

445 Hamilton Avenue, 14th Floor White Plains, New York 10601 Tel 914.761.1300 Fax 914.761.5372 www.cuddyfeder.com

March 24, 2014

VIA E-MAIL AND OVERNIGHT DELIVERY

Ms. Melanie Bachman
Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, Connecticut 06051



Re: Do

Docket 423

North Atlantic Towers, LLC and New Cingular Wireless PCS, LLC ("AT&T")

Certificate of Environmental Compatibility and Public Need

Route 198, Woodstock, Connecticut

D&M Revision and

Request for an Extension of Time for the Facility to Become Operational

Dear Ms. Bachman:

On behalf of AT&T, please accept for review and Council approval this *revised* Development Management Plan ("D&M Plan") filing for the above-captioned Facility as approved in Docket No. 423. This submission is a revision to the D&M Plan approved on September 21, 2012 and includes an update to AT&T's equipment for the provision of LTE (Long Term Evolution) services.

On behalf of AT&T, we also respectfully request that the time for the facility to become operational as part of North Atlantic Towers' Certificate be extended to December 21, 2014.

D&M Revision

Antennas & Other Equipment

Enclosed are fifteen (15) sets of 11"x 17" sized revised construction drawings being filed in accordance with the Siting Council's ("Council") Decision and Order dated June 21, 2012. Two (2) sets of full sized drawings will be forwarded shortly under separate cover.

The revised D&M Plan drawings incorporate revised specifications for AT&T's LTE facility. The revisions include the addition of one panel antenna per sector, for a total of twelve antennas. The revisions also include the addition of Remote Radiohead Units ("RRUs") for a total of twenty-one (21). As shown in the enclosed drawings, the RRUs will be installed behind AT&T's twelve panel antennas on the antenna platform mount at a centerline height of approximately 106' above grade level on the existing 110' tall tower. The enclosed drawings also included updated specifications for AT&T's panel antennas.

C&F: 2389309.2



March 24, 2014 Page 2 of **2**

Additionally, enclosed is a structural analysis prepared by Michael F. Plahovinsak, P.E. dated February 10, 2014, which confirms that the tower facility can structurally accommodate AT&T's upgraded facility.

Also enclosed is a cumulative power density analysis, which concludes that the worst-case emissions from AT&T's equipment at the Facility will be 13.9% of the Federal Communications Commission's Maximum Permissible Exposure standard for RF emissions.

Required Notifications

The supervisor for all construction related matters for AT&T's facility is Bryon Morawski of SAI. Mr. Morawski is located at 500 Enterprise Drive, Suite 3A, Rocky Hill, CT 06067 and can be reached by telephone at (860) 513-7223.

Extension of Time

Condition 7 of the Decision and Order dated June 21, 2012 required the approved Facility to be fully constructed and providing wireless services within eighteen months from the date of the mailing of the Council's Findings of Fact, Opinion, and Decision and Order. On September 23, 2013, the Council granted an extension of time for AT&T to make the facility operational to June 21, 2014.

We are advised that current scheduling demands and logistical considerations for AT&T to install its updated facility as detailed in the enclosed materials will require additional time to bring the fully constructed facility on-air. Accordingly, we respectfully request a six month extension of time for the facility to be operational to December 21, 2014.

We respectfully request that this matter be included on the Council's next available agenda for review. Should Council members or Staff have any questions regarding the foregoing please do not hesitate to contact me.

Thank you for your consideration of the enclosed.

Very truly yours,

Lucia Chiocchio

Enclosures

ee: Allan D. Walker Jr., First Selectman Town of Woodstock

Michele Briggs, AT&T

Chris Policinski, Centerline Communications

Keith Coppins, North Atlantic Towers

Christopher B. Fisher, Esq.

