	33	0.300	-1.974	0.010		
			Max. Vx	20	0.141	-0.076	-1.125		
		Diagonal	Max Tension	12	8.456	0.000	0.000		
			Max. Compression	12	-8.446	0.000	0.000		
T9	40 - 20	Diagonal	Max. Mx	29	1.270	0.320	0.042		
			Max. My	30	-1.155	0.297	0.048		
		Leg	Max. Vy	29	0.141	0.318	0.046		
			Max. Vx	30	-0.008	0.000	0.000		
			Max Tension	7	203.314	-1.049	0.010		
			Max. Compression	18	-249.733	1.710	-0.021		
			Max. Mx	37	14.985	-4.036	-0.010		
			Max. My	20	-20.403	-0.128	-1.388		
			Max. Vy	33	0.660	-4.019	0.009		
			Max. Vx	20	-0.195	-0.128	-1.388		
			Diagonal	Max Tension	12	9.745	0.000	0.000	
				Max. Compression	12	-10.043	0.000	0.000	
		T10	20 - 0	Diagonal	Max. Mx	27	1.962	0.388	-0.041
					Max. My	30	3.730	0.333	0.046
Leg	Max. Vy			29	0.148	0.387	-0.040		
	Max. Vx			30	-0.007	0.000	0.000		
	Max Tension			7	226.920	-1.111	0.015		
	Max. Compression			18	-279.877	0.000	0.000		
	Max. Mx			35	-127.281	4.116	0.006		
	Max. My			20	-23.914	-0.216	-2.517		
	Max. Vy			33	-0.790	-4.019	0.009		
	Max. Vx			20	-0.361	-0.216	-2.517		
	Diagonal			Max Tension	12	10.107	0.000	0.000	
				Max. Compression	10	-10.583	0.000	0.000	
	Max. Mx			29	-0.493	0.517	0.050		
	Max. My			30	5.206	0.336	0.059		
	Max. Vy	29	0.170	0.517	0.050				
	Max. Vx	30	-0.009	0.000	0.000				

## Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	18	287.371	28.165	-16.621
	Max. H <sub>x</sub>	18	287.371	28.165	-16.621
	Max. H <sub>z</sub>	7	-232.650	-23.650	14.003
	Min. Vert	7	-232.650	-23.650	14.003
	Min. H <sub>x</sub>	7	-232.650	-23.650	14.003
	Min. H <sub>z</sub>	18	287.371	28.165	-16.621
Leg B	Max. Vert	10	286.110	-27.937	-16.676
	Max. H <sub>x</sub>	23	-232.270	23.450	14.070
	Max. H <sub>z</sub>	23	-232.270	23.450	14.070



<b><i>tnxTower</i></b>  <b><i>MTS Engineering, P.L.L.C.</i></b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	27 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

<i>Location</i>	<i>Condition</i>	<i>Gov. Load Comb.</i>	<i>Vertical K</i>	<i>Horizontal, X K</i>	<i>Horizontal, Z K</i>
Leg A	Min. Vert	23	-232.270	23.450	14.070
	Min. H <sub>x</sub>	10	286.110	-27.937	-16.676
	Min. H <sub>z</sub>	10	286.110	-27.937	-16.676
	Max. Vert	2	281.110	0.263	31.937
	Max. H <sub>x</sub>	21	18.570	4.624	1.538
	Max. H <sub>z</sub>	2	281.110	0.263	31.937
	Min. Vert	15	-225.923	-0.273	-26.727
	Min. H <sub>x</sub>	8	25.927	-4.635	2.145
	Min. H <sub>z</sub>	15	-225.923	-0.273	-26.727

## Tower Mast Reaction Summary

<i>Load Combination</i>	<i>Vertical K</i>	<i>Shear<sub>x</sub> K</i>	<i>Shear<sub>z</sub> K</i>	<i>Overturning Moment, M<sub>x</sub> kip-ft</i>	<i>Overturning Moment, M<sub>z</sub> kip-ft</i>	<i>Torque kip-ft</i>
Dead Only	62.650	0.000	0.000	-7.312	4.079	0.000
1.2 Dead+1.0 Wind 0 deg - No Ice	75.180	0.062	-50.350	-5554.739	-5.954	11.653
0.9 Dead+1.0 Wind 0 deg - No Ice	56.385	0.062	-50.350	-5552.545	-7.177	11.653
1.2 Dead+1.0 Wind 30 deg - No Ice	75.180	24.329	-42.046	-4665.780	-2693.439	13.606
0.9 Dead+1.0 Wind 30 deg - No Ice	56.385	24.329	-42.046	-4663.586	-2694.663	13.606
1.2 Dead+1.0 Wind 60 deg - No Ice	75.180	42.399	-24.482	-2736.607	-4718.759	-13.370
0.9 Dead+1.0 Wind 60 deg - No Ice	56.385	42.399	-24.482	-2734.414	-4719.983	-13.370
1.2 Dead+1.0 Wind 90 deg - No Ice	75.180	49.909	-0.057	-18.816	-5574.005	-43.194
0.9 Dead+1.0 Wind 90 deg - No Ice	56.385	49.909	-0.057	-16.623	-5575.228	-43.194
1.2 Dead+1.0 Wind 120 deg - No Ice	75.180	44.506	25.630	2818.581	-4911.991	-20.396
0.9 Dead+1.0 Wind 120 deg - No Ice	56.385	44.506	25.630	2820.775	-4913.215	-20.396
1.2 Dead+1.0 Wind 150 deg - No Ice	75.180	24.383	42.235	4687.994	-2706.332	-5.937
0.9 Dead+1.0 Wind 150 deg - No Ice	56.385	24.383	42.235	4690.188	-2707.556	-5.937
1.2 Dead+1.0 Wind 180 deg - No Ice	75.180	-0.083	47.693	5306.695	19.006	-11.798
0.9 Dead+1.0 Wind 180 deg - No Ice	56.385	-0.083	47.693	5308.889	17.782	-11.798
1.2 Dead+1.0 Wind 210 deg - No Ice	75.180	-24.348	42.064	4651.128	2706.209	-13.813
0.9 Dead+1.0 Wind 210 deg - No Ice	56.385	-24.348	42.064	4653.321	2704.985	-13.813
1.2 Dead+1.0 Wind 240 deg - No Ice	75.180	-44.729	25.830	2837.346	4932.741	13.160
0.9 Dead+1.0 Wind 240 deg - No Ice	56.385	-44.729	25.830	2839.540	4931.517	13.160
1.2 Dead+1.0 Wind 270 deg - No Ice	75.180	-49.932	0.066	2.687	5587.294	42.998
0.9 Dead+1.0 Wind 270 deg - No Ice	56.385	-49.932	0.066	4.880	5586.070	42.998
1.2 Dead+1.0 Wind 300 deg - No Ice	75.180	-42.208	-24.281	-2717.692	4722.596	20.204

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	28 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	15:20:18 09/13/23
	Crown Castle	<b>Designed by</b>
		R AITHAL

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
No Ice						
0.9 Dead+1.0 Wind 300 deg - No Ice	56.385	-42.208	-24.281	-2715.498	4721.372	20.204
1.2 Dead+1.0 Wind 330 deg - No Ice	75.180	-24.372	-42.244	-4706.947	2714.381	6.061
0.9 Dead+1.0 Wind 330 deg - No Ice	56.385	-24.372	-42.244	-4704.753	2713.157	6.061
1.2 Dead+1.0 Ice+1.0 Temp	181.116	0.000	0.000	-45.596	-24.558	0.000
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	181.116	0.012	-15.537	-1771.006	-26.710	3.992
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	181.116	7.600	-13.147	-1515.724	-875.192	3.076
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	181.116	13.121	-7.576	-899.932	-1504.097	-6.535
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	181.116	15.360	-0.011	-47.570	-1761.019	-11.159
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	181.116	13.611	7.846	834.402	-1552.664	-4.913
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	181.116	7.660	13.268	1441.728	-883.180	-0.587
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	181.116	-0.016	15.114	1644.258	-21.682	-4.024
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	181.116	-7.604	13.151	1425.172	826.737	-3.122
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	181.116	-13.493	7.791	827.190	1486.787	6.489
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	181.116	-15.365	0.013	-43.308	1712.679	11.116
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	181.116	-13.246	-7.630	-907.111	1472.853	4.870
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	181.116	-7.658	-13.270	-1533.232	833.679	0.615
Dead+Wind 0 deg - Service	62.650	0.017	-13.945	-1529.934	1.127	3.171
Dead+Wind 30 deg - Service	62.650	6.742	-11.653	-1286.223	-736.916	3.699
Dead+Wind 60 deg - Service	62.650	11.749	-6.784	-756.335	-1292.970	-3.646
Dead+Wind 90 deg - Service	62.650	13.825	-0.015	-10.045	-1527.506	-11.764
Dead+Wind 120 deg - Service	62.650	12.322	7.096	768.797	-1345.559	-5.558
Dead+Wind 150 deg - Service	62.650	6.757	11.704	1282.421	-740.425	-1.620
Dead+Wind 180 deg - Service	62.650	-0.022	13.222	1452.580	7.920	-3.211
Dead+Wind 210 deg - Service	62.650	-6.747	11.658	1272.387	745.885	-3.755
Dead+Wind 240 deg - Service	62.650	-12.383	7.151	773.904	1356.700	3.589
Dead+Wind 270 deg - Service	62.650	-13.831	0.018	-4.193	1536.617	11.710
Dead+Wind 300 deg - Service	62.650	-11.697	-6.729	-751.187	1299.509	5.506
Dead+Wind 330 deg - Service	62.650	-6.754	-11.707	-1297.427	748.109	1.654

## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-62.650	0.000	0.000	62.650	0.000	0.000%
2	0.062	-75.180	-50.350	-0.062	75.180	50.350	0.000%
3	0.062	-56.385	-50.350	-0.062	56.385	50.350	0.000%
4	24.329	-75.180	-42.046	-24.329	75.180	42.046	0.000%
5	24.329	-56.385	-42.046	-24.329	56.385	42.046	0.000%
6	42.399	-75.180	-24.482	-42.399	75.180	24.482	0.000%
7	42.399	-56.385	-24.482	-42.399	56.385	24.482	0.000%
8	49.909	-75.180	-0.057	-49.909	75.180	0.057	0.000%

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	29 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	15:20:18 09/13/23
	Crown Castle	<b>Designed by</b>
		R AITHAL

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
9	49.909	-56.385	-0.057	-49.909	56.385	0.057	0.000%
10	44.506	-75.180	25.630	-44.506	75.180	-25.630	0.000%
11	44.506	-56.385	25.630	-44.506	56.385	-25.630	0.000%
12	24.383	-75.180	42.235	-24.383	75.180	-42.235	0.000%
13	24.383	-56.385	42.235	-24.383	56.385	-42.235	0.000%
14	-0.083	-75.180	47.693	0.083	75.180	-47.693	0.000%
15	-0.083	-56.385	47.693	0.083	56.385	-47.693	0.000%
16	-24.348	-75.180	42.064	24.348	75.180	-42.064	0.000%
17	-24.348	-56.385	42.064	24.348	56.385	-42.064	0.000%
18	-44.729	-75.180	25.830	44.729	75.180	-25.830	0.000%
19	-44.729	-56.385	25.830	44.729	56.385	-25.830	0.000%
20	-49.932	-75.180	0.066	49.932	75.180	-0.066	0.000%
21	-49.932	-56.385	0.066	49.932	56.385	-0.066	0.000%
22	-42.208	-75.180	-24.281	42.208	75.180	24.281	0.000%
23	-42.208	-56.385	-24.281	42.208	56.385	24.281	0.000%
24	-24.372	-75.180	-42.244	24.372	75.180	42.244	0.000%
25	-24.372	-56.385	-42.244	24.372	56.385	42.244	0.000%
26	0.000	-181.116	0.000	-0.000	181.116	-0.000	0.000%
27	0.012	-181.116	-15.537	-0.012	181.116	15.537	0.000%
28	7.600	-181.116	-13.147	-7.600	181.116	13.147	0.000%
29	13.121	-181.116	-7.576	-13.121	181.116	7.576	0.000%
30	15.360	-181.116	-0.011	-15.360	181.116	0.011	0.000%
31	13.611	-181.116	7.846	-13.611	181.116	-7.846	0.000%
32	7.660	-181.116	13.268	-7.660	181.116	-13.268	0.000%
33	-0.016	-181.116	15.114	0.016	181.116	-15.114	0.000%
34	-7.604	-181.116	13.151	7.604	181.116	-13.151	0.000%
35	-13.493	-181.116	7.791	13.493	181.116	-7.791	0.000%
36	-15.365	-181.116	0.013	15.365	181.116	-0.013	0.000%
37	-13.246	-181.116	-7.630	13.246	181.116	7.630	0.000%
38	-7.658	-181.116	-13.270	7.658	181.116	13.270	0.000%
39	0.017	-62.650	-13.945	-0.017	62.650	13.945	0.000%
40	6.742	-62.650	-11.653	-6.742	62.650	11.653	0.000%
41	11.749	-62.650	-6.784	-11.749	62.650	6.784	0.000%
42	13.825	-62.650	-0.015	-13.825	62.650	0.015	0.000%
43	12.322	-62.650	7.096	-12.322	62.650	-7.096	0.000%
44	6.757	-62.650	11.704	-6.757	62.650	-11.704	0.000%
45	-0.022	-62.650	13.222	0.022	62.650	-13.222	0.000%
46	-6.747	-62.650	11.658	6.747	62.650	-11.658	0.000%
47	-12.383	-62.650	7.151	12.383	62.650	-7.151	0.000%
48	-13.831	-62.650	0.018	13.831	62.650	-0.018	0.000%
49	-11.697	-62.650	-6.729	11.697	62.650	6.729	0.000%
50	-6.754	-62.650	-11.707	6.754	62.650	11.707	0.000%

## Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	192 - 180	4.207	47	0.199	0.021
T2	180 - 160	3.707	47	0.196	0.021
T3	160 - 140	2.905	47	0.174	0.019
T4	140 - 120	2.210	47	0.147	0.016
T5	120 - 100	1.617	47	0.122	0.013
T6	100 - 80	1.125	47	0.100	0.010
T7	80 - 60	0.731	47	0.077	0.008
T8	60 - 40	0.428	47	0.057	0.005

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	30 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T9	40 - 20	0.209	47	0.038	0.003
T10	20 - 0	0.063	47	0.020	0.002

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
190.000	APXVTM14-ALU-I20 w/ Mount Pipe	47	4.124	0.199	0.021	610355
180.000	(2) LPA-80080/4CF	47	3.707	0.196	0.021	214899
171.000	(3) 7130.16.33.00 w/ Mount Pipe	47	3.337	0.188	0.020	67597
162.000	HBX-6516DS-VTM w/ Mount Pipe	47	2.981	0.176	0.019	39351
158.000	VHLP2-11W/A	47	2.830	0.171	0.019	37011
156.000	APXVAARR24_43-U-NA20 w/ Mount Pipe	47	2.756	0.168	0.019	37761
140.000	7770.00 w/ Mount Pipe	47	2.210	0.147	0.016	49135
130.000	MX08FRO665-21 w/ Mount Pipe	47	1.902	0.134	0.015	48292
98.000	58532A	47	1.081	0.097	0.010	47780

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	192 - 180	15.397	18	0.725	0.075
T2	180 - 160	13.566	18	0.717	0.076
T3	160 - 140	10.629	19	0.635	0.070
T4	140 - 120	8.083	19	0.539	0.060
T5	120 - 100	5.911	19	0.446	0.048
T6	100 - 80	4.107	19	0.364	0.037
T7	80 - 60	2.668	19	0.280	0.028
T8	60 - 40	1.560	19	0.207	0.020
T9	40 - 20	0.760	19	0.140	0.013
T10	20 - 0	0.228	19	0.071	0.006

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
190.000	APXVTM14-ALU-I20 w/ Mount Pipe	18	15.092	0.725	0.076	188593
180.000	(2) LPA-80080/4CF	18	13.566	0.717	0.076	64354
171.000	(3) 7130.16.33.00 w/ Mount Pipe	19	12.212	0.688	0.074	18992
162.000	HBX-6516DS-VTM w/ Mount Pipe	19	10.908	0.645	0.070	10915
158.000	VHLP2-11W/A	19	10.355	0.626	0.069	10242
156.000	APXVAARR24_43-U-NA20 w/ Mount Pipe	19	10.085	0.616	0.068	10442

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	31 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

Elevation	Appurtenance	Gov. Load Comb.	Deflection	Tilt	Twist	Radius of Curvature
ft			in	°	°	ft
140.000	7770.00 w/ Mount Pipe	19	8.083	0.539	0.060	13505
130.000	MX08FRO665-21 w/ Mount Pipe	19	6.952	0.492	0.054	13233
98.000	58532A	19	3.947	0.356	0.036	13049

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load per Bolt K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	192	Leg	A325N	0.625	4	0.795	20.340	0.039 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	1.164	7.116	0.164 ✓	1.05	Member Block Shear
		Top Girt	A325N	0.625	1	0.080	7.116	0.011 ✓	1.05	Member Block Shear
T2	180	Leg	A325N	0.625	4	5.398	20.340	0.265 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	3.367	8.135	0.414 ✓	1.05	Member Block Shear
		Top Girt	A325N	0.625	1	0.612	8.135	0.075 ✓	1.05	Member Block Shear
T3	160	Leg	A325N	0.875	4	11.525	41.556	0.277 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	5.025	10.440	0.481 ✓	1.05	Gusset Bearing
T4	140	Leg	A325N	1.000	4	18.477	54.517	0.339 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.625	1	6.437	10.440	0.617 ✓	1.05	Gusset Bearing
T5	120	Leg	A325N	1.000	6	17.193	54.517	0.315 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.750	1	6.907	14.137	0.489 ✓	1.05	Member Bearing
T6	100	Leg	A325N	1.000	6	21.363	54.517	0.392 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.750	1	7.972	14.137	0.564 ✓	1.05	Member Bearing
T7	80	Leg	A325N	1.000	8	19.366	54.517	0.355 ✓	1.05	Bolt Tension
		Diagonal	A325N	0.750	1	8.662	14.137	0.613 ✓	1.05	Member Bearing
T8	60	Leg	A325N	1.000	8	22.379	54.517	0.410 ✓	1.05	Bolt Tension
		Diagonal	A325X	0.750	1	8.456	17.672	0.479 ✓	1.05	Member Bearing
T9	40	Leg	A325N	1.000	8	25.414	54.517	0.466 ✓	1.05	Bolt Tension
		Diagonal	A325X	0.750	1	9.745	17.672	0.551 ✓	1.05	Member Bearing
T10	20	Diagonal	A325X	0.750	1	10.107	18.922	0.534 ✓	1.05	Gusset Bearing

### Compression Checks

### Leg Design Data (Compression)

<b>tnxTower</b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	32 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	192 - 180	ROHN 2.5 STD	12.000	4.000	50.7 K=1.00	1.704	-5.741	63.560	0.090 <sup>1</sup>
T2	180 - 160	ROHN 2.5 STD	20.032	5.008	63.4 K=1.00	1.704	-29.959	57.139	0.524 <sup>1</sup>
T3	160 - 140	ROHN 3 EH	20.036	6.679	70.5 K=1.00	3.016	-59.379	94.337	0.629 <sup>1</sup>
T4	140 - 120	ROHN 4 EH	20.038	6.679	54.3 K=1.00	4.407	-94.442	159.899	0.591 <sup>1</sup>
T5	120 - 100	ROHN 5 EH	20.036	6.679	43.6 K=1.00	6.112	-128.236	239.378	0.536 <sup>1</sup>
T6	100 - 80	ROHN 6 EHS	20.036	10.018	54.0 K=1.00	6.713	-157.572	244.047	0.646 <sup>1</sup>
T7	80 - 60	ROHN 6 EH	20.032	10.016	54.8 K=1.00	8.405	-189.468	303.757	0.624 <sup>1</sup>
T8	60 - 40	ROHN 8 EHS	20.042	10.021	41.2 K=1.00	9.719	-219.603	386.354	0.568 <sup>1</sup>
T9	40 - 20	ROHN 8 EHS	20.031	10.015	41.2 K=1.00	9.719	-249.733	386.409	0.646 <sup>1</sup>
T10	20 - 0	ROHN 8 EHS	20.033	10.017	41.2 K=1.00	9.719	-279.877	386.397	0.724 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls



### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	192 - 180	L1 3/4x1 3/4x3/16	7.700	3.585	125.3 K=1.00	0.621	-1.215	11.328	0.107 <sup>1</sup>
T2	180 - 160	L2x2x3/16	9.686	4.721	143.8 K=1.00	0.715	-3.299	9.897	0.333 <sup>1</sup>
T3	160 - 140	L2 1/2x2 1/2x1/4	12.241	6.028	147.3 K=1.00	1.190	-4.976	15.695	0.317 <sup>1</sup>
T4	140 - 120	L2 1/2x2 1/2x1/4	14.067	6.897	168.6 K=1.00	1.190	-6.527	11.987	0.544 <sup>1</sup>
T5	120 - 100	L3x3x1/4	15.944	7.773	157.6 K=1.00	1.440	-6.866	16.602	0.414 <sup>1</sup>
T6	100 - 80	L3 1/2x3 1/2x1/4	19.209	9.452	163.4 K=1.00	1.690	-8.013	18.110	0.442 <sup>1</sup>
T7	80 - 60	L4x4x1/4	20.935	10.297	155.4 K=1.00	1.940	-8.807	22.986	0.383 <sup>1</sup>
T8	60 - 40	L4x4x5/16	22.872	11.214	170.1 K=1.00	2.400	-8.446	23.735	0.356 <sup>1</sup>
T9	40 - 20	L4x4x5/16	24.688	12.078	183.2 K=1.00	2.400	-10.043	20.461	0.491 <sup>1</sup>
T10	20 - 0	L4x4x3/8	26.510	13.002	198.0 K=1.00	2.860	-10.583	20.882	0.507 <sup>1</sup>

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	<b>Page</b>	33 of 35
	<b>Project</b>		<b>Date</b>	15:20:18 09/13/23
	<b>Client</b>	Crown Castle	<b>Designed by</b>	R AITHAL

<sup>1</sup>  $P_u / \phi P_n$  controls











### Top Girt Design Data (Compression)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>K</i>	$\phi P_n$ <i>K</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	192 - 180	L1 3/4x1 3/4x3/16	6.580	6.090	212.8 K=1.00	0.621	-0.069	3.926	0.018 <sup>1</sup> 
T2	180 - 160	KL/R > 200 (C) - 5 L2x2x3/16	6.580	6.090	185.5 K=1.00	0.715	-0.643	5.948	0.108 <sup>1</sup> 

<sup>1</sup>  $P_u / \phi P_n$  controls

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>K</i>	$\phi P_n$ <i>K</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	192 - 180	ROHN 2.5 STD	12.000	4.000	50.7	1.704	3.178	76.682	0.041 <sup>1</sup> 
T2	180 - 160	ROHN 2.5 STD	20.032	5.008	63.4	1.704	21.593	76.682	0.282 <sup>1</sup> 
T3	160 - 140	ROHN 3 EH	20.036	6.679	70.5	3.016	46.102	135.717	0.340 <sup>1</sup> 
T4	140 - 120	ROHN 4 EH	20.038	6.679	54.3	4.407	73.909	198.335	0.373 <sup>1</sup> 
T5	120 - 100	ROHN 5 EH	20.036	6.679	43.6	6.112	103.159	275.039	0.375 <sup>1</sup> 
T6	100 - 80	ROHN 6 EHS	20.036	10.018	54.0	6.713	128.180	302.097	0.424 <sup>1</sup> 
T7	80 - 60	ROHN 6 EH	20.032	10.016	54.8	8.405	154.929	378.222	0.410 <sup>1</sup> 
T8	60 - 40	ROHN 8 EHS	20.042	10.021	41.2	9.719	179.030	437.369	0.409 <sup>1</sup> 
T9	40 - 20	ROHN 8 EHS	20.031	10.015	41.2	9.719	203.314	437.369	0.465 <sup>1</sup> 
T10	20 - 0	ROHN 8 EHS	20.033	10.017	41.2	9.719	226.920	437.369	0.519 <sup>1</sup> 

<sup>1</sup>  $P_u / \phi P_n$  controls

<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	34 of 35
	<b>Project</b>	<b>Date</b>
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

### Diagonal Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>K</i>	$\phi P_n$ <i>K</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	192 - 180	L1 3/4x1 3/4x3/16	7.700	3.585	82.9	0.360	1.164	15.675	0.074 <sup>1</sup>
T2	180 - 160	L2x2x3/16	9.686	4.721	94.3	0.431	3.367	18.739	0.180 <sup>1</sup>
T3	160 - 140	L2 1/2x2 1/2x1/4	11.669	5.746	91.6	0.752	5.025	32.707	0.154 <sup>1</sup>
T4	140 - 120	L2 1/2x2 1/2x1/4	14.067	6.897	109.6	0.752	6.437	32.707	0.197 <sup>1</sup>
T5	120 - 100	L3x3x1/4	15.944	7.773	102.0	0.916	6.907	44.652	0.155 <sup>1</sup>
T6	100 - 80	L3 1/2x3 1/2x1/4	19.209	9.452	105.5	1.103	7.972	53.793	0.148 <sup>1</sup>
T7	80 - 60	L4x4x1/4	20.935	10.297	100.1	1.291	8.662	62.933	0.138 <sup>1</sup>
T8	60 - 40	L4x4x5/16	22.872	11.214	109.8	1.595	8.456	77.752	0.109 <sup>1</sup>
T9	40 - 20	L4x4x5/16	24.688	12.078	118.2	1.595	9.745	77.752	0.125 <sup>1</sup>
T10	20 - 0	L4x4x3/8	26.510	13.002	128.2	1.899	10.107	92.572	0.109 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Top Girt Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L<sub>u</sub></i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in<sup>2</sup></i>	<i>P<sub>u</sub></i> <i>K</i>	$\phi P_n$ <i>K</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	192 - 180	L1 3/4x1 3/4x3/16	6.580	6.090	141.7	0.360	0.080	15.675	0.005 <sup>1</sup>
T2	180 - 160	L2x2x3/16	6.580	6.090	123.3	0.431	0.612	18.739	0.033 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Section Capacity Table

Section No.	Elevation <i>ft</i>	Component Type	Size	Critical Element	<i>P</i> <i>K</i>	$\phi P_{allow}$ <i>K</i>	% Capacity	Pass Fail
T1	192 - 180	Leg	ROHN 2.5 STD	1	-5.741	66.738	8.6	Pass
T2	180 - 160	Leg	ROHN 2.5 STD	25	-29.959	59.996	49.9	Pass
T3	160 - 140	Leg	ROHN 3 EH	55	-59.379	99.054	59.9	Pass
T4	140 - 120	Leg	ROHN 4 EH	76	-94.442	167.894	56.3	Pass
T5	120 - 100	Leg	ROHN 5 EH	97	-128.236	251.347	51.0	Pass
T6	100 - 80	Leg	ROHN 6 EHS	118	-157.572	256.249	61.5	Pass



