



Crown Castle  
3 Corporate Park Drive, Suite 101  
Clifton Park, NY 12065

August 8, 2018

Melanie A. Bachman  
Acting Executive Director  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

**RE: Notice of Exempt Modification for Sprint DO Macro: 876345**  
**Sprint Site ID: CT03XC204**  
**3 Janowski Road, Ashford, CT 06419**  
**Latitude: 41° 57' 7.7" / Longitude: -72° 11' 43.9"**

Dear Ms. Bachman:

Sprint currently maintains six (6) antennas at the 192-foot level of the existing 192-foot self-support tower located at 33 Janoski Road, Ashford, CT. The tower is owned by Crown Castle. The property is owned by David H Martin C/O Sprint Spectrum CT-03XC204. Sprint now intends to replace six (6) antennas with six (6) new antennas. These antennas would be installed at the 192-foot level of the tower. Sprint also intends to install twelve (12) RRH's, add four (4) Hybrid cables and remove six (6) existing coaxial cables

The facility was approved by the Town of Ashford Planning and Zoning Commission at the annual meeting on November 12, 1996 with no conditions.

Please accept this letter as notification pursuant to Regulations of Connecticut State Agencies § 16-50j-73, for construction that constitutes an exempt modification pursuant to R.C.S.A. § 16-50j-72(b)(2). In accordance with R.S.C.A. § 16-50j-73, a copy of this letter is being sent to the First Selectman, Mr. Michael J. Zambo, Zoning Enforcement Officer, Mr. Michael Gardner, the land owner is Sprint Spectrum and Crown Castle is the tower owner.

1. The proposed modifications will not result in an increase in the height of the existing tower.
2. The proposed modifications will not require the extension of the site boundary.
3. The proposed modification will not increase noise levels at the facility by six decibels or more, or to levels that exceed state and local criteria.
4. The operation of the replacement antennas will not increase radio frequency emissions at the facility to a level at or above the Federal Communication Commission safety standard.

Melanie A. Bachman

August 8, 2018

Page 2

5. The proposed modifications will not cause a change or alteration in the physical or environmental characteristics of the site.
6. The existing structure and its foundation can support the proposed loading.

For the foregoing reasons, Sprint respectfully submits that the proposed modifications to the above-reference telecommunications facility constitutes an exempt modification under R.C.S.A. § 16-50j-72(b)(2). Please send approval/rejection letter to Attn: Jeffrey Barbadora.

Sincerely,



Jeffrey Barbadora

Real Estate Specialist

12 Gill Street, Suite 5800, Woburn, MA 01801

781-729-0053

[Jeff.Barbadora@crowncastle.com](mailto:Jeff.Barbadora@crowncastle.com)

Attachments:

Tab 1: Exhibit-1: Compound plan and elevation depicting the planned changes

Tab 2: Exhibit-2: Structural Modification Report

Tab 3: Exhibit-3: General Power Density Table Report (RF Emissions Analysis Report)

cc: Mr. Michael J. Zambo  
Town of Ashford-First Selectman  
5 Town Hall Road  
Ashford, CT 06278  
(860) 487-4400

Mr. Michael Gardner  
Zoning Enforcement Officer  
5 Town Hall Road  
Ashford, CT 06278  
(860) 487-4415

FILE SITE # 204

SKY HILL

ZONING

RECEIVED

11-13-96 *ljf*

## MINUTES - ASHFORD PLANNING AND ZONING COMMISSION

Annual Meeting - November 12, 1996

Members present: Organ, Lawrence, Nagy, Levaur, Rossman, McCarthy & White.

Alternates present: Bartok & Specyalski.

The meeting was called to order at 9:55 p.m. after the public hearing (Sprint Spectrum, tower & Moratorium, Lake Chaffee).

Specyalski is the voting alternate for this meeting.

At the Annual Town meeting, Alex Hastillo and Kevin McCarthy were elected to 4 year terms on the Commission ending in the year 2000 and Bartok was elected to a 3 year term as Alternate ending in 1999.

Moved and seconded to consider Old and New Business first. Passed without dissent.

The Commission considered the Sprint Spectrum application for a communications tower to be located on Sky Hill. There were no objections at tonight's public hearing. The tower will be able to hold three sets of antennas. Sprint Spectrum will operate a PCS digital system. It is regulated by the FCC. There will be no lights on the tower. Access will be off Frontage Road to Janowski Road to avoid the wetlands on the east end of Janowski Road. Moved and seconded to approve with conditions the application for a Special Exception under Section 5.2.3 by Sprint Spectrum L.P., Meriden, CT for a 200' communications tower to be located on land leased from David H. Martin off Janowski Road on Sky Hill.

The conditions are:

1. Utilities to the site which is approximately 2500' from Janowski Road will be located underground in the right of way.
2. Space and installation of fire, emergency and municipal communications equipment to meet present and future needs will be provided at no cost.
3. A copy of the liability insurance will be submitted to the Commission.
4. A site plan including driveway design and sedimentation and erosion control measures will be submitted to the Commission before the construction begins.
5. A copy of the lease will be part of the land records.

Motion passed without dissent.

The Commission considered the proposed Moratorium at Lake Chaffee. Tim Backus, Chairman of the Water Pollution Control Authority was the only person to speak at the public hearing. Moved and seconded to approve the following:

Moratorium at Lake Chaffee

WHEREAS, the Department of Environmental Protection has cited the Town of Ashford and the Lake Chaffee Improvement Association, Inc. to study and report upon potential pollution at Lake Chaffee resulting from construction around the lake; and

WHEREAS, the Department of Environmental Protection has found pollution in the tributaries leading to the lake, and

WHEREAS, there is a reasonable expectation that the recommendation of the study may be to limit new construction in that area, or as an alternative to require that homes in the area be connected to an alternative type of sewage disposal system, and

WHEREAS, this Commission does not want to allow any deterioration of the water in the lake or tributaries;

The Planning and Zoning Commission of the Town of Ashford, pursuant to the authority vested in it by Section 8-2 of Connecticut General Statutes, hereby amends the zoning regulations of the Town of Ashford by adoption of the following Moratorium:

"Until December 31, 1997, there shall be no new house construction allowed within the area of Lake Chaffee Improvement Association, Inc. nor any enclosed addition to any existing house in that area. The Zoning Enforcement Officer may not in that period certify that any new construction is in conformity with the zoning regulations of the town."

Motion passed without dissent.

The reasons for reinstating the moratorium include:

1. There is need for more testing of the water and septic systems in the area.
2. There have been minimal applications for construction since the last moratorium was lifted.
3. The WPCA is seeking on-site solutions.
4. There are several sets of vacant lots that may be valuable for sewage disposal systems.

Specyalski stepped down for the next item of business.

Brialee Campground - Brian Specyalski submitted a plan for a six additional campsites at the campground. It was noted that three of these butt onto adjoining property that is owned by the State of Connecticut. The others have a 100' setback that has been the minimum acceptable to the Commission. Moved and seconded to receive the plan and hold a public hearing on December 9th. Passed without dissent. A new map showing only the three sites that meet the setback requirements will be submitted. The Commission will walk the site at 7 a.m. on Saturday November 16th.

The Commission returned to the top of the agenda.

Moved and seconded to approve the minutes of the October 15th meeting. Passed without dissent.

Moved and seconded to send a letter of appreciation to George Quirk Sr., retiring member for his many years of service to the Commission. Passed without dissent.

There were no bills.

A copy of the revised Small Cities Housing Plan was received from the Office of the Selectmen. It will go to a public hearing in December. Copies will be distributed to the Commission members for review.

The revised fee schedule was approved by Town Meeting in October.

Moved and seconded to add to the agenda the election of officers and reappointment of employees. Passed without dissent.

Moved and seconded to reelect the following officers to serve until the next annual meeting of the Commission: Sidney E. Organ, Chairman, Alex Hastillo, Vice Chairman and John Bartok, Secretary. Passed without dissent. The Secretary will cast one ballot for each.

Moved and seconded to reappoint Rudolph Makray, Zoning Enforcement Officer and John Bartok, Recording Secretary for one year or until the next annual meeting. Passed without dissent.

The Commission agreed to hold a Special Meeting on Monday, December 16th at 7 p.m. to review the draft of the revised Plan of Development.

The meeting adjourned at 10:55 p.m.

Respectfully submitted.



John W. Bartok, Jr.  
Recording Secretary

## LEGAL NOTICE

Town of Ashford

The Ashford Planning and Zoning Commission at its meeting on November 12, 1996 took the following actions:

APPROVED with conditions the application of Sprint Spectrum, L.P., Meriden, CT for a 200' communications tower to be built on the David Matin property located off Route 89 on Sky Hill.

APPROVED a request by the Ashford Water Pollution Control Authority to reenstate the moratorium at Lake Chaffee until December 31, 1997 that prohibits construction of new houses or enclosed additions to any existing house.

Dated in Ashford, Connecticut this 14th day of November, 1996.

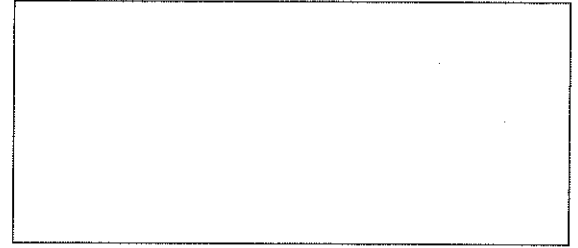
John W. Bartok, Jr., Sec.  
Ashford Planning and  
Zoning Commission


:



**33 Janoski Rd, Ashford, CT 06278**

Location: 41.952633, -72.192901



  
**33 Janoski Rd,  
Ashford, CT 06278**

Ferrence Rd

Mount Hope River

Ferrence Rd



250 feet 50 m

© 2018 HERE, © OpenStreetMap

# 33 JANOSKI RD

**Location** 33 JANOSKI RD

**Mblu** 02/ F/ 1.1/ /

**Acct#** 00007410

**Owner** MARTIN DAVID H

**Taxable Status**

**Assessment** \$252,200

**Appraisal** \$360,200

**PID** 65

**Building Count** 1

**Legal Description**

**Lot Type**

topoTopo

**Location**

## Current Value

Appraisal					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2018	\$0	\$0	\$183,100	\$177,100	\$360,200

Assessment					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2018	\$0	\$0	\$128,200	\$124,000	\$252,200

## Parcel Addresses

Additional Addresses		
Address	City, State Zip	Type
33 JANOSKI RD		Primary

## Owner of Record

<b>Owner</b>	MARTIN DAVID H	<b>Sale Price</b>	\$0
<b>Co-Owner</b>	C/O SPRINT SPECTRUM CT-03XC204	<b>Certificate</b>	C
<b>Care Of</b>		<b>Book &amp; Page</b>	109/ 811
<b>Address</b>	PO BOX 8430 KANSAS CITY, MO 641148430	<b>Sale Date</b>	09/30/1996
		<b>Qualified</b>	U

## Ownership History

Ownership History
No Data for Ownership History



## Building Information

### Building 1 : Section 1

**Year Built:**

**Living Area:** 0

**Replacement Cost:** \$0

**Building Percent**

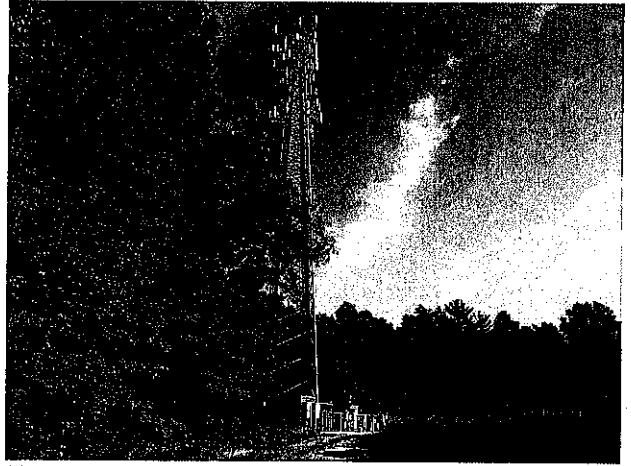
**Good:**

**Replacement Cost**

**Less Depreciation:** \$0


Building Attributes	
Field	Description
Style	Outbuildings
Model	
Grade:	
Stories:	
Occupancy	
Exterior Wall 1	
Exterior Wall 2	
Roof Structure:	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Total Bthrms:	
Total Half Baths:	
Total Xtra Fixtrs:	
Total Rooms:	
Bath Style:	
Kitchen Style:	
Bsmt. Garages	

### Building Photo



(<http://images.vgsi.com/photos/AshfordCTPhotos//\00\00\25\30>)

### Building Layout

 Building Layout

([http://images.vgsi.com/photos/AshfordCTPhotos//Sketches/65\\_](http://images.vgsi.com/photos/AshfordCTPhotos//Sketches/65_))

Building Sub-Areas (sq ft)	Legend
No Data for Building Sub-Areas	

### Extra Features

Extra Features	Legend
No Data for Extra Features	

## Parcel Information

**Use Code** 201  
**Description** Commercial Vacant  
**Deeded Acres** 0.7

## Land

### Land Use

**Use Code** 201  
**Description** Commercial Vacant  
**Zone**  
**Neighborhood** C3  
**Alt Land Appr** No  
**Category**

### Land Line Valuation

**Size (Acres)** 0.7  
**Frontage**  
**Depth**  
**Assessed Value** \$124,000  
**Appraised Value** \$177,100

## Outbuildings

Outbuildings								Legend
Code	Description	Sub Code	Sub Description	Size	Value	Assessed Value	Bldg #	Comment
TWR1	Cell Tower			192 HEIGHT	\$73,400	\$51,400	1	
SHD2	Pre Cast Cell			240 S.F.	\$34,400	\$24,100	1	
FN3	Fence 6'			260 L.F.	\$3,600	\$2,500	1	
SHD2	Pre Cast Cell			360 S.F.	\$34,400	\$24,100	1	
SHD2	Pre Cast Cell			260 S.F.	\$37,300	\$26,100	1	

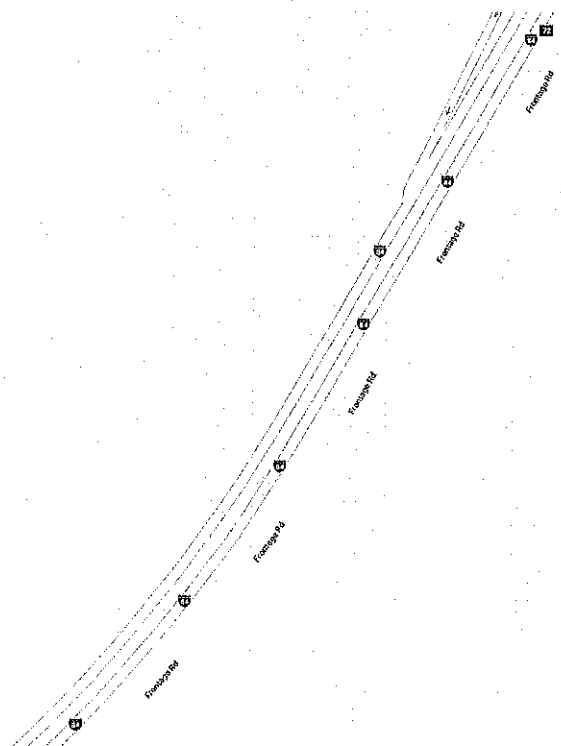
## Valuation History

Appraisal					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2017	\$0	\$0	\$183,100	\$177,100	\$360,200
2016	\$0	\$0	\$183,100	\$177,100	\$360,200
2015	\$0	\$0	\$182,200	\$189,000	\$371,200

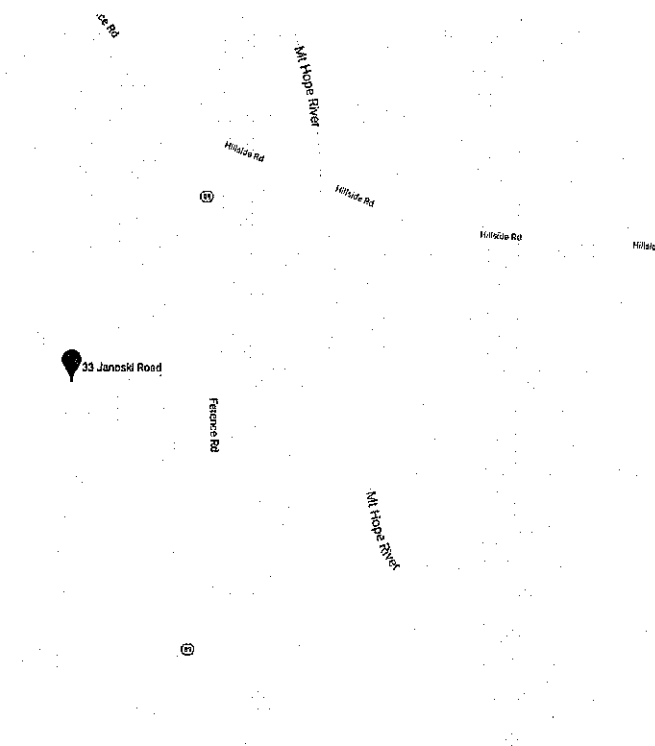
Assessment					
Valuation Year	Building	Extra Features	Outbuildings	Land	Total
2017	\$0	\$0	\$128,200	\$124,000	\$252,200
2016	\$0	\$0	\$128,200	\$124,000	\$252,200
2015	\$0	\$0	\$127,600	\$132,300	\$259,900

(c) 2016 Vision Government Solutions, Inc. All rights reserved.

# Google Maps 33 Janoski Rd



Google



Map data ©2018 Google 200 ft



**33 Janoski Rd**  
Ashford, CT 06278

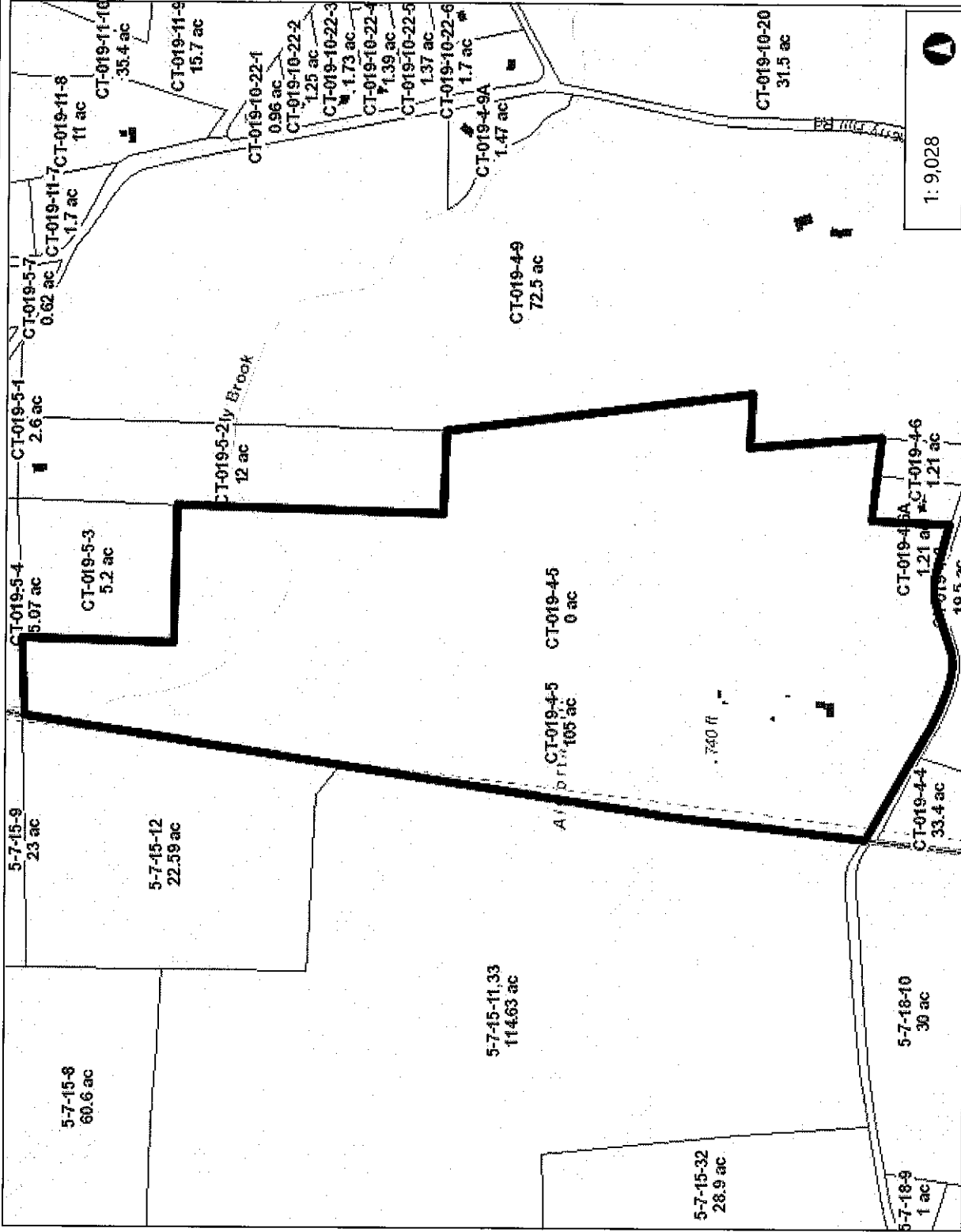


# Necog GIS Site



### Legend

- Town
- Buildings 2012
- Parcels



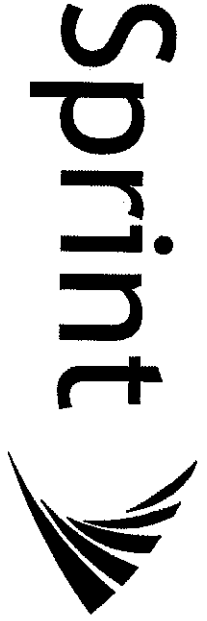
1: 9,028



This map is a user generated static output from an Internet mapping site and is for reference only. Data layers that appear on this map may or may not be accurate, current, or otherwise reliable.

**THIS MAP IS NOT TO BE USED FOR NAVIGATION**

Notes  
Enter Map Description



**PROJECT:** DO MACRO UPGRADE  
**SITE NAME:** SKY HILL  
**SITE CASCADE:** CT03XC204  
**SITE NUMBER:** 876345  
**SITE ADDRESS:** 33 JANOWSKI ROAD  
 ASHFORD, CT 06278  
**SITE TYPE:** SELF SUPPORT TOWER  
**MARKET:** NEW ENGLAND/UPSTATE NY

**SITE INFORMATION**

**TOWER OWNER:**  
 GROWN CASTLE COMPANY LLC  
 5000 RAYMOND AVENUE  
 CHANDLER, AZ 85024  
 (704) 485-8555

**LATITUDE (NAD83):**  
 41° 07' 27" N  
 41.124175

**LONGITUDE (NAD83):**  
 -72° 11' 24.3" W  
 -72.189925

**COUNTY:**  
 WINDHAM

**ZONING JURISDICTION:**  
 TOWN OF ASHFORD

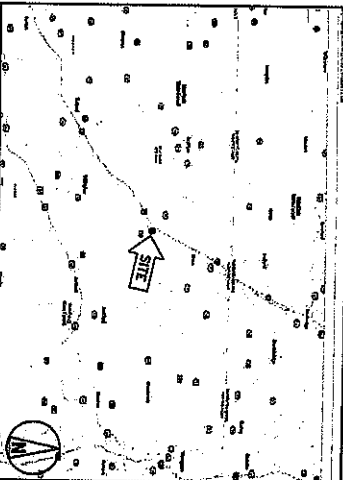
**ZONING DISTRICT:**  
 C3

**POWER COMPANY:**  
 BORGESSE  
 800-388-3000

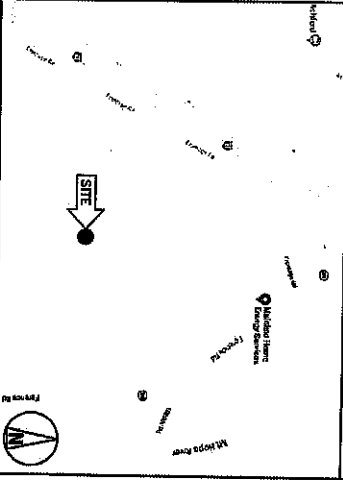
**SPRINT CONSTRUCTION:**  
 TBD

**GROWN E.M.**  
 SCOTT MANORON  
 (201) 226-9228

**AREA MAP**



**LOCATION MAP**



**PROJECT DESCRIPTION**

- PROFIT PROGRESS TO ADDRY AN EXISTING UNMANNED TELECOMMUNICATIONS FACILITY.
- INSTALL 2A EQUIPMENT RISER EXISTING HW. HANG CHASSET
- REMOVE (6) PANEL ANTENNAS
- INSTALL (6) PANEL ANTENNAS (3 800/1900, 3 2500)
- INSTALL (12) BRVS TO TOWER (4 800, 3 1900, 3 2500)
- INSTALL (3) HBRD CABLES
- REMOVE (6) COV CABLES

THESE PLANS HAVE BEEN REVIEWED FOR THE ADEQUACY OF AN EXISTING UNMANNED TELECOMMUNICATIONS FACILITY. THE REVIEW WAS LIMITED TO THE SCOPE OF WORK PROVIDED BY SPRINT. THE REVIEWER HAS NOT CONDUCTED THE SCOPE OF WORK IN THE PLANS. THESE PLANS ARE NOT A SUBSTITUTE FOR A PROFESSIONAL STRUCTURAL ANALYSIS PERFORMED BY A LICENSED PROFESSIONAL STRUCTURAL ENGINEER. ANALYSIS MUST INCLUDE BOTH STRUCTURE AND WINDS.

**APPLICABLE CODES**

- INTERNATIONAL BUILDING CODE (2015 IBC)
- INTERNATIONAL WIND LOADS AND EFFECTS CODE (2010 IBC)
- 2011 NATIONAL ELECTRICAL CODE OR LATEST EDITION
- 2011 NATIONAL ELECTRICAL CODE OR LATEST EDITION
- ANY OTHER NATIONAL OR LOCAL APPLICABLE CODES, LOCAL BUILDING CODES
- CITY/COUNTY ORDINANCES

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE SCOPE OF WORK PROVIDED BY SPRINT. THE REVIEWER HAS NOT CONDUCTED THE SCOPE OF WORK IN THE PLANS. THESE PLANS ARE NOT A SUBSTITUTE FOR A PROFESSIONAL STRUCTURAL ANALYSIS PERFORMED BY A LICENSED PROFESSIONAL STRUCTURAL ENGINEER. ANALYSIS MUST INCLUDE BOTH STRUCTURE AND WINDS.