C&F: 2389309.2

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Structural Analysis 110-ft Monopole

Prepared For:
Florida Tower Partners, LLC
1001 3rd Ave. West, Suite 420
Bradenton, FL 34205

MFP Project #40914-015

Site Location:
CT1182 Woodstock
Windham Co., Connecticut
Lat/Long: 41°56'21.6", -72°4'54.6"

Analysis Type: ANSI/TIA-222-G

February 10, 2014



Michael F. Plahovinsak, P.E. 1830| State Route 161 W, Plain City, 0H 43064 614-398-6250 - mike@mfpeng.com

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Project Summary:

I have completed a structural analysis of the existing monopole for the following new configuration:

- 105' AT&T:
 - o (12) CCI HPA-65R-BUU-H8 Panel +
 - o (9) RRUS-11 + (6) RRUS-12 + (6) RRUS-A2 + (3) RRUS-E2 + (3) RRUS-32
 - o (4) Raycap DC6-48-60-18-8F Suppresor
 - \circ (2) 7/16" + (8) 34" + (3) 1/2" Cable / Low Profile Platform

The pole has been analyzed in accordance with the requirements of the International Building Code per IBC section 3108.4, and the recommendations of the Telecommunications Industry Association "Structural Standard for Steel Antenna Supporting Structures" ANSI/TIA-222-G.

This analysis may be considered a "Rigorous Structural Analysis" as defined in ANSI/TIA-222-G 15.5.2.

As indicated in the conclusions of this analysis, I have determined that the existing pole and foundation have *sufficient capacity* to support the existing, reserved and proposed antenna loads as detailed herein. Based on the results of my analysis, structural modifications are not required at this time.

Source of Data:

Resource	Source	Job Number	Date
Pole and Foundation Drawings	Nello Inc	182019	08/22/12
Geotechnical Report	Berkshire Geo-Tech	n/a	07/16/12

Analysis Criteria:

International Building Code (All Versions) Section 3108.4 Structural Standards for Steel Antenna Supporting Structures **ANSI/TIA-222-G 2**

Basic Wind Speed

100 mph (3-Sec Gust)

Basic Wind Speed w/ 1" Ice

50 mph (3-Sec Gust)

Operational Wind Speed

60 mph (3-Sec Gust)

Structure Class	Exposure Category	Topographic Category
$\Pi (I = 1.0)$	В	3 (Crest 150')

Michael F. Plahovinsak, P.E. - 2014

mike@mfpeng.com

Appurtenance Listing:

Status	Elev.	Antenna / Mounting	Coax	Owner
Proposed	105'	(12) CCI HPA-65R-BUU-H8 Panel + (4) DC6-48-60-18-8F (9) RRUS-11 + (6) RRUS-12 + (6) RRUS-A2 (3) RRUS-E2 + (3) RRUS-32 Low Profile Platform	(2) 7/16" + (8) 3/4" + (3) 1/2"	AT&T

All antenna lines assumed internally mounted, not exposed to the wind.

Foundation Analysis:

The existing monopole foundation design was analyzed in conjunction with site specific geotechnical report. The existing foundation has sufficient capacity to support the pole with the proposed antenna configuration.

Conclusion:

I have completed a structural analysis of the existing monopole and foundation in accordance with the project specifics outlined above. My analysis indicates that the existing monopole and foundation is stressed to a maximum of 45.7% of its usable capacity when considering the existing plus proposed loading. Please refer to the attached calculations for an itemized listing of all member stress ratios. The existing pole is safe and adequate to support the proposed loads, and no structural reinforcing is required to support the above loading.

If you have any questions about the contents of this structural report or require any additional information, please feel free to contact my office.