<b><i>tnxTower</i></b>  <b>MTS Engineering, P.L.L.C.</b> 1717 S. Boulder, Suite 300 Tulsa, OK 74119 Phone: (918) 587-4630 FAX: (918) 587-4630	<b>Job</b>	<b>Page</b>
	77921.018.01.0001 - SKY HILL, CT (BU# 876345)	35 of 35
	<b>Project</b>	<b>Date</b>
		15:20:18 09/13/23
	<b>Client</b>	<b>Designed by</b>
	Crown Castle	R AITHAL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T7	80 - 60	Leg	ROHN 6 EH	133	-189.468	318.945	59.4	Pass
T8	60 - 40	Leg	ROHN 8 EHS	148	-219.603	405.672	54.1	Pass
T9	40 - 20	Leg	ROHN 8 EHS	163	-249.733	405.729	61.6	Pass
T10	20 - 0	Leg	ROHN 8 EHS	178	-279.877	405.717	69.0	Pass
T1	192 - 180	Diagonal	L1 3/4x1 3/4x3/16	7	-1.215	11.895	10.2	Pass
T2	180 - 160	Diagonal	L2x2x3/16	36	-3.299	10.392	31.7	Pass
T3	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-4.976	16.480	30.2	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	81	-6.527	12.587	51.9	Pass
T5	120 - 100	Diagonal	L3x3x1/4	102	-6.866	17.432	39.4	Pass
T6	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	123	-8.013	19.016	42.1	Pass
T7	80 - 60	Diagonal	L4x4x1/4	138	-8.807	24.136	36.5	Pass
T8	60 - 40	Diagonal	L4x4x5/16	153	-8.446	24.922	33.9	Pass
T9	40 - 20	Diagonal	L4x4x5/16	168	-10.043	21.484	46.7	Pass
T10	20 - 0	Diagonal	L4x4x3/8	183	-10.583	21.926	48.3	Pass
T1	192 - 180	Top Girt	L1 3/4x1 3/4x3/16	5	-0.069	4.122	1.7	Pass
T2	180 - 160	Top Girt	L2x2x3/16	29	-0.643	6.245	10.3	Pass
							Summary	
							Leg (T10)	69.0 Pass
							Diagonal (T4)	51.9 Pass
							Top Girt (T2)	10.3 Pass
							Bolt Checks	58.7 Pass
							<b>RATING =</b>	<b>69.0 Pass</b>

**APPENDIX B**  
**BASE LEVEL DRAWING**

(OTHER CONSIDERED EQUIPMENT)  
(1) 1-1/2" TO 130 FT LEVEL

(PROPOSED EQUIPMENT CONFIGURATION)  
(2) 7/32" TO 156 FT LEVEL  
(2) 21/64" TO 156 FT LEVEL  
(3) 1-5/8" TO 156 FT LEVEL

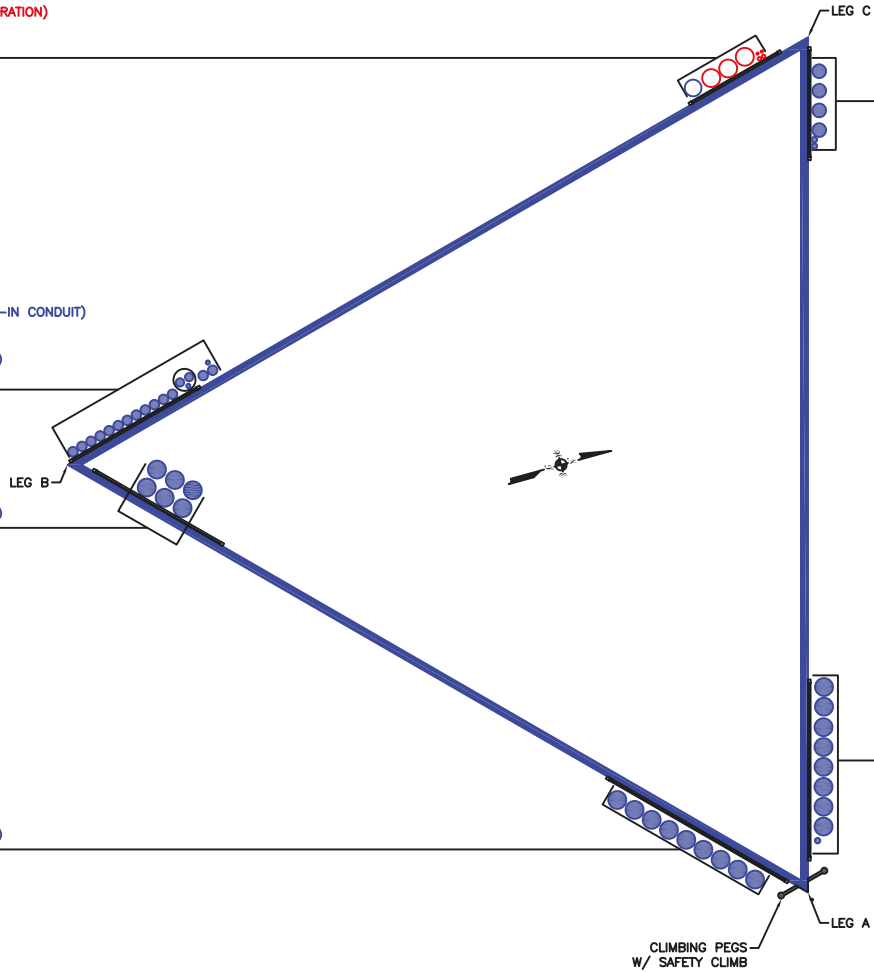
(OTHER CONSIDERED EQUIPMENT—IN CONDUIT)  
(1) 3/8" TO 140 FT LEVEL  
(2) 3/4" TO 140 FT LEVEL  
(OTHER CONSIDERED EQUIPMENT)  
(1) 3/8" TO 140 FT LEVEL  
(14) 7/8" TO 140 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)  
(6) 1-5/8" TO 162 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)  
(9) 1-5/8" TO 171 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)  
(1) 1/2" TO 190 FT LEVEL  
(4) 1-1/4" TO 190 FT LEVEL  
(OTHER CONSIDERED EQUIPMENT)  
(1) 1/2" TO 98 FT LEVEL

(OTHER CONSIDERED EQUIPMENT)  
(1) 1/2" TO 180 FT LEVEL  
(8) 1-5/8" TO 180 FT LEVEL



BUSINESS UNIT: 876345

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

## Self Support Anchor Rod Capacity



Site Info		
BU #	876345	
Site Name	SKY HILL, CT	
Order #	655749, Rev. 0	

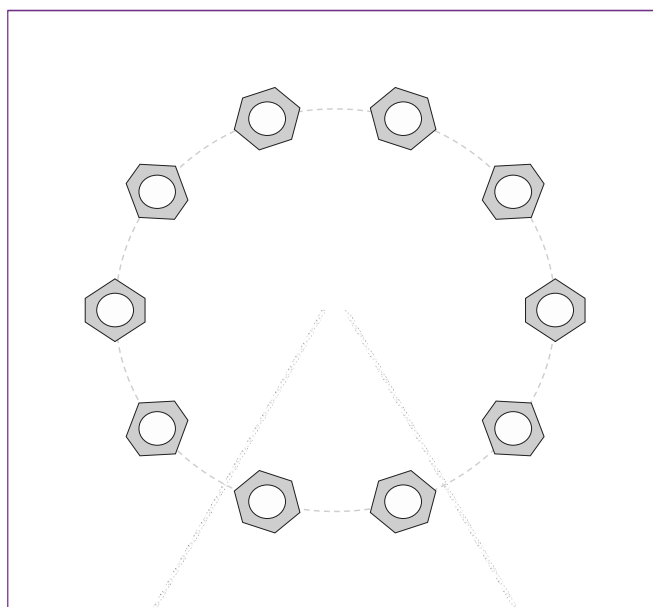
Analysis Considerations		
TIA-222 Revision	H	
Grout Considered:	Yes	
$l_{ar}$ (in)	0	

Applied Loads		
	Comp.	Uplift
Axial Force (kips)	287.37	232.65
Shear Force (kips)	32.70	27.48

\*TIA-222-H Section 15.5 Applied

Considered Eccentricity		
Leg Mod Eccentricity (in)	0.000	
Anchor Rod N.A Shift (in)	0.000	
Total Eccentricity (in)	0.000	

\*Anchor Rod Eccentricity Applied



### Connection Properties

### Analysis Results

#### Anchor Rod Data

(10) 1"  $\phi$  bolts (A354-BC N;  $F_y=109$  ksi,  $F_u=125$  ksi)  
 $l_{ar}$  (in): 0

#### Anchor Rod Summary

(units of kips, kip-in)

$Pu_t = 23.27$	$\phi Pn_t = 56.81$	<b>Stress Rating</b>
$Vu = 2.75$	$\phi Vn = 36.82$	<b>39.0%</b>
$Mu = n/a$	$\phi Mn = n/a$	<b>Pass</b>

## Drilled Pier Foundation

BU # :	876345
Site Name:	SKY HILL, CT
Order Number:	655749,Rev# 0
TIA-222 Revision:	H
Tower Type:	Self Support

Applied Loads		
	Comp.	Uplift
Moment (kip-ft)	0	0
Axial Force (kips)	287.37	232.65
Shear Force (kips)	32.7	27.48

Material Properties	
Concrete Strength, f <sub>c</sub> :	3 ksi
Rebar Strength, F <sub>y</sub> :	60 ksi
Tie Yield Strength, F <sub>y</sub> :	60 ksi

Pier Design Data	
Depth	26 ft
Ext. Above Grade	0.5 ft
Pier Section 1	
From 0.5' above grade to 26' below grade	
Pier Diameter	5 ft
Rebar Quantity	18
Rebar Size	9
Rebar Cage Diameter	51 in
Tie Size	5
Tie Spacing	12 in

Rebar & Pier Options

Embedded Pole Inputs

Belled Pier Inputs

Analysis Results		
Soil Lateral Check		
	Compression	Uplift
D <sub>reqd</sub> (ft from TOC)	11.53	11.53
Soil Safety Factor	43.00	51.17
Max Moment (kip-ft)	260.67	219.06
Rating*	2.9%	2.5%
Soil Vertical Check		
	Compression	Uplift
Skin Friction (kips)	520.43	520.43
End Bearing (kips)	375.00	-
Weight of Concrete (kips)	93.66	70.24
Total Capacity (kips)	895.43	590.68
Axial (kips)	381.03	232.65
Rating*	40.5%	37.5%
Reinforced Concrete Flexure		
	Compression	Uplift
Critical Depth (ft from TOC)	11.81	10.88
Critical Moment (kip-ft)	260.49	218.24
Critical Moment Capacity	2313.70	1700.08
Rating*	10.7%	12.2%
Reinforced Concrete Shear		
	Compression	Uplift
Critical Depth (ft from TOC)	19.01	0.00
Critical Shear (kip)	34.82	27.48
Critical Shear Capacity	483.55	278.04
Rating*	6.9%	9.4%

Structural Foundation Rating*	12.2%
Soil Interaction Rating*	40.5%

\*Rating per TIA-222-H Section 15.5



Check Limitation	
Apply TIA-222-H Section 15.5:	<input checked="" type="checkbox"/>
N/A	<input type="checkbox"/>
Additional Longitudinal Rebar	
Input Effective Depths (else Actual):	<input type="checkbox"/>
Shear Design Options	
Check Shear along Depth of Pier:	<input checked="" type="checkbox"/>
Utilize Shear-Friction Methodology:	<input type="checkbox"/>
Override Critical Depth:	<input type="checkbox"/>

[Go to Soil Calculations](#)

Soil Profile														
Groundwater Depth		N/A		# of Layers		4								
Layer	Top (ft)	Bottom (ft)	Thickness (ft)	Y <sub>soil</sub> (pcf)	Y <sub>concrete</sub> (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	2	2	120	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
2	2	3.33	1.33	130	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
3	3.33	5	1.67	130	150	3	0	1.650	1.650	0.00	0.00			Cohesive
4	5	26	21	135	150	5	0	2.321	2.321	2.10	2.10	25.46479		Cohesive

# ASCE 7 Hazards Report

**Address:**

No Address at This Location

**Standard:**

ASCE/SEI 7-16

**Risk Category:** II**Soil Class:**

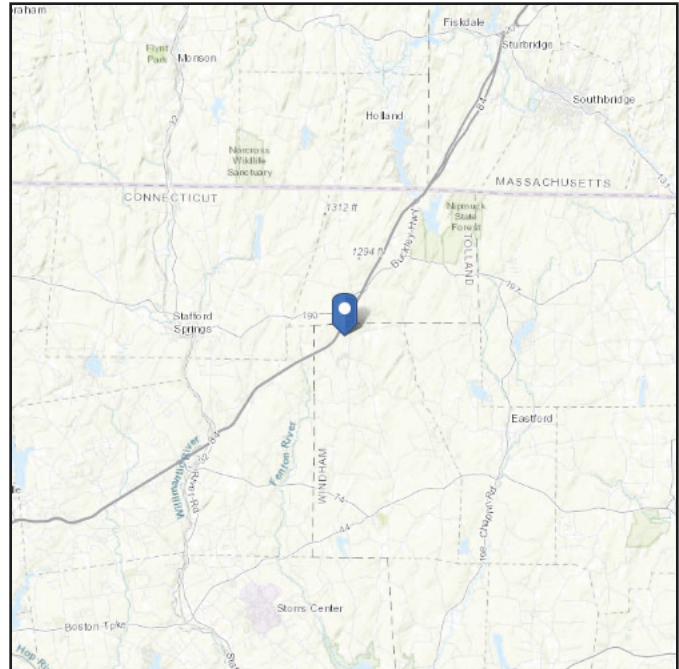
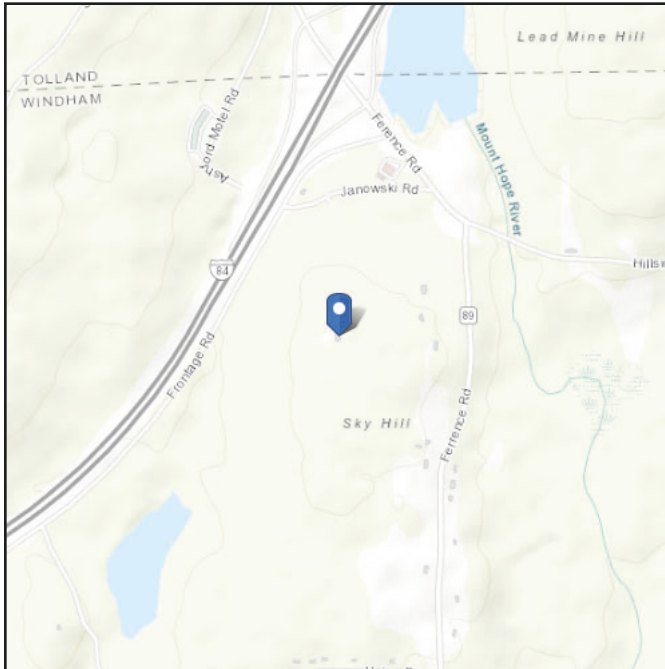
D - Default (see  
Section 11.4.3)