**DRAWING INDEX**

SHEET NO.	TITLE SHEET & PROJECT DATA	REV.
T-1	TITLE SHEET & PROJECT DATA	1
SP-1	SPRINT SPECIFICATIONS	1
SP-2	SPRINT SPECIFICATIONS	1
SP-3	SPRINT SPECIFICATIONS	1
A-1	SITE PLAN	1
A-2	TOWER ELEVATION & CABLE PLAN	1
A-3	EQUIPMENT & MOUNTING DETAILS	1
A-4	ONL. DETAILS	1
A-5	PLUMBING DETAILS	1
A-6	ELECTRICAL & GROUNTING DETAILS	1
E-2	ELECTRICAL & GROUNTING DETAILS	1



PLANS PROVIDED FOR:  
**Sprint**  
 6301 Sprint Parkway  
 Overland Park, Kansas 66251

FROM ZERO TO INFINIGY  
**INFINIGY**  
 The solutions are endless  
 1033 Generational Square W. | 1st Floor | 150-000-0000  
 Phone: 516-452-9719 | Fax: 516-452-9729  
 40 RIVERSIDE ST. 11TH FL. NEW YORK, NY 10013

**CROWN CASTLE**

STATE OF CONNECTICUT  
 PROFESSIONAL ENGINEER  
 No. 24705  
 MICHAEL J. SHERBET  
 REGISTERED PROFESSIONAL ENGINEER

**DRAWING NOTE:**  
 THESE DOCUMENTS ARE CONFIDENTIAL AND ARE THE SOLE PROPERTY OF SPRINT AND SHALL NOT BE REPRODUCED, COPIED, OR DISTRIBUTED WITHOUT THE WRITTEN CONSENT OF SPRINT.

REVISION	DESCRIPTION	DATE	BY	REV.
1	ISSUE FOR CONSTRUCTION	02/20/15	MSJ	1
2	ISSUE FOR CONSTRUCTION	02/20/15	MSJ	2
3	ISSUE FOR REVIEW	02/20/15	MSJ	3

**SITE NAME:** SKY HILL  
**SITE CASCADE:** CT03XC204  
**SITE ADDRESS:** 33 JANOWSKI ROAD  
 ASHFORD, CT 06278

**SHEET DESCRIPTION:** TITLE SHEET & PROJECT DATA  
**SHEET NUMBER:** T-1

THESE OUTLINE SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS, INCLUDING CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.

SECTION 01 100 - SCOPE OF WORK

PART 1 - GENERAL

- 1.1 THE WORK, THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES ARE INCLUDED WITH THE SPRINT CONSTRUCTION SPECIFICATIONS DESCRIBING THE WORK TO BE PERFORMED BY THE CONTRACTOR.
- 1.2 RELATED DOCUMENTS:
  - A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SECTION.
  - B. SPRINT STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWIT.
- 1.3 PRECEDENCE: SHOULD CONFLICTS OCCUR BETWEEN THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES INCLUDING THE SPRINT CONSTRUCTION SPECIFICATIONS AND THE SPRINT CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES, THE SPRINT CONSTRUCTION SPECIFICATIONS SHALL TAKE PRECEDENCE. IN THE EVENT OF CONFLICT, THE SPRINT CONSTRUCTION SPECIFICATIONS SHALL TAKE PRECEDENCE.
- 1.4 MANUALLY REWORKED CODES AND STANDARDS:
  - A. THE WORK SHALL CONFORM WITH APPROVED NATIONAL AND LOCAL CODES AND STANDARDS, LATEST EDITION, AND PORTIONS THEREOF, INCLUDED BUT NOT LIMITED TO THE FOLLOWING:
    1. 68-31-CODE BOOK REQUIREMENTS: PHYSICAL PROTECTION
    2. 68-31-CODE BOOK REQUIREMENTS FOR THE PHYSICAL DESIGN AND MAINTENANCE OF TELECOMMUNICATIONS EQUIPMENT.
    3. SPRINT-CODE BOOK REQUIREMENTS FOR THE PHYSICAL DESIGN AND MAINTENANCE OF TELECOMMUNICATIONS EQUIPMENT.
    4. NATIONAL FIRE PROTECTION ASSOCIATION CODES AND STANDARDS (NFPA) (LIVE SPRINT CODES).
    5. AMERICAN SOCIETY FOR TESTING OF MATERIALS (ASTM)
    6. INSTITUTE OF ELECTRIC AND ELECTRICAL ENGINEERS (IEEE)
    7. AMERICAN CONCRETE INSTITUTE (ACI)
    8. AMERICAN WIRE PRODUCTS ASSOCIATION (AWPA)
    9. CONCRETE REINFORCING STEEL INSTITUTE (CRSI)
    10. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO)
    11. PORTLAND CEMENT ASSOCIATION (PCA)
    12. NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA)
    13. BRICK INDUSTRY ASSOCIATION (BIA)
    14. AMERICAN WELDING SOCIETY (AWS)
    15. NATIONAL ARCHITECTURAL COMMUNICATIONS ASSOCIATION (NACA)
    16. SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION (SMACNA)
    17. DOOR AND HARDWARE INSTITUTE (DHI)
    18. OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA)
    19. APPLICABLE BUILDING CODES INCLUDING NATIONAL BUILDING CODE, SOUTHERN BUILDING CODE, BOCA, AND THE INTERNATIONAL BUILDING CODE.

- 1.5 DEFINITIONS:
  - A. WORK, THE TITLE OF TITLES AND RESPONSIBILITIES IDENTIFIED IN THE CONTRACT DOCUMENTS.
  - B. CONTRACTOR: SPRINT CORPORATION
  - C. ENGINEER: SYNOPSIS WITH ARCHITECT & ENGINEERS AND PART, THE DESIGN PROFESSIONAL HAVING PROFESSIONAL RESPONSIBILITY FOR DESIGN OF THE PROJECT.
  - D. CONTRACTOR: CONSTRUCTION CONTRACTOR, CONSTRUCTION VENDOR, INDIVIDUAL, OR FIRM.
  - E. THIRD PARTY: PERSON OR ENTITY A PERSON OR ENTITY ENGAGED SEPARATELY BY THE CONTRACTOR OR ENGINEER FOR THE PROJECT, INCLUDING BUT NOT LIMITED TO ARCHITECT, SPECIALISTS RELATED TO BUT NOT INCLUDED IN THE WORK.
  - F. OTHER OWNER PROVIDED: CONTRACTOR INSTALLED EQUIPMENT.
- 1.6 CONSTRUCTION MANAGER - ALL PROJECTS RELATED COMPLIANCE TO FOLLOW THROUGH SPRINT REPRESENTATIVE IN CHARGE OF PROJECT.

- 1.7 POINT OF CONTACT: COMMUNICATION BETWEEN SPRINT AND THE CONTRACTOR SHALL FOLLOW THROUGH THE SINGLE SPRINT CONSTRUCTION MANAGER APPOINTED TO MANAGE THE PROJECT FOR SPRINT.
- 1.8 ON-SITE SUPERVISION: THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK THROUGHOUT THE CONSTRUCTION PERIOD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTION METHOD, SELECTION, TECHNIQUES, MATERIALS, AND QUALITY CONTROL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.9 CONSTRUCTION SPECIFICATIONS AND DETAILS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.10 USE OF JOB SITE: THE CONTRACTOR SHALL CONFORM ALL CONSTRUCTION AND RELATED OPERATIONS INCLUDING STAGING AND STORAGE OF MATERIALS AND EQUIPMENT TO THE CONSTRUCTION SITE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.11 UTILITIES: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.12 RESULTS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.13 CONSTRUCTION SHALL TAKE ALL NECESSARY AND PROTECT ALL MATERIALS, EQUIPMENT AND PROPERTY.
- 1.14 METHOD OF PROTECTION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.

- 1.15 USE OF ELECTRONIC PROJECT MANAGER SYSTEMS:
    - A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- PART 2 - PRODUCTS (NOT USED)
- 2.1 MATERIALS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
  - 2.2 ACCESS TO WORK: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
  - 2.3 TESTING: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
  - 2.4 HANDLING: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.

SECTION 01 200 - COMPLANT FURNISHED MATERIAL AND EQUIPMENT

PART 1 - GENERAL

- 1.1 THE WORK, THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONJUNCTION WITH THE SPRINT STANDARD CONSTRUCTION SPECIFICATIONS DESCRIBING THE WORK TO BE PERFORMED BY THE CONTRACTOR.
- 1.2 RELATED DOCUMENTS:
  - A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SECTION.
  - B. SPRINT STANDARD CONSTRUCTION DETAILS FOR WIRELESS SITES ARE INCLUDED IN AND MADE A PART OF THESE SPECIFICATIONS HEREWIT.
- 1.3 PRECEDENCE: SHOULD CONFLICTS OCCUR BETWEEN THE STANDARD CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES INCLUDING THE SPRINT CONSTRUCTION SPECIFICATIONS AND THE SPRINT CONSTRUCTION SPECIFICATIONS FOR WIRELESS SITES, THE SPRINT CONSTRUCTION SPECIFICATIONS SHALL TAKE PRECEDENCE. IN THE EVENT OF CONFLICT, THE SPRINT CONSTRUCTION SPECIFICATIONS SHALL TAKE PRECEDENCE.
- 1.4 MANUALLY REWORKED CODES AND STANDARDS:
  - A. THE WORK SHALL CONFORM WITH APPROVED NATIONAL AND LOCAL CODES AND STANDARDS, LATEST EDITION, AND PORTIONS THEREOF, INCLUDED BUT NOT LIMITED TO THE FOLLOWING:
    1. 68-31-CODE BOOK REQUIREMENTS: PHYSICAL PROTECTION
    2. 68-31-CODE BOOK REQUIREMENTS FOR THE PHYSICAL DESIGN AND MAINTENANCE OF TELECOMMUNICATIONS EQUIPMENT.
    3. SPRINT-CODE BOOK REQUIREMENTS FOR THE PHYSICAL DESIGN AND MAINTENANCE OF TELECOMMUNICATIONS EQUIPMENT.
    4. NATIONAL FIRE PROTECTION ASSOCIATION CODES AND STANDARDS (NFPA) (LIVE SPRINT CODES).
    5. AMERICAN SOCIETY FOR TESTING OF MATERIALS (ASTM)
    6. INSTITUTE OF ELECTRIC AND ELECTRICAL ENGINEERS (IEEE)
    7. AMERICAN CONCRETE INSTITUTE (ACI)
    8. AMERICAN WIRE PRODUCTS ASSOCIATION (AWPA)
    9. CONCRETE REINFORCING STEEL INSTITUTE (CRSI)
    10. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO)
    11. PORTLAND CEMENT ASSOCIATION (PCA)
    12. NATIONAL CONCRETE MASONRY ASSOCIATION (NCMA)
    13. BRICK INDUSTRY ASSOCIATION (BIA)
    14. AMERICAN WELDING SOCIETY (AWS)
    15. NATIONAL ARCHITECTURAL COMMUNICATIONS ASSOCIATION (NACA)
    16. SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION (SMACNA)
    17. DOOR AND HARDWARE INSTITUTE (DHI)
    18. OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA)
    19. APPLICABLE BUILDING CODES INCLUDING NATIONAL BUILDING CODE, SOUTHERN BUILDING CODE, BOCA, AND THE INTERNATIONAL BUILDING CODE.

- 1.5 DEFINITIONS:
  - A. WORK, THE TITLE OF TITLES AND RESPONSIBILITIES IDENTIFIED IN THE CONTRACT DOCUMENTS.
  - B. CONTRACTOR: SPRINT CORPORATION
  - C. ENGINEER: SYNOPSIS WITH ARCHITECT & ENGINEERS AND PART, THE DESIGN PROFESSIONAL HAVING PROFESSIONAL RESPONSIBILITY FOR DESIGN OF THE PROJECT.
  - D. CONTRACTOR: CONSTRUCTION CONTRACTOR, CONSTRUCTION VENDOR, INDIVIDUAL, OR FIRM.
  - E. THIRD PARTY: PERSON OR ENTITY A PERSON OR ENTITY ENGAGED SEPARATELY BY THE CONTRACTOR OR ENGINEER FOR THE PROJECT, INCLUDING BUT NOT LIMITED TO ARCHITECT, SPECIALISTS RELATED TO BUT NOT INCLUDED IN THE WORK.
  - F. OTHER OWNER PROVIDED: CONTRACTOR INSTALLED EQUIPMENT.
- 1.6 CONSTRUCTION MANAGER - ALL PROJECTS RELATED COMPLIANCE TO FOLLOW THROUGH SPRINT REPRESENTATIVE IN CHARGE OF PROJECT.

- 1.7 POINT OF CONTACT: COMMUNICATION BETWEEN SPRINT AND THE CONTRACTOR SHALL FOLLOW THROUGH THE SINGLE SPRINT CONSTRUCTION MANAGER APPOINTED TO MANAGE THE PROJECT FOR SPRINT.
- 1.8 ON-SITE SUPERVISION: THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK THROUGHOUT THE CONSTRUCTION PERIOD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTION METHOD, SELECTION, TECHNIQUES, MATERIALS, AND QUALITY CONTROL. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.9 CONSTRUCTION SPECIFICATIONS AND DETAILS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.10 USE OF JOB SITE: THE CONTRACTOR SHALL CONFORM ALL CONSTRUCTION AND RELATED OPERATIONS INCLUDING STAGING AND STORAGE OF MATERIALS AND EQUIPMENT TO THE CONSTRUCTION SITE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.11 UTILITIES: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.12 RESULTS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- 1.13 CONSTRUCTION SHALL TAKE ALL NECESSARY AND PROTECT ALL MATERIALS, EQUIPMENT AND PROPERTY.
- 1.14 METHOD OF PROTECTION: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.

- 1.15 USE OF ELECTRONIC PROJECT MANAGER SYSTEMS:
    - A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
- PART 2 - PRODUCTS (NOT USED)
- 2.1 MATERIALS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
  - 2.2 ACCESS TO WORK: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
  - 2.3 TESTING: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.
  - 2.4 HANDLING: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION OF THE WORK.



FROM ZERO TO INFINIGY  
 THE SOLUTIONS YOU NEED  
 TO GET THE MOST OUT OF YOUR NETWORK



THESE DOCUMENTS ARE CONFIDENTIAL AND ARE THE SOLE PROPERTY OF SPRINT AND MAY NOT BE REPRODUCED, DISSEMINATED OR DISTRIBUTED WITHOUT THE WRITTEN AUTHORITY OF SPRINT.

DESCRIPTION	DATE	BY	HOW
ISSUE LIST CORRECTION	07/20/11	MS	2
ISSUE LIST CORRECTION	07/20/11	MS	1
ISSUE LIST CORRECTION	07/17/11	MS	1
ISSUE LIST CORRECTION	07/17/11	MS	1

SKY HILL  
 CT09XC204

33 LANOWSKI ROAD  
 ASHFORD, CT 06278

SPRINT SPECIFICATIONS  
 SP-1

**CONTINUE FROM SP-1**

1. PERSONNEL REQUIRED FOR ENVIRONMENTAL MONITORING.
2. PREPARE GROUND TESTS, PROVIDE DE-CORROSION AND PROTECT AND PAINT GROUNDING, AND CONDUCT SURFACE TREATMENTS.
3. UNLOAD AND CONDUIT ALL ACTIVITIES FOR INSTALLATION OF UTILITIES INCLUDING ELECTRICAL AND TOWER BUCKING.
4. INSTALL UNDERGROUND FACILITIES INCLUDING UNDERGROUND POWER AND COMMUNICATIONS CONDUITS, AND UNDERGROUND GROUNDING SYSTEMS.
5. INSTALL ABOVE GROUND GROUNDING SYSTEMS.
6. PROVIDE NEW HANG INSTALLATIONS AND MODIFICATIONS.
7. ANTENNA, TOWER, CHIMNEYS AND SHELTERS AS INDICATED.
8. INSTALL TOWER, ACCESS WAYS, CHIMNEYS AND SHELTERS AS INDICATED.
9. ACCOMPLISH REQUIRED MODIFICATION OF EXISTING FACILITIES.
10. PROVIDE ANTENNA SUPPORT STRUCTURE FOUNDATIONS.
11. PROVIDE SCAFFS AND EQUIPMENT PLATFORMS.
12. INSTALL CONDUIT FEMING, SHIRT SHEETING, LANDSCAPING AND ACCESS BARRIERS.
13. PERSONNEL INSPECTION AND MATERIAL TESTING AS REQUIRED HEREINAFTER.
14. CONDUCT SITE RESISTANCE TO EARTH TESTING AS REQUIRED HEREINAFTER.
15. INSTALL FIELD DEVELOPER SETS AND OTHER SURVEY POWER SOLUTIONS.
16. INSTALL TOWERS, ANTENNA SUPPORT STRUCTURES AND PLATFORMS ON EXISTING TOWERS AS REQUIRED.
17. INSTALL CELL SITE ROOFS, APPROXIMATE ONE (1) CONCRETE ANTENNA CROSS BARS AND COLUMNS, TOWER TOP AIRLIFTERS, LOW NOISE MUFFLERS AND RELATED EQUIPMENT.
18. PERSONNEL, DOCUMENT, AND CLOSE OUT ANY CONSTRUCTION CONTROL DOCUMENTS THAT MAY BE REQUIRED BY GOVERNMENT AGENCIES AND LOCALITIES.
19. PERSONNEL ANTENNA AND CABLE SHEET TESTING AND WAVE ANALYSIS AND NECESSARY CORRECTIONS.
20. REMAIN ON SITE UNTIL THE TOWER AND ANTENNA ARE COMPLETED AND ALL NECESSARY CORRECTIONS ARE MADE.

**SECTION 01 400 - SUBMITTALS & TESTS**

- PART 1 - GENERAL**
- 1.1 THE WORK THESE STANDARD CONSTRUCTION SPECIFICATIONS IN CONNECTION WITH THE OTHER CONTRACT DOCUMENTS AND THE CONSTRUCTION DRAWINGS DESCRIBE THE WORK TO BE PERFORMED BY THE CONTRACTOR.
  - 1.2 RELATED DOCUMENTS:
    - A. THE REQUIREMENTS OF THIS SECTION APPLY TO ALL SECTIONS IN THIS SPECIFICATION.
    - B. SPECIFIC CONSTRUCTION DETAILS FOR MATERIALS SHALL BE INCLUDED IN THE CONSTRUCTION DRAWINGS.
  - 1.3 SUBSTITUTIONS:
    - A. THE WORK IN ALL SECTIONS SHALL COMPLY WITH THE CONSTRUCTION DRAWINGS AND THESE SPECIFICATIONS.
    - B. SUBMIT THE FOLLOWING TO COMPANY REPRESENTATIVE FOR APPROVAL:
      1. CONCRETE MIXTURES FOR TOWER FOUNDATIONS, ANCHORS, PILES, AND CONCRETE RAFTERS.
      2. CONCRETE BRICK TESTS AS SPECIFIED HEREIN.
      3. SPECIAL FINISHES FOR MASONRY SURFACES, IF ANY.
      4. ALL EQUIPMENT AND MATERIALS SO IDENTIFIED ON THE CONSTRUCTION DRAWINGS.
      5. CHECKOUT DRAWING DESIGN.
    - C. ATTESTATION OF THE COMPANY'S REQUEST ANY ALTERATIONS TO THE MATERIALS MANUFACTURER'S SPECIFICATIONS SHALL BE SUBMITTED TO SPECIFIC CONSTRUCTION DIVISIONS FOR APPROVAL. APPROVAL SHALL BE GRANTED ONLY IF THESE MATERIALS ARE IN ACCORDANCE WITH THE CONSTRUCTION DRAWINGS AND THE CONSTRUCTION SPECIFICATIONS. APPROVAL SHALL INCLUDE A STATEMENT OF COST REDUCTION PROVIDED FOR USE OF ALTERNATE PRODUCT.
  - 1.4 TESTS AND INSPECTIONS:
    - A. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION TESTS, INSPECTIONS AND PROJECT DOCUMENTATION.
    - B. CONSTRUCTION SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
      1. CORE SAMPLES AND BEAR TESTS PER CLIENT VERSION OF SPANTS TEST-ON-ROOF-TO-ROOF ANTENNA LINE ACCEPTANCE STANDARDS.
      2. ALL ZEMMEL AND REMAY USING GEOTECHNICAL CONTRACTOR.
      3. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL CORRECTIONS TO ANY WORK AREA OF TESTING.
    - C. REQUIRED CHECKOUT DOCUMENTATION INCLUDES, BUT IS NOT LIMITED TO THE FOLLOWING:
      1. ANTENNA REPORT, AND - AT LEAST REPORT FROM ANTENNA ALIGNMENT TOOL TO VERIFY THE ANTENNA IS IN THE CORRECT POSITION AND THE ANTENNA IS IN CONTACT WITH THE BR DATA SHEETS, SWEET AND BRICK TESTS.
      2. SCHEDULED EQUIPMENT.
      3. ALL AVAILABLE DIMENSIONAL INFORMATION.
      4. PHOTO COPY OF BIDDING PROVIDED IN FIELD.


**PART 2 - PRODUCTS (NOT USED)**

- PART 3 - EXECUTION**
- 3.1 REQUIREMENTS FOR TESTING:
    - A. THIRD PARTY TESTING AGENCY:
      1. WHEN THE USE OF A THIRD PARTY INDEPENDENT TESTING AGENCY IS REQUIRED, THE AGENCY SHALL BE SELECTION BY THE CONTRACTOR FOR A REPUTABLE BASIS IN THE STATE WHERE THE PROJECT IS LOCATED AND HAVE A PROVEN RECORD UNDERSTANDING OF LOCAL AVAILABLE MATERIALS, INCLUDING THE SOIL, ROCK, AND ENVIRONMENTAL CONDITIONS.
      2. THE THIRD PARTY TESTING AGENCY IS TO BE FAMILIAR WITH THE APPLICABLE SPECIFICATIONS IN SOILS, CONCRETE, MASONRY, ASBESTOS, AND ASPHALT TESTING USING ASTM, AASHTO, AND OTHER METHODS IS FAMILIAR.
      3. EXPERIENCE IN SOILS, CONCRETE, MASONRY, ASBESTOS, AND ASPHALT TESTING USING ASTM, AASHTO, AND OTHER METHODS IS FAMILIAR.
      4. TESTING USING SOIL, CONCRETE, MASONRY, ASBESTOS, AND ASPHALT TESTING USING ASTM, AASHTO, AND OTHER METHODS IS FAMILIAR.
  - 3.2 REQUIRED TESTS:
    - A. CONSTRUCTION SHALL ACCOMPLISH TESTING INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
      1. CONCRETE COMPRESSIVE STRENGTH TESTS FOR THE TOWER AND ANCHOR.
      2. ASPHALT ROADWAY COMPACTION TESTS FOR THE TOWER FOUNDATION.
      3. FIELD QUALITY CONTROL TESTING AS SPECIFIED IN SECTION 01 40 00 ASPHALT CONCRETE FINISH.
      4. TESTING REQUIRED UNDER SECTION: ASBESTOS BASE FOR ACCESS ROADS, PILES AND PAVEMENT DOCUMENTS.
      5. STRUCTURAL BACKFILL COMPACTION TESTS FOR THE TOWER FOUNDATION, SITE RESISTANCE TO EARTH TESTING PER CLIENT, CELL SITE FOUNDING SYSTEM DESIGN.
      6. ANTENNA AND CABLE SHEET TESTS PER CLIENT: ANTENNA TRANSMISSION LINE ACCEPTANCE STANDARDS.
      7. OCCUPANCY AT ANTENNA WANTS FOR OPS AND AIRBORNE.
      8. ALL OTHER TESTS REQUIRED BY COMPANY OR JURISDICTION.
  - 3.3 REQUIRED INSPECTIONS:
    - A. SCHEDULED INSPECTIONS WITH COMPANY REPRESENTATIVE.
    - B. CONDUCT INSPECTIONS INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
      1. GROUNDING SYSTEM INSTALLATION FROM TO EARTH CONNECTION OF SCAFFS AND TOWER FOUNDATION.
      2. FORMING FOR CONCRETE AND BEARER PLACEMENT FROM TO FOUR OR SPREAD REINFORCEMENT.
      3. COMPARISON OF GROUND MATERIALS, ASBESTOS BASE FOR ROADS, PILES AND ANCHORS, ASPHALT FINISH, AND SHEET PILING FOR BRICK AND PILE-AND-PILE-CONSTRUCTION REPORT AND STRUCTURAL INSPECTIONS ON EXISTING FACILITIES.
      4. TOWER ERECTION SECTION STAGING AND MATERIALS AT CHECKOUT DOCUMENTED BY CLIENT PHOTOGRAPHS BY THIRD PARTY AGENCY.
      5. ANTENNA ZEMMEL, DOWN THE AND SET CHECKOUT TOOL SUNSHINE INSTRUMENTS - ANTENNA ALIGNMENT TOOL (AAI)

PLANS REVIEWED FOR:  
  
 5501 Sprint Parkway  
 Olathe, OK, Kansas 66201

FROM ZERO TO INFINIGY  
  
 1311 Westchester Street, Suite 100, Lawrence, KS 66044  
 Phone: 785-843-2222  
 Fax: 785-843-2222

DISBURSING LICENSE:  
  
 10000 E. Eastman Avenue  
 Suite 1000  
 Denver, CO 80231

STATE OF CONNECTICUT  
 DEPARTMENT OF CONSTRUCTION  
  
 REGISTERED PROFESSIONAL ENGINEER  
 No. 10000  
 JOHN S. STEVENS, JR.  
 10000 E. Eastman Avenue  
 Suite 1000  
 Denver, CO 80231

REVISIONS:

NO.	DATE	BY	REVISION
1	02/01/01	MS	ISSUED FOR CONSTRUCTION
2	06/27/01	MS	REVISED FOR CONSTRUCTION
3	09/10/01	MS	ISSUED FOR CONSTRUCTION
4	09/10/01	MS	ISSUED FOR CONSTRUCTION
5	09/10/01	MS	ISSUED FOR CONSTRUCTION