Sincerely,

Michael F. Plahovinsak, P.E.

mike@mfpeng.com - 614.398-6250

Michael F. Plahovinsak, P.E. - 2014

mike@mfpeng.com

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Standard Conditions for Providing Structural Consulting Services on Existing Structures

1. The following standard conditions are a general overview of key issues regarding the work product supplied.

- 2. If the existing conditions are not as represented in this structural report or attached sketches, I should be contacted to evaluate the significance of the deviation and revise the structural assessment accordingly.
- 3. The structural analysis has been performed assuming that the structure is in "like new" condition. No allowance was made for excessive corrosion, damaged or missing structural members, loose bolts, etc. If there are any known deficiencies in the structure that potentially compromise structural integrity, I should be made aware of the deficiencies. If I am aware of a deficiency that exists in a structure at the time of my analysis, a general explanation of the structural concern due to the deficiency will be included in the structural report, but the deficiency will not be reflected in capacity calculations.
- 4. The structural analysis provided is an assessment of the primary load carrying capacity of the structure. I provide a limited scope of service in that I have not verified the capacity of every weld, plate, connection detail, etc. In most cases, structural fabrication details are unknown at the time of my analysis, and the detailed field measurement of this information is beyond the scope of my services. In instances where I have not performed connection capacity calculations, it is assumed that existing manufactured connections develop the full capacity of the primary members being connected.
- 5. The structural integrity of the existing foundation system can only be verified if exact foundation sizes and soils conditions are known. I will not accept any responsibility for the adequacy of the existing foundations unless this site-specific data is supplied.
- 6. Miscellaneous items such as antenna mounts, coax supports, etc. have not been designed, detailed, or specified as part of my work. It is assumed that material of adequate size and strength will be purchased from a reputable component manufacturer. The attached report and sketches are schematic in nature and should not be used to fabricate or purchase hardware and accessories to be attached to the structure. I recommend field measurement of the structure before fabricating or purchasing new hardware and accessories. I am not responsible for proper fit and clearance of hardware and accessory items in the field.
- 7. The structural analysis has been performed considering minimum code requirements or recommendations. If alternate wind, ice, or deflection criteria are to be considered, then I shall be made aware of the alternate criteria.

Michael F. Plahovinsak, P.E. - 2014

mike@mfpeng.com

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Number of Sides Thickness (in)		48,89	S3.00
kness (in)	18	18	
!	0.3750	0.3750	0.2500
Sacket Length (ff)		6.75	5.50
Top Dia (in)	44.8751	37,8533	29,4000
Bot Dia (in)	48.5000	46.8700	39.3900
Grade		A572-65	
Weight (K) 17.0	3.8	8.3	4.9
<u>0.0 ft</u>	13.6 ft		57.0 ft
REACTIONS - 100 mph WIND	AXIAL 55 K SHEAR 7 K 506 kip-R 50 mph WIND - 1.0000 in ICE AXIAL 25 K SHEAR 3 MOMENT 1658 kip-ft	ALL REACTIONS ARE FACTORED	(4) CCI pipe (AT (3) Erics (2) Erics (2) Erics Ericsson Ericsson (4) CCI pipe (AT (3) Erics (2) Erics (2) Erics (2) Erics (2) Erics (3) Erics (4) CCI pipe (AT (4) CCI pipe (AT (5) Erics (6) Erics (7) Erics (8) Erics (9) Erics (10) Erics (11) Erics (12) Erics (12) Erics (13) Erics (14) Erics (15) Erics (16) Erics (17) Erics (17) Erics (18) Eri

Michael F. Plahovinsak, P.E.	lob: 110-ft Monopole - MFP	#40914-015
	Project. CT1182 Woodstock	
Plain City, OH 43064	^{Client} Florida Tower Partners ^{Draw}	^{n by:} Mike App'd:
Phone: 614-398-6250	Code: TIA-222-G Date:	02/10/14 Scale:
FAX: mike@mfneng.com	Path:	Dwg N