**Latitude:**

41.952139

**Longitude:** -72.195528**Elevation:**

1066.1276209828807 ft  
(NAVD 88)



## Wind

**Results:**

Wind Speed	118 Vmph
10-year MRI	75 Vmph
25-year MRI	84 Vmph
50-year MRI	90 Vmph
100-year MRI	98 Vmph

**Data Source:**

ASCE/SEI 7-16, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2

**Date Accessed:**

Wed Sep 13 2023

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-16 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

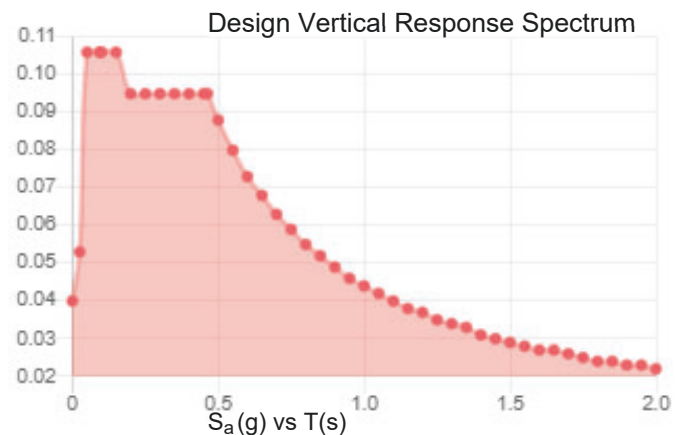
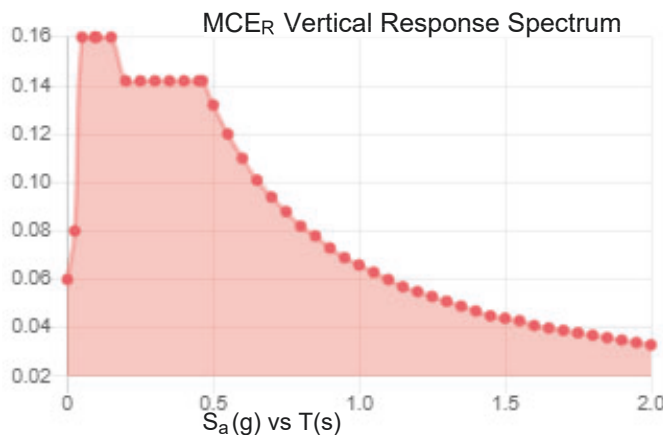
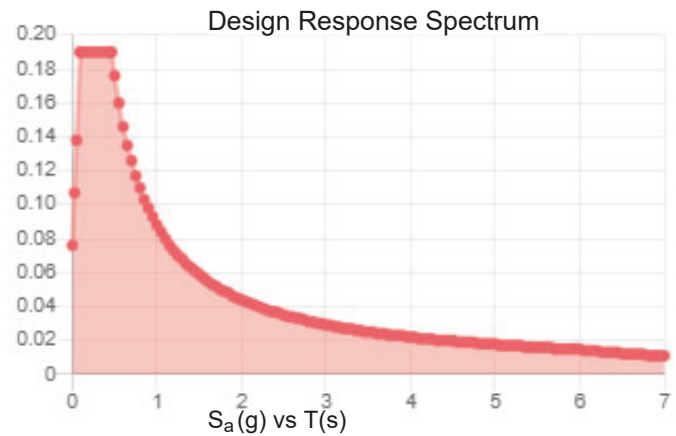
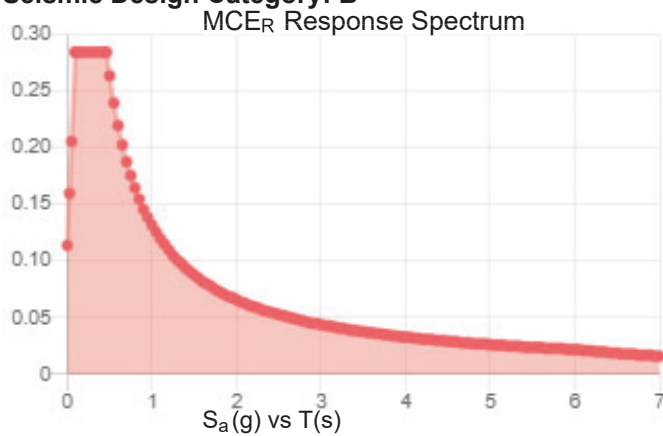
Site is in a hurricane-prone region as defined in ASCE/SEI 7-16 Section 26.2. Glazed openings need not be protected against wind-borne debris.

**Site Soil Class:**

**Results:**

$S_S$ :	0.178	$S_{D1}$ :	0.088
$S_1$ :	0.055	$T_L$ :	6
$F_a$ :	1.6	PGA :	0.095
$F_v$ :	2.4	PGA <sub>M</sub> :	0.151
$S_{MS}$ :	0.285	$F_{PGA}$ :	1.6
$S_{M1}$ :	0.132	$I_e$ :	1
$S_{DS}$ :	0.19	$C_v$ :	0.7

**Seismic Design Category: B**



**Data Accessed:**

**Wed Sep 13 2023**

**Date Source:**

**USGS Seismic Design Maps based on ASCE/SEI 7-16 and ASCE/SEI 7-16 Table 1.5-2. Additional data for site-specific ground motion procedures in accordance with ASCE/SEI 7-16 Ch. 21 are available from USGS.**



**Results:**

Ice Thickness: 1.50 in.  
Concurrent Temperature: 5 F  
Gust Speed 50 mph

**Data Source:** Standard ASCE/SEI 7-16, Figs. 10-2 through 10-8

**Date Accessed:** Wed Sep 13 2023

Ice thicknesses on structures in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

Values provided are equivalent radial ice thicknesses due to freezing rain with concurrent 3-second gust speeds, for a 500-year mean recurrence interval, and temperatures concurrent with ice thicknesses due to freezing rain. Thicknesses for ice accretions caused by other sources shall be obtained from local meteorological studies. Ice thicknesses in exposed locations at elevations higher than the surrounding terrain and in valleys and gorges may exceed the mapped values.

---

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

ASCE does not intend, nor should anyone interpret, the results provided by this Tool to replace the sound judgment of a competent professional, having knowledge and experience in the appropriate field(s) of practice, nor to substitute for the standard of care required of such professionals in interpreting and applying the contents of this Tool or the ASCE 7 standard.

In using this Tool, you expressly assume all risks associated with your use. Under no circumstances shall ASCE or its officers, directors, employees, members, affiliates, or agents be liable to you or any other person for any direct, indirect, special, incidental, or consequential damages arising from or related to your use of, or reliance on, the Tool or any information obtained therein. To the fullest extent permitted by law, you agree to release and hold harmless ASCE from any and all liability of any nature arising out of or resulting from any use of data provided by the ASCE 7 Hazard Tool.

THE COMPOUND ADJUST WAS COMPLETED ON  
09/22/2023. THE CONSTRUCTION DRAWING  
REFLECTS CONDITIONS AT TIME OF ADJUST.

# T Mobile

**T-MOBILE SITE NUMBER:** CT11353C  
**T-MOBILE SITE NAME:** ASHFORD/I-84\_1  
**T-MOBILE PROJECT:** ANCHOR  
**T-MOBILE RAN:** 67D5D998E 6160  
**T-MOBILE A&L:** 67D5998E\_1xAIR+1OP+1QP

**BUSINESS UNIT #:** 876345  
**SITE ADDRESS:** 36 JANOSKI RD  
ASHFORD, CT 06278  
**COUNTY:** WINDHAM  
**SITE TYPE:** SELF SUPPORT TOWER  
**TOWER HEIGHT:** 192'-0"

# T Mobile

**CROWN CASTLE**

**PM&A**  
P. MARSHALL & ASSOCIATES

1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

**T-MOBILE SITE NUMBER:**  
**CT11353C**

**BU #:** 876345  
**CROWN CASTLE SITE NAME:**  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

**EXISTING 192'-0" SELF SUPPORT TOWER**

#### ISSUED FOR:

REV	DATE	ISSUED	DESCRIPTION	DESIGN
1	12/26/2023	ISS	FINAL	JS
2	01/02/2024	JS	REWORKED NOTES	JS
3	01/09/2024	JS	ANTENNA MOVERS	JS

#### APPLICABLE CODES & REFERENCE DOCUMENTS

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITY. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

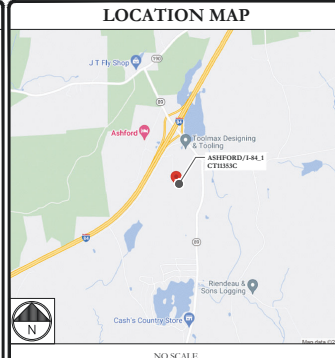
CODE TYPE	CODE
BUILDING	2021 IBC
MECHANICAL	2021 IMC
ELECTRICAL	2020 NEC

#### REFERENCE DOCUMENTS:

STRUCTURAL ANALYSIS:	B+T GROUP # 77921.018.01.0001
DATE:	09/13/2023
MOUNT ANALYSIS:	TRYLON # 211375
DATE:	09/14/2023
RFDS REVISION:	6
DATE:	08/03/2023
ORDER ID:	617436
REVISION:	0

PM&A PROJECT NUMBER: 23CCTCTM-0004

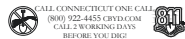
EXISTING T-MOBILE ELECTRIC SERVICE  
METER AND DISCONNECT: 200A 120/240V-60PH  
PPC: MANUFACTURER 200A 120/240V-60PH FAULT CURRENT RATING 65A, 200A GENERATOR PLEIG, 200A MAXIMUM BRANCH CIRCUIT SIZE & 10-AC BREAKER POSITIONS  
PPC UPGRADE IS REQUIRED (10 POSITION TO 15 POSITION)



#### DRAWING INDEX

SHEET #	SHEET DESCRIPTION
T-1	TITLE SHEET
T-2	GENERAL NOTES
G-1.1	COMPOUND PLAN
G-1.2	EXISTING EQUIPMENT PLAN
G-1.3	FINAL EQUIPMENT PLAN
G-2	TOWER ELEVATIONS
C-3	ANTENNA PLANS
C-4	FINAL EQUIPMENT SCHEDULE
C-5.1	TOWER EQUIPMENT DETAILS & SPECIFICATIONS
C-5.2	TOWER EQUIPMENT DETAILS & SPECIFICATIONS
C-6.1	ENCLOSURE CLEARANCES
C-6.2	STEEL SUPPORT CABINET SPECIFICATIONS
C-6.3	BATTERY CABINET SPECIFICATIONS
C-7	RF EQUIPMENT DETAILS & SPECIFICATIONS
E-1	PANEL SCHEDULES & ONE-LINE DIAGRAM
E-2	UTILITY ROUTING & GROUNDING PLAN
G-1	TYPICAL GROUNDING SCHEMATIC
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS

ALL DRAWINGS CONTAINED HEREIN ARE FORMATTED FOR 22X34. CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.



#### PROJECT TEAM

**A&E FIRM:** P. MARSHALL & ASSOCIATES, LLC  
3545 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NC 28273  
PROJECT ENGINEER - TREVOR MCALLISTER  
478-542-3291

**CROWN CASTLE:** 1500 CORPORATE DRIVE  
CANONSBURG, PA 15317

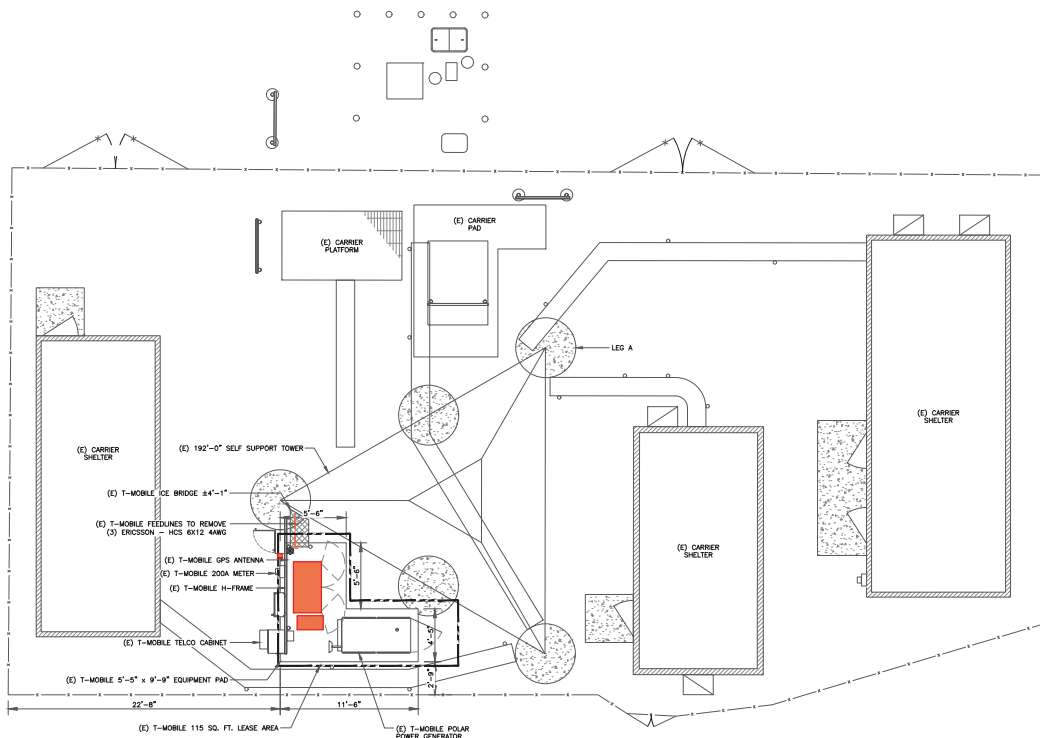
**CONTACTS:** Teicia Polon - PROJECT MANAGER  
Teicia.Polon@crowncastle.com  
Israel Carey - CONSTRUCTION MANAGER  
Israel.Carey@crowncastle.com  
Susan Palm - A&E  
Susan.Palm@crowncastle.com

PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE CROWN NOC AT (800) 788-7011 & CROWN CONSTRUCTION MANAGER.