SKY HILL

CT09XC204

33 JANOWSKI ROAD  
 ASHFORD, CT 06278

SPRINT SPECIFICATIONS  
 SP-2







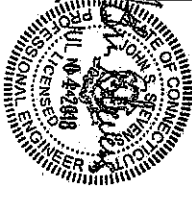
6800 Spalding Parkway  
Overland Park, Kansas 66251

FINFINIGY  
FROM ZERO TO INFINIGY  
The solutions are endless

023 Westerville Station Rd, Albany, NY 12245  
Phone: 518-435-1111  
www.infingy.com  
400 NORTH 2ND ST  
ALBANY, NY 12202



PROFESSIONAL ENGINEER



DESIGNED BY: JAMES J. ALBANO  
DATE: 08/20/14  
CHECKED BY: JAMES J. ALBANO  
DATE: 08/20/14  
ISSUED FOR REVIEW: 08/20/14

REVISION	DESCRIPTION	DATE	BY	CHK
1	ISSUED FOR CONSTRUCTION	08/20/14	JAS	JAS
2	ISSUED FOR CONSTRUCTION	08/20/14	JAS	JAS
3	ISSUED FOR CONSTRUCTION	08/20/14	JAS	JAS
4	ISSUED FOR REVIEW	08/20/14	JAS	JAS
5	ISSUED FOR REVIEW	08/20/14	JAS	JAS

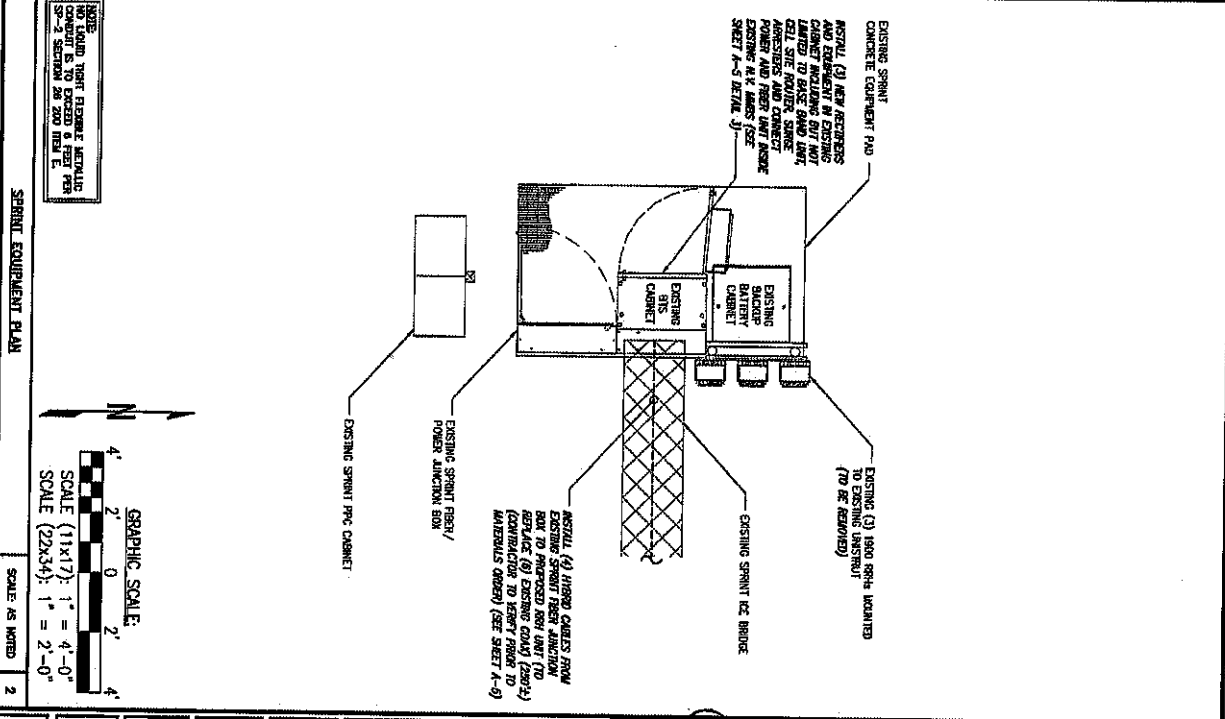
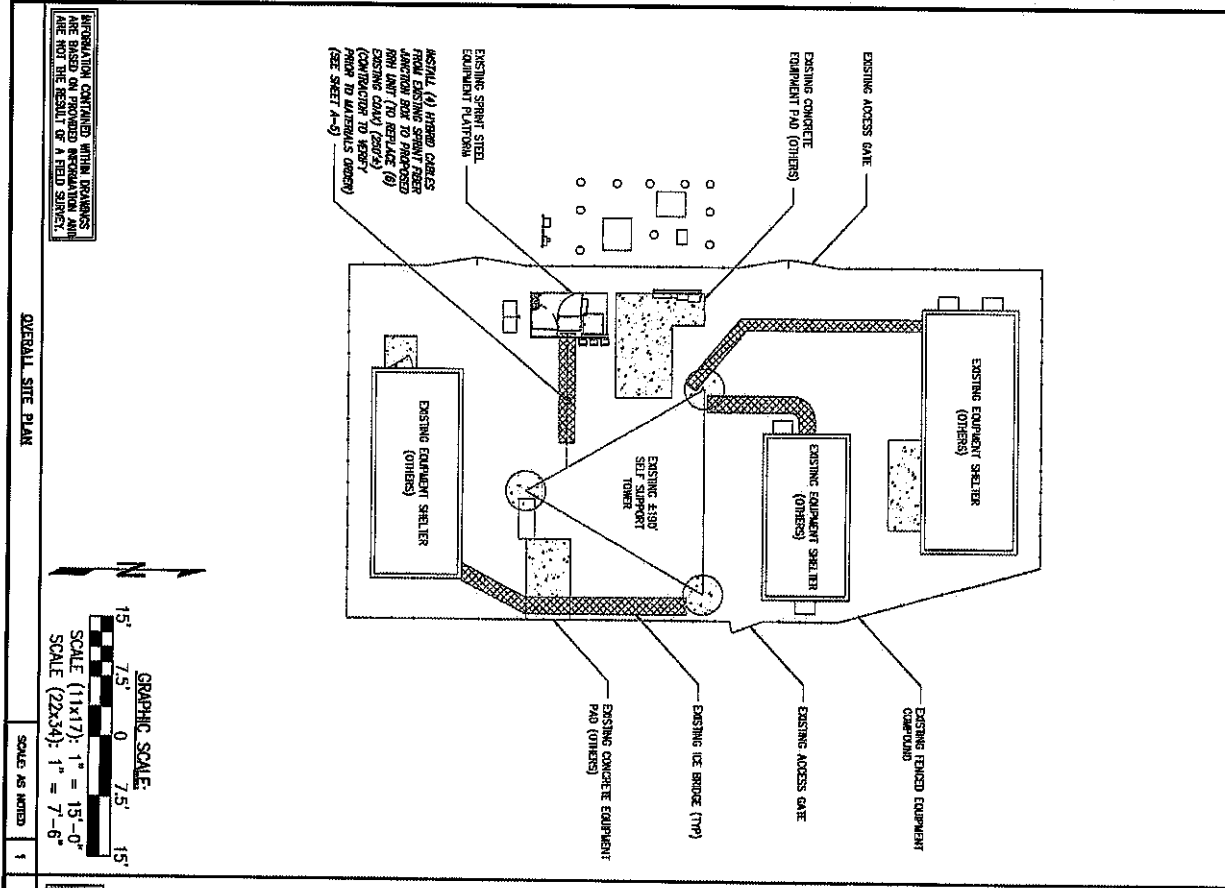
SKY HILL

CT103XC204

33 JANOWSKI ROAD  
ASHROD, CT 06278

SITE PLAN

A-1



INFORMATION CONCERNING VERTICAL DIMENSIONS ARE BASED ON PROVIDED INFORMATION AND ARE NOT THE RESULT OF A FIELD SURVEY.

NOTE:  
NO LUMEN TIGHT FIBER OR METALLIC SP-2 SPLICING ARE 200 MICRONS.



PLANS REVIEWED FOR:



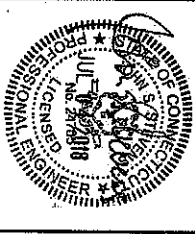
5500 Sprint Parkway  
Overland Park, Kansas 66201

PLANS REVIEWED BY:  
**INFENIGY**  
1011 Westport Blvd., Suite 100  
Overland Park, Kansas 66201  
Phone: 913-441-4100, Fax: 913-441-4000  
www.infengy.com  
2011-2012

FROM ZERO TO INFENIGY  
This position ends with  
1011 Westport Blvd., Suite 100  
Overland Park, Kansas 66201  
Phone: 913-441-4100, Fax: 913-441-4000  
www.infengy.com  
2011-2012



ENGINEERING LICENSE



THESE DOCUMENTS ARE CONFIDENTIAL AND ARE THE SOLE PROPERTY OF SPRINT AND MAY NOT BE REPRODUCED, COPIED, DISTRIBUTED OR OTHERWISE USED WITHOUT THE EXPRESS WRITTEN CONSENT OF SPRINT.

REVISION	DATE	BY	HOW
ISSUED FOR CONSTRUCTION	02/03/10	ME 2	1
ISSUED FOR CONSTRUCTION	02/03/10	ME 1	1
ISSUED FOR CONSTRUCTION	02/03/10	ME 1	1
ISSUED FOR CONSTRUCTION	02/03/10	ME 1	1

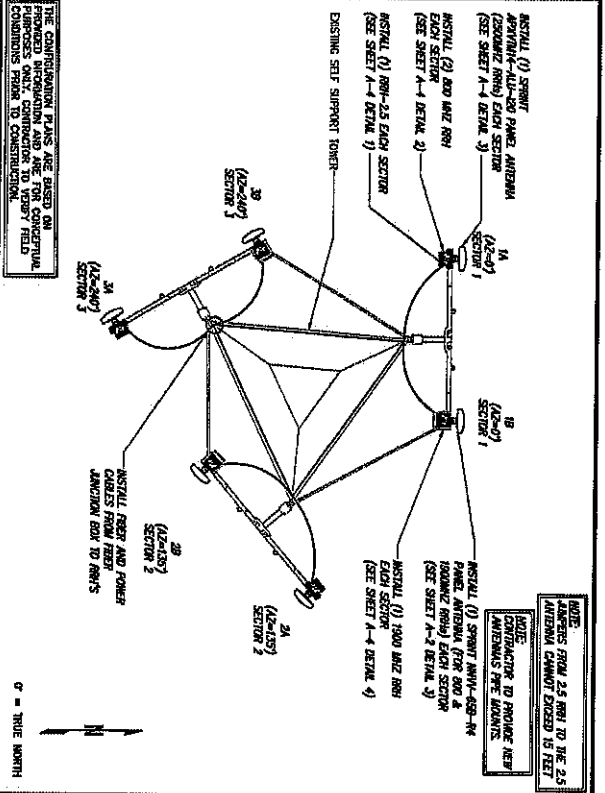
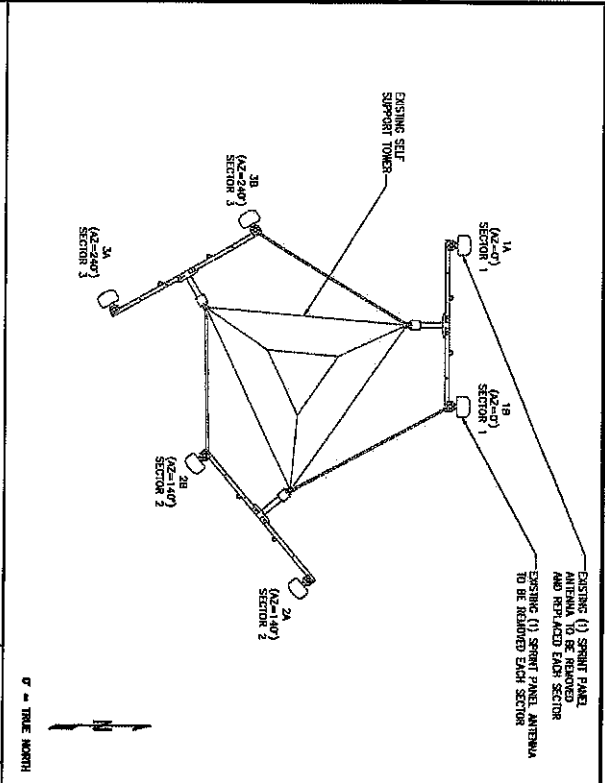
PROJECT NAME:  
**SKY HILL**

WTR DRAWING:  
**CT103XC204**

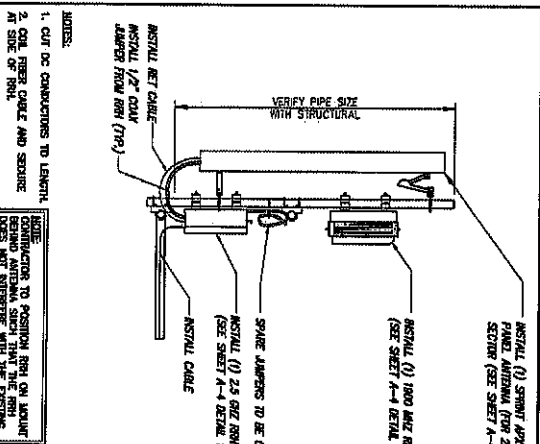
SITE ADDRESS:  
**33 JANDOWSKI ROAD  
ASHFORD, CT 06278**

SHEET DESCRIPTION:  
**ANTENNA LAYOUT  
& MOUNTING DETAILS**

**A-3**



- NOTES:**
1. ALL ANTENNA HEIGHTS ARE TO CENTER OF HORIZONTAL ANTENNA.
  2. VERIFY HEIGHT AND CL HEIGHT WITH AS-BUILT DRAWINGS IF AVAILABLE.
  3. NO OBJECT IS TO BE WITHIN 45 DEGREES OF ROSE-SIGHT OF 2.5G OR ANY OTHER TOWER ANTENNA. IF NECESSARY, 2.5G ANTENNA CAN BE PLACED AT THE SIDE OF HORIZONTAL ANTENNA MOUNT MEMBER FOR CLEAR LINE OF SIGHT OR EVEN ON ANOTHER SECTOR FOR CLEAR LINE OF SIGHT.
  4. 2.5G ANTENNA MUST BE AT LEAST 6' FROM HORIZONTAL ANTENNA, 30" FROM DOWNZ ANTENNA AND 5.0' FROM SELF-SUPPORT TOWER.
  5. IF ANTENNAS ARE MOUNTED ON A FACE SURFACE SUCH AS A BUILDING WALL, PERMIT SHALL BE OBTAINED BEFORE ANY WORK BEGINS. THE EXACT LOCATION OF WHERE ANTENNA IS TO BE LOCATED, CONTACT FROM THE SUBMITTER OF THIS DRAWING IS REQUESTED.
  6. GENERAL CONTRACTOR TO FIELD VERIFY HEIGHT AND CL HEIGHT AND RECORDING DOWNHILL. IF DOWNHILL RECORDING IS NOT AVAILABLE, THE CONTRACTOR SHALL VERIFY THE HEIGHT BY MEANS OF A THEODOLITE OR ANOTHER SURVEYING INSTRUMENT. THE CONTRACTOR SHALL VERIFY THE HEIGHT BY MEANS OF A THEODOLITE OR ANOTHER SURVEYING INSTRUMENT. THE CONTRACTOR SHALL VERIFY THE HEIGHT BY MEANS OF A THEODOLITE OR ANOTHER SURVEYING INSTRUMENT.
  7. ALL NEW VERTICALLY OPERATING IS TO BE PERFORMED AFTER FINAL INSTALLATION OF ANTENNAS AND RRH CABLES HAVE BEEN CONNECTED. VERIFY OPERATION OF ALL EXISTING STRAINS, EXAMINE FOR DAMAGE AND REPAIR AS NECESSARY. VERIFY OPERATION OF ALL EXISTING STRAINS, EXAMINE FOR DAMAGE AND REPAIR AS NECESSARY.
  8. GENERAL CONTRACTOR MUST ASSURE THAT NO OBJECT IS LOCATED IN FRONT OF ANTENNA THIS HEIGHTS NO OBJECT IS TO BE WITHIN 45 DEGREES OF ROSE-SIGHT OF 2.5G OR ANY OTHER TOWER ANTENNA. IF NECESSARY, 2.5G ANTENNA CAN BE PLACED AT THE SIDE OF HORIZONTAL ANTENNA MOUNT MEMBER FOR CLEAR LINE OF SIGHT OR EVEN ON ANOTHER SECTOR FOR CLEAR LINE OF SIGHT.
  9. GENERAL CONTRACTOR IS REQUIRED TO USE A TOWER ATTACHMENT TOOL TO SET ANTENNA, RRH AND DOWNHILL. DOWNHILL ATTACHMENT IS TO BE WITHIN 1 DEGREE DOWNHILL AND RRH (LEFT, RIGHT OR CENTER) IS TO BE WITHIN 0.1 DEGREES. IF FOR SOME REASON THE ATTACHMENT CANNOT BE ACHIEVED, ATTACHMENT TOOL ON DOWNHILL TOOL. <http://www.teltek.com/antenna-attachment-tool/>



- NOTES:**
1. CUT DC CONDUITS TO LENGTH.
  2. COIL FIBER CABLE AND SECURE TO ANTENNA.
  3. DO NOT EXCEED BEND RADIUS.
- NOTE:** CONTRACTOR TO POSITION RRH ON MOUNT MEMBER ANTENNA SUCH THAT THE EXISTING DOWNHILL/RRH MOUNTING MEMBER IS NOT DAMAGED.
- NOTE:** SPARE DC CABLES ARE COILED UP ON RRH'S AT SIGHT AND TO BE USED TO POWER UP ANTENNAS FROM THE EXISTING DOWNHILL/RRH MOUNTING MEMBER.
- NOTE:** GENERAL IS FOR CONSTRUCTION PURPOSES ONLY. CONTRACTOR IS TO VERIFY ALL MOUNTING AND RRH MOUNTING DETAILS.

**NOTES**

NO SCALE

3

EXISTING ANTENNA LAYOUT

NO SCALE

1

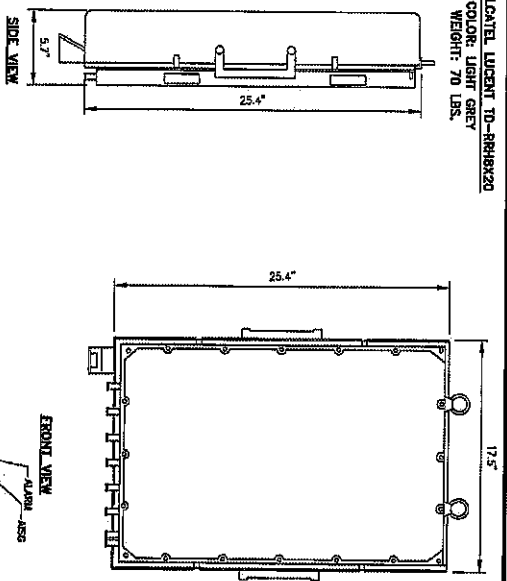
FINAL ANTENNA & RRH LAYOUT

NO SCALE

2

RFR: ALCATEL LUCENT TD-RFRHX20

COLOR: LIGHT GREY  
WEIGHT: 70 LBS.



**NOTES**  
CONSULT WITH MANUFACTURER'S INSTRUCTIONS TO ENSURE THAT ALL WIRING RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING. DO NOT OPEN RFR PACKAGES IN THE RFR.

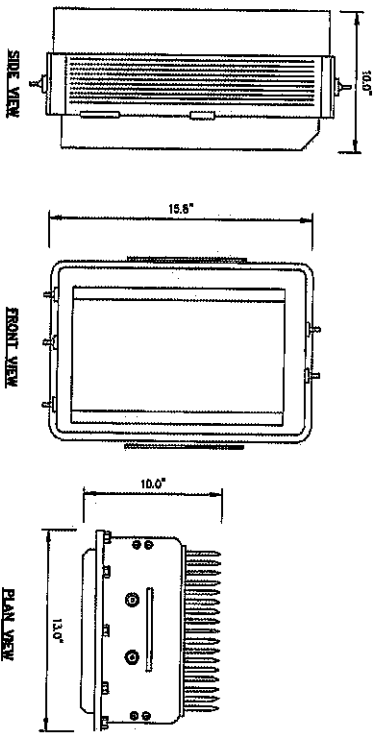
2.5. RFR'S

NO SCALE

1

RFR: ALCATEL LUCENT RFR 800 MHz 2x50W

COLOR: LIGHT GREY  
WEIGHT: 53 LBS.



**NOTES**  
CONSULT WITH MANUFACTURER'S INSTRUCTIONS TO ENSURE THAT ALL WIRING RECEIVE ELECTRICAL POWER WITHIN 24 HOURS OF BEING REMOVED FROM THE MANUFACTURER'S PACKAGING. DO NOT OPEN RFR PACKAGES IN THE RFR.

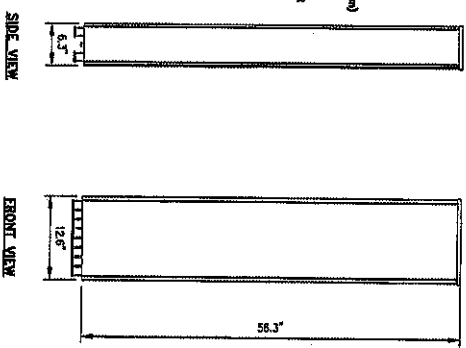
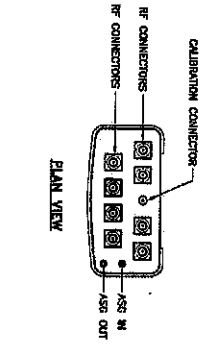
800 MHz RFR

NO SCALE

2

ANTENNA RFS APVYMH4-ALU-120

ROOM: MATERIAL: ASA  
COLOR: LIGHT GREY  
DIMENSIONS: (Height/Width) 56.3"x12.8"x6.3" (1430x320x160mm)  
WEIGHT: 58.2 lbs  
CONNECTORS: (8) 41/9.5 DIN FEMALE  
(1) N - CALIBRATION CONNECTOR

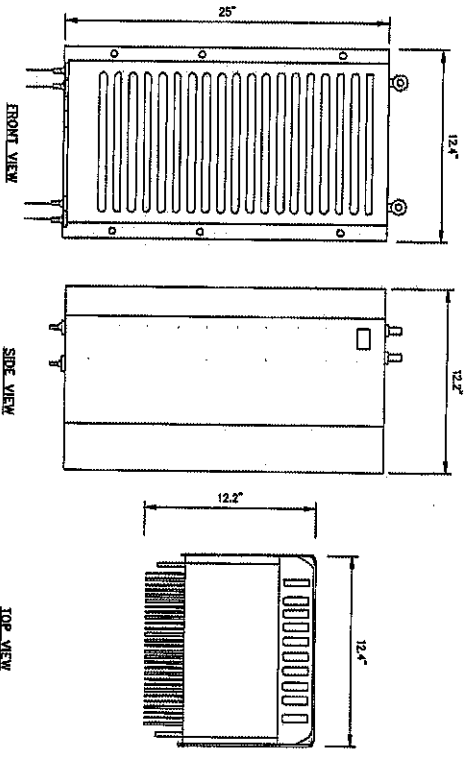


2.5. ANTENNA

NO SCALE

3

RFR: ALCATEL LUCENT 1800 MHz  
COLOR: LIGHT GREY  
WEIGHT: 70 LBS.  
(INCLUDING OPTIONAL SOLAR SHIELD)



1800 MHz RFR

NO SCALE

4

PLANS PROVIDED FOR:



PLANS PROVIDED BY:  
**INFINIGY**  
FROM ZERO TO INFINIGY  
The solutions you envision  
1500 Sprint Parkway, Suite 1100  
Overland Park, Kansas 66251  
www.infinigy.com  
Tel: 913.241.1100  
Fax: 913.241.1101



**WARNING NOTICE:**  
THESE DOCUMENTS ARE CONFIDENTIAL AND ARE THE SOLE PROPERTY OF CROWN CASTLE. ANY REPRODUCTION, DISSEMINATION OR DISTRIBUTION WITHOUT THE EXPRESS WRITTEN CONSENT OF CROWN CASTLE IS PROHIBITED.

REVISION	DESCRIPTION	DATE	BY	APP
1	ISSUED FOR CONSTRUCTION	07/27/07	MS	A
2	ISSUED FOR CONSTRUCTION	07/27/07	MS	A
3	ISSUED FOR CONSTRUCTION	07/27/07	MS	A

SITE NAME:  
**SKY HILL**

SITE LOCATION:  
**CT03XC204**

SITE ADDRESS:  
**33 JANOWSKI ROAD  
ASHFORD, CT 06278**

SHEET NUMBER:  
**EQUIPMENT &  
MOUNTING DETAILS**

**A-4**

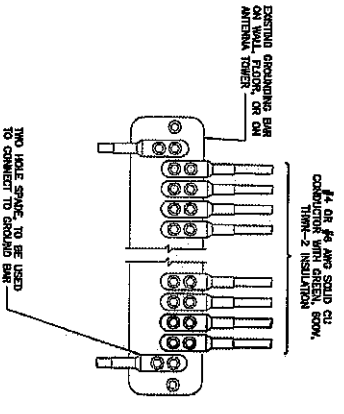
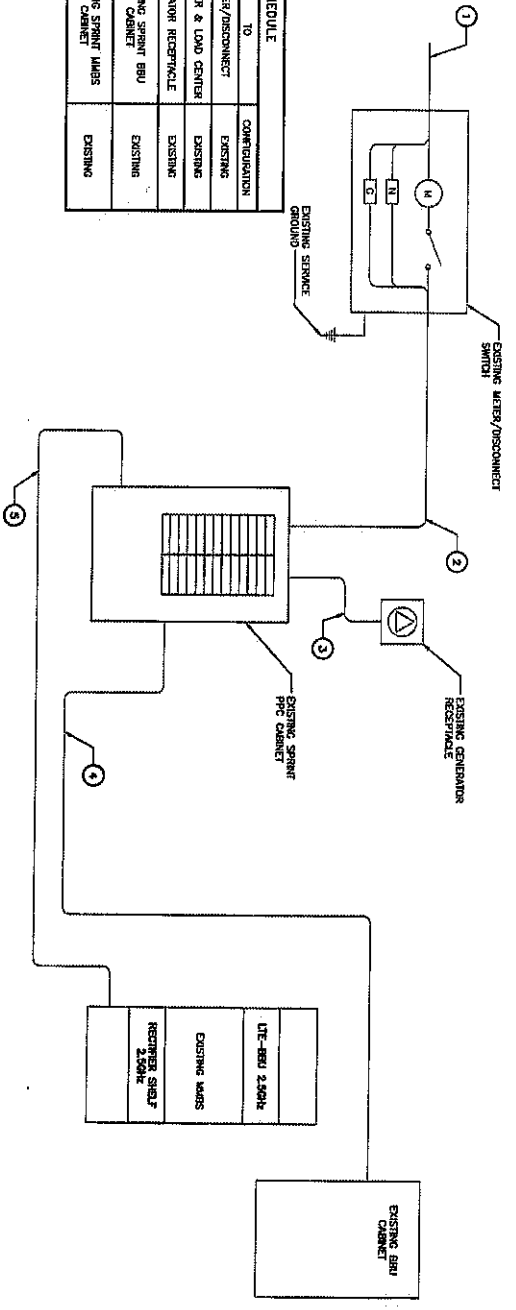




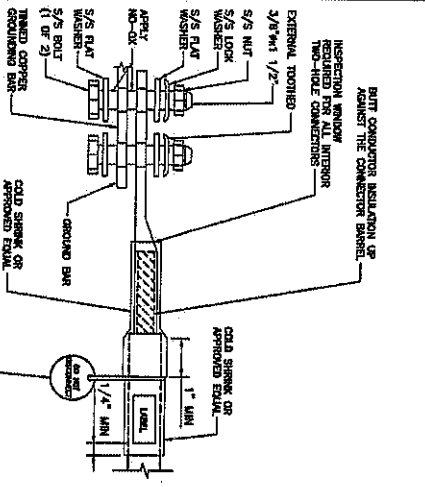


**NOTES**  
 02 SHALL REFERENCE ALL SPECS FOR CONNECTIONS TO POWER SUPPLY OF THE NEW INSTALLATION DOCUMENTS, FOR ALL CONNECTION SPECIFICATIONS.