4) CCI HPA-65R-BUU-H8 w/ ipe (ATT) 3) Ericsson RRUS-11 (ATT) 2) Ericsson RRUS 12 (ATT) 2) Ericsson RRUS A2 (ATT) cricsson RRUS E2 (ATT)		/AΠON	TYI		ELEVATION
3) Enicsson RRUS-11 (ATT) 2) Ericsson RRUS 12 (ATT) 2) Ericsson RRUS A2 (ATT) (ricsson RRUS-E2 (ATT)	/mount 105		son RRUS-E2 (105
2) Ericsson RRUS 12 (ATT) 2) Ericsson RRUS A2 (ATT) ricsson RRUS E2 (ATT)	105		son RRUS-32 (105
) Ericsson RRUSA2 (ATT) icsson RRUS-E2 (ATT)	105		CI HPA-65R-BL (ATT)	JU-H8 w/ mount	105
icsson RRUS-E2 (ATT)	105		ricsson RRUS-	11 (ATT)	105
	105		ricsson RRUS		105
icsson RRUS-32 (ATT)	105	(2) E	ricsson RRUS	12 (ATT)	105
) CCI HPA-65R-BUU-H8 w/	/ mount 105		son RRUS-E2 (105
pe (ATT)			son RRUS-32 (105
) Ericsson RRUS-11 (ATT)	105	(4) R	aycap DC6-48-	60-18-8F	105
) Ericsson RRUS 12 (ATT)) Ericsson RRUS A2 (ATT)	105 105		essor (ATT) ow Profile Platfo	orm (ATT)	105
Elicssuffrosaz (ATT)	100		W Florie Flau	AIII (XI I)	100
		TERIAL STE			
GRADE F ₃ 572-65 65 ksi	9 80 ksi	Fu GI	RADE	Fy	Fu
Tower is located in	Windham Coun		i.		
Tower designed for Tower designed for Tower is also designed	r a 100 mph basi	ic wind in acco	rdance with		
increase in thickne	ss with height.		ווון טט.ו ווווי	ce. Ice is cuit	sidered to
Deflections are bas Tower Structure Cla		ph wind.			
Topographic Categ		Height of 150	00 ft		
TOWER RATING:			••		
			•		
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-					
-1					
p-ft					
p-ft	×				
p-ft					
p-ft	`				
ENT ip-ft					
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tnx	1	0	W	e	r

Michael F. Plahovinsak, P.E. 18301 State Route 161 W

Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

Job		Page
	110-ft Monopole - MFP #40914-015	1 of 6
Project	CT1182 Woodstock	Date 18:44:48 02/10/14
Client	Florida Tower Partners	Designed by Mike

Tower Input Data

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Tower is located in Windham County, Connecticut. Basic wind speed of 100 mph.

Structure Class II.

Exposure Category B.
Topographic Category 3.

Crest Height 150.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph. A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feedline supports, and appurtenance mounts are not considered.

Tapered Pole Section Geometry

Section	Elevation	Section	Splice	Number	Top	Bottom	Wall	Bend	Pole Grade
		Length	Ĺength	of	Diameter	Diameter	Thickness	Radius	
	ft	ft	ft	Sides	in	in	in	in	
Ll	110.00-57.00	53.00	5.50	18	29.4000	39.3900	0.2500	1.0000	A572-65 (65 ksi)
L2	57.00-13.61	48.89	6.75	18	37.8533	46.8700	0.3750	1.5000	A572-65 (65 ksi)
L3	13.61-0.00	20.36		18	44.8751	48.5000	0.3750	1.5000	Á572-65 (65 ksi)

Tapered Pole Properties

	**************************************	antina antina paramenta de mentro de la		CANADA NO DINING MANAGEMENT		whenen the sales and sales are sales and sales are		************	OLINANA DI KANDANI KAND	Mark the Control of t
Section	Tip Dia.	Area	I	r	C	I/C	J	It/Q	w	w/t
	în	in^2	in ⁴	in	in	in^3	in ⁴	in ²	in	
L1	29.8535	23.1305	2483,1334	10.3483	14.9352	166.2605	4969.5343	11.5675	4.7344	18.938
	39.9977	31.0576	6011.0016	13.8947	20,0101	300.3981	12029.9127	15.5317	6.4926	25.971
L2	39.4673	44.6085	7916.1744	13.3048	19.2295	411.6687	15842.7652	22.3085	6.0022	16.006
	47.5930	55.3407	15114.5213	16.5057	23.8100	634.7983	30248.9306	27.6756	7.5891	20.238
L3	46.7877	52.9663	13251.3117	15.7975	22.7966	581.2857	26520.0598	26.4882	7.2380	19.301
	49.2482	57.2808	16760.5346	17.0844	24.6380	680.2717	33543.1232	28.6458	7.8760	21.003