**SHEET NUMBER:** **T-1** **REVISION:** **2**



T-MOBILE NATIONAL ANCHOR



EQUIPMENT LEGEND:	
[White Box]	EXISTING
[Red Box]	TO BE RELOCATED/REMOVED
[Blue Box]	NEW/RELOCATED

**T Mobile**

**CROWN CASTLE**

**PM&A**  
P. MARSHALL & ASSOCIATES  
1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

T-MOBILE SITE NUMBER:  
**CT11353C**

BU #: **876345**  
CROWN CASTLE SITE  
NAME:  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:				
REV	DATE	ISSUED	DESCRIPTION	DRS, CLM
1	12/20/2023	NO	FINAL	JS
2	01/02/2024	YES	REMOVED BIDS	JS
3	01/02/2024	YES	ANTENNA MODEL	JS

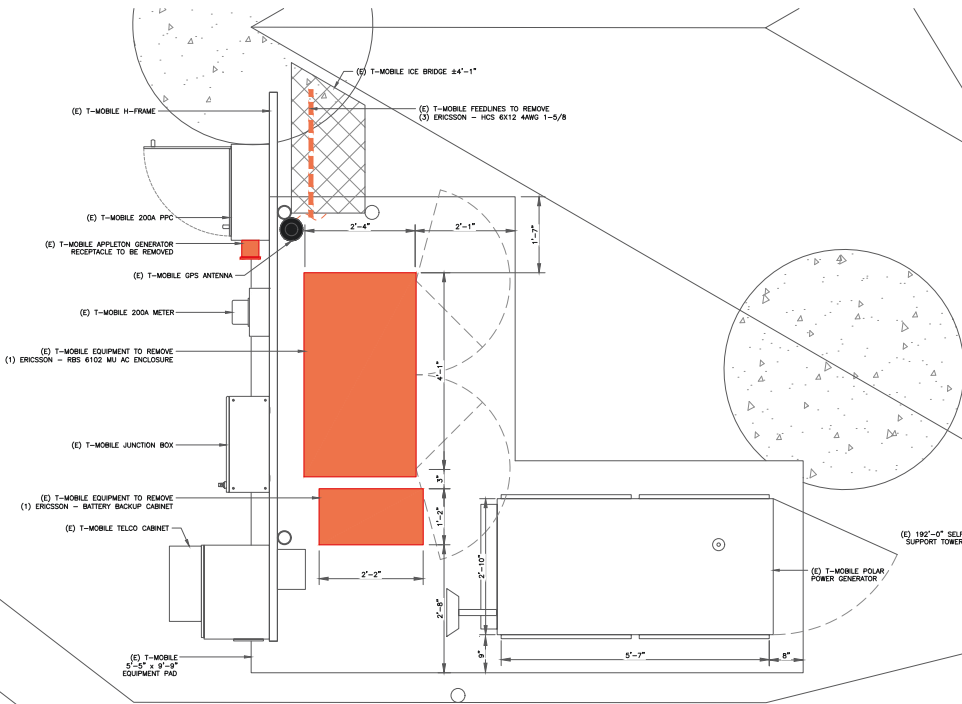
STATE OF CONNECTICUT  
SHERK & CREASEL  
PROFESSIONAL ENGINEER

IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

SHEET NUMBER: **C-1.1** REVISION: **2**

1 COMPOUND PLAN  
SCALE: 1/4"=1'-0" (FULL SIZE)  
1/8"=1'-0" (1 1/2")





EQUIPMENT LEGEND:		
[White Box]	EXISTING	
[Red Box]	TO BE RELOCATED/REMOVED	
[Blue Box]	NEW/RELOCATED	



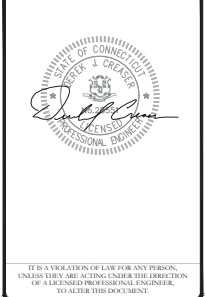
T-MOBILE SITE NUMBER:  
**CT11353C**

BU #: **876345**  
CROWN CASTLE SITE  
NAME:  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:				
REV	DATE	ISSUED	DESCRIPTION	DESIGN
1	12/20/2023	NO	FINAL	JS
2	01/02/2024	JS	REWORKED BIDS	JS
3	01/09/2024	JS	ANTENNA MODEL	JS



IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

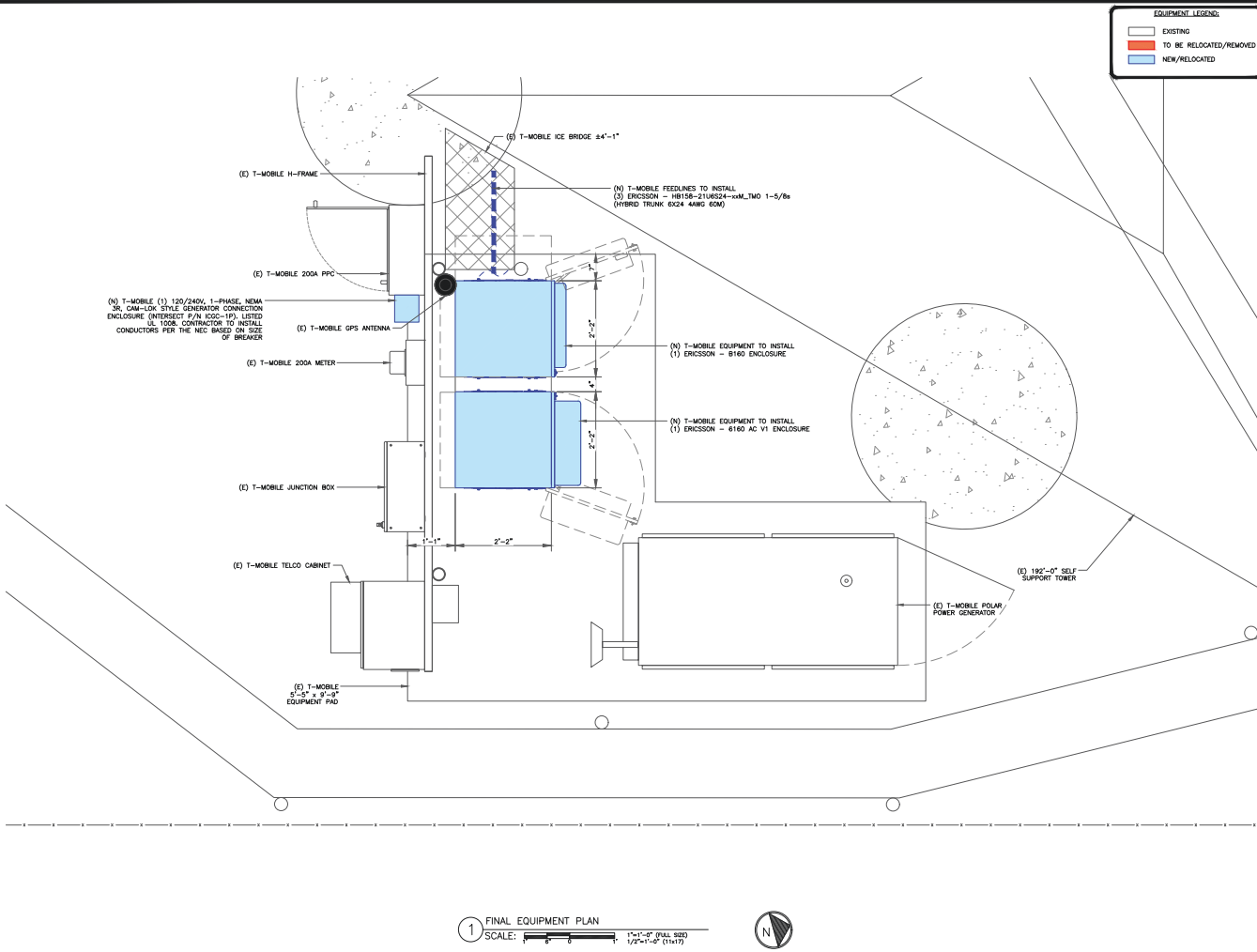
SHEET NUMBER: **C-1.2** REVISION: **2**

1 EXISTING EQUIPMENT PLAN  
SCALE: 1/8"=1'-0" (P.L. 300)  
1/2"=1'-0" (1:60)



T-MOBILE NATIONAL ANCHOR

T-MOBILE NATIONAL ANCHOR



EQUIPMENT LEGEND:		
[White Box]	EXISTING	
[Red Box]	TO BE RELOCATED/REMOVED	
[Blue Box]	NEW/RELOCATED	



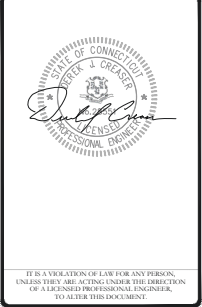
T-MOBILE SITE NUMBER:  
**CT11353C**

BU #: **876345**  
CROWN CASTLE SITE  
NAME:  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:				
REV	DATE	ISSUED	DESCRIPTION	DESIGN
1	12/20/2023	NO	FINAL	JS
2	01/02/2024	JS	REMOVED BIDS	JS
3	01/09/2024	JS	ANTENNA MODEL	JS

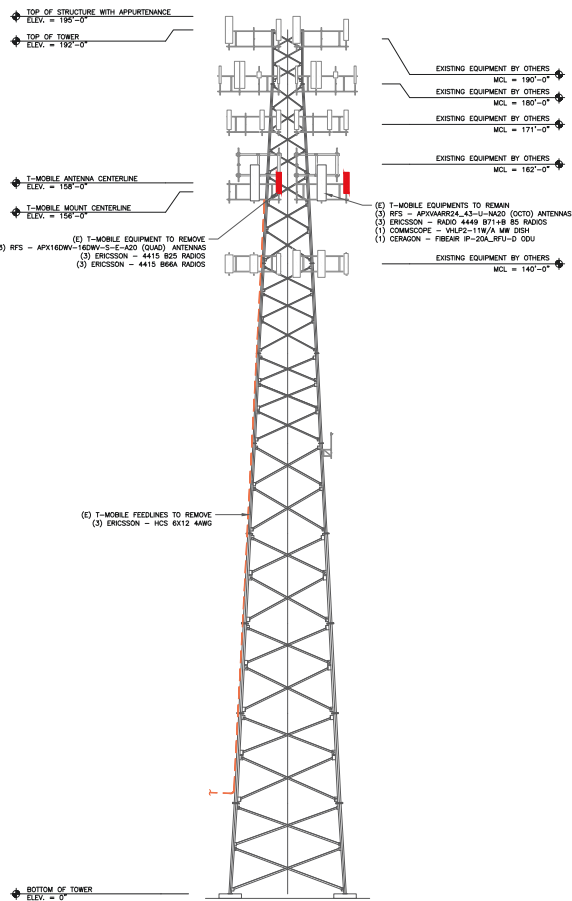


IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

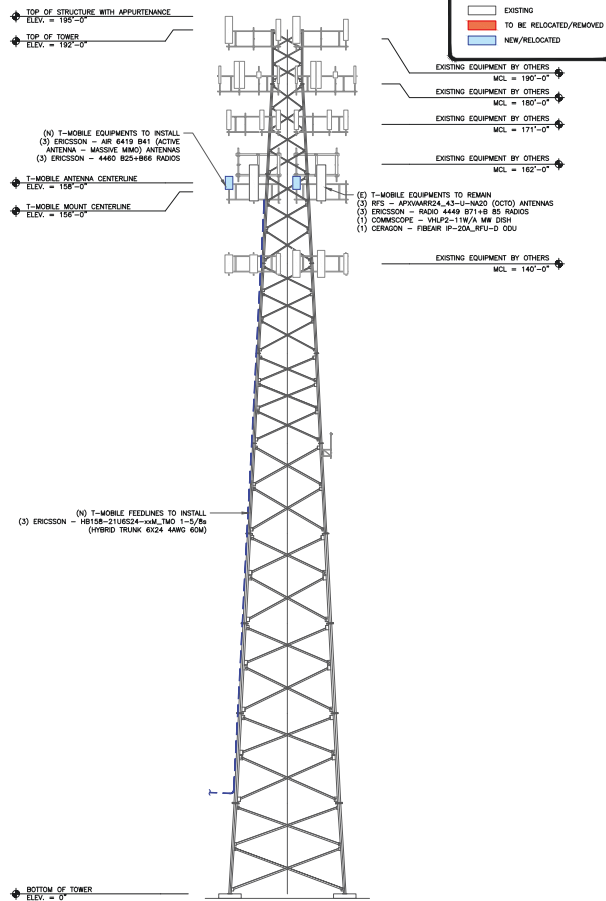
SHEET NUMBER: <b>C-1.3</b>	REVISION: <b>2</b>
-------------------------------	-----------------------

1 FINAL EQUIPMENT PLAN  
SCALE: 1"=4'-0" (FULL SIZE)  
1/2"=1'-0" (1:6)





1 EXISTING TOWER ELEVATION  
SCALE: 3/16"=1'-0" (ALL SIZES)  
3/32"=1'-0" (1/4"=1")



2 FINAL TOWER ELEVATION  
SCALE: 3/16"=1'-0" (ALL SIZES)  
3/32"=1'-0" (1/4"=1")

EQUIPMENT LEGEND:	
<span style="display:inline-block; width:10px; height:10px; border:1px solid black; background-color:white;"></span>	EXISTING
<span style="display:inline-block; width:10px; height:10px; border:1px solid black; background-color:orange;"></span>	TO BE RELOCATED/REMOVED
<span style="display:inline-block; width:10px; height:10px; border:1px solid black; background-color:blue;"></span>	NEW/RELOCATED