CIRCUIT SCHEDULE			
NO	FROM	TO	COMPARISON
1	UTILITY SOURCE	METER/DISCONNECT	EXISTING
2	METER/DISCONNECT	TRANSFER & LOAD CENTER	EXISTING
3	TRANSFER & LOAD CENTER	GENERATOR RECEPTACLE	EXISTING
4	TRANSFER & LOAD CENTER	EXISTING SPRINT BRU CABINET	EXISTING
5	TRANSFER & LOAD CENTER	EXISTING SPRINT LABS CABINET	EXISTING



**NOTES**  
 1. APPLY NO-OX TO LUG AND BAR CONTACT SURFACE. DO NOT OIL NUTS LUG.  
 2. IF SHOULD BE REPLACED THEORETICAL ROD W/ SPRINT OR FOR REPLACEMENT THEORETICAL ROD W/.



**INSTALLATION OF GROUNDING BAR CONDUCTOR TO GROUNDING BAR**

**TWO HOLE LUGS**

**GROUNDING RISER DIAGRAM**

**ELECTRICAL & GROUNDING DETAILS**

PLANS REVIEWED FOR: **Sprint**  
 5500 South Park Drive  
 Overland Park, Kansas 66201

PLAN REVIEWED BY: **INFINIGY**  
 FROM ZERO TO INFINIGY  
 The solutions are endless  
 1021 Woodloch Hill Drive  
 Overland Park, Kansas 66201  
 Phone: 913-442-0276 Fax: 913-442-0273  
 Email: info@infinigy.com  
 www.infinigy.com

PROFESSIONAL ENGINEER  
 STATE OF KANSAS  
 No. 11478  
 J. S. CROWN

ENGINEERING LICENSE  
**CROWN CASTLE**

REVISIONS:

NO.	DESCRIPTION	DATE	BY	REV
1	ISSUED FOR CONSTRUCTION	02/20/20	JSC	1
2	ISSUED FOR CONSTRUCTION	02/20/20	JSC	2
3	ISSUED FOR CONSTRUCTION	02/20/20	JSC	3
4	ISSUED FOR CONSTRUCTION	02/20/20	JSC	4
5	ISSUED FOR CONSTRUCTION	02/20/20	JSC	5
6	ISSUED FOR CONSTRUCTION	02/20/20	JSC	6
7	ISSUED FOR CONSTRUCTION	02/20/20	JSC	7
8	ISSUED FOR CONSTRUCTION	02/20/20	JSC	8
9	ISSUED FOR CONSTRUCTION	02/20/20	JSC	9
10	ISSUED FOR CONSTRUCTION	02/20/20	JSC	10

DATE DRAWN: **CT103XC204**

PROJECT ADDRESS: **33 JANOWSKI ROAD ASHFORD, CT 06278**

CLIENT NAME: **SKY HILL**

PROJECT NUMBER: **E-2**





Date: May 31, 2018

Denice Nicholson  
Crown Castle  
3 Corporate Park Drive Suite 101  
Clifton Park, NY 12065

Crown Castle  
2000 Corporate Drive  
Canonsburg, PA 15317  
(724) 416-2000

**Subject:** Structural Analysis Report

**Carrier Designation:** Sprint PCS Co-Locate  
**Carrier Site Number:** CT03XC204  
**Carrier Site Name:** CT03XC204

**Crown Castle Designation:** Crown Castle BU Number: 876345  
Crown Castle Site Name: SKY HILL  
Crown Castle JDE Job Number: 505815  
Crown Castle Work Order Number: 1580544  
Crown Castle Order Number: 441316 Rev. 0

**Engineering Firm Designation:** Crown Castle Project Number: 1580544

**Site Data:** 33 Janowski Road, Ashford, Windham County, CT  
Latitude 41° 57' 7.7", Longitude -72° 11' 43.9"  
192 Foot - Self Support Tower

Dear Denice Nicholson,

Crown Castle is pleased to submit this "Structural Analysis Report" to determine the structural integrity of the above mentioned tower. This analysis has been performed in accordance with the Crown Castle Structural 'Statement of Work' and the terms of Crown Castle Purchase Order Number 1580544, in accordance with order 441316, revision 0.

The purpose of the analysis is to determine acceptability of the tower stress level. Based on our analysis we have determined the tower stress level for the structure and foundation, under the following load case, to be:

LC7: Existing + Reserved + Proposed Equipment **Sufficient Capacity**  
Note: See Table I and Table II for the proposed and existing/reserved loading, respectively.

This analysis has been performed in accordance with the 2016 Connecticut State Building Code based upon an ultimate 3-second gust wind speed of 130 mph converted to a nominal 3-second gust wind speed of 101 mph per Section 1609.3 and Appendix N as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category B and Risk Category II were used in this analysis.

All modifications and equipment proposed in this report shall be installed in accordance with the attached drawings for the determined available structural capacity to be effective.

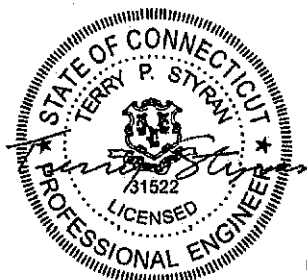
We at Crown Castle appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Structural analysis prepared by: Mahdis Arianpour / KB

Respectfully submitted by:

Terry P. Styran, P.E.  
Senior Project Engineer

tnxTower Report - version 7.0.5.1



6/5/2018

## TABLE OF CONTENTS

### 1) INTRODUCTION

### 2) ANALYSIS CRITERIA

Table 1 - Proposed Antenna and Cable Information

Table 2 - Existing and Reserved Antenna and Cable Information

Table 3 - Design Antenna and Cable Information

### 3) ANALYSIS PROCEDURE

Table 4 - Documents Provided

3.1) Analysis Method

3.2) Assumptions

### 4) ANALYSIS RESULTS

Table 5 - Section Capacity (Summary)

Table 6 – Tower Components vs. Capacity- LC7

4.1) Recommendations

### 5) APPENDIX A

tnxTower Output

### 6) APPENDIX B

Base Level Drawing

### 7) APPENDIX C

Additional Calculations

**1) INTRODUCTION**

This tower is a 192 ft Self Support tower designed by Rohn in December of 1996. The tower was originally designed for a wind speed of 90 mph per TIA/EIA-222-E.

**2) ANALYSIS CRITERIA**

The structural analysis was performed for this tower in accordance with the requirements of TIA-222-G Structural Standards for Steel Antenna Towers and Antenna Supporting Structures using a 3-second gust wind speed of 101 mph with no ice, 50 mph with 1 inch ice thickness and 60 mph under service loads, exposure category B.

**Table 1 - Proposed Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
190.0	192.0	3	alcatel lucent	PCS 1900MHz 4x45W-65MHz	4	1-1/4	-
		6	alcatel lucent	RRH2X50-800			
		3	alcatel lucent	TD-RRH8x20-25			
		3	commscope	NNVV-65B-R4 w/ Mount Pipe			
		3	rfs celwave	APXVTM14-ALU-I20 w/ Mount Pipe			

**Table 2 - Existing and Reserved Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
190.0	192.0	6	decibel	DB980H90E-M w/ Mount Pipe	6	1-5/8	3
	190.0	1	tower mounts	Sector Mount [SM 506-3]	-	-	1
180.0	181.0	1	symmetricom	58532A	6	1-5/8	1
		6	antel	LPA-80080/4CF	1	1/2	
		3	alcatel lucent	RRH2X60-700	2	1-5/8	2
		3	alcatel lucent	RRH4X45-AWS4 B66			
		6	commscope	JAHH-65B-R3B			
		3	nokia	BAND 5 AHCA RRH4X40			
	2	raycap	RC3DC-3315-PF-48				
180.0	1	tower mounts	Sector Mount [SM 304-3]	-	-	1	
170.0	172.0	9	allgon	7130.16.33.00 w/ Mount Pipe	9	1-5/8	4
	170.0	1	tower mounts	Sector Mount [SM 502-3]			
160.0	160.0	3	andrew	HBX-6516DS-VTM w/ Mount Pipe	6	1-5/8	1
		1	tower mounts	Sector Mount [SM 104-3]			
153.0	153.0	2	commscope	ATBT-BOTTOM-24V	8	7/8	1
		2	commscope	LNx-6515DS-VTM w/ Mount Pipe			

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)	Note
		2	ems wireless	RR90-17-02DP w/ Mount Pipe			
		2	ericsson	KRY 112 144/1			
		2	tower mounts	Side Arm Mount [SO 301-1]			
140.0	141.0	3	communication components inc.	DTMABP7819VG12A	12 2 1 1	7/8 3/4 3/8 Conduit	1
		6	ericsson	RRUS-11			
		3	kathrein	800 10121 w/ Mount Pipe			
		4	kmw communications	AM-X-CD-14-65-00T-RET w/ Mount Pipe			
		2	kmw communications	AM-X-CD-16-65-00T-RET w/ Mount Pipe			
		3	powerwave technologies	7020.00			
		3	powerwave technologies	LGP13519			
		1	raycap	DC6-48-60-18-8F			
	140.0	1	tower mounts	Sector Mount [SM 504-3]			
98.0	102.0	1	symmetricom	58532A	-	-	1
	98.0	1	tower mounts	Side Arm Mount [SO 301-1]			

- Notes:  
 1) Existing Equipment  
 2) Reserved Equipment  
 3) Equipment to be Removed, Considered in this Analysis  
 4) Abandoned Equipment; Considered in this Analysis

**Table 3 - Design Antenna and Cable Information**

Mounting Level (ft)	Center Line Elevation (ft)	Number of Antennas	Antenna Manufacturer	Antenna Model	Number of Feed Lines	Feed Line Size (in)
189	189	12	Decibel	DB980H90E-M	12	2-1/4
170	170	1	generic	Mounting Frame	12	1 5/8
		12	Swedcom	ALP9212		
150	150	12	Swedcom	ALP9212	12	1 5/8
80	80	1	generic	12' Gate Boom	1	7/8
		1	generic	GPS Antenna		

### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	FDH Engineering	2189896	CCI Sites
4-TOWER MANUFACTURER DRAWINGS	Rohn	1631630	CCI Sites
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Rohn	1631622	CCI Sites

#### 3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) The existing base plate grout was considered in this analysis. Grout must be maintained and inspected periodically and must be replaced if damaged or cracked. Refer to Crown Castle document ENG-PRC-10012, Base Plate Grout Repair.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	192 - 180	Leg	ROHN 2.5 STD	1	-8.388	63.560	13.2	Pass
T2	180 - 160	Leg	ROHN 2.5 STD	27	-40.997	57.139	71.8	Pass
T3	160 - 140	Leg	ROHN 3 EH	57	-75.920	94.337	80.5	Pass
T4	140 - 120	Leg	ROHN 4 EH	78	-116.623	159.899	72.9	Pass
T5	120 - 100	Leg	ROHN 5 EH	99	-155.166	239.348	64.8	Pass
T6	100 - 80	Leg	ROHN 6 EHS	120	-189.129	244.047	77.5	Pass
T7	80 - 60	Leg	ROHN 6 EH	135	-226.462	303.757	74.6	Pass
T8	60 - 40	Leg	ROHN 8 EHS	150	-261.827	393.649	66.5	Pass
T9	40 - 20	Leg	ROHN 8 EHS	165	-297.486	393.703	75.6	Pass
T10	20 - 0	Leg	ROHN 8 EHS	180	-333.211	393.691	84.6	Pass
T1	192 - 180	Diagonal	L1 3/4x1 3/4x3/16	11	-1.838	8.789	20.9 27.0 (b)	Pass
T2	180 - 160	Diagonal	L2x2x3/16	36	-4.691	7.794	60.2 60.8 (b)	Pass
T3	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-6.691	12.367	54.1 63.0 (b)	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	84	-7.908	9.447	83.7	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T5	120 - 100	Diagonal	L3x3x1/4	105	-8.418	13.104	64.2	Pass
T6	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	126	-9.824	14.295	68.7 72.4 (b)	Pass
T7	80 - 60	Diagonal	L4x4x1/4	141	-10.765	18.143	59.3 78.8 (b)	Pass
T8	60 - 40	Diagonal	L4x4x5/16	156	-10.319	18.754	55.0 76.5 (b)	Pass
T9	40 - 20	Diagonal	L4x4x5/16	171	-12.234	16.165	75.7 88.7 (b)	Pass
T10	20 - 0	Diagonal	L4x4x3/8	183	-12.710	16.496	77.0 92.2 (b)	Pass
T1	192 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.119	3.099	3.9	Pass
T2	180 - 160	Top Girt	L2x2x3/16	28	-1.010	4.694	21.5	Pass
							Summary	
							Leg (T10)	84.6 Pass
							Diagonal (T10)	92.2 Pass
							Top Girt (T2)	21.5 Pass
							Bolt Checks	92.2 Pass
							Rating =	92.2 Pass

**Table 6 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	57.9	Pass
1	Base Foundation (Structure)	0	16.7	Pass
1	Base Foundation (Soil Interaction)	0	46.6	Pass

<b>Structure Rating (max from all components) =</b>	<b>92.2%</b>
---	--------------

Notes:

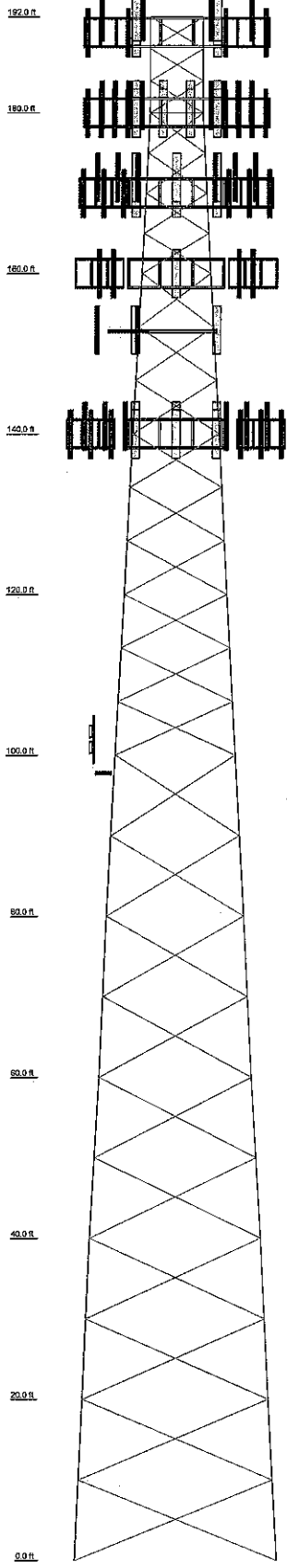
- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

**4.1) Recommendations**

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX A**  
**TNXTOWER OUTPUT**

Section	T19	T18	T17	T16	T15	T14	T13	T12	T11
Leg Code	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH	ROHN-B-CH
Diagonal	L6x4x1/4	L6x4x1/4	L6x4x1/4	L6x4x1/4	L6x4x1/4	L6x4x1/4	L6x4x1/4	L6x4x1/4	L6x4x1/4
Original Girth	A572-50	A572-50	A572-50	A572-50	A572-50	A572-50	A572-50	A572-50	A572-50
Top Chl	10 @ 10	10 @ 10	10 @ 10	10 @ 10	10 @ 10	10 @ 10	10 @ 10	10 @ 10	10 @ 10
Face Width (ft)	26.5	26.5	26.5	26.5	26.5	26.5	26.5	26.5	26.5
# Panels @ (ft)	4.4	4.4	4.4	4.4	4.4	4.4	4.4	4.4	4.4
Weight (K)	26.5	26.5	26.5	26.5	26.5	26.5	26.5	26.5	26.5



**DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
NHVV-65B-R4 w/ Mount Pipe	190	HBX-651SDS-VTM w/ Mount Pipe	160
NHVV-65B-R4 w/ Mount Pipe	180	HBX-651SDS-VTM w/ Mount Pipe	150
NHVV-65B-R4 w/ Mount Pipe	170	6" x 2" Mount Pipe	160
APXVTM14-ALLU-120 w/ Mount Pipe	190	6" x 2" Mount Pipe	160
APXVTM14-ALLU-120 w/ Mount Pipe	180	Sector Mount (SM 504-3)	160
TD-RRH-20-25	190	RR90-17-202P w/ Mount Pipe	153
TD-RRH-20-25	180	RR90-17-202P w/ Mount Pipe	153
TD-RRH-20-25	170	LHX-651SDS-VTM w/ Mount Pipe	153
FCS 1900MHz 4x45W-65MHz	190	LHX-651SDS-VTM w/ Mount Pipe	153
FCS 1900MHz 4x45W-65MHz	180	KRY 112 144F	153
FCS 1900MHz 4x45W-65MHz	170	KRY 112 144F	153
(2) RRH-2050-800	190	ATB-BOTTOM-24V	153
(2) RRH-2050-800	180	Side Arm Mount (SO 301-1)	153
(2) RRH-2050-800	170	Side Arm Mount (SO 301-1)	153
Sector Mount (SM 504-3)	190	Side Arm Mount (SO 301-1)	153
58532A	180	(2) AMX-CD-16-65-00-FRET w/ Mount Pipe	140
(2) LPA-403094CF	180	(2) AMX-CD-16-65-00-FRET w/ Mount Pipe	140
(2) LPA-403094CF	170	(2) AMX-CD-16-65-00-FRET w/ Mount Pipe	140
(2) LPA-403094CF	160	800 10121 w/ Mount Pipe	140
(2) JAHH-65B-R3B	180	800 10121 w/ Mount Pipe	140
(2) JAHH-65B-R3B	170	800 10121 w/ Mount Pipe	140
(2) JAHH-65B-R3B	160	DCS-48-60-18-8F	140
BAND 5 AHCA-RRH4040	180	(2) RRUS-11	140
BAND 5 AHCA-RRH4040	170	(2) RRUS-11	140
BAND 5 AHCA-RRH4040	160	(2) RRUS-11	140
RRH2060-700	180	7620 00	140
RRH2060-700	170	7620 00	140
RRH2060-700	160	7620 00	140
RRH4046-AW/84 EBS	180	LGP13519	140
RRH4046-AW/84 EBS	170	LGP13519	140
RRH4046-AW/84 EBS	160	LGP13519	140
(2) RC3DC-3315-PF-48	180	DTMABP7818V12A	140
Sector Mount (SM 304-3)	180	DTMABP7818V12A	140
(3) 7130.16.33.00 w/ Mount Pipe	170	DTMABP7818V12A	140
(3) 7130.16.33.00 w/ Mount Pipe	160	Sector Mount (SM 504-3)	140
(3) 7130.16.33.00 w/ Mount Pipe	150	58532A	99
Sector Mount (SM 503-3)	170	Side Arm Mount (SO 301-1)	99
HBX-651SDS-VTM w/ Mount Pipe	160		

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A572-50	50 ksi	58 ksi	A36	36 ksi	58 ksi

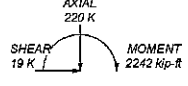
**TOWER DESIGN NOTES**

1. Tower is located in Windham County, Connecticut.
2. Tower designed for Exposure B to the TIA-222-G Standard.
3. Tower designed for a 101 mph basic wind in accordance with the TIA-222-G Standard.
4. Tower is also designed for a 50 mph basic wind with 1.00 in. ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 90 mph wind.
6. Tower Structure Class II.
7. Topographic Category 1 with Crest Height of 0.000 ft
8. TOWER RATING: 92.2%

ALL REACTIONS ARE FACTORED

MAX. CORNER REACTIONS AT BASE:  
DOWN: 342 K  
SHEAR: 39 K

UPLIFT: -289 K  
SHEAR: 34 K



TORQUE 5 kip-ft  
50 mph WIND - 1,000 in ICE



TORQUE 16 kip-ft  
REACTIONS - 101 mph WIND

<p><b>Crown Castle</b> 2000 Corporate Drive Canonsburg, PA 15317 Phone: (724) 416-2000 FAX: (724) 416-6223</p>	<p>Proj: BU# 876345</p>
	<p>Client: Crown Castle</p>
	<p>Code: TIA-222-G</p>
	<p>Drawn by: Mahdis Arianpour</p>
	<p>Date: 05/31/18</p>
<p>Scale: NTS</p>	
<p>Dwg No: E-1</p>	



## Tower Input Data

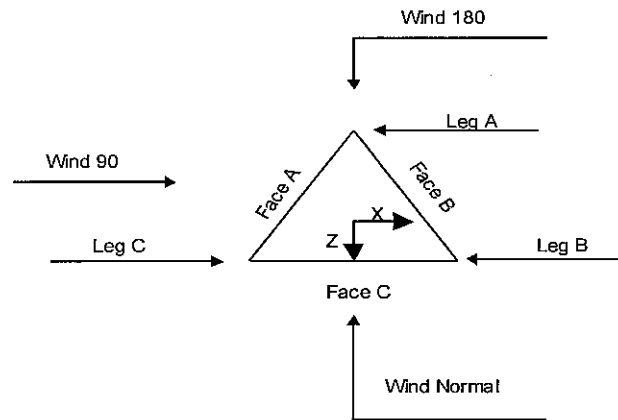
The main tower is a 3x free standing tower with an overall height of 192.000 ft above the ground line.  
 The base of the tower is set at an elevation of 0.000 ft above the ground line.  
 The face width of the tower is 6.580 ft at the top and 25.050 ft at the base.  
 This tower is designed using the TIA-222-G standard.