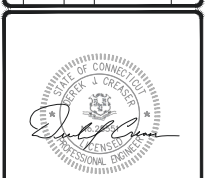
**T-Mobile**

**CROWN CASTLE**

**PM&A**  
P. MARSHALL & ASSOCIATES  
1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

T-MOBILE SITE NUMBER:  
**CT11353C**  
BU #: **876345**  
CROWN CASTLE SITE NAME:  
**SKY HILL**  
36 JANOSKI RD  
ASHFORD, CT 06278  
EXISTING 192'-0" SELF SUPPORT TOWER

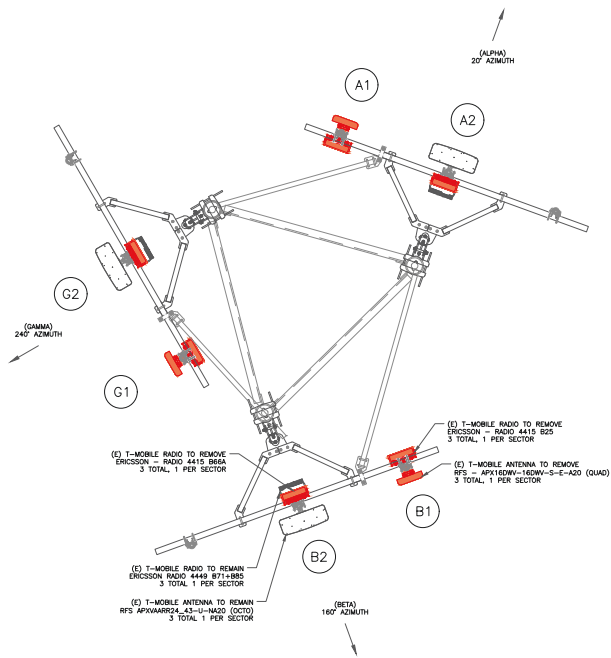
ISSUED FOR:				
REV	DATE	BY	DESCRIPTION	CHKD/APP
1	12/20/2023	MM	FINAL	JS
2	01/02/2024	JS	REMOVED RADIOS	JS
3	01/09/2024	JS	ANTENNA MOVED	JS



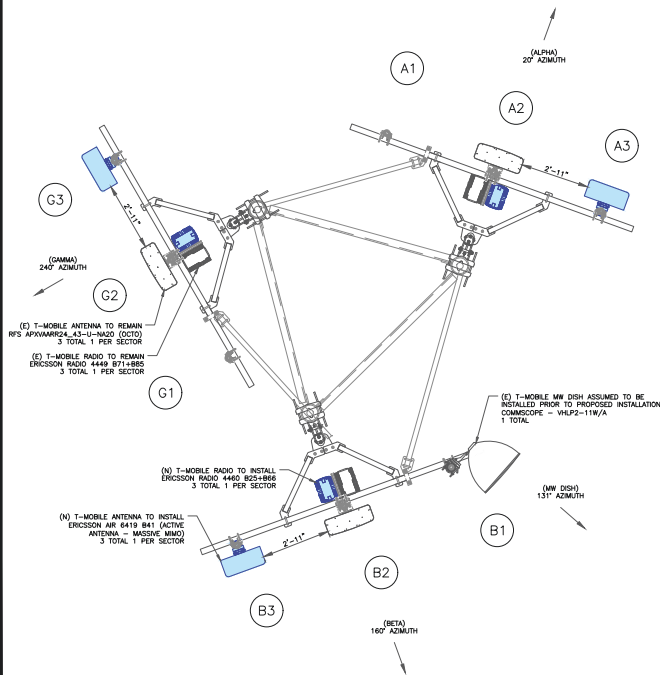
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: **C-2** REVISION: **2**





1 EXISTING ANTENNA PLAN  
SCALE: 1-1/2"=1'-0" (FULL SIZE)  
3/4"=1'-0" (1/4"=1')



2 FINAL ANTENNA PLAN  
SCALE: 1-1/2"=1'-0" (FULL SIZE)  
3/4"=1'-0" (1/4"=1')

EQUIPMENT LEGEND:	
<span style="color: red;">■</span>	EXISTING
<span style="color: blue;">■</span>	TO BE RELOCATED/REMOVED
<span style="color: green;">■</span>	NEW/RELOCATED



T-MOBILE SITE NUMBER:  
**CT11353C**

BU #: **876345**  
CROWN CASTLE SITE  
NAME:  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:				
REV	DATE	ISSUED	DESCRIPTION	DESIGN
1	12/20/2023	NO	FINAL	JS
2	01/02/2024	YES	REWORKED BIDS	JS
3	01/09/2024	YES	ANTENNA MODEL	JS

IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

SHEET NUMBER:	REVISION:
<b>C-3</b>	<b>2</b>



FINAL EQUIPMENT SCHEDULE  
(VERIFY WITH CURRENT RFDS)

POSITION	ANTENNA				RADIO			DIPLEXER			TMA		SURGE PROTECTION		CABLES				
	TECH	STATUS/MANUFACTURER MODEL	AZIMUTH	RAD CENTER	QTY.	STATUS/MODEL	LOCATION	QTY.	STATUS	LOCATION	QTY.	STATUS	QTY.	STATUS/MODEL	QTY.	STATUS/TYPE	SIZE	LENGTH	
ALPHA																			
A1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
A2	L700/N600 L600 L1900 R1900 G1900 L2100	(E) RFS - APXVAMRR24_A3-U-NA20 (DCTO)	20°	158°-0°	1	(E) 4449 871+885	TOWER	-	-	-	-	-	-	-	1	(N) HYBRID	1-5/8	208'-0"	
A3	N2500 L2500	(N) ERICSSON - AIR6419 B41 (ACTIVE ANTENNA MASSIVE MIMO)	20°	158°-0°	1	(N) 4460 825+866	TOWER	-	-	-	-	-	-	-	-	-	-	-	
BETA																			
B1	-	(E) COMMSCOPE - VHLF2-11W/A	131°	158°-0°	-	(E) FIBEAR IP-20A-RFU-D	TOWER	-	-	-	-	-	-	-	2 2	(E) FIBER (E) POWER	21/64 7/32	208'-0"	
B2	L700/N600 L600 L1900 R1900 G1900 L2100	(E) RFS - APXVAMRR24_A3-U-NA20 (DCTO)	20°	158°-0°	1	(E) 4449 871+885	TOWER	-	-	-	-	-	-	-	1	(N) HYBRID	1-5/8	208'-0"	
B3	N2500 L2500	(N) ERICSSON - AIR6419 B41 (ACTIVE ANTENNA MASSIVE MIMO)	20°	158°-0°	1	(N) 4460 825+866	TOWER	-	-	-	-	-	-	-	-	-	-	-	
GAMMA																			
G1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
G2	L700/N600 L600 L1900 R1900 G1900 L2100	(E) RFS - APXVAMRR24_A3-U-NA20 (DCTO)	240°	158°-0°	1	(E) 4449 871+885	TOWER	-	-	-	-	-	-	-	1	(N) HYBRID	1-5/8	208'-0"	
G3	N2500 L2500	(N) ERICSSON - AIR6419 B41 (ACTIVE ANTENNA MASSIVE MIMO)	240°	158°-0°	1	(N) 4460 825+866	TOWER	-	-	-	-	-	-	-	-	-	-	-	
														UNUSED FEEDLINES		-	-	-	-
																-	-	-	-

1 FINAL EQUIPMENT SCHEDULE  
SCALE: NOT TO SCALE

T Mobile

CROWN CASTLE

PM&A  
P. MARSHALL & ASSOCIATES  
1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

T-MOBILE SITE NUMBER:  
CT11353C

BU #: 876345  
CROWN CASTLE SITE  
NAME:  
SKY HILL

36 JANOSKI RD  
ASHFORD, CT 06278  
EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:

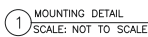
REV	DATE	ISSUED	DESCRIPTION	DESIGN
1	12/20/2023	ISS	FINAL	JS
2	01/02/2024	JS	REWORKED BIDS	JS
3	01/09/2024	JS	ANTENNA MODEL	JS

STATE OF CONNECTICUT  
J. CREAGER  
PROFESSIONAL ENGINEER  
No. 10000

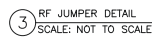
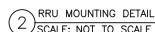
IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

SHEET NUMBER: C-4  
REVISION: 2

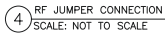
1. COMPLY WITH MANUFACTURERS INSTRUCTIONS TO ENSURE THAT ALL RRU's RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING.
2. DO NOT OPEN RRU PACKAGES IN THE RAIN.
3. ALL PIPES, BRACKETS, AND MISCELLANEOUS HARDWARE TO BE GALVANIZED UNLESS NOTED OTHERWISE.



1. COMPLY WITH MANUFACTURERS INSTRUCTIONS TO ENSURE THAT ALL RRUs RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING.
2. DO NOT OPEN RRU PACKAGES IN THE RAIN.
3. ALL PIPES, BRACKETS, AND MISCELLANEOUS HARDWARE TO BE GALVANIZED UNLESS NOTED OTHERWISE.
4. ANTENNA NOT SHOWN FOR CLARITY



JUMPERS TO BE TORQUED TO 221.27 IN/LBS

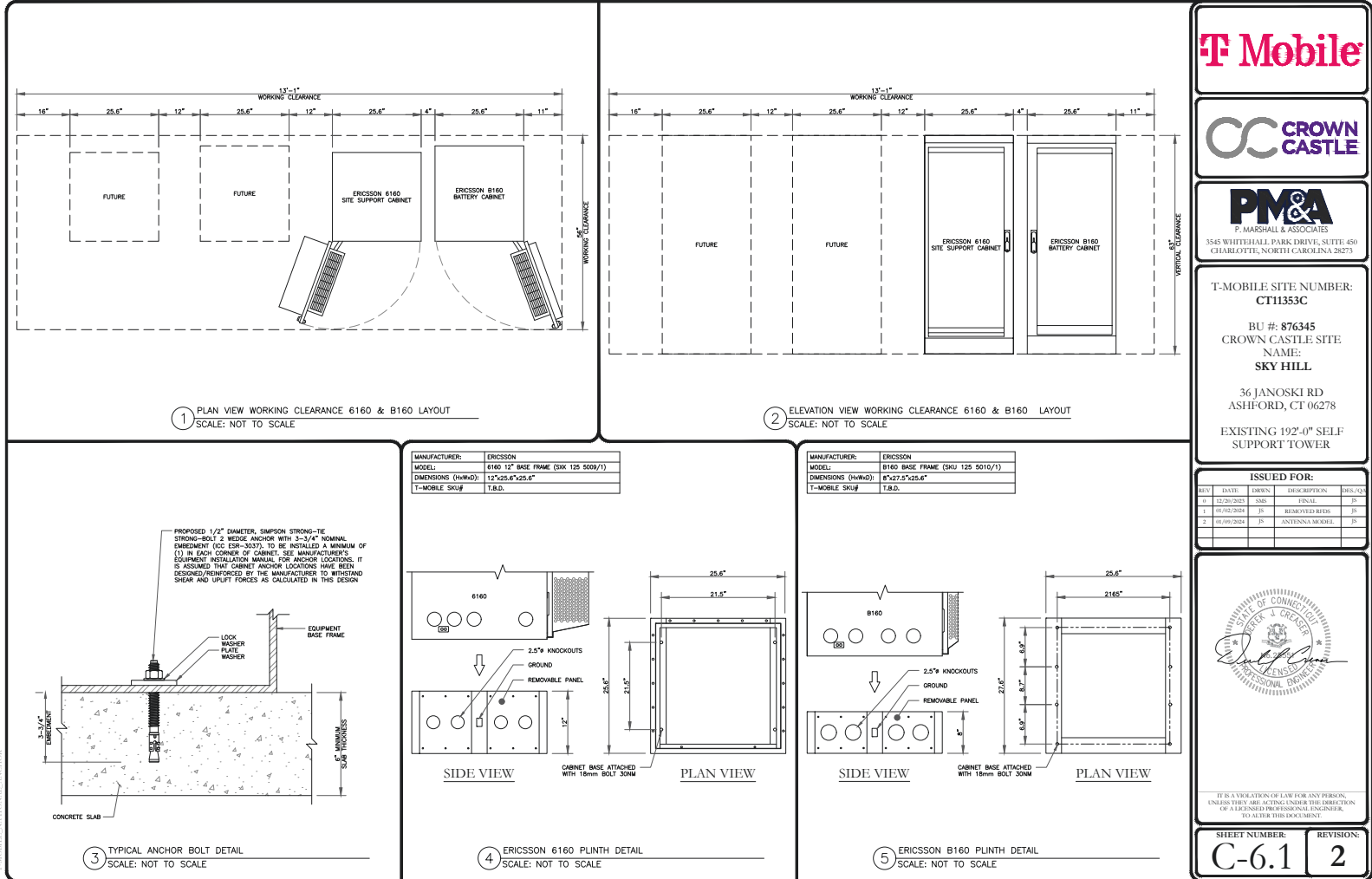


5 NOT USED  
SCALE: NOT TO SCALE

6 6x24 HYBRID TRUNK CROSS SECTION  
SCALE: NOT TO SCALE

C-5.1 | 2





T-MOBILE SITE NUMBER:  
**CT11353C**

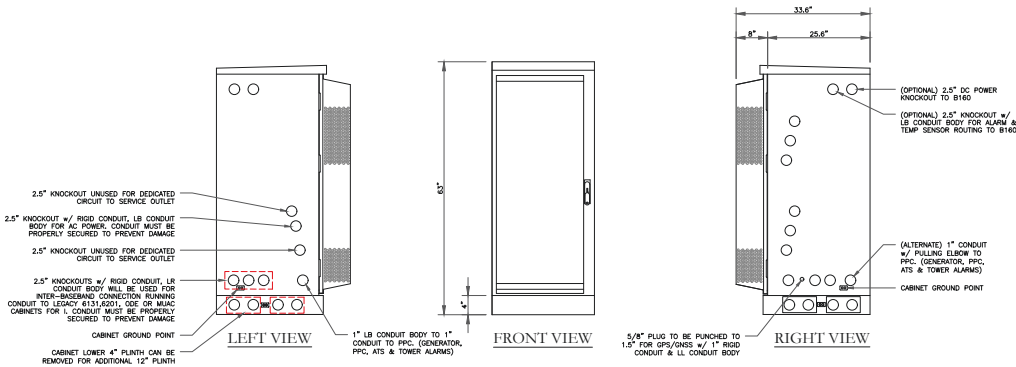
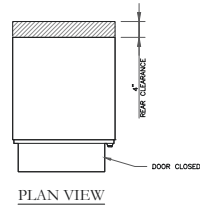
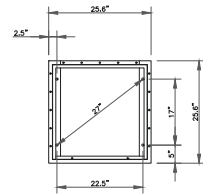
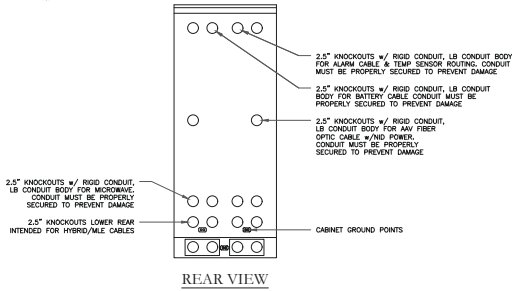
BU #: **876345**  
CROWN CASTLE SITE NAME:  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

EXISTING 192'-0" SELF SUPPORT TOWER

MANUFACTURER:	ERICSSON
MODEL:	(UT6160_ENCL_AC) V1 CABINET
DIMENSIONS (HxWxD):	63"x25.6"x33.6"
WEIGHT:	373 LBS
SKU #:	T.B.D.

NOTE:  
CORRECT KNOCKOUT TOOL REQUIRED FOR PUNCHING KNOCKOUTS. DO NOT DRILL THROUGH KNOCKOUTS.  
CONDUIT MUST BE PROPERLY SECURED TO PREVENT DAMAGE TO CABINETS AND/OR CABLING.  
GROUNDING NOTE:  
CABINET GROUNDING TO USE A SINGLE #2 B7CW CONDUCTOR, W/ 2-HOLE, 1" C-C, LONG BARREL WINDOW LUG, IN 3/4" UPVC TO GROUND RING. PLINTH GROUNDING IS NOT REQUIRED.



1 6160 ERICSSON SITE SUPPORT CABINET  
SCALE: 1"=1'-0" (FULL SIZE)  
1/2"=1'-0" (1/4"=1')

T Mobile

CROWN CASTLE

PM&A  
P. MARSHALL & ASSOCIATES  
1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

T-MOBILE SITE NUMBER:  
CT11353C  
BU #: 876345  
CROWN CASTLE SITE  
NAME:  
SKY HILL  
36 JANOSKI RD  
ASHFORD, CT 06278  
EXISTING 192'-0" SELF  
SUPPORT TOWER

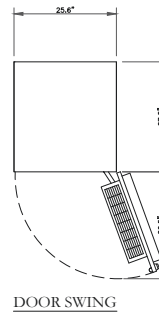
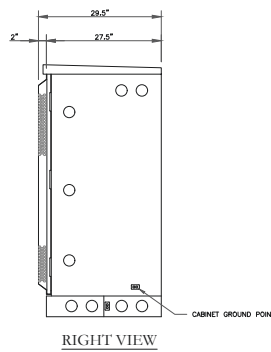
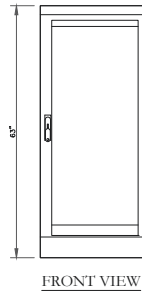
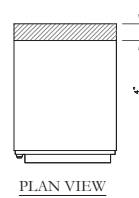
ISSUED FOR:				
REV	DATE	BY	DESCRIPTION	CHKD/APP
1	12/20/2023	MM	FINAL	JS
2	01/02/2024	JS	REMOVED BIDS	JS
3	01/02/2024	JS	ANTENNA MOVED	JS

STATE OF CONNECTICUT  
J. CREAGER  
PROFESSIONAL ENGINEER  
No. 12345  
EXPIRATION DATE 12/31/2025

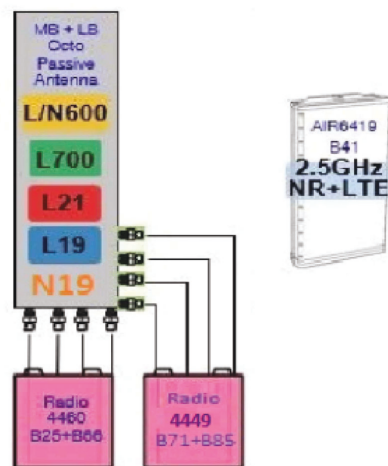
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: C-6.2	REVISION: 2
------------------------	----------------

NOTE:  
CORRECT KNOCKOUT TOOL REQUIRED FOR PUNCHING KNOCKOUTS. DO NOT DRILL THROUGH KNOCKOUTS  
CONDUIT MUST BE PROPERLY SECURED TO PREVENT DAMAGE TO CABINETS AND/OR CABLEING  
GROUNDING NOTE:  
CABINET GROUNDING TO USE A SINGLE, #2 BTWC CONDUCTOR, W/ 2-HOLE, 1" C-C, LONG BARREL, WINDOW LUG, IN 3/4" LFLC TO GROUND RING. PLINTH GROUNDING IS NOT REQUIRED.



## 67D5D998E 6160



T-MOBILE SITE NUMBER:  
CT11353C

BU #: 876345  
CROWN CASTLE SITE  
NAME:  
SKY HILL

36 JANOSKI RD  
ASHFORD, CT 06278

EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:				
REV	DATE	DRWN	DESCRIPTION	DES./QA
0	12/20/2023	SMS	FINAL	JS
1	01/02/2024	JS	REMOVED REFS	JS
2	01/09/2024	JS	ANTENNA MODEL	JS



IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

SHEET NUMBER: REVISION:

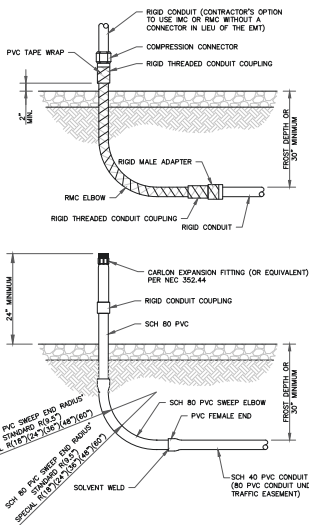
C-7 | 2



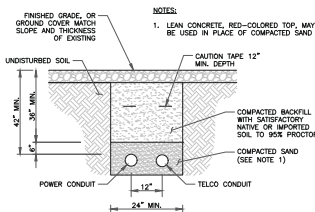


# INSTALLER NOTES

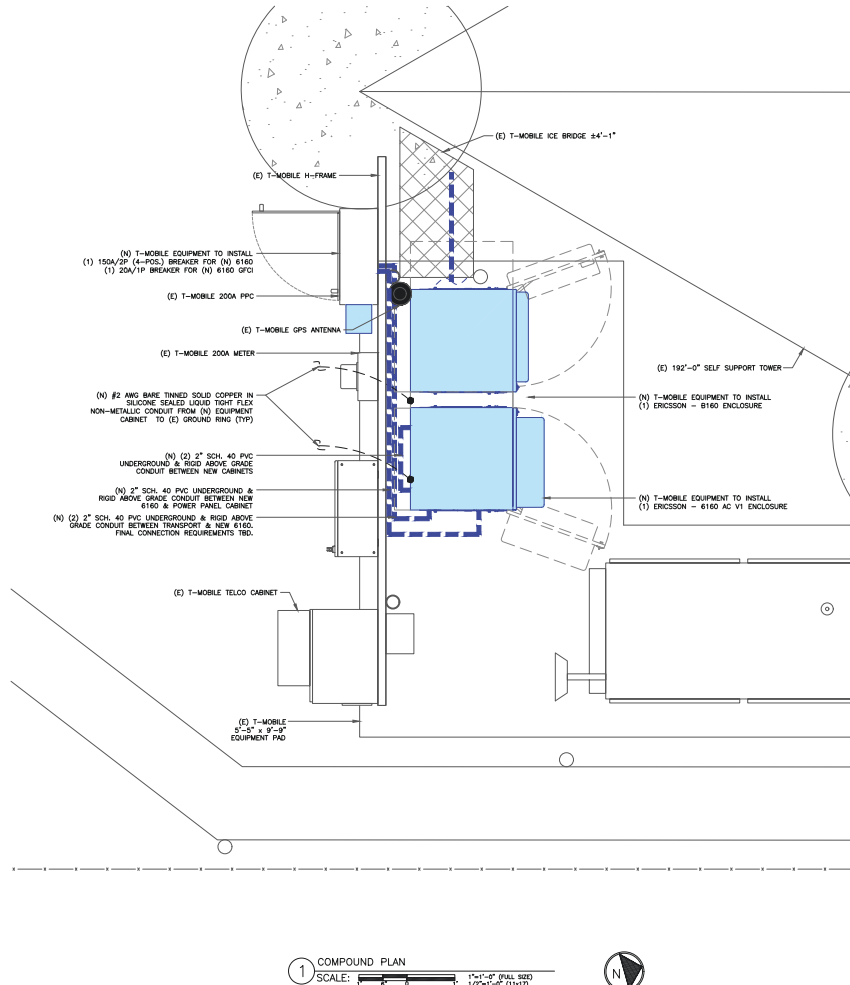
ALL METAL CONDUIT INSTALLED IN DIRECT CONTACT WITH THE EARTH SHALL BE CONSIDERED TO BE INSTALLED IN A SEVERELY CORROSIVE ENVIRONMENT AND IS REQUIRED TO HAVE SUPPLEMENTAL PROTECTION AGAINST CORROSION (NEC ARTICLE 342.10(B) & 344.10(B)(1)). THIS PROTECTION SHALL EITHER BE AN APPROVED MANUFACTURER INSTALLED PROTECTIVE COATING ON THE CONDUIT OR SHALL BE (2) LAYERS OF 10 MIL PVC PIPE WRAP TAPE INSTALLED USING OVERLAPPING SPIRAL WRAPS ON VERTICAL PIPE. THE OUTSIDE LAYER OF TAPE SHALL BE WRAPPED SO AS TO PROVIDE SHEDDING OF WATER (I.E. TAPE SHOULD WRAP IN AN UPWARD DIRECTION WITH LOWER WRAP BEING BENEATH THE WRAP ABOVE). SPIRAL WRAPS SHALL HAVE A MINIMUM OF 1/4" OVERLAP WITH THE PRECEDING TAPE WRAP. ANY OTHER METHODS OF CORROSION PROTECTION SHALL REQUIRE APPROVAL BY THE ENGINEER OF RECORD PRIOR TO BEING USED.



1 CONDUIT STUB UP DETAILS  
SCALE: NOT TO SCALE



2 TRENCH DETAIL  
SCALE: NOT TO SCALE



3 COMPOUND PLAN  
SCALE: 1\"/>

**T Mobile**

**CROWN CASTLE**

**PM&A**  
P. MARSHALL & ASSOCIATES  
1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

T-MOBILE SITE NUMBER:  
**CT11353C**  
BU #: **876345**  
CROWN CASTLE SITE NAME:  
**SKY HILL**  
36 JANOSKI RD  
ASHFORD, CT 06278  
EXISTING 192'-0\"/>

ISSUED FOR:				
REV	DATE	BY	DESCRIPTION	CHKD/APP
1	12/20/2023	MM	FINAL	JS
2	01/02/2024	JS	REMOVED BIDS	JS
3	01/02/2024	JS	ANTENNA MODEL	JS

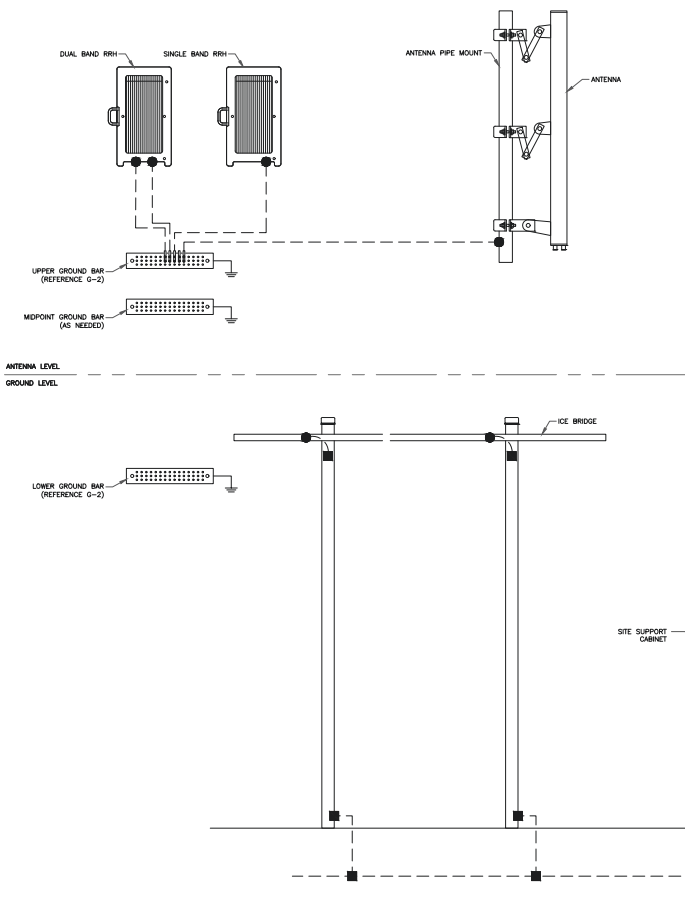
(E) T-MOBILE POLAR POWER GENERATOR

STATE OF CONNECTICUT  
OFFICE OF THE REGISTERED PROFESSIONAL ENGINEER  
Professional Seal of P. Marshall & Associates, Inc.  
P. Marshall & Associates, Inc.  
1345 Whitehall Park Drive, Suite 450  
Charlotte, North Carolina 28273

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER THIS DOCUMENT.