The following design criteria apply:

- 1) Tower is located in Windham County, Connecticut.
- 2) Basic wind speed of 101 mph.
- 3) Structure Class II.
- 4) Exposure Category B.
- 5) Topographic Category 1.
- 6) Crest Height 0.000 ft.
- 7) Nominal ice thickness of 1.000 in.
- 8) Ice thickness is considered to increase with height.
- 9) Ice density of 56.000 pcf.
- 10) A wind speed of 50 mph is used in combination with ice.
- 11) Temperature drop of 50.000 °F.
- 12) Deflections calculated using a wind speed of 60 mph.
- 13) Pressures are calculated at each section.
- 14) Stress ratio used in tower member design is 1.
- 15) Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

- |  |  |   |
|--|--|---|
| Consider Moments - Legs<br>Consider Moments - Horizontals<br>Consider Moments - Diagonals<br>Use Moment Magnification<br>✓ Use Code Stress Ratios<br>✓ Use Code Safety Factors - Guys<br>Escalate Ice<br>Always Use Max Kz<br>Use Special Wind Profile<br><br>✓ Include Bolts In Member Capacity<br><br>Leg Bolts Are At Top Of Section<br>✓ Secondary Horizontal Braces Leg<br>Use Diamond Inner Bracing (4 Sided)<br>SR Members Have Cut Ends<br>SR Members Are Concentric | Distribute Leg Loads As Uniform<br>Assume Legs Pinned<br>✓ Assume Rigid Index Plate<br>✓ Use Clear Spans For Wind Area<br>✓ Use Clear Spans For KL/r<br>Retension Guys To Initial Tension<br>✓ Bypass Mast Stability Checks<br>✓ Use Azimuth Dish Coefficients<br>✓ Project Wind Area of Appurt.<br><br>Autocalc Torque Arm Areas<br><br>Add IBC .6D+W Combination<br>✓ Sort Capacity Reports By Component<br>Triangulate Diamond Inner Bracing<br>Treat Feed Line Bundles As Cylinder | Use ASCE 10 X-Brace Ly Rules<br>✓ Calculate Redundant Bracing Forces<br>Ignore Redundant Members in FEA<br>✓ SR Leg Bolts Resist Compression<br>All Leg Panels Have Same Allowable<br>Offset Girt At Foundation<br>✓ Consider Feed Line Torque<br>✓ Include Angle Block Shear Check<br>Use TIA-222-G Bracing Resist.<br>Exemption<br>Use TIA-222-G Tension Splice<br>Exemption<br>Poles<br>Include Shear-Torsion Interaction<br>Always Use Sub-Critical Flow<br>Use Top Mounted Sockets |
|--|--|---|



**Triangular Tower**

**Tower Section Geometry**

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	192.000-180.000			6.580	1	12.000
T2	180.000-160.000			6.580	1	20.000
T3	160.000-140.000			8.540	1	20.000
T4	140.000-120.000			10.610	1	20.000
T5	120.000-100.000			12.740	1	20.000
T6	100.000-80.000			14.830	1	20.000
T7	80.000-60.000			16.920	1	20.000
T8	60.000-40.000			18.880	1	20.000
T9	40.000-20.000			21.130	1	20.000
T10	20.000-0.000			23.050	1	20.000

**Tower Section Geometry (cont'd)**

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in.	in.
T1	192.000-180.000	4.000	X Brace	No	No	0.000	0.000
T2	180.000-160.000	5.000	X Brace	No	No	0.000	0.000
T3	160.000-140.000	6.667	X Brace	No	No	0.000	0.000
T4	140.000-120.000	6.667	X Brace	No	No	0.000	0.000
T5	120.000-100.000	6.667	X Brace	No	No	0.000	0.000
T6	100.000-80.000	10.000	X Brace	No	No	0.000	0.000
T7	80.000-60.000	10.000	X Brace	No	No	0.000	0.000
T8	60.000-40.000	10.000	X Brace	No	No	0.000	0.000
T9	40.000-20.000	10.000	X Brace	No	No	0.000	0.000

Tower Section	Tower Elevation ft	Diagonal Spacing ft	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset in	Bottom Girt Offset in
T10	20.000-0.000	10.000	X Brace	No	No	0.000	0.000

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 192.000-180.000	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)
T2 180.000-160.000	Pipe	ROHN 2.5 STD	A572-50 (50 ksi)	Equal Angle	L2x2x3/16	A36 (36 ksi)
T3 160.000-140.000	Pipe	ROHN 3 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T4 140.000-120.000	Pipe	ROHN 4 EH	A572-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x1/4	A36 (36 ksi)
T5 120.000-100.000	Pipe	ROHN 5 EH	A572-50 (50 ksi)	Equal Angle	L3x3x1/4	A572-50 (50 ksi)
T6 100.000-80.000	Pipe	ROHN 6 EHS	A572-50 (50 ksi)	Equal Angle	L3 1/2x3 1/2x1/4	A572-50 (50 ksi)
T7 80.000-60.000	Pipe	ROHN 6 EH	A572-50 (50 ksi)	Equal Angle	L4x4x1/4	A572-50 (50 ksi)
T8 60.000-40.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x5/16	A572-50 (50 ksi)
T9 40.000-20.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x5/16	A572-50 (50 ksi)
T10 20.000-0.000	Pipe	ROHN 8 EHS	A572-50 (50 ksi)	Equal Angle	L4x4x3/8	A572-50 (50 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 192.000-180.000	Equal Angle	L1 3/4x1 3/4x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)
T2 180.000-160.000	Equal Angle	L2x2x3/16	A36 (36 ksi)	Single Angle		A36 (36 ksi)

### Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft <sup>2</sup>	Gusset Thickness in	Gusset Grade	Adjust. Factor A <sub>r</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
T1 192.000-180.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T2 180.000-160.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T3 160.000-140.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T4 140.000-120.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T5 120.000-100.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T6 100.000-80.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T7 80.000-60.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T8 60.000-40.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt
T9 40.000-20.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A <sub>r</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
T10 20.000-0.000	0.000	0.250	A36 (36 ksi)	1.05	1	1.05	Mid-Pt	Mid-Pt	Mid-Pt

### Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors <sup>1</sup>							
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
				X Y	X Y	X Y	X Y	X Y	X Y	X Y	
T1 192.000-180.000	Yes	No	1	1	1	1	1	1	1	1	1
T2 180.000-160.000	Yes	No	1	1	1	1	1	1	1	1	1
T3 160.000-140.000	Yes	No	1	1	1	1	1	1	1	1	1
T4 140.000-120.000	Yes	No	1	1	1	1	1	1	1	1	1
T5 120.000-100.000	Yes	No	1	1	1	1	1	1	1	1	1
T6 100.000-80.000	Yes	No	1	1	1	1	1	1	1	1	1
T7 80.000-60.000	Yes	No	1	1	1	1	1	1	1	1	1
T8 60.000-40.000	Yes	No	1	1	1	1	1	1	1	1	1
T9 40.000-20.000	Yes	No	1	1	1	1	1	1	1	1	1
T10 20.000-0.000	Yes	No	1	1	1	1	1	1	1	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 192.000-180.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T2 180.000-160.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T3 160.000-140.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T4 140.000-120.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T5 120.000-100.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T6 100.000-80.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T7 80.000-60.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T8 60.000-40.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T9 40.000-20.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75
T10 20.000-0.000	0.000	1	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75	0.000	0.75

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg Connection Type	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
		Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 192.000-180.000	Flange	0.625 A325N	4	0.625 A325N	1	0.625 A325N	1	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T2 180.000-160.000	Flange	0.625 A325N	4	0.625 A325N	1	0.625 A325N	1	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T3 160.000-140.000	Flange	0.875 A325N	4	0.625 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T4 140.000-120.000	Flange	1.000 A325N	4	0.625 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T5 120.000-100.000	Flange	1.000 A325N	6	0.750 A325N	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T6 100.000-80.000	Flange	1.000 A325N	6	0.750 A325N	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T7 80.000-60.000	Flange	1.000 A325N	8	0.750 A325N	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T8 60.000-40.000	Flange	1.000 A325N	8	0.750 A325X	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T9 40.000-20.000	Flange	1.000 A325N	8	0.750 A325X	1	0.625 A325N	0	0.625 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0
T10 20.000-0.000	Flange	1.000 A354-BC	10	0.750 A325X	1	0.625 A325N	0	0.000 A325N	0	0.625 A325X	0	0.625 A325N	0	0.625 A325X	0

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter r in	Weight klf
HB114-1-0813U4-M5J(1-1/4) *****	A	No	Ar (CaAa)	190.000 - 0.000	2.000	-0.45	4	4	0.850 0.750	1.540		0.001
LDF4-50A(1/2") *****	A	No	Ar (CaAa)	180.000 - 0.000	0.000	0.45	1	1	0.850 0.750	0.630		0.000
LDF7-50A(1-5/8") *****	A	No	Ar (CaAa)	180.000 - 0.000	0.000	0.41	8	8	0.850 0.750	1.980		0.001
LDF7-50A(1-5/8") *****	B	No	Ar (CaAa)	170.000 - 0.000	0.000	-0.4	9	9	0.850 75.000	1.980		0.001
FXL 1873 PE(1 5/8") *****	B	No	Ar (CaAa)	160.000 - 0.000	-2.000	0.45	6	3	0.850 0.750	1.980		0.000
AVA5-50(7/8") *****	C	No	Ar (CaAa)	153.000 - 0.000	0.000	0.4	8	8	0.850 0.750	1.102		0.000
FLC 78-50J(7/8")	C	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.45	12	12	0.850 0.750	1.112		0.000
FB-L98B-002-75000(3/8)	C	No	Ar (CaAa)	140.000 - 0.000	1.500	-0.46	1	1	0.300	0.394		0.000
WR-VG86ST-BRD(3/4)	C	No	Ar (CaAa)	140.000 - 0.000	1.500	-0.47	2	2	0.300	0.795		0.001
2" Rigid Conduit *****	C	No	Ar (CaAa)	140.000 - 0.000	0.000	-0.405	1	1	2.000	2.000		0.003
Feedline Ladder (Af)	A	No	Af (CaAa)	190.000 - 0.000	0.000	-0.45	1	1	3.000	3.000		0.008
Feedline Ladder (Af)	A	No	Af (CaAa)	180.000 - 0.000	0.000	0.41	1	1	3.000	3.000		0.008

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Face Offset in	Lateral Offset (Frac FW)	#	# Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight kif
***** Feedline Ladder (Af)	B	No	Af (CaAa)	170.000 - 0.000	0.000	-0.4	1	1	3.000	3.000		0.008
Feedline Ladder (Af)	B	No	Af (CaAa)	160.000 - 0.000	-1.000	0.45	1	1	3.000	3.000		0.008
***** Feedline Ladder (Af)	C	No	Af (CaAa)	150.000 - 0.000	0.000	0.4	1	1	3.000	3.000		0.008
Feedline Ladder (Af)	C	No	Af (CaAa)	140.000 - 0.000	0.000	-0.45	1	1	3.000	3.000		0.008
***** Thin Flat Bar Climbing Ladder	A	No	Af (CaAa)	192.000 - 0.000	-6.000	0.45	1	1	2.000	2.000		0.004
Safety Line 3/8	A	No	Ar (CaAa)	192.000 - 0.000	-6.000	0.45	1	1	0.375	0.375		0.000
*****												

### Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	CAAA ft <sup>2</sup> /ft	Weight kif
*****							

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	CAAA In Face ft <sup>2</sup>	CAAA Out Face ft <sup>2</sup>	Weight K
T1	192.000-180.000	A	0.000	0.000	15.610	0.000	0.183
		B	0.000	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000	0.000
T2	180.000-160.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	22.820	0.000	0.158
		C	0.000	0.000	0.000	0.000	0.000
T3	160.000-140.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	16.461	0.000	0.115
T4	140.000-120.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561
T5	120.000-100.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561
T6	100.000-80.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561
T7	80.000-60.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561
T8	60.000-40.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561
T9	40.000-20.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561
T10	20.000-0.000	A	0.000	0.000	72.677	0.000	0.651
		B	0.000	0.000	79.400	0.000	0.484
		C	0.000	0.000	72.287	0.000	0.561

**Feed Line/Linear Appurtenances Section Areas - With Ice**

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	192.000-180.000	A	2.378	0.000	0.000	44.197	0.000	0.929
		B		0.000	0.000	0.000	0.000	0.000
		C		0.000	0.000	0.000	0.000	0.000
T2	180.000-160.000	A	2.356	0.000	0.000	181.157	0.000	3.699
		B		0.000	0.000	47.320	0.000	0.968
		C		0.000	0.000	0.000	0.000	0.000
T3	160.000-140.000	A	2.327	0.000	0.000	180.198	0.000	3.651
		B		0.000	0.000	152.291	0.000	3.137
		C		0.000	0.000	42.568	0.000	0.794
T4	140.000-120.000	A	2.294	0.000	0.000	179.117	0.000	3.597
		B		0.000	0.000	151.597	0.000	3.095
		C		0.000	0.000	202.655	0.000	3.713
T5	120.000-100.000	A	2.256	0.000	0.000	177.875	0.000	3.535
		B		0.000	0.000	150.800	0.000	3.048
		C		0.000	0.000	201.319	0.000	3.648
T6	100.000-80.000	A	2.211	0.000	0.000	176.410	0.000	3.463
		B		0.000	0.000	149.860	0.000	2.992
		C		0.000	0.000	199.744	0.000	3.572
T7	80.000-60.000	A	2.156	0.000	0.000	174.618	0.000	3.375
		B		0.000	0.000	148.710	0.000	2.924
		C		0.000	0.000	197.817	0.000	3.480
T8	60.000-40.000	A	2.085	0.000	0.000	172.291	0.000	3.264
		B		0.000	0.000	147.216	0.000	2.837
		C		0.000	0.000	195.314	0.000	3.362
T9	40.000-20.000	A	1.981	0.000	0.000	168.906	0.000	3.104
		B		0.000	0.000	145.043	0.000	2.712
		C		0.000	0.000	191.676	0.000	3.194
T10	20.000-0.000	A	1.775	0.000	0.000	162.202	0.000	2.800
		B		0.000	0.000	140.738	0.000	2.470
		C		0.000	0.000	184.471	0.000	2.872

**Feed Line Center of Pressure**

Section	Elevation ft	CP <sub>x</sub> in	CP <sub>z</sub> in	CP <sub>x</sub> Ice in	CP <sub>z</sub> Ice in
T1	192.000-180.000	-3.243	0.243	-2.365	-1.087
T2	180.000-160.000	-2.129	-5.892	-1.892	-5.713
T3	160.000-140.000	-0.450	-4.328	-1.103	-4.736
T4	140.000-120.000	1.483	-2.018	0.986	-2.178
T5	120.000-100.000	1.679	-2.282	1.129	-2.511
T6	100.000-80.000	1.919	-2.604	1.311	-2.926
T7	80.000-60.000	2.115	-2.867	1.455	-3.251
T8	60.000-40.000	2.271	-3.078	1.589	-3.551
T9	40.000-20.000	2.485	-3.365	1.744	-3.884
T10	20.000-0.000	2.680	-3.628	1.900	-4.186

**Shielding Factor Ka**

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T1	2	HB114-1-0813U4-M5J(1-1/4)	180.00 - 190.00	0.6000	0.5011
T1	21	Feedline Ladder (Af)	180.00 - 190.00	0.6000	0.5011
T1	30	Thin Flat Bar Climbing Ladder	180.00 - 192.00	0.6000	0.5011
T1	31	Safety Line 3/8	180.00 - 192.00	0.6000	0.5011
T2	2	HB114-1-0813U4-M5J(1-1/4)	160.00 - 180.00	0.6000	0.5800
T2	7	LDF4-50A(1/2")	160.00 -	0.6000	0.5800

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>s</sub> No Ice	K <sub>s</sub> Ice
T2	8	LDF7-50A(1-5/8")	180.00 - 160.00	0.6000	0.5800
T2	10	LDF7-50A(1-5/8")	180.00 - 170.00	0.6000	0.5800
T2	21	Feedline Ladder (Af)	180.00 - 160.00	0.6000	0.5800
T2	22	Feedline Ladder (Af)	180.00 - 160.00	0.6000	0.5800
T2	24	Feedline Ladder (Af)	180.00 - 170.00	0.6000	0.5800
T2	30	Thin Flat Bar Climbing Ladder	180.00 - 160.00	0.6000	0.5800
T2	31	Safety Line 3/8	180.00 - 160.00	0.6000	0.5800
T3	2	HB114-1-0813U4-M5J(1-1/4)	160.00 - 140.00	0.6000	0.6000
T3	7	LDF4-50A(1/2")	160.00 - 140.00	0.6000	0.6000
T3	8	LDF7-50A(1-5/8")	160.00 - 140.00	0.6000	0.6000
T3	10	LDF7-50A(1-5/8")	160.00 - 140.00	0.6000	0.6000
T3	12	FXL 1873 PE(1 5/8")	160.00 - 140.00	0.6000	0.6000
T3	14	AVA5-50( 7/8")	153.00 - 140.00	0.6000	0.6000
T3	21	Feedline Ladder (Af)	160.00 - 140.00	0.6000	0.6000
T3	22	Feedline Ladder (Af)	160.00 - 140.00	0.6000	0.6000
T3	24	Feedline Ladder (Af)	160.00 - 140.00	0.6000	0.6000
T3	25	Feedline Ladder (Af)	160.00 - 140.00	0.6000	0.6000
T3	27	Feedline Ladder (Af)	160.00 - 150.00	0.6000	0.6000
T3	30	Thin Flat Bar Climbing Ladder	160.00 - 140.00	0.6000	0.6000
T3	31	Safety Line 3/8	160.00 - 140.00	0.6000	0.6000
T4	2	HB114-1-0813U4-M5J(1-1/4)	140.00 - 120.00	0.6000	0.6000
T4	7	LDF4-50A(1/2")	140.00 - 120.00	0.6000	0.6000
T4	8	LDF7-50A(1-5/8")	140.00 - 120.00	0.6000	0.6000
T4	10	LDF7-50A(1-5/8")	140.00 - 120.00	0.6000	0.6000
T4	12	FXL 1873 PE(1 5/8")	140.00 - 120.00	0.6000	0.6000
T4	14	AVA5-50( 7/8")	140.00 - 120.00	0.6000	0.6000
T4	16	FLC 78-50J(7/8")	140.00 - 120.00	0.6000	0.6000
T4	17	FB-L98B-002-75000(3/8)	140.00 - 120.00	0.0000	0.0000
T4	18	WR-VG86ST-BRD(3/4)	140.00 - 120.00	0.0000	0.0000
T4	19	2" Rigid Conduit	140.00 - 120.00	0.6000	0.6000
T4	21	Feedline Ladder (Af)	140.00 - 120.00	0.6000	0.6000
T4	22	Feedline Ladder (Af)	140.00 - 120.00	0.6000	0.6000
T4	24	Feedline Ladder (Af)	140.00 - 120.00	0.6000	0.6000
T4	25	Feedline Ladder (Af)	140.00 - 120.00	0.6000	0.6000



Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T4	27	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T4	28	Feedline Ladder (Af)	120.00 - 140.00	0.6000	0.6000
T4	30	Thin Flat Bar Climbing Ladder	120.00 - 140.00	0.6000	0.6000
T4	31	Safety Line 3/8	120.00 - 140.00	0.6000	0.6000
T5	2	HB114-1-0813U4-M5J(1-1/4)	100.00 - 120.00	0.6000	0.6000
T5	7	LDF4-50A(1/2")	100.00 - 120.00	0.6000	0.6000
T5	8	LDF7-50A(1-5/8")	100.00 - 120.00	0.6000	0.6000
T5	10	LDF7-50A(1-5/8")	100.00 - 120.00	0.6000	0.6000
T5	12	FXL 1873 PE(1 5/8")	100.00 - 120.00	0.6000	0.6000
T5	14	AVA5-50( 7/8")	100.00 - 120.00	0.6000	0.6000
T5	16	FLC 78-50J(7/8")	100.00 - 120.00	0.6000	0.6000
T5	17	FB-L98B-002-75000(3/8)	100.00 - 120.00	0.0000	0.0000
T5	18	WR-VG86ST-BRD(3/4)	100.00 - 120.00	0.0000	0.0000
T5	19	2" Rigid Conduit	100.00 - 120.00	0.6000	0.6000
T5	21	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T5	22	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T5	24	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T5	25	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T5	27	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T5	28	Feedline Ladder (Af)	100.00 - 120.00	0.6000	0.6000
T5	30	Thin Flat Bar Climbing Ladder	100.00 - 120.00	0.6000	0.6000
T5	31	Safety Line 3/8	100.00 - 120.00	0.6000	0.6000
T6	2	HB114-1-0813U4-M5J(1-1/4)	80.00 - 100.00	0.6000	0.6000
T6	7	LDF4-50A(1/2")	80.00 - 100.00	0.6000	0.6000
T6	8	LDF7-50A(1-5/8")	80.00 - 100.00	0.6000	0.6000
T6	10	LDF7-50A(1-5/8")	80.00 - 100.00	0.6000	0.6000
T6	12	FXL 1873 PE(1 5/8")	80.00 - 100.00	0.6000	0.6000
T6	14	AVA5-50( 7/8")	80.00 - 100.00	0.6000	0.6000
T6	16	FLC 78-50J(7/8")	80.00 - 100.00	0.6000	0.6000
T6	17	FB-L98B-002-75000(3/8)	80.00 - 100.00	0.0000	0.0000
T6	18	WR-VG86ST-BRD(3/4)	80.00 - 100.00	0.0000	0.0000
T6	19	2" Rigid Conduit	80.00 - 100.00	0.6000	0.6000
T6	21	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	22	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	24	Feedline Ladder (Af)	80.00 -	0.6000	0.6000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T6	25	Feedline Ladder (Af)	100.00 - 80.00	0.6000	0.6000
T6	27	Feedline Ladder (Af)	100.00 - 80.00	0.6000	0.6000
T6	28	Feedline Ladder (Af)	100.00 - 80.00	0.6000	0.6000
T6	30	Thin Flat Bar Climbing Ladder	100.00 - 80.00	0.6000	0.6000
T6	31	Safety Line 3/8	100.00 - 80.00	0.6000	0.6000
T7	2	HB114-1-0813U4-M5J(1-1/4)	100.00 - 60.00	0.6000	0.6000
T7	7	LDF4-50A(1/2")	80.00 - 60.00	0.6000	0.6000
T7	8	LDF7-50A(1-5/8")	80.00 - 60.00	0.6000	0.6000
T7	10	LDF7-50A(1-5/8")	80.00 - 60.00	0.6000	0.6000
T7	12	FXL 1873 PE(1 5/8")	80.00 - 60.00	0.6000	0.6000
T7	14	AVA5-50( 7/8")	80.00 - 60.00	0.6000	0.6000
T7	16	FLC 78-50J(7/8")	80.00 - 60.00	0.6000	0.6000
T7	17	FB-L98B-002-75000(3/8)	80.00 - 60.00	0.0000	0.0000
T7	18	WR-VG86ST-BRD(3/4)	80.00 - 60.00	0.0000	0.0000
T7	19	2" Rigid Conduit	80.00 - 60.00	0.6000	0.6000
T7	21	Feedline Ladder (Af)	80.00 - 60.00	0.6000	0.6000
T7	22	Feedline Ladder (Af)	80.00 - 60.00	0.6000	0.6000
T7	24	Feedline Ladder (Af)	80.00 - 60.00	0.6000	0.6000
T7	25	Feedline Ladder (Af)	80.00 - 60.00	0.6000	0.6000
T7	27	Feedline Ladder (Af)	80.00 - 60.00	0.6000	0.6000
T7	28	Feedline Ladder (Af)	80.00 - 60.00	0.6000	0.6000
T7	30	Thin Flat Bar Climbing Ladder	80.00 - 60.00	0.6000	0.6000
T7	31	Safety Line 3/8	80.00 - 60.00	0.6000	0.6000
T8	2	HB114-1-0813U4-M5J(1-1/4)	60.00 - 40.00	0.6000	0.6000
T8	7	LDF4-50A(1/2")	60.00 - 40.00	0.6000	0.6000
T8	8	LDF7-50A(1-5/8")	60.00 - 40.00	0.6000	0.6000
T8	10	LDF7-50A(1-5/8")	60.00 - 40.00	0.6000	0.6000
T8	12	FXL 1873 PE(1 5/8")	60.00 - 40.00	0.6000	0.6000
T8	14	AVA5-50( 7/8")	60.00 - 40.00	0.6000	0.6000
T8	16	FLC 78-50J(7/8")	60.00 - 40.00	0.6000	0.6000
T8	17	FB-L98B-002-75000(3/8)	60.00 - 40.00	0.0000	0.0000
T8	18	WR-VG86ST-BRD(3/4)	60.00 - 40.00	0.0000	0.0000
T8	19	2" Rigid Conduit	60.00 - 40.00	0.6000	0.6000
T8	21	Feedline Ladder (Af)	60.00 - 40.00	0.6000	0.6000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T8	22	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	24	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	25	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	27	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	28	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	30	Thin Flat Bar Climbing Ladder	40.00 - 60.00	0.6000	0.6000
T8	31	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T9	2	HB114-1-0813U4-M5J(1-1/4)	20.00 - 40.00	0.6000	0.6000
T9	7	LDF4-50A(1/2")	20.00 - 40.00	0.6000	0.6000
T9	8	LDF7-50A(1-5/8")	20.00 - 40.00	0.6000	0.6000
T9	10	LDF7-50A(1-5/8")	20.00 - 40.00	0.6000	0.6000
T9	12	FXL 1873 PE(1 5/8")	20.00 - 40.00	0.6000	0.6000
T9	14	AVA5-50( 7/8")	20.00 - 40.00	0.6000	0.6000
T9	16	FLC 78-50J(7/8")	20.00 - 40.00	0.6000	0.6000
T9	17	FB-L98B-002-75000(3/8)	20.00 - 40.00	0.0000	0.0000
T9	18	WR-VG86ST-BRD(3/4)	20.00 - 40.00	0.0000	0.0000
T9	19	2" Rigid Conduit	20.00 - 40.00	0.6000	0.6000
T9	21	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	22	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	24	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	25	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	27	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	28	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	30	Thin Flat Bar Climbing Ladder	20.00 - 40.00	0.6000	0.6000
T9	31	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T10	2	HB114-1-0813U4-M5J(1-1/4)	0.00 - 20.00	0.6000	0.6000
T10	7	LDF4-50A(1/2")	0.00 - 20.00	0.6000	0.6000
T10	8	LDF7-50A(1-5/8")	0.00 - 20.00	0.6000	0.6000
T10	10	LDF7-50A(1-5/8")	0.00 - 20.00	0.6000	0.6000
T10	12	FXL 1873 PE(1 5/8")	0.00 - 20.00	0.6000	0.6000
T10	14	AVA5-50( 7/8")	0.00 - 20.00	0.6000	0.6000
T10	16	FLC 78-50J(7/8")	0.00 - 20.00	0.6000	0.6000
T10	17	FB-L98B-002-75000(3/8)	0.00 - 20.00	0.0000	0.0000
T10	18	WR-VG86ST-BRD(3/4)	0.00 - 20.00	0.0000	0.0000
T10	19	2" Rigid Conduit	0.00 - 20.00	0.6000	0.6000
T10	21	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	22	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	24	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	25	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	27	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	28	Feedline Ladder (Af)	0.00 - 20.00	0.6000	0.6000
T10	30	Thin Flat Bar Climbing Ladder	0.00 - 20.00	0.6000	0.6000

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K <sub>a</sub> No Ice	K <sub>a</sub> Ice
T10	31	Safety Line 3/8	0.00 - 20.00	0.6000	0.6000

### Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets:			Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight
			Horz	Lateral	Vert					
			ft	ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K
NNVV-65B-R4 w/ Mount Pipe	A	From Leg	4.000	0.000	190.000	No Ice	12.509	7.413	0.103	
			0.000			1/2"	13.108	8.598	0.194	
			2.000			Ice	13.672	9.496	0.293	
						1" Ice				
NNVV-65B-R4 w/ Mount Pipe	B	From Leg	4.000	0.000	190.000	No Ice	12.509	7.413	0.103	
			0.000			1/2"	13.108	8.598	0.194	
			2.000			Ice	13.672	9.496	0.293	
						1" Ice				
NNVV-65B-R4 w/ Mount Pipe	C	From Leg	4.000	0.000	190.000	No Ice	12.509	7.413	0.103	
			0.000			1/2"	13.108	8.598	0.194	
			2.000			Ice	13.672	9.496	0.293	
						1" Ice				
APXVTM14-ALU-I20 w/ Mount Pipe	A	From Leg	4.000	0.000	190.000	No Ice	6.580	4.959	0.077	
			0.000			1/2"	7.031	5.754	0.132	
			2.000			Ice	7.473	6.472	0.193	
						1" Ice				
APXVTM14-ALU-I20 w/ Mount Pipe	B	From Leg	4.000	0.000	190.000	No Ice	6.580	4.959	0.077	
			0.000			1/2"	7.031	5.754	0.132	
			2.000			Ice	7.473	6.472	0.193	
						1" Ice				
APXVTM14-ALU-I20 w/ Mount Pipe	C	From Leg	4.000	0.000	190.000	No Ice	6.580	4.959	0.077	
			0.000			1/2"	7.031	5.754	0.132	
			2.000			Ice	7.473	6.472	0.193	
						1" Ice				
TD-RRH8x20-25	A	From Leg	4.000	0.000	190.000	No Ice	4.045	1.535	0.070	
			0.000			1/2"	4.298	1.714	0.097	
			2.000			Ice	4.557	1.901	0.128	
						1" Ice				
TD-RRH8x20-25	B	From Leg	4.000	0.000	190.000	No Ice	4.045	1.535	0.070	
			0.000			1/2"	4.298	1.714	0.097	
			2.000			Ice	4.557	1.901	0.128	
						1" Ice				
TD-RRH8x20-25	C	From Leg	4.000	0.000	190.000	No Ice	4.045	1.535	0.070	
			0.000			1/2"	4.298	1.714	0.097	
			2.000			Ice	4.557	1.901	0.128	
						1" Ice				
PCS 1900MHz 4x45W-65MHz	A	From Leg	4.000	0.000	190.000	No Ice	2.322	2.238	0.060	
			0.000			1/2"	2.527	2.441	0.083	
			2.000			Ice	2.739	2.651	0.110	
						1" Ice				
PCS 1900MHz 4x45W-65MHz	B	From Leg	4.000	0.000	190.000	No Ice	2.322	2.238	0.060	
			0.000			1/2"	2.527	2.441	0.083	
			2.000			Ice	2.739	2.651	0.110	
						1" Ice				
PCS 1900MHz 4x45W-65MHz	C	From Leg	4.000	0.000	190.000	No Ice	2.322	2.238	0.060	
			0.000			1/2"	2.527	2.441	0.083	
			2.000			Ice	2.739	2.651	0.110	
						1" Ice				
(2) RRH2X50-800	A	From Leg	4.000	0.000	190.000	No Ice	1.701	1.282	0.053	
			0.000			1/2"	1.864	1.428	0.070	
			2.000			Ice	2.035	1.580	0.090	
						1" Ice				
(2) RRH2X50-800	B	From Leg	4.000	0.000	190.000	No Ice	1.701	1.282	0.053	
			0.000			1/2"	1.864	1.428	0.070	
			2.000			Ice	2.035	1.580	0.090	
						1" Ice				

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustment	Placement ft		C <sub>AA</sub> Front ft <sup>2</sup>	C <sub>AA</sub> Side ft <sup>2</sup>	Weight K
(2) RRH2X50-800	C	From Leg	4.000 0.000 2.000	0.000	190.000	No Ice 1/2" Ice 1" Ice	1.701 1.864 2.035	1.282 1.428 1.580	0.053 0.070 0.090
Sector Mount [SM 506-3]	C	None		0.000	190.000	No Ice 1/2" Ice 1" Ice	35.470 50.600 65.730	35.470 50.600 65.730	1.742 2.348 2.953
***** 58532A	C	From Leg	4.000 0.000 4.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	0.189 0.248 0.315	0.189 0.248 0.315	0.000 0.003 0.006
(2) LPA-80080/4CF	A	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	2.619 2.922 3.232	5.399 5.726 6.061	0.012 0.045 0.083
(2) LPA-80080/4CF	B	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	2.619 2.922 3.232	5.399 5.726 6.061	0.012 0.045 0.083
(2) LPA-80080/4CF	C	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	2.619 2.922 3.232	5.399 5.726 6.061	0.012 0.045 0.083
(2) JAHH-65B-R3B	A	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	9.113 9.579 10.052	5.983 6.442 6.909	0.061 0.119 0.183
(2) JAHH-65B-R3B	B	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	9.113 9.579 10.052	5.983 6.442 6.909	0.061 0.119 0.183
(2) JAHH-65B-R3B	C	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	9.113 9.579 10.052	5.983 6.442 6.909	0.061 0.119 0.183
BAND 5 AHCA RRH4X40	A	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	1.313 1.456 1.607	0.746 0.860 0.982	0.040 0.052 0.066
BAND 5 AHCA RRH4X40	B	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	1.313 1.456 1.607	0.746 0.860 0.982	0.040 0.052 0.066
BAND 5 AHCA RRH4X40	C	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	1.313 1.456 1.607	0.746 0.860 0.982	0.040 0.052 0.066
RRH2X60-700	A	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	3.500 3.761 4.029	1.816 2.052 2.289	0.060 0.083 0.109
RRH2X60-700	B	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	3.500 3.761 4.029	1.816 2.052 2.289	0.060 0.083 0.109
RRH2X60-700	C	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	3.500 3.761 4.029	1.816 2.052 2.289	0.060 0.083 0.109
RRH4X45-AWS4 B66	A	From Leg	4.000 0.000 1.000	0.000	180.000	No Ice 1/2" Ice 1" Ice	2.660 2.878 3.104	1.586 1.769 1.959	0.064 0.084 0.108

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>A</sub> A <sub>A</sub> Front	C <sub>A</sub> A <sub>A</sub> Side	Weight	
			Horz	Lateral Vert						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
RRH4X45-AWS4 B66	B	From Leg	4.000	0.000	0.000	180.000	No Ice	2.660	1.586	0.064
			0.000				1/2"	2.878	1.769	0.084
			1.000				Ice	3.104	1.959	0.108
							1" Ice			
RRH4X45-AWS4 B66	C	From Leg	4.000	0.000	0.000	180.000	No Ice	2.660	1.586	0.064
			0.000				1/2"	2.878	1.769	0.084
			1.000				Ice	3.104	1.959	0.108
							1" Ice			
(2) RC3DC-3315-PF-48	A	From Leg	4.000	0.000	0.000	180.000	No Ice	3.792	2.512	0.032
			0.000				1/2"	4.044	2.725	0.063
			1.000				Ice	4.303	2.945	0.099
							1" Ice			
Sector Mount [SM 304-3]	C	None			0.000	180.000	No Ice	44.820	44.820	1.920
							1/2"	63.480	63.480	2.772
							Ice	82.140	82.140	3.624
							1" Ice			
*****										
(3) 7130.16.33.00 w/ Mount Pipe	A	From Leg	4.000	0.000	0.000	170.000	No Ice	5.555	6.584	0.037
			0.000				1/2"	5.968	7.295	0.096
			2.000				Ice	6.382	7.978	0.162
							1" Ice			
(3) 7130.16.33.00 w/ Mount Pipe	B	From Leg	4.000	0.000	0.000	170.000	No Ice	5.555	6.584	0.037
			0.000				1/2"	5.968	7.295	0.096
			2.000				Ice	6.382	7.978	0.162
							1" Ice			
(3) 7130.16.33.00 w/ Mount Pipe	C	From Leg	4.000	0.000	0.000	170.000	No Ice	5.555	6.584	0.037
			0.000				1/2"	5.968	7.295	0.096
			2.000				Ice	6.382	7.978	0.162
							1" Ice			
Sector Mount [SM 502-3]	C	None			0.000	170.000	No Ice	33.020	33.020	1.673
							1/2"	47.360	47.360	2.224
							Ice	61.700	61.700	2.775
							1" Ice			
*****										
HBX-6516DS-VTM w/ Mount Pipe	A	From Leg	4.000	0.000	0.000	160.000	No Ice	3.598	3.241	0.029
			0.000				1/2"	3.998	3.914	0.062
			0.000				Ice	4.389	4.564	0.101
							1" Ice			
HBX-6516DS-VTM w/ Mount Pipe	B	From Leg	4.000	0.000	0.000	160.000	No Ice	3.598	3.241	0.029
			0.000				1/2"	3.998	3.914	0.062
			0.000				Ice	4.389	4.564	0.101
							1" Ice			
HBX-6516DS-VTM w/ Mount Pipe	C	From Leg	4.000	0.000	0.000	160.000	No Ice	3.598	3.241	0.029
			0.000				1/2"	3.998	3.914	0.062
			0.000				Ice	4.389	4.564	0.101
							1" Ice			
6' x 2" Mount Pipe	A	From Leg	4.000	0.000	0.000	160.000	No Ice	1.425	1.425	0.022
			0.000				1/2"	1.925	1.925	0.033
			0.000				Ice	2.294	2.294	0.048
							1" Ice			
6' x 2" Mount Pipe	B	From Leg	4.000	0.000	0.000	160.000	No Ice	1.425	1.425	0.022
			0.000				1/2"	1.925	1.925	0.033
			0.000				Ice	2.294	2.294	0.048
							1" Ice			
6' x 2" Mount Pipe	C	From Leg	4.000	0.000	0.000	160.000	No Ice	1.425	1.425	0.022
			0.000				1/2"	1.925	1.925	0.033
			0.000				Ice	2.294	2.294	0.048
							1" Ice			
Sector Mount [SM 104-3]	C	None			0.000	160.000	No Ice	30.020	30.020	0.953
							1/2"	40.480	40.480	1.405
							Ice	50.940	50.940	1.857
							1" Ice			
*****										
RR90-17-02DP w/ Mount Pipe	A	From Leg	3.000	0.000	0.000	153.000	No Ice	4.593	3.319	0.034
			0.000				1/2"	5.018	4.089	0.072

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert ft ft ft	Azimuth Adjustmen t °	Placement ft	Ice	C <sub>A</sub> A <sub>1</sub> Front ft <sup>2</sup>	C <sub>A</sub> A <sub>1</sub> Side ft <sup>2</sup>	Weight K
			0.000			Ice	5.436	4.784	0.115
RR90-17-02DP w/ Mount Pipe	C	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	4.593	3.319	0.034
						No Ice	5.018	4.089	0.072
						1/2"	5.436	4.784	0.115
LNX-6515DS-VTM w/ Mount Pipe	A	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	11.683	9.842	0.083
						No Ice	12.404	11.366	0.173
						1/2"	13.135	12.914	0.273
LNX-6515DS-VTM w/ Mount Pipe	C	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	11.683	9.842	0.083
						No Ice	12.404	11.366	0.173
						1/2"	13.135	12.914	0.273
KRY 112 144/1	A	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	0.350	0.175	0.011
						No Ice	0.426	0.234	0.014
						1/2"	0.509	0.301	0.019
KRY 112 144/1	C	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	0.350	0.175	0.011
						No Ice	0.426	0.234	0.014
						1/2"	0.509	0.301	0.019
ATBT-BOTTOM-24V	A	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	0.104	0.065	0.003
						No Ice	0.148	0.102	0.004
						1/2"	0.199	0.147	0.006
ATBT-BOTTOM-24V	C	From Leg	3.000 0.000 0.000	0.000	153.000	1" Ice	0.104	0.065	0.003
						No Ice	0.148	0.102	0.004
						1/2"	0.199	0.147	0.006
Side Arm Mount [SO 301-1]	A	From Leg	1.500 0.000 0.000	0.000	153.000	1" Ice	1.000	0.900	0.023
						No Ice	1.390	1.420	0.033
						1/2"	1.780	1.940	0.042
Side Arm Mount [SO 301-1]	C	From Leg	1.500 0.000 0.000	0.000	153.000	1" Ice	1.000	0.900	0.023
						No Ice	1.390	1.420	0.033
						1/2"	1.780	1.940	0.042
*****									
(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	A	From Leg	4.000 0.000 1.000	0.000	140.000	1" Ice	8.262	6.304	0.074
						No Ice	8.822	7.479	0.139
						1/2"	9.346	8.368	0.212
(2) AM-X-CD-14-65-00T-RET w/ Mount Pipe	B	From Leg	4.000 0.000 1.000	0.000	140.000	1" Ice	5.232	4.015	0.035
						No Ice	5.618	4.633	0.080
						1/2"	6.012	5.257	0.131
(2) AM-X-CD-14-65-00T-RET w/ Mount Pipe	C	From Leg	4.000 0.000 1.000	0.000	140.000	1" Ice	5.232	4.015	0.035
						No Ice	5.618	4.633	0.080
						1/2"	6.012	5.257	0.131
800 10121 w/ Mount Pipe	A	From Leg	4.000 0.000 1.000	0.000	140.000	1" Ice	5.388	4.600	0.066
						No Ice	5.813	5.351	0.114
						1/2"	6.234	6.046	0.168
800 10121 w/ Mount Pipe	B	From Leg	4.000 0.000 1.000	0.000	140.000	1" Ice	5.388	4.600	0.066
						No Ice	5.813	5.351	0.114
						1/2"	6.234	6.046	0.168
800 10121 w/ Mount Pipe	C	From Leg	4.000 0.000 1.000	0.000	140.000	1" Ice	5.388	4.600	0.066
						No Ice	5.813	5.351	0.114
						1/2"	6.234	6.046	0.168
DC6-48-60-18-8F	A	From Leg	4.000 0.000	0.000	140.000	1" Ice	0.791	0.791	0.020
						No Ice	1.274	1.274	0.035

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub>		Weight	
			Horz Lateral	Vert			Front	Side		
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
				1.000				1.450	1.450	0.053
(2) RRUS-11	A	From Leg		4.000	0.000	140.000	Ice	2.784	1.187	0.048
				0.000			No Ice	2.992	1.334	0.068
				1.000			1/2"	3.207	1.490	0.092
(2) RRUS-11	B	From Leg		4.000	0.000	140.000	Ice	2.784	1.187	0.048
				0.000			No Ice	2.992	1.334	0.068
				1.000			1/2"	3.207	1.490	0.092
(2) RRUS-11	C	From Leg		4.000	0.000	140.000	Ice	2.784	1.187	0.048
				0.000			No Ice	2.992	1.334	0.068
				1.000			1/2"	3.207	1.490	0.092
7020.00	A	From Leg		4.000	0.000	140.000	Ice	0.102	0.175	0.002
				0.000			No Ice	0.147	0.239	0.005
				1.000			1/2"	0.199	0.311	0.009
7020.00	B	From Leg		4.000	0.000	140.000	Ice	0.102	0.175	0.002
				0.000			No Ice	0.147	0.239	0.005
				1.000			1/2"	0.199	0.311	0.009
7020.00	C	From Leg		4.000	0.000	140.000	Ice	0.102	0.175	0.002
				0.000			No Ice	0.147	0.239	0.005
				1.000			1/2"	0.199	0.311	0.009
LGP13519	A	From Leg		4.000	0.000	140.000	Ice	0.290	0.181	0.005
				0.000			No Ice	0.362	0.241	0.008
				1.000			1/2"	0.441	0.310	0.012
LGP13519	B	From Leg		4.000	0.000	140.000	Ice	0.290	0.181	0.005
				0.000			No Ice	0.362	0.241	0.008
				1.000			1/2"	0.441	0.310	0.012
LGP13519	C	From Leg		4.000	0.000	140.000	Ice	0.290	0.181	0.005
				0.000			No Ice	0.362	0.241	0.008
				1.000			1/2"	0.441	0.310	0.012
DTMABP7819VG12A	A	From Leg		4.000	0.000	140.000	Ice	0.976	0.339	0.019
				0.000			No Ice	1.100	0.419	0.026
				1.000			1/2"	1.232	0.510	0.036
DTMABP7819VG12A	B	From Leg		4.000	0.000	140.000	Ice	0.976	0.339	0.019
				0.000			No Ice	1.100	0.419	0.026
				1.000			1/2"	1.232	0.510	0.036
DTMABP7819VG12A	C	From Leg		4.000	0.000	140.000	Ice	0.976	0.339	0.019
				0.000			No Ice	1.100	0.419	0.026
				1.000			1/2"	1.232	0.510	0.036
Sector Mount [SM 504-3]	C	None			0.000	140.000	Ice	34.250	34.250	1.708
							No Ice	48.980	48.980	2.286
							1/2"	63.710	63.710	2.864
***** 58532A	C	From Leg		3.000	0.000	98.000	Ice	0.189	0.189	0.000
				0.000			No Ice	0.248	0.248	0.003
				4.000			1/2"	0.315	0.315	0.006
Side Arm Mount [SO 301-1]	C	From Leg		1.500	0.000	98.000	Ice	1.000	0.900	0.023
				0.000			No Ice	1.390	1.420	0.033
				0.000			1/2"	1.780	1.940	0.042
*****							Ice			



### Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 30 deg - No Ice
5	0.9 Dead+1.6 Wind 30 deg - No Ice
6	1.2 Dead+1.6 Wind 60 deg - No Ice
7	0.9 Dead+1.6 Wind 60 deg - No Ice
8	1.2 Dead+1.6 Wind 90 deg - No Ice
9	0.9 Dead+1.6 Wind 90 deg - No Ice
10	1.2 Dead+1.6 Wind 120 deg - No Ice
11	0.9 Dead+1.6 Wind 120 deg - No Ice
12	1.2 Dead+1.6 Wind 150 deg - No Ice
13	0.9 Dead+1.6 Wind 150 deg - No Ice
14	1.2 Dead+1.6 Wind 180 deg - No Ice
15	0.9 Dead+1.6 Wind 180 deg - No Ice
16	1.2 Dead+1.6 Wind 210 deg - No Ice
17	0.9 Dead+1.6 Wind 210 deg - No Ice
18	1.2 Dead+1.6 Wind 240 deg - No Ice
19	0.9 Dead+1.6 Wind 240 deg - No Ice
20	1.2 Dead+1.6 Wind 270 deg - No Ice
21	0.9 Dead+1.6 Wind 270 deg - No Ice
22	1.2 Dead+1.6 Wind 300 deg - No Ice
23	0.9 Dead+1.6 Wind 300 deg - No Ice
24	1.2 Dead+1.6 Wind 330 deg - No Ice
25	0.9 Dead+1.6 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	192 - 180	Leg	Max Tension	23	5.743	-0.106	-0.065
			Max. Compression	18	-8.388	0.079	-0.058
			Max. Mx	20	-1.251	-1.008	0.002
			Max. My	2	-0.722	-0.035	-1.002
			Max. Vy	20	-0.741	0.485	-0.041
		Diagonal	Max. Vx	2	-0.749	-0.004	0.510
			Max Tension	4	1.844	0.000	0.000
			Max. Compression	4	-1.838	0.000	0.000
			Max. Mx	36	0.239	0.031	0.000

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft			
T2	180 - 160	Top Girt	Max. My	16	1.821	0.004	-0.002			
			Max. Vy	36	-0.036	0.031	0.000			
			Max. Vx	16	-0.001	0.004	-0.002			
		Leg	Max Tension	14	0.131	0.000	0.000			
			Max. Compression	3	-0.119	0.000	0.000			
			Max. Mx	26	-0.006	-0.091	0.000			
			Max. Vy	26	0.055	0.000	0.000			
			Max Tension	23	32.760	-0.104	-0.013			
			Max. Compression	2	-40.997	0.126	0.000			
			Max. Mx	10	-40.123	0.131	0.013			
			Max. My	20	-3.111	-0.017	-0.153			
			Max. Vy	14	-1.500	-0.071	-0.005			
			Diagonal	Max. Vx	8	1.498	0.011	-0.030		
				Max Tension	16	4.758	0.000	0.000		
Max. Compression	16	-4.766		0.000	0.000					
Max. Mx	27	1.402		0.058	0.006					
Top Girt	Max. My	28	-1.731	0.029	0.007					
	Max. Vy	27	-0.048	0.058	0.006					
	Max. Vx	28	-0.003	0.000	0.000					
	Max Tension	3	0.977	0.000	0.000					
	Max. Compression	14	-1.010	0.000	0.000					
	Max. Mx	26	-0.060	-0.097	0.000					
	Max. My	26	-0.056	0.000	0.003					
	Max. Vy	26	0.059	0.000	0.000					
	Max. Vx	26	-0.002	0.000	0.000					
	T3	160 - 140	Leg	Max Tension	23	63.875	-0.177	-0.009		
Max. Compression				2	-75.920	0.175	-0.007			
Max. Mx				14	39.613	-0.206	-0.027			
Diagonal			Max. My	20	-5.962	-0.002	-0.266			
			Max. Vy	14	-0.532	-0.104	-0.001			
			Max. Vx	19	-0.549	-0.114	-0.186			
			Max Tension	17	6.575	0.000	0.000			
			Max. Compression	16	-6.691	0.000	0.000			
			Max. Mx	27	1.534	0.106	-0.013			
			Max. My	36	1.668	0.100	-0.014			
			Max. Vy	37	0.072	0.093	0.013			
			Max. Vx	36	0.004	0.000	0.000			
			T4	140 - 120	Leg	Max Tension	23	98.702	-0.241	-0.008
						Max. Compression	2	-116.623	0.388	-0.002
Max. Mx	2	-116.623				0.388	-0.002			
Diagonal	Max. My	20			-8.769	0.009	-0.361			
	Max. Vy	14			-1.046	-0.181	0.006			
	Max. Vx	8			1.029	-0.008	0.069			
	Max Tension	16			7.848	0.000	0.000			
	Max. Compression	16			-7.908	0.000	0.000			
	Max. Mx	27			2.008	0.130	-0.016			
	Max. My	35			1.556	0.123	-0.017			
	Max. Vy	37			0.086	0.127	0.016			
	Max. Vx	35			0.005	0.000	0.000			
	T5	120 - 100			Leg	Max Tension	23	132.529	-0.365	-0.007
						Max. Compression	2	-155.166	0.798	0.002
Max. Mx			3	-152.495		0.798	0.002			
Diagonal			Max. My	20	-10.487	-0.001	-0.645			
			Max. Vy	19	-0.125	0.797	-0.020			
			Max. Vx	8	-0.123	0.001	0.644			
			Max Tension	16	8.434	0.000	0.000			
			Max. Compression	16	-8.418	0.000	0.000			
			Max. Mx	27	2.513	0.181	-0.022			
			Max. My	29	-2.375	0.162	0.024			
			Max. Vy	37	0.111	0.180	-0.023			
			Max. Vx	29	-0.006	0.000	0.000			
			T6	100 - 80	Leg	Max Tension	7	162.115	-0.660	0.010
						Max. Compression	2	-189.128	0.971	-0.011
Max. Mx	2	-189.128				0.971	-0.011			
Diagonal	Max. My	4			-10.491	-0.056	-1.084			
	Max. Vy	18			-0.139	0.971	-0.014			
	Max. Vx	20			-0.160	-0.055	-1.083			
	Max Tension	16			9.769	0.000	0.000			
	Max. Compression	16			-9.824	0.000	0.000			

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T7	80 - 60	Leg	Max. Mx	27	2.741	0.289	-0.038
			Max. My	36	2.626	0.282	-0.039
			Max. Vy	29	0.141	0.284	-0.037
			Max. Vx	36	0.008	0.000	0.000
			Max Tension	7	194.226	-0.617	0.007
			Max. Compression	2	-226.462	1.310	-0.009
			Max. Mx	2	-226.462	1.310	-0.009
		Diagonal	Max. My	20	-14.136	0.030	-1.056
			Max. Vy	18	-0.174	1.308	-0.021
			Max. Vx	20	0.135	-0.062	-0.864
			Max Tension	16	10.629	0.000	0.000
			Max. Compression	16	-10.765	0.000	0.000
			Max. Mx	27	2.942	0.373	-0.045
			Max. My	36	2.794	0.365	-0.046
T8	60 - 40	Leg	Max. Vy	29	0.171	0.368	-0.044
			Max. Vx	36	0.009	0.000	0.000
			Max Tension	7	223.513	-1.453	0.009
			Max. Compression	2	-261.827	1.231	-0.008
			Max. Mx	37	18.277	-2.093	-0.027
			Max. My	4	-14.837	-0.073	-1.346
			Max. Vy	33	0.314	-2.081	0.009
		Diagonal	Max. Vx	20	0.144	-0.069	-1.342
			Max Tension	16	10.316	0.000	0.000
			Max. Compression	16	-10.319	0.000	0.000
			Max. Mx	29	2.667	0.450	0.063
			Max. My	29	-2.415	0.411	0.065
			Max. Vy	29	0.197	0.450	0.063
			Max. Vx	29	-0.011	0.000	0.000
T9	40 - 20	Leg	Max Tension	7	253.031	-1.292	0.009
			Max. Compression	2	-297.486	2.086	-0.009
			Max. Mx	37	19.982	-4.211	-0.012
			Max. My	4	-17.229	-0.120	-1.667
			Max. Vy	33	0.683	-4.193	0.003
			Max. Vx	20	-0.209	-0.116	-1.660
			Max Tension	16	11.967	0.000	0.000
		Diagonal	Max. Compression	16	-12.234	0.000	0.000
			Max. Mx	29	1.740	0.518	-0.056
			Max. My	30	4.237	0.476	0.062
			Max. Vy	29	0.204	0.478	-0.060
			Max. Vx	30	-0.010	0.000	0.000
			Max Tension	7	281.913	-1.340	0.016
			Max. Compression	2	-333.211	0.000	-0.000
T10	20 - 0	Leg	Max. Mx	27	-174.224	4.256	0.017
			Max. My	4	-19.965	-0.198	-3.067
			Max. Vy	33	-0.815	-4.193	0.003
			Max. Vx	20	-0.409	-0.196	-3.055
			Max Tension	12	12.437	0.000	0.000
			Max. Compression	12	-12.710	0.000	0.000
			Max. Mx	29	-0.093	0.648	0.065
		Diagonal	Max. My	30	5.755	0.481	0.076
			Max. Vy	29	0.220	0.648	0.065
			Max. Vx	30	-0.011	0.000	0.000

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg C	Max. Vert	18	340.216	33.588	-19.767
	Max. H <sub>x</sub>	18	340.216	33.588	-19.767
	Max. H <sub>z</sub>	7	-289.016	-29.070	17.114
	Min. Vert	7	-289.016	-29.070	17.114
	Min. H <sub>x</sub>	7	-289.016	-29.070	17.114
	Min. H <sub>z</sub>	18	340.216	33.588	-19.767
Leg B	Max. Vert	10	339.488	-33.522	-19.766
	Max. H <sub>x</sub>	23	-288.475	29.010	17.108

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg A	Max. H <sub>z</sub>	23	-288.475	29.010	17.108
	Min. Vert	23	-288.475	29.010	17.108
	Min. H <sub>x</sub>	10	339.488	-33.522	-19.766
	Min. H <sub>z</sub>	10	339.488	-33.522	-19.766
	Max. Vert	2	342.098	0.032	39.047
	Max. H <sub>x</sub>	21	16.639	4.731	1.391
	Max. H <sub>z</sub>	2	342.098	0.032	39.047
	Min. Vert	15	-288.713	-0.025	-33.767
	Min. H <sub>x</sub>	8	23.022	-4.733	1.930
	Min. H <sub>z</sub>	15	-288.713	-0.025	-33.767

### Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturing Moment, M <sub>x</sub> kip-ft	Overturing Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	54.488	0.000	0.000	-15.690	1.110	0.000
1.2 Dead+1.6 Wind 0 deg - No Ice	65.386	0.051	-62.660	-6948.625	-6.446	1.506
0.9 Dead+1.6 Wind 0 deg - No Ice	49.039	0.051	-62.660	-6943.918	-6.779	1.506
1.2 Dead+1.6 Wind 30 deg - No Ice	65.386	30.097	-52.223	-5870.472	-3367.954	-6.518
0.9 Dead+1.6 Wind 30 deg - No Ice	49.039	30.097	-52.223	-5865.765	-3368.287	-6.518
1.2 Dead+1.6 Wind 60 deg - No Ice	65.386	51.184	-29.665	-3341.598	-5722.448	-12.777
0.9 Dead+1.6 Wind 60 deg - No Ice	49.039	51.184	-29.665	-3336.891	-5722.781	-12.777
1.2 Dead+1.6 Wind 90 deg - No Ice	65.386	60.104	-0.051	-26.607	-6723.767	-15.777
0.9 Dead+1.6 Wind 90 deg - No Ice	49.039	60.104	-0.051	-21.900	-6724.100	-15.777
1.2 Dead+1.6 Wind 120 deg - No Ice	65.386	54.094	31.286	3439.333	-5972.510	-14.832
0.9 Dead+1.6 Wind 120 deg - No Ice	49.039	54.094	31.286	3444.040	-5972.844	-14.832
1.2 Dead+1.6 Wind 150 deg - No Ice	65.386	30.008	52.171	5825.036	-3354.480	-8.792
0.9 Dead+1.6 Wind 150 deg - No Ice	49.039	30.008	52.171	5829.743	-3354.814	-8.792
1.2 Dead+1.6 Wind 180 deg - No Ice	65.386	-0.051	59.240	6613.238	9.111	-1.121
0.9 Dead+1.6 Wind 180 deg - No Ice	49.039	-0.051	59.240	6617.945	8.778	-1.121
1.2 Dead+1.6 Wind 210 deg - No Ice	65.386	-30.097	52.223	5832.815	3370.619	6.518
0.9 Dead+1.6 Wind 210 deg - No Ice	49.039	-30.097	52.223	5837.522	3370.286	6.518
1.2 Dead+1.6 Wind 240 deg - No Ice	65.386	-54.146	31.375	3452.806	5982.954	13.345
0.9 Dead+1.6 Wind 240 deg - No Ice	49.039	-54.146	31.375	3457.513	5982.621	13.345
1.2 Dead+1.6 Wind 270 deg - No Ice	65.386	-60.104	0.051	-11.050	6726.432	15.777
0.9 Dead+1.6 Wind 270 deg - No Ice	49.039	-60.104	0.051	-6.343	6726.099	15.777
1.2 Dead+1.6 Wind 300 deg - No Ice	65.386	-51.133	-29.576	-3328.125	5717.334	13.879
0.9 Dead+1.6 Wind 300 deg - No Ice	49.039	-51.133	-29.576	-3323.418	5717.001	13.879
1.2 Dead+1.6 Wind 330 deg - No Ice	65.386	-30.008	-52.171	-5862.693	3357.146	8.792
0.9 Dead+1.6 Wind 330 deg - No Ice	49.039	-30.008	-52.171	-5857.986	3356.812	8.792
1.2 Dead+1.0 Ice+1.0 Temp	220.265	0.000	0.000	-97.856	-44.306	0.000

Load Combination	Vertical	Shear <sub>x</sub>	Shear <sub>y</sub>	Overturing Moment, M <sub>x</sub>	Overturing Moment, M <sub>y</sub>	Torque
	K	K	K	kip-ft	kip-ft	kip-ft
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp	220.265	-0.004	-18.842	-2241.813	-43.726	-0.242
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp	220.265	9.220	-15.981	-1927.299	-1099.370	-2.673
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp	220.265	15.928	-9.198	-1151.030	-1867.598	-4.358
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp	220.265	18.446	0.004	-97.277	-2155.439	-4.909
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp	220.265	16.312	9.424	974.624	-1899.879	-4.198
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp	220.265	9.227	15.985	1732.166	-1100.374	-2.205
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp	220.265	0.004	18.404	2009.495	-44.885	0.275
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp	220.265	-9.220	15.981	1731.586	1010.759	2.673
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp	220.265	-16.308	9.417	973.620	1810.688	4.451
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp	220.265	-18.446	-0.004	-98.436	2066.827	4.909
1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp	220.265	-15.932	-9.205	-1152.034	1779.567	4.072
1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp	220.265	-9.227	-15.985	-1927.879	1011.763	2.205
Dead+Wind 0 deg - Service	54.488	0.011	-13.821	-1544.172	-0.605	0.332
Dead+Wind 30 deg - Service	54.488	6.638	-11.519	-1306.368	-742.042	-1.438
Dead+Wind 60 deg - Service	54.488	11.289	-6.543	-748.582	-1261.364	-2.818
Dead+Wind 90 deg - Service	54.488	13.257	-0.011	-17.406	-1482.222	-3.480
Dead+Wind 120 deg - Service	54.488	11.931	6.901	747.065	-1316.520	-3.271
Dead+Wind 150 deg - Service	54.488	6.619	11.507	1273.271	-739.070	-1.939
Dead+Wind 180 deg - Service	54.488	-0.011	13.066	1447.122	2.826	-0.247
Dead+Wind 210 deg - Service	54.488	-6.638	11.519	1274.987	744.262	1.438
Dead+Wind 240 deg - Service	54.488	-11.943	6.920	750.036	1320.456	2.944
Dead+Wind 270 deg - Service	54.488	-13.257	0.011	-13.975	1484.443	3.480
Dead+Wind 300 deg - Service	54.488	-11.278	-6.523	-745.611	1261.869	3.061
Dead+Wind 330 deg - Service	54.488	-6.619	-11.507	-1304.652	741.291	1.939

### Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.000	-54.488	0.000	0.000	54.488	-0.000	0.000%
2	0.051	-65.386	-62.660	-0.051	65.386	62.660	0.000%
3	0.051	-49.039	-62.660	-0.051	49.039	62.660	0.000%
4	30.097	-65.386	-52.223	-30.097	65.386	52.223	0.000%
5	30.097	-49.039	-52.223	-30.097	49.039	52.223	0.000%
6	51.184	-65.386	-29.665	-51.184	65.386	29.665	0.000%
7	51.184	-49.039	-29.665	-51.184	49.039	29.665	0.000%
8	60.104	-65.386	-0.051	-60.104	65.386	0.051	0.000%
9	60.104	-49.039	-0.051	-60.104	49.039	0.051	0.000%
10	54.094	-65.386	31.286	-54.094	65.386	-31.286	0.000%
11	54.094	-49.039	31.286	-54.094	49.039	-31.286	0.000%
12	30.008	-65.386	52.171	-30.008	65.386	-52.171	0.000%
13	30.008	-49.039	52.171	-30.008	49.039	-52.171	0.000%
14	-0.051	-65.386	59.240	0.051	65.386	-59.240	0.000%
15	-0.051	-49.039	59.240	0.051	49.039	-59.240	0.000%
16	-30.097	-65.386	52.223	30.097	65.386	-52.223	0.000%
17	-30.097	-49.039	52.223	30.097	49.039	-52.223	0.000%

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
18	-54.146	-65.386	31.375	54.146	65.386	-31.375	0.000%
19	-54.146	-49.039	31.375	54.146	49.039	-31.375	0.000%
20	-60.104	-65.386	0.051	60.104	65.386	-0.051	0.000%
21	-60.104	-49.039	0.051	60.104	49.039	-0.051	0.000%
22	-51.133	-65.386	-29.576	51.133	65.386	29.576	0.000%
23	-51.133	-49.039	-29.576	51.133	49.039	29.576	0.000%
24	-30.008	-65.386	-52.171	30.008	65.386	52.171	0.000%
25	-30.008	-49.039	-52.171	30.008	49.039	52.171	0.000%
26	0.000	-220.265	0.000	-0.000	220.265	-0.000	0.000%
27	-0.004	-220.265	-18.842	0.004	220.265	18.842	0.000%
28	9.220	-220.265	-15.981	-9.220	220.265	15.981	0.000%
29	15.928	-220.265	-9.198	-15.928	220.265	9.198	0.000%
30	18.446	-220.265	0.004	-18.446	220.265	-0.004	0.000%
31	16.312	-220.265	9.424	-16.312	220.265	-9.424	0.000%
32	9.227	-220.265	15.985	-9.227	220.265	-15.985	0.000%
33	0.004	-220.265	18.404	-0.004	220.265	-18.404	0.000%
34	-9.220	-220.265	15.981	9.220	220.265	-15.981	0.000%
35	-16.308	-220.265	9.417	16.308	220.265	-9.417	0.000%
36	-18.446	-220.265	-0.004	18.446	220.265	0.004	0.000%
37	-15.932	-220.265	-9.205	15.932	220.265	9.205	0.000%
38	-9.227	-220.265	-15.985	9.227	220.265	15.985	0.000%
39	0.011	-54.488	-13.821	-0.011	54.488	13.821	0.000%
40	6.638	-54.488	-11.519	-6.638	54.488	11.519	0.000%
41	11.289	-54.488	-6.543	-11.289	54.488	6.543	0.000%
42	13.257	-54.488	-0.011	-13.257	54.488	0.011	0.000%
43	11.931	-54.488	6.901	-11.931	54.488	-6.901	0.000%
44	6.619	-54.488	11.507	-6.619	54.488	-11.507	0.000%
45	-0.011	-54.488	13.066	0.011	54.488	-13.066	0.000%
46	-6.638	-54.488	11.519	6.638	54.488	-11.519	0.000%
47	-11.943	-54.488	6.920	11.943	54.488	-6.920	0.000%
48	-13.257	-54.488	0.011	13.257	54.488	-0.011	0.000%
49	-11.278	-54.488	-6.523	11.278	54.488	6.523	0.000%
50	-6.619	-54.488	-11.507	6.619	54.488	11.507	0.000%

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	192 - 180	4.285	39	0.211	0.012
T2	180 - 160	3.753	39	0.208	0.012
T3	160 - 140	2.907	39	0.180	0.010
T4	140 - 120	2.192	39	0.150	0.008
T5	120 - 100	1.594	39	0.122	0.005
T6	100 - 80	1.103	39	0.099	0.004
T7	80 - 60	0.714	39	0.076	0.003
T8	60 - 40	0.417	39	0.055	0.002
T9	40 - 20	0.203	39	0.038	0.001
T10	20 - 0	0.061	39	0.019	0.000

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
190.000	NNVV-65B-R4 w/ Mount Pipe	39	4.196	0.211	0.012	409868
180.000	58532A	39	3.753	0.208	0.012	151635
170.000	(3) 7130.16.33.00 w/ Mount Pipe	39	3.318	0.196	0.011	50344
160.000	HBX-6516DS-VTM w/ Mount Pipe	39	2.907	0.180	0.010	30868
153.000	RR90-17-02DP w/ Mount Pipe	39	2.641	0.169	0.009	33495
140.000	(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	39	2.192	0.150	0.008	44370
98.000	58532A	39	1.060	0.097	0.004	47133

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	192 - 180	19.200	2	0.940	0.056
T2	180 - 160	16.824	2	0.926	0.056
T3	160 - 140	13.048	2	0.805	0.046
T4	140 - 120	9.852	2	0.672	0.036
T5	120 - 100	7.168	2	0.549	0.024
T6	100 - 80	4.964	2	0.445	0.017
T7	80 - 60	3.216	2	0.340	0.012
T8	60 - 40	1.878	2	0.249	0.008
T9	40 - 20	0.915	2	0.169	0.005
T10	20 - 0	0.274	2	0.086	0.002

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
190.000	NNVV-65B-R4 w/ Mount Pipe	2	18.803	0.939	0.056	98217
180.000	58532A	2	16.824	0.926	0.056	35891
170.000	(3) 7130.16.33.00 w/ Mount Pipe	2	14.884	0.876	0.052	11417
160.000	HBX-6516DS-VTM w/ Mount Pipe	2	13.048	0.805	0.046	6971
153.000	RR90-17-02DP w/ Mount Pipe	2	11.861	0.757	0.043	7575
140.000	(2) AM-X-CD-16-65-00T-RET w/ Mount Pipe	2	9.852	0.672	0.036	10081
98.000	58532A	2	4.769	0.435	0.016	10526

### Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
T1	192	Leg	A325N	0.625	4	1.436	20.709	0.069	1	Bolt Tension
		Diagonal	A325N	0.625	1	1.844	6.831	0.270	1	Member Block Shear
		Top Girt	A325N	0.625	1	0.131	7.116	0.018	1	Member Block Shear
T2	180	Leg	A325N	0.625	4	8.190	20.709	0.395	1	Bolt Tension
		Diagonal	A325N	0.625	1	4.758	7.830	0.608	1	Member Bearing
		Top Girt	A325N	0.625	1	0.977	8.135	0.120	1	Member Block Shear
T3	160	Leg	A325N	0.875	4	15.969	40.589	0.393	1	Bolt Tension
		Diagonal	A325N	0.625	1	6.575	10.440	0.630	1	Member Bearing
T4	140	Leg	A325N	1.000	4	24.675	53.014	0.465	1	Bolt Tension
		Diagonal	A325N	0.625	1	7.848	10.440	0.752	1	Member Bearing
T5	120	Leg	A325N	1.000	6	22.088	53.014	0.417	1	Bolt Tension
		Diagonal	A325N	0.750	1	8.434	13.485	0.625	1	Gusset Bearing
T6	100	Leg	A325N	1.000	6	27.019	53.014	0.510	1	Bolt Tension
		Diagonal	A325N	0.750	1	9.769	13.485	0.724	1	Gusset Bearing
T7	80	Leg	A325N	1.000	8	24.278	53.014	0.458	1	Bolt Tension
		Diagonal	A325N	0.750	1	10.630	13.485	0.788	1	Gusset Bearing
T8	60	Leg	A325N	1.000	8	27.939	53.014	0.527	1	Bolt Tension
		Diagonal	A325X	0.750	1	10.316	13.485	0.765	1	Gusset Bearing
T9	40	Leg	A325N	1.000	8	31.629	53.014	0.597	1	Bolt Tension
		Diagonal	A325X	0.750	1	11.967	13.485	0.887	1	Gusset Bearing
T10	20	Leg	A354-BC	1.000	10	28.191	55.223	0.510	1	Bolt Tension
		Diagonal	A325X	0.750	1	12.437	13.485	0.922	1	Gusset Bearing

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt K	Allowable Load K	Ratio Load Allowable	Allowable Ratio	Criteria
-------------	-----------------	----------------	------------	-----------------	-----------------	----------------------------	---------------------	-------------------------	-----------------	----------

### Compression Checks

### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> φP <sub>n</sub>
T1	192 - 180	ROHN 2.5 STD	12.000	4.000	50.7 K=1.00	1.704	-8.388	63.560	0.132 <sup>1</sup>
T2	180 - 160	ROHN 2.5 STD	20.032	5.008	63.4 K=1.00	1.704	-40.997	57.139	0.718 <sup>1</sup>
T3	160 - 140	ROHN 3 EH	20.036	6.679	70.5 K=1.00	3.016	-75.920	94.337	0.805 <sup>1</sup>
T4	140 - 120	ROHN 4 EH	20.038	6.679	54.3 K=1.00	4.407	-116.623	159.899	0.729 <sup>1</sup>
T5	120 - 100	ROHN 5 EH	20.036	6.679	43.6 K=1.00	6.111	-155.166	239.348	0.648 <sup>1</sup>
T6	100 - 80	ROHN 6 EHS	20.036	10.018	54.0 K=1.00	6.713	-189.129	244.047	0.775 <sup>1</sup>
T7	80 - 60	ROHN 6 EH	20.032	10.016	54.8 K=1.00	8.405	-226.462	303.757	0.746 <sup>1</sup>
T8	60 - 40	ROHN 8 EHS	20.042	10.021	40.6 K=1.00	9.867	-261.827	393.649	0.665 <sup>1</sup>
T9	40 - 20	ROHN 8 EHS	20.031	10.015	40.5 K=1.00	9.867	-297.486	393.703	0.756 <sup>1</sup>
T10	20 - 0	ROHN 8 EHS	20.033	10.017	40.6 K=1.00	9.867	-333.211	393.691	0.846 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> φP <sub>n</sub>
T1	192 - 180	L1 3/4x1 3/4x3/16	7.700	3.590	125.4 K=1.00	0.621	-1.838	8.789	0.209 <sup>1</sup>
T2	180 - 160	L2x2x3/16	9.686	4.727	144.0 K=1.00	0.715	-4.691	7.794	0.602 <sup>1</sup>
T3	160 - 140	L2 1/2x2 1/2x1/4	12.241	6.033	147.4 K=1.00	1.190	-6.691	12.367	0.541 <sup>1</sup>
T4	140 - 120	L2 1/2x2 1/2x1/4	14.067	6.902	168.7 K=1.00	1.190	-7.908	9.447	0.837 <sup>1</sup>
T5	120 - 100	L3x3x1/4	15.944	7.773	157.6 K=1.00	1.440	-8.418	13.104	0.642 <sup>1</sup>
T6	100 - 80	L3 1/2x3 1/2x1/4	19.209	9.452	163.4 K=1.00	1.690	-9.824	14.295	0.687 <sup>1</sup>
T7	80 - 60	L4x4x1/4	20.935	10.297	155.4 K=1.00	1.940	-10.765	18.143	0.593 <sup>1</sup>
T8	60 - 40	L4x4x5/16	22.872	11.208	170.0 K=1.00	2.400	-10.319	18.754	0.550 <sup>1</sup>
T9	40 - 20	L4x4x5/16	24.688	12.072	183.1 K=1.00	2.400	-12.234	16.165	0.757 <sup>1</sup>
T10	20 - 0	L4x4x3/8	26.510	12.996	197.9 K=1.00	2.860	-12.710	16.496	0.770 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls



### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
T1	192 - 180	L1 3/4x1 3/4x3/16	6.580	6.090	212.8 K=1.00	0.621	-0.119	3.099	0.039 <sup>1</sup>
T2	180 - 160	KL/R > 200 (C) - 4 L2x2x3/16	6.580	6.090	185.5 K=1.00	0.715	-1.010	4.694	0.215 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Tension Checks

### Leg Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
T1	192 - 180	ROHN 2.5 STD	12.000	4.000	50.7	1.704	5.743	76.682	0.075 <sup>1</sup>
T2	180 - 160	ROHN 2.5 STD	20.032	5.008	63.4	1.704	32.760	76.682	0.427 <sup>1</sup>
T3	160 - 140	ROHN 3 EH	20.036	6.679	70.5	3.016	63.875	135.717	0.471 <sup>1</sup>
T4	140 - 120	ROHN 4 EH	20.038	6.679	54.3	4.407	98.702	198.335	0.498 <sup>1</sup>
T5	120 - 100	ROHN 5 EH	20.036	6.679	43.6	6.111	132.529	275.012	0.482 <sup>1</sup>
T6	100 - 80	ROHN 6 EHS	20.036	10.018	54.0	6.713	162.115	302.097	0.537 <sup>1</sup>
T7	80 - 60	ROHN 6 EH	20.032	10.016	54.8	8.405	194.226	378.222	0.514 <sup>1</sup>
T8	60 - 40	ROHN 8 EHS	20.042	10.021	40.6	9.867	223.513	443.995	0.503 <sup>1</sup>
T9	40 - 20	ROHN 8 EHS	20.031	10.015	40.5	9.867	253.031	443.995	0.570 <sup>1</sup>
T10	20 - 0	ROHN 8 EHS	20.033	10.017	40.6	9.867	281.913	443.995	0.635 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
T1	192 - 180	L1 3/4x1 3/4x3/16	7.700	3.590	82.9	0.360	1.844	15.675	0.118 <sup>1</sup>
T2	180 - 160	L2x2x3/16	9.686	4.727	94.3	0.431	4.758	18.739	0.254 <sup>1</sup>
T3	160 - 140	L2 1/2x2 1/2x1/4	12.241	6.033	96.0	0.752	6.575	32.707	0.201 <sup>1</sup>
T4	140 - 120	L2 1/2x2 1/2x1/4	14.067	6.902	109.6	0.752	7.848	32.707	0.240 <sup>1</sup>
T5	120 - 100	L3x3x1/4	15.944	7.773	102.0	0.916	8.434	44.652	0.189 <sup>1</sup>
T6	100 - 80	L3 1/2x3 1/2x1/4	19.209	9.452	105.5	1.103	9.769	53.793	0.182 <sup>1</sup>
T7	80 - 60	L4x4x1/4	20.935	10.297	100.1	1.291	10.630	62.933	0.169 <sup>1</sup>
T8	60 - 40	L4x4x5/16	22.872	11.208	109.8	1.595	10.316	77.752	0.133 <sup>1</sup>
T9	40 - 20	L4x4x5/16	24.688	12.072	118.1	1.595	11.967	77.752	0.154 <sup>1</sup>
T10	20 - 0	L4x4x3/8	26.510	12.996	128.1	1.899	12.437	92.572	0.134 <sup>1</sup>

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio P <sub>u</sub> / φP <sub>n</sub>
T1	192 - 180	L1 3/4x1 3/4x3/16	6.580	6.090	141.7	0.360	0.131	15.675	0.008 <sup>1</sup>
T2	180 - 160	L2x2x3/16	6.580	6.090	123.3	0.431	0.977	18.739	0.052 <sup>1</sup>

<sup>1</sup>  $P_u / \phi P_n$  controls

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
T1	192 - 180	Leg	ROHN 2.5 STD	1	-8.388	63.560	13.2	Pass
T2	180 - 160	Leg	ROHN 2.5 STD	27	-40.997	57.139	71.8	Pass
T3	160 - 140	Leg	ROHN 3 EH	57	-75.920	94.337	80.5	Pass
T4	140 - 120	Leg	ROHN 4 EH	78	-116.623	159.899	72.9	Pass
T5	120 - 100	Leg	ROHN 5 EH	99	-155.166	239.348	64.8	Pass
T6	100 - 80	Leg	ROHN 6 EHS	120	-189.129	244.047	77.5	Pass
T7	80 - 60	Leg	ROHN 6 EH	135	-226.462	303.757	74.6	Pass
T8	60 - 40	Leg	ROHN 8 EHS	150	-261.827	393.649	66.5	Pass
T9	40 - 20	Leg	ROHN 8 EHS	165	-297.486	393.703	75.6	Pass
T10	20 - 0	Leg	ROHN 8 EHS	180	-333.211	393.691	84.6	Pass
T1	192 - 180	Diagonal	L1 3/4x1 3/4x3/16	11	-1.838	8.789	20.9	Pass
							27.0 (b)	
T2	180 - 160	Diagonal	L2x2x3/16	36	-4.691	7.794	60.2	Pass
							60.8 (b)	
T3	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-6.691	12.367	54.1	Pass
							63.0 (b)	
T4	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	84	-7.908	9.447	83.7	Pass
T5	120 - 100	Diagonal	L3x3x1/4	105	-8.418	13.104	64.2	Pass
T6	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	126	-9.824	14.295	68.7	Pass
							72.4 (b)	
T7	80 - 60	Diagonal	L4x4x1/4	141	-10.765	18.143	59.3	Pass
							78.8 (b)	
T8	60 - 40	Diagonal	L4x4x5/16	156	-10.319	18.754	55.0	Pass
							76.5 (b)	
T9	40 - 20	Diagonal	L4x4x5/16	171	-12.234	16.165	75.7	Pass
							88.7 (b)	
T10	20 - 0	Diagonal	L4x4x3/8	183	-12.710	16.496	77.0	Pass
							92.2 (b)	
T1	192 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.119	3.099	3.9	Pass
T2	180 - 160	Top Girt	L2x2x3/16	28	-1.010	4.694	21.5	Pass
							Summary	
						Leg (T10)	84.6	Pass
						Diagonal (T10)	92.2	Pass
						Top Girt (T2)	21.5	Pass
						Bolt	92.2	Pass
						Checks		
						<b>RATING =</b>	<b>92.2</b>	<b>Pass</b>

### 3) ANALYSIS PROCEDURE

**Table 4 - Documents Provided**

Document	Remarks	Reference	Source
4-GEOTECHNICAL REPORTS	FDH Engineering	2189896	CCI Sites
4-TOWER MANUFACTURER DRAWINGS	Rohn	1631630	CCI Sites
4-TOWER FOUNDATION DRAWINGS/DESIGN/SPECS	Rohn	1631622	CCI Sites

#### 3.1) Analysis Method

tnxTower (version 7.0.5.1), a commercially available analysis software package, was used to create a three-dimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is included in Appendix A.

#### 3.2) Assumptions

- 1) Tower and structures were built in accordance with the manufacturer's specifications.
- 2) The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Tables 1 and 2 and the referenced drawings.
- 4) The existing base plate grout was considered in this analysis. Grout must be maintained and inspected periodically and must be replaced if damaged or cracked. Refer to Crown Castle document ENG-PRC-10012, Base Plate Grout Repair.

This analysis may be affected if any assumptions are not valid or have been made in error. Crown Castle should be notified to determine the effect on the structural integrity of the tower.

### 4) ANALYSIS RESULTS

**Table 5 - Section Capacity (Summary)**

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T1	192 - 180	Leg	ROHN 2.5 STD	1	-8.388	63.560	13.2	Pass
T2	180 - 160	Leg	ROHN 2.5 STD	27	-40.997	57.139	71.8	Pass
T3	160 - 140	Leg	ROHN 3 EH	57	-75.920	94.337	80.5	Pass
T4	140 - 120	Leg	ROHN 4 EH	78	-116.623	159.899	72.9	Pass
T5	120 - 100	Leg	ROHN 5 EH	99	-155.166	239.348	64.8	Pass
T6	100 - 80	Leg	ROHN 6 EHS	120	-189.129	244.047	77.5	Pass
T7	80 - 60	Leg	ROHN 6 EH	135	-226.462	303.757	74.6	Pass
T8	60 - 40	Leg	ROHN 8 EHS	150	-261.827	393.649	66.5	Pass
T9	40 - 20	Leg	ROHN 8 EHS	165	-297.486	393.703	75.6	Pass
T10	20 - 0	Leg	ROHN 8 EHS	180	-333.211	393.691	84.6	Pass
T1	192 - 180	Diagonal	L1 3/4x1 3/4x3/16	11	-1.838	8.789	20.9 27.0 (b)	Pass
T2	180 - 160	Diagonal	L2x2x3/16	36	-4.691	7.794	60.2 60.8 (b)	Pass
T3	160 - 140	Diagonal	L2 1/2x2 1/2x1/4	63	-6.691	12.367	54.1 63.0 (b)	Pass
T4	140 - 120	Diagonal	L2 1/2x2 1/2x1/4	84	-7.908	9.447	83.7	Pass

Section No.	Elevation (ft)	Component Type	Size	Critical Element	P (K)	SF*P_allow (K)	% Capacity	Pass / Fail
T5	120 - 100	Diagonal	L3x3x1/4	105	-8.418	13.104	64.2	Pass
T6	100 - 80	Diagonal	L3 1/2x3 1/2x1/4	126	-9.824	14.295	68.7 72.4 (b)	Pass
T7	80 - 60	Diagonal	L4x4x1/4	141	-10.765	18.143	59.3 78.8 (b)	Pass
T8	60 - 40	Diagonal	L4x4x5/16	156	-10.319	18.754	55.0 76.5 (b)	Pass
T9	40 - 20	Diagonal	L4x4x5/16	171	-12.234	16.165	75.7 88.7 (b)	Pass
T10	20 - 0	Diagonal	L4x4x3/8	183	-12.710	16.496	77.0 92.2 (b)	Pass
T1	192 - 180	Top Girt	L1 3/4x1 3/4x3/16	4	-0.119	3.099	3.9	Pass
T2	180 - 160	Top Girt	L2x2x3/16	28	-1.010	4.694	21.5	Pass
							Summary	
							Leg (T10)	84.6 Pass
							Diagonal (T10)	92.2 Pass
							Top Girt (T2)	21.5 Pass
							Bolt Checks	92.2 Pass
							Rating =	92.2 Pass

**Table 6 - Tower Component Stresses vs. Capacity – LC7**

Notes	Component	Elevation (ft)	% Capacity	Pass / Fail
1	Anchor Rods	0	57.9	Pass
1	Base Foundation (Structure)	0	16.7	Pass
1	Base Foundation (Soil Interaction)	0	46.6	Pass

<b>Structure Rating (max from all components) =</b>	<b>92.2%</b>
---	--------------

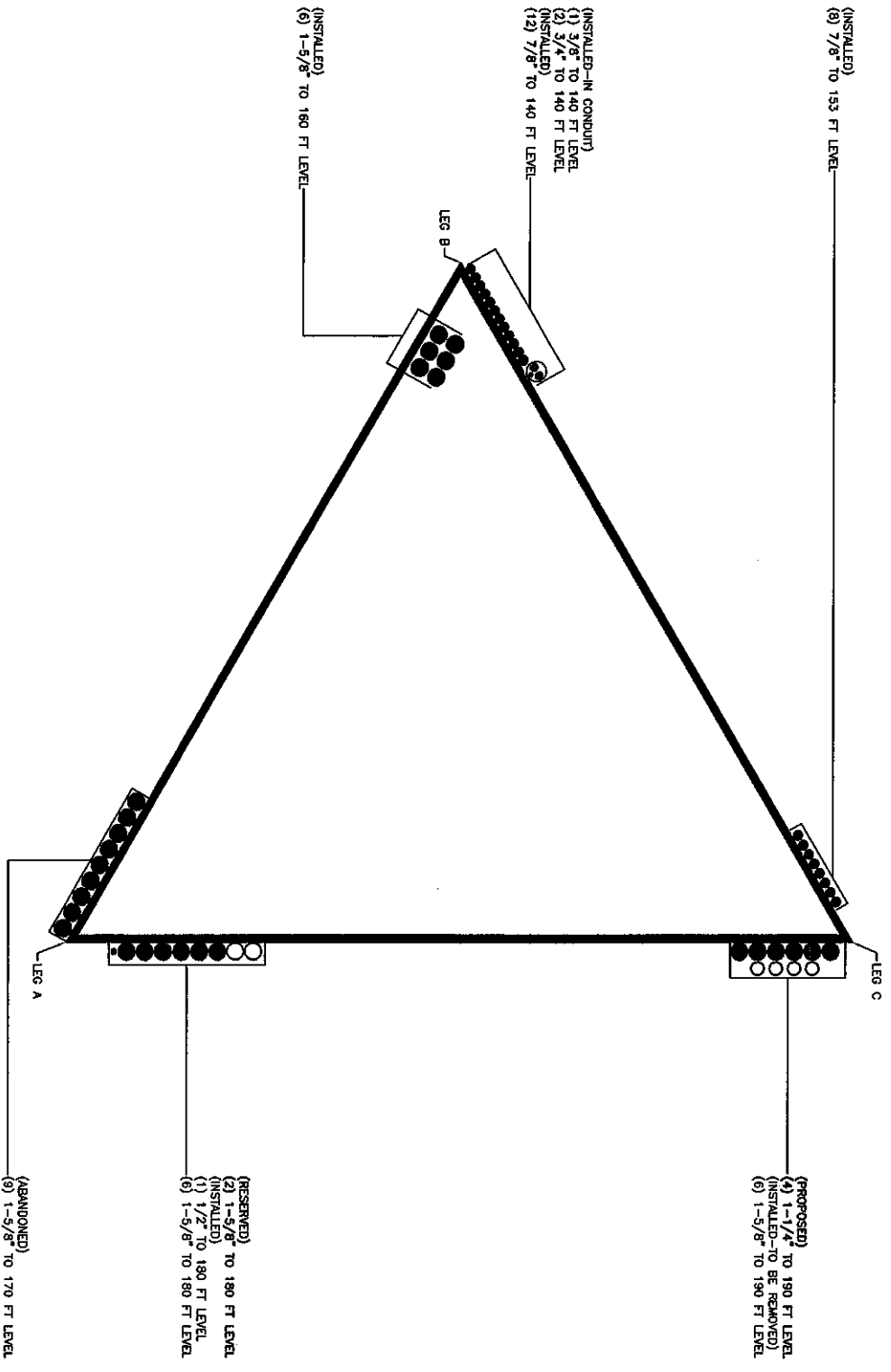
Notes:

- 1) See additional documentation in "Appendix C – Additional Calculations" for calculations supporting the % capacity consumed.

**4.1) Recommendations**

The tower and its foundation have sufficient capacity to carry the proposed load configuration. No modifications are required at this time.

**APPENDIX B**  
**BASE LEVEL DRAWING**



BUSINESS UNIT: 876346 TOWER ID: C\_BASLEVEL

**APPENDIX C**  
**ADDITIONAL CALCULATIONS**

# Anchor Rod Check for Self Supporting Towers

TIA-222-G, Section 4.9.9

Rev. 6.1



Site Data	
BU#:	<b>876345</b>
Site Name:	<b>SKY HILL</b>
App #:	<b>441316 Rev. 0</b>

Reactions		
Eta Factor, $\eta$	<b>0.55</b>	Detail Type
Uplift, $P_u$ :	<b>289</b>	kips
Shear, $V_u$ :	<b>34</b>	kips

Anchor Rod Data		
Qty:	<b>10</b>	
Diam:	<b>1</b>	in
Rod Material:	<b>A354 Gr. BC (1/4 to 2-1/2 incl.)</b>	
Strength ( $F_u$ ):	<b>125</b>	ksi
Yield ( $F_y$ ):	<b>109</b>	ksi

$l_{ar}$ :		in
$M_u = 0.65 * l_{ar} * V_u$		ft-kips

* Rod Circle:		in
* e:		in
* # of Rods		1 or 2

### Anchor Rod Results:

Max Rod ( $C_u + V_u/\eta$ ):	<b>35.1</b>	Kips
Design Axial, $\Phi * F_u * A_{net}$ :	<b>60.6</b>	Kips
Anchor Rod Stress Ratio:	<b>57.9%</b>	

$M_u = P_u \times e$ :		ft-kips
------------------------	--	---------

\* Only enter rod circle, offset (e) and number of anchor rods at the extreme fiber to consider if eccentric load due to leg reinforcement exist.

### If Applicable;

### Anchor Rod Results with Bending Considered:

When the clear distance from the top of concrete to the bottom of level nut exceeds 1.0 times the diameter of the anchor rod, the following interaction equation shall also be satisfied (see Figure 4-4 of Rev. G):

$$(V_u/\phi R_{nv})^2 + [(P_u/\phi R_{nt}) + (M_u/\phi R_{nm})]^2 \leq 1$$

$\phi R_{nv} = \phi * 0.45 * F_{ub} * A_b =$		kips
$\phi R_{nt} = \phi * F_u * A_{net} =$		kips
$\phi R_{nm} = \phi * F_y * Z =$		ft-kips

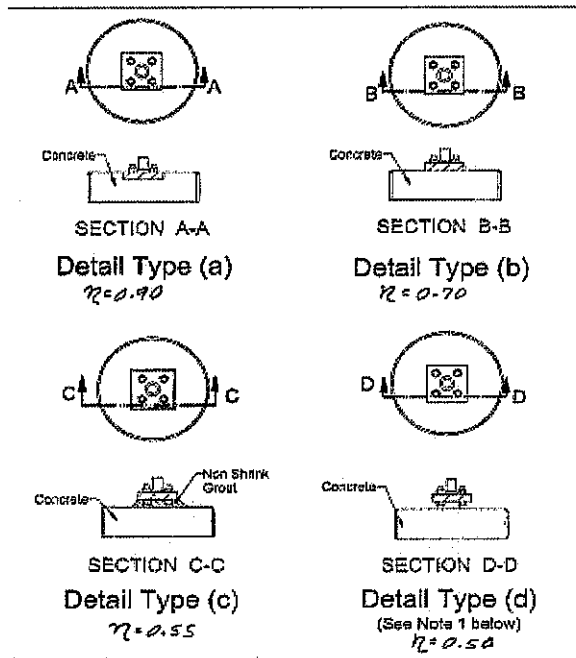


Figure 4-4 of TIA-222-G

Maximum Acceptable Ratio: **105** %

Governing Stress Ratio: **57.9%** Pass



# Drilled Pier Foundation

BU #: 876345  
 Site Name: SKY HILL  
 App. Number: 441316 Rev.0

TIA-222 Revision: G  
 Tower Type: Self Support



## Analysis Results

Soil Lateral Capacity		Compression	Uplift
$D_{90}$ (ft from TOC)	11.53		11.53
Soil Safety Factor	36.18		41.50
Max. Moment (kip-ft)	309.84		270.12
Rating	3.7%		3.2%
Soil Vertical Capacity		Compression	Uplift
Skin Friction (kips)	560.77		560.77
End Bearing (kips)	374.93		-
Weight of Concrete (kips)	93.66		70.24
Total Capacity (kips)	935.70		631.02
Axial (kips)	435.66		289.00
Rating	46.6%		45.8%
Reinforced Concrete Capacity		Compression	Uplift
Critical Depth (ft from TOC)	11.87		10.67
Critical Moment (kip-ft)	309.51		268.33
Critical Moment Capacity	2385.66		1607.61
Rating	73.0%		16.7%
Soil Interaction Rating		46.6%	
Structural Foundation Rating			16.7%

Applied Loads		Comp.	Uplift
Moment (kip-ft)	0		0
Axial Force (kips)	342		289
Shear Force (kips)	39		34

Material Properties	
Concrete Strength, $f_c$	3 ksi
Rebar Strength, $F_y$	60 ksi

Pier Design Data	
Depth	26 ft
Ext. Above Grade	0.5 ft
Pier Section 1	
<i>From 0.5' above grade to 26' below grade</i>	
Pier Diameter	5 ft
Rebar Quantity	18
Rebar Size	9
Rebar Cage Diameter	51 in
Tie Size	5

## Soil Profile

# of Layers: 5

Groundwater Depth: N/A ft

Layer	Top (ft)	Bottom (ft)	Thickness (ft)	$\gamma_{soil}$ (pcf)	$\gamma_{concrete}$ (pcf)	Cohesion (ksf)	Angle of Friction (degrees)	Calculated Ultimate Skin Friction Comp (ksf)	Calculated Ultimate Skin Friction Uplift (ksf)	Ultimate Skin Friction Comp Override (ksf)	Ultimate Skin Friction Uplift Override (ksf)	Ult. Gross Bearing Capacity (ksf)	SPT Blow Count	Soil Type
1	0	2	2	120	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
2	2	3.3	1.3	130	150	0	0	0.000	0.000	0.00	0.00			Cohesionless
3	3.3	5	1.7	130	150	3	0	1.650	1.650	0.00	0.00			Cohesive
4	5	6	1	135	150	5	0	2.321	2.321	0.00	0.00			Cohesive
5	6	26	20	135	150	5	0	2.321	2.321	2.38	2.38	25.46		Cohesive

# CCISeismic - Design Category

Per 2012/2015 IBC

Site BU: 876345  
 Work Order: 1580544  
 Application: 441316 Rev. 0



	Degrees	Minutes	Seconds		
Site Latitude =	41	57	7.70	0.0000	degrees
Site Longitude =	-72	11	43.90	0.0000	degrees
Ground Supported Structure =	Yes				
Structure Class =	II				(Table 2-1)
Site Class =	D - Stiff Soil				(Table 2-11)
Spectral response acceleration short periods, $S_s$ =	0.173				USGS Seismic Tool
Spectral response acceleration 1 s period, $S_1$ =	0.063				
Importance Factor, $I$ =	1.0				(Table 2-3)
Acceleration-based site coefficient, $F_a$ =	1.6				(Table 2-12)
Velocity-based site coefficient, $F_v$ =	2.4				(Table 2-13)
Design spectral response acceleration short period, $S_{DS}$ =	0.185				(2.7.6)
Design spectral response acceleration 1 s period, $S_{D1}$ =	0.101				(2.7.6)
Seismic Design Category - Short Period Response =	B				ASCE 7-05 Table 11.6-1
Seismic Design Category - 1s Period Response =	B				ASCE 7-05 Table 11.6-2
Worst Case Seismic Design Category =	B				ASCE 7-05 Tables 11.6-1 and 6-2



# EBI Consulting

environmental | engineering | due diligence

---

## RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

SPRINT Existing Facility

Site ID: CT03XC204

Sky Hill  
33 Janowski Road  
Ashford, CT 06278

**July 31, 2018**

**EBI Project Number: 6218005226**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>8.09 %</b>



# EBI Consulting

environmental | engineering | due diligence

July 31, 2018

SPRINT

Attn: RF Engineering Manager  
1 International Boulevard, Suite 800  
Mahwah, NJ 07495

## Emissions Analysis for Site: **CT03XC204 – Sky Hill**

EBI Consulting was directed to analyze the proposed SPRINT facility located at **33 Janowski Road, Ashford, CT**, for the purpose of determining whether the emissions from the Proposed SPRINT Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

General population exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limits for the 850 MHz Band is approximately  $567 \mu\text{W}/\text{cm}^2$ . The general population exposure limit for the 1900 MHz (PCS) and 2500 MHz (BRS) bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



# EBI Consulting

environmental | engineering | due diligence

---

Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed SPRINT Wireless antenna facility located at **33 Janowski Road, Ashford, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since SPRINT is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 1 CDMA channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.
- 2) 2 LTE channels (850 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 50 Watts per Channel.
- 3) 5 CDMA channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 16 Watts per Channel.
- 4) 2 LTE channels (1900 MHz (PCS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 40 Watts per Channel.
- 5) 8 LTE channels (2500 MHz (BRS)) were considered for each sector of the proposed installation. These Channels have a transmit power of 20 Watts per Channel.



- 6) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.
- 7) For the following calculations, the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 8) The antennas used in this modeling are the **Commscope NNVV-65B-R4** and the **RFS APXVTM14-ALU-I20** for transmission in the 850 MHz, 1900 MHz (PCS) and 2500 MHz (BRS) frequency bands. This is based on feedback from the carrier with regards to anticipated antenna selection. Maximum gain values for all antennas are listed in the Inventory and Power Data table below. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB for directional panel antennas, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 9) The antenna mounting height centerlines of the proposed panel antennas are **192 feet** above ground level (AGL) for **Sector A**, **192 feet** above ground level (AGL) for **Sector B** and **192 feet** above ground level (AGL) for Sector C.
- 10) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.

All calculations were done with respect to uncontrolled / general population threshold limits.



# EBI Consulting

environmental | engineering | due diligence

## SPRINT Site Inventory and Power Data by Antenna

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	Commscope NNVV-65B-R4	Make / Model:	Commscope NNVV-65B-R4	Make / Model:	Commscope NNVV-65B-R4
Gain:	12.75 / 15.05 dBd	Gain:	12.75 / 15.05 dBd	Gain:	12.75 / 15.05 dBd
Height (AGL):	192 feet	Height (AGL):	192 feet	Height (AGL):	192 feet
Frequency Bands:	850 MHz / 1900 MHz (PCS)	Frequency Bands:	850 MHz / 1900 MHz (PCS)	Frequency Bands:	850 MHz / 1900 MHz (PCS)
Channel Count:	10	Channel Count:	10	Channel Count:	10
Total TX Power(W):	280 Watts	Total TX Power(W):	280 Watts	Total TX Power(W):	280 Watts
ERP(W):	7,378.61	ERP(W):	7,378.61	ERP(W):	7,378.61
Antenna A1 MPE%:	0.94 %	Antenna B1 MPE%:	0.94 %	Antenna C1 MPE%:	0.94 %
Antenna #:	2	Antenna #:	2	Antenna #:	2
Make / Model:	RFS APXVTM14-ALU-I20	Make / Model:	RFS APXVTM14-ALU-I20	Make / Model:	RFS APXVTM14-ALU-I20
Gain:	15.9 dBd	Gain:	15.9 dBd	Gain:	15.9 dBd
Height (AGL):	192 feet	Height (AGL):	192 feet	Height (AGL):	192 feet
Frequency Bands:	2500 MHz (BRS)	Frequency Bands:	2500 MHz (BRS)	Frequency Bands:	2500 MHz (BRS)
Channel Count:	8	Channel Count:	8	Channel Count:	8
Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts	Total TX Power(W):	160 Watts
ERP(W):	6,224.72	ERP(W):	6,224.72	ERP(W):	6,224.72
Antenna A2 MPE%:	0.65 %	Antenna B2 MPE%:	0.65 %	Antenna C2 MPE%:	0.65 %

Site Composite MPE%	
Carrier	MPE%
SPRINT – Max per sector	1.59 %
AT&T	2.14 %
Verizon Wireless	2.80 %
T-Mobile	1.35 %
Nextel	0.21 %
<b>Site Total MPE %:</b>	<b>8.09 %</b>

SPRINT Sector A Total:	1.59 %
SPRINT Sector B Total:	1.59 %
SPRINT Sector C Total:	1.59 %
<b>Site Total:</b>	<b>8.09 %</b>

SPRINT Frequency Band / Technology (All Sectors)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
Sprint 850 MHz CDMA	1	376.73	192	0.39	850 MHz	567	0.06%
Sprint 850 MHz LTE	2	941.82	192	1.96	850 MHz	567	0.34%
Sprint 1900 MHz (PCS) CDMA	5	511.82	192	2.66	1900 MHz (PCS)	1000	0.27%
Sprint 1900 MHz (PCS) LTE	2	1,279.56	192	2.66	1900 MHz (PCS)	1000	0.27%
Sprint 2500 MHz (BRS) LTE	8	778.09	192	6.47	2500 MHz (BRS)	1000	0.65%
						<b>Total:</b>	<b>1.59%</b>



# EBI Consulting

environmental | engineering | due diligence

## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the SPRINT facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

SPRINT Sector	Power Density Value (%)
Sector A:	1.59 %
Sector B:	1.59 %
Sector C:	1.59 %
SPRINT Maximum MPE % (per sector):	1.59 %
Site Total:	8.09 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **8.09 %** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

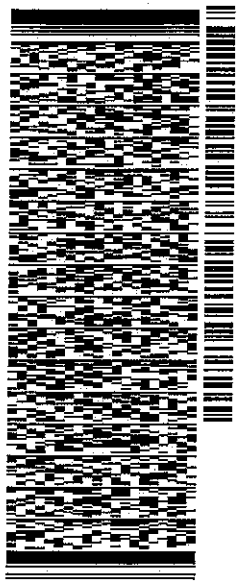


ORIGIN D:BEDA (781) 970-0053  
JEFF BARADORA  
CROWN CASTLE  
12 GILL STREET  
SUITE 5800  
WOBURN, MA 01801  
UNITED STATES US

SHIP DATE: 08AUG18  
ACTWGT: 0.50 LB  
CAD: 104924191IN/ET4040  
BILL SENDER

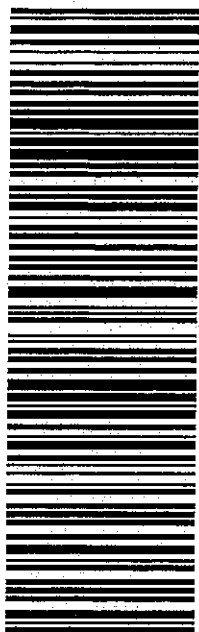
TO MICHAEL GARDNER-ZONE OFFICER  
TOWN OF ASHFORD  
5 TOWN HALL ROAD

ASHFORD CT 06278  
(880) 487-4415 REF: 17656980  
DEPT: NV



TRK# 7729 2834 3018 THU - 09 AUG 4:30P  
0201 PRIORITY OVERNIGHT

EB GONA 06278  
CT-US BDL



552J1/3309/DCA5

**After printing this label:**

1. Use the 'Print' button on this page to print your label to your laser or inkjet printer.
2. Fold the printed page along the horizontal line.
3. Place label in shipping pouch and affix it to your shipment so that the barcode portion of the label can be read and scanned.

**Warning:** Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.

Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com. FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

**Barbadora, Jeff**

**From:** TrackingUpdates@fedex.com  
**Sent:** Friday, August 10, 2018 11:59 AM  
**To:** Barbadora, Jeff  
**Subject:** FedEx Shipment 772928343018 Delivered

## Your package has been delivered

Tracking # 772928343018

Ship date:  
**Wed, 8/8/2018**

**Jeff Barbadora**  
Crown Castle  
WOBURN, MA 01801  
US

Delivery date:  
**Fri, 8/10/2018 11:56 am**

**Michael Gardner-Zone Officer**  
Town of Ashford  
5 Town Hall Road  
ASHFORD, CT 06278  
US



### Shipment Facts

Our records indicate that the following package has been delivered.

**Tracking number:** [772928343018](#)  
**Status:** Delivered: 08/10/2018 11:56 AM  
Signed for By: B.FERENNCE  
**Reference:** 1766.6680  
**Signed for by:** B.FERENNCE  
**Delivery location:** ASHFORD, CT  
**Delivered to:** Receptionist/Front Desk  
**Service type:** FedEx Priority Overnight®  
**Packaging type:** FedEx® Envelope  
**Number of pieces:** 1  
**Weight:** 2.00 lb.  
**Special handling/Services:** Deliver Weekday  
**Standard transit:** 8/10/2018 by 4:30 pm

Please do not respond to this message. This email was sent from an unattended mailbox. This report was generated at approximately 10:58 AM CDT on 08/10/2018.

All weights are estimated.

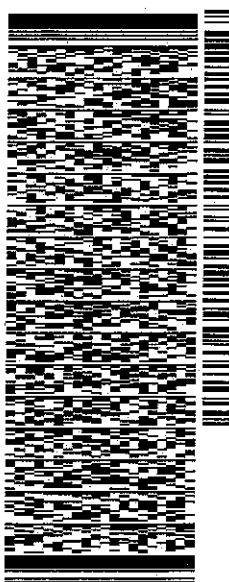
To track the latest status of your shipment, click on the tracking number above.

ORIGIN ID: BEDA (781) 970-0053  
JEFF BARBADORA  
CROWN CASTLE  
12 GIL STREET  
SUITE 5800  
WOBBURN, MA 01801  
UNITED STATES US

SHIP DATE: 08AUG18  
ACTWT: 0.50 LB  
CAD: 1049241911NET14040  
BILL SENDER

TO MICHAEL ZAMBO-FIRST SELECTMAN  
TOWN OF ASHFORD  
5 TOWN HALL ROAD

ASHFORD CT 06278  
(860) 487-4400 REF: 17656690  
INVT DEPT:  
PO



J182016072201ur

TRK# 7729 2822 1293 THU - 09 AUG 4:30P  
0201 PRIORITY OVERNIGHT

EB GONA 06278  
CT-US BDL



552J1/3309/DC/5

**After printing this label:**

1. Use the 'Print' button on this page to print your label to your laser or inkjet printer.
2. Fold the printed page along the horizontal line.
3. Place label in shipping pouch and affix it to your shipment so that the barcode portion of the label can be read and scanned.

**Warning:** Use only the printed original label for shipping. Using a photocopy of this label for shipping purposes is fraudulent and could result in additional billing charges, along with the cancellation of your FedEx account number.

Use of this system constitutes your agreement to the service conditions in the current FedEx Service Guide, available on fedex.com. FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1,000, e.g. jewelry, precious metals, negotiable instruments and other items listed in our ServiceGuide. Written claims must be filed within strict time limits, see current FedEx Service Guide.

## Barbadora, Jeff

**From:** TrackingUpdates@fedex.com  
**Sent:** Friday, August 10, 2018 11:59 AM  
**To:** Barbadora, Jeff  
**Subject:** FedEx Shipment 772928221293 Delivered

# Your package has been delivered

Tracking # 772928221293

Ship date:  
**Wed, 8/8/2018**

Jeff Barbadora  
Crown Castle  
WOBURN, MA 01801  
US

Delivery date:  
**Fri, 8/10/2018 11:55 am**

Michael Zambo-First  
Selectman  
Town of Ashford  
5 Town Hall Road  
ASHFORD, CT 06278  
US



## Shipment Facts

Our records indicate that the following package has been delivered.

**Tracking number:** 772928221293

**Status:** Delivered: 08/10/2018 11:55 AM  
Signed for By: A.RUCHOLL

**Reference:** 1766.6680

**Signed for by:** A.RUCHOLL

**Delivery location:** ASHFORD, CT

**Delivered to:** Receptionist/Front Desk

**Service type:** FedEx Priority Overnight®

**Packaging type:** FedEx® Envelope

**Number of pieces:** 1

**Weight:** 2.00 lb.

**Special handling/Services:** Deliver Weekday

**Standard transit:** 8/10/2018 by 4:30 pm

✉ Please do not respond to this message. This email was sent from an unattended mailbox. This report was generated at approximately 10:58 AM CDT on 08/10/2018.

All weights are estimated.