SHEET NUMBER: **E-2** REVISION: **2**

T-MOBILE NATIONAL ANCHOR



1 TYPICAL FINAL GROUNDING SCHEMATIC  
SCALE: NOT TO SCALE

**GROUNDING PLAN LEGEND:**

- #6 STRANDED COPPER WITH GREEN INSULATION GROUND WIRE
- - - #2 STRANDED COPPER WITH GREEN INSULATION GROUND WIRE
- ... #2 BARE, SOLDERED, TINNED COPPER GROUND WIRE
- EXOTHERMIC WELD
- MECHANICAL CONNECTION
- ⊙ COPPER GROUND ROD
- ⊗ GROUND ROD W/ TEST WELL

**NOTE:**  
SEE FINAL EQUIPMENT PLAN FOR NEW EQUIPMENT  
REQUIRING GROUNDING. CONTRACTOR TO VERIFY  
EXISTING EQUIPMENT GROUNDING IN FIELD.  
CONTRACTOR TO VERIFY IN FIELD AND INSTALL ANY  
MISSING T-MOBILE GROUND BARS ON SITE.

P. MARSHALL & ASSOCIATES  
1345 WHITEHALL PARK DRIVE, SUITE 450  
CHARLOTTE, NORTH CAROLINA 28273

T-MOBILE SITE NUMBER:  
**CT11353C**

BU #: **876345**  
CROWN CASTLE SITE NAME:  
**SKY HILL**

36 JANOSKI RD  
ASHFORD, CT 06278

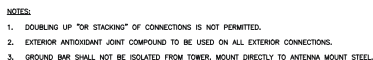
EXISTING 192'-0" SELF  
SUPPORT TOWER

ISSUED FOR:				
REV	DATE	BY	DESCRIPTION	DRS, CLM
1	12/20/2023	MS	FINAL	JS
1	01/02/2024	JS	REMOVED BIDS	JS
2	01/09/2024	JS	ANTENNA MODEL	JS

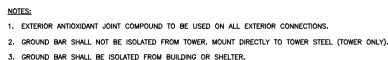
IT IS A VIOLATION OF LAW FOR ANY PERSON,  
UNLESS THEY ARE ACTING UNDER THE DIRECTION  
OF A LICENSED PROFESSIONAL ENGINEER,  
TO ALTER THIS DOCUMENT.

SHEET NUMBER: **G-1**

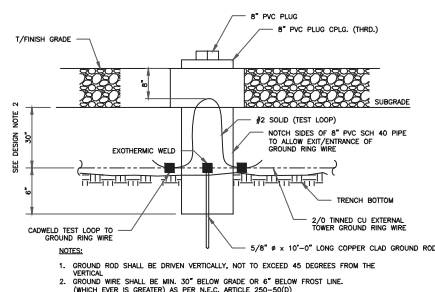
REVISION: **2**



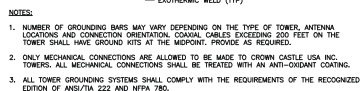
1 ANTENNA SECTOR GROUND BAR DETAIL  
SCALE: NOT TO SCALE



② TOWER/SHELTER GROUND BAR DETAIL  
SCALE: NOT TO SCALE



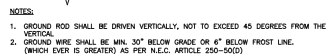
3 INSPECTION WELL DETAIL  
SCALE: NOT TO SCALE



4 TYPICAL ANTENNA CABLE GROUNDING  
SCALE: NOT TO SCALE



5 HARDWARE DETAIL FOR EXTERIOR CONNECTIONS  
SCALE: NOT TO SCALE

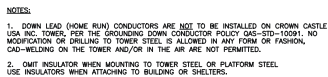
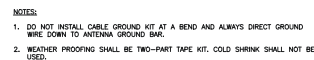
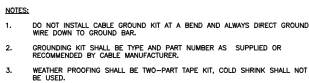


6 GROUND ROD DETAIL  
SCALE: NOT TO SCALE

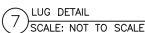
2



(1) SCALE: NOT TO SCALE

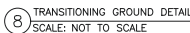
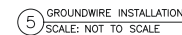


6 SCALE: NOT TO SCALE



**NOTES:**

1. ALL GROUNDING LUGS ARE TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS. ALL HARDWARE BOLTS, NUTS, LOCK WASHERS SHALL BE STAINLESS STEEL. ALL HARDWARE ARE TO BE AS FOLLOWS: BOLT, FLAT WASHER, GROUND BAR, GROUND LUG FLAT WASHER AND NUT.





FOX HILL TELECOM

## Radio Frequency Emissions Analysis Report



Site ID: CT11353C

Ashford/ I-84\_1 33  
Janoski Road Ashford,  
CT 06278

September 29, 2023

Fox Hill Telecom Project Number: 230996

Site Compliance Summary	
Compliance Status:	COMPLIANT
Site total MPE% of FCC general population allowable limit:	9.122 %



September 29, 2023

T-MOBILE  
Attn: RF Manager  
35 Griffin Road South  
Bloomfield, CT 06009

## Emissions Analysis for Site: **CT11353C – Ashford/ I-84\_1**

Fox Hill Telecom, Inc (“Fox Hill”) was directed to analyze the proposed upgrades to the T-MOBILE facility located at **33 Janoski Road, Ashford, CT**, for the purpose of determining whether the emissions from the Proposed T-MOBILE Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

General population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 600 MHz & 700 MHz bands are approximately  $400 \mu\text{W}/\text{cm}^2$  and  $467 \mu\text{W}/\text{cm}^2$  respectively. The general population exposure limit for the 1900 MHz (PCS), 2100 MHz (AWS), 2500 MHz (BRS) & 11 GHz microwave bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report the percentage of MPE rather than power density.



# FOX HILL TELECOM

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.



## CALCULATIONS

Calculations were performed for the proposed upgrades to the T-MOBILE antenna facility located at **33 Janoski Road, Ashford, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65 for far field modeling calculations.

In OET-65, plane wave power densities in the Far Field of an antenna are calculated by considering antenna gain and reflective waves that would contribute to exposure.

Since the radiation pattern of an antenna has developed in the **Far Field** region the power gain in specific directions needs to be considered in exposure predictions to yield an Effective Radiated Power (ERP) in each specific direction from the antenna. Also, since the vertical radiation pattern of the antenna is considered, the exposure calculations would most likely be reduced significantly at ground level, resulting in a more realistic estimate of the actual exposure levels. To determine a worst-case scenario at each point along the calculation radials, each point was calculated using the antenna gain value at each angle of incident and compared against the result using an isotropic radiator at the antenna height with the greater of the two used to yield the more pessimistic far field value for each point along the calculation radial.

Additionally, to model a truly "worst case" prediction of exposure levels at or near a surface, such as at ground-level or on a rooftop, reflection off the surface of antenna radiation power can be assumed, resulting in a potential 1.6 times increase in power density in calculating far field power density values.

With these factors Considered, the worst case **Far Field prediction model** utilized in this analysis is determined by the following equation:

Equation 9 per FCC OET65 for Far Field Modeling

$$S = \frac{33.4 \text{ ERP}}{R^2}$$

S = Power Density (in  $\mu\text{W}/\text{cm}^2$ )

ERP = Effective Radiated Power from antenna (watts)

R = Distance from the antenna (meters)

Predicted far field power density values for all carriers identified in this report were calculated 6 feet above the ground level and are displayed as a percentage of the applicable FCC standards. All emissions values for other carriers were calculated using the same Far Field model outlined above, using industry standard radio configurations and frequency band selection based upon available licenses in this geographic area for emissions contribution estimates.





For each T-Mobile sector the following channel counts, frequency bands and power levels were utilized as shown in *Table 1*:

Technology	Frequency Band	Channel Count	Transmit Power per Channel (W)
LTE / 5G NR	600 MHz	2	60
LTE	700 MHz	2	20
LTE	1900 MHz (PCS)	4	40
GSM	1900 MHz (PCS)	1	15
LTE	2100 MHz (AWS)	4	40
LTE / 5G NR	2500 MHz (BRS)	8	20
Microwave (Sector A)	11 GHz	1	1

*Table 1: Channel Data Table*



The following T-Mobile antennas listed in *Table 2* were used in the modeling for transmission in the 600 MHz, 700 MHz, 1900 MHz (PCS), 2100 MHz (AWS), 2500 MHz (BRS) and 11 GHz microwave frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below.

Sector	Antenna Number	Antenna Make / Model	Antenna Centerline (ft)
A	1	RFS APXVAARR24 43-U-NA20	158
A	2	Ericsson AIR6419 B41	158
A	3	Commscope VHLP2-11W/A	158
B	1	RFS APXVAARR24 43-U-NA20	158
B	2	Ericsson AIR6419 B41	158
C	1	RFS APXVAARR24 43-U-NA20	158
C	2	Ericsson AIR6419 B41	158

*Table 2: Antenna Data*

All calculations were done with respect to uncontrolled / general population threshold limits.



## RESULTS

Per the calculations completed for the proposed T-MOBILE configurations *Table 3* shows resulting emissions power levels and percentages of the FCC's allowable general population limit.

Antenna ID	Antenna Make / Model	Frequency Bands	Antenna Gain (dBd)	Channel Count	Total TX Power (W)	ERP (W)	MPE %
Antenna A1	RFS APXVAARR24 43-U-NA20	600 MHz / 700 MHz / 1900 MHz (PCS) / 2100 MHz (AWS)	12.95 / 13.35 / 15.65 / 16.35	13	495	16,563.74	1.120
Antenna A2	Ericsson AIR6419 B41	2500 MHz (BRS)	21.5	8	160	22,600.60	0.990
Antenna A3	Commscope VHLP2-11W/A (Microwave)	11 GHz	32.35	1	1	1,717.91	0.002
Sector A Composite MPE%							<b>2.112</b>
Antenna B1	RFS APXVAARR24 43-U-NA20	600 MHz / 700 MHz / 1900 MHz (PCS) / 2100 MHz (AWS)	12.95 / 13.35 / 15.65 / 16.35	13	495	16,563.74	1.120
Antenna B2	Ericsson AIR6419 B41	2500 MHz (BRS)	21.5	8	160	22,600.60	0.990
Sector B Composite MPE%							<b>2.110</b>
Antenna C1	RFS APXVAARR24 43-U-NA20	600 MHz / 700 MHz / 1900 MHz (PCS) / 2100 MHz (AWS)	12.95 / 13.35 / 15.65 / 16.35	13	495	16,563.74	1.12
Antenna C2	Ericsson AIR6419 B41	2500 MHz (BRS)	21.5	8	160	22,600.60	0.9900
Sector C Composite MPE%							<b>2.110</b>

*Table 3: T-MOBILE Emissions Levels*



# FOX HILL TELECOM

The Following table (*table 4*) shows all additional identified carriers on site and their emissions contribution estimates, along with the newly calculated maximum T-MOBILE MPE contributions per this report. FCC OET 65 specifies that for carriers utilizing directional antennas that the highest recorded sector value be used for composite site MPE values due to their greatly reduced emissions contributions in the directions of the adjacent sectors. For this site, the T-Mobile sector with the largest calculated MPE% is **Sector A**. *Table 5* below shows a summary for each T-MOBILE Sector as well as the composite estimated MPE value for the site.

Site Composite MPE%	
Carrier	MPE%
T-MOBILE – Max Value (Sector A)	<b>2.112 %</b>
AT&T	4.960 %
Verizon Wireless	2.050 %
<b>Site Total MPE %:</b>	<b>9.122 %</b>

*Table 4: All Carrier MPE Contributions*

T-MOBILE Sector A Total:	<b>2.112 %</b>
T-MOBILE Sector B Total:	2.110 %
T-MOBILE Sector C Total:	2.110 %
Site Total:	9.12 %

*Table 5: Site MPE Summary*



Table 6 below details a breakdown by frequency band and technology for the MPE power values for the maximum calculated T-MOBILE sector(s). For this site, the T-Mobile sector with the largest calculated MPE% is **Sector A**.

T-MOBILE _ Frequency Band / Technology Max Power Values (Sector A)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
T-Mobile 600 MHz LTE / 5G NR	2	1,183.45	158	1.88	600 MHz	400	0.470%
T-Mobile 700 MHz LTE	2	432.54	158	0.61	700 MHz	467	0.130%
T-Mobile 1900 MHz (PCS) LTE / 5G NR	4	1,469.13	158	2.50	1900 MHz (PCS)	1000	0.250%
T-Mobile 1900 MHz (PCS) GSM	1	550.92	158	0.20	1900 MHz (PCS)	1000	0.020%
T-Mobile 2100 MHz (AWS) LTE	4	1,726.08	158	2.50	2100 MHz (AWS)	1000	0.250%
T-Mobile 2500 MHz (BRS) LTE / 5G NR	8	2,825.08	158	9.90	2500 MHz (BRS)	1000	0.990%
T-Mobile 11 GHz Microwave	1	1,717.91	158	0.02	11 GHz	1000	0.002%
						<b>Total:</b>	<b>2.112 %</b>

Table 6: T-MOBILE Maximum Sector MPE Power Values



## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-MOBILE facility as well as the site composite emissions estimates value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-MOBILE Sector	Power Density Value (%)
Sector A:	2.112 %
Sector B:	2.110 %
Sector C:	2.110 %
T-MOBILE Maximum Total (Sector A):	2.112 %
Site Total:	9.122%
Site Compliance Status:	<b>COMPLIANT</b>

The estimated composite MPE value for this site assuming all carriers present is **9.122%** of the allowable FCC established general population limit sampled at the ground level. This is based upon the far field calculations performed for all carriers identified in this report.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite estimated values calculated were well within the allowable 100% threshold standard per the federal government.

Scott Heffernan  
Principal RF Engineer  
**Fox Hill Telecom, Inc**  
Worcester, MA 01609  
(978)660-3998