tnxTower	Job	110-ft Monopole - MFP #40914-015	Page 2 of 6
fichael F. Plahovinsak, P.E. 18301 State Route 161 W	Project	CT1182 Woodstock	Date 18:44:48 02/10/14
Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Client	Florida Tower Partners	Designed by Mike

	Feed	Line/Line	ar Appurt	enances	- Entered	As Area
Description	Face Allow	Component	Placement	Total		Weight

Description	Face or	Allow Shield	Component Type	Placement	Total Number		C_AA_A	Weigh
	Leg		7.	ft			ft²/ft	plf
7/16"	С	No	Inside Pole	105.00 - 0.00	2	No Ice	0.00	0.15
(ATT)						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15
3/4"	C	No	Inside Pole	105.00 - 0.00	8	No Ice	0.00	0.33
(ATT)						1/2" Ice	0.00	0.33
. ,						1" Ice	0.00	0.33
1/2"	C	No	Inside Pole	105.00 - 0.00	3	No Ice	0.00	0.15
(ATT)						1/2" Ice	0.00	0.15
						1" Ice	0.00	0.15

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			Vert ft ft	٥	ft		ft²	ft²	K
			ft						
(4) CCI HPA-65R-BUU-H8	A	From Face	3.00	0.0000	105.00	No Ice	13,62	9.18	0.10
w/ mount pipe			0.00			1/2" Ice	14.35	10.58	0.19
(ATT)			0.00			I" Ice	15.09	11.83	0.29
(3) Ericsson RRUS-11	Α	From Face	3.00	0.0000	105.00	No Ice	2.55	0.92	0.05
(ATT)			0.00			1/2" Ice	2.77	1.07	0.06
, ,			0.00			1" Ice	2.99	1.23	0.08
(2) Ericsson RRUS 12	Α	From Face	3.00	0.0000	105.00	No Ice	3.67	1.46	0.06
(ATT)			0.00			1/2" Ice	3.92	1.64	0.08
` ,			0.00			1" Ice	4.19	1.84	0.11
(2) Ericsson RRUS A2	Α	From Face	3.00	0.0000	105.00	No Ice	1.87	0.50	0.03
(ATT)			0.00			1/2" Ice	2.05	0.62	0.04
· · · · · · · · · · · · · · · · · · ·			0.00			1" Ice	2.24	0.75	0.05
Ericsson RRUS-E2	Α	From Face	3.00	0.0000	105.00	No Ice	3.67	1.49	0.06
(ATT)			0.00			1/2" Ice	3.93	1.67	0.08
,			0.00			1" Ice	4.19	1.87	0.11
Ericsson RRUS-32	Α	From Face	3.00	0.0000	105.00	No Ice	3.87	2.76	0.08
(ATT)			0.00			1/2" Ice	4.15	3.02	0.10
. ,			0.00			1" Ice	4.44	3.29`	0.14
(4) CCI HPA-65R-BUU-H8	В	From Face	3.00	0.0000	105.00	No Ice	13.62	9.18	0.10
w/ mount pipe			0.00			1/2" Ice	14.35	10.58	0.19
(ATT)			0.00			1" Ice	15.09	11.83	0.29
(3) Ericsson RRUS-11	В	From Face	3.00	0.0000	105.00	No Ice	2.55	0.92	0.05
(ATT)			0.00			1/2" Ice	2.77	1.07	0.06
, ,			0.00			1" Ice	2.99	1.23	0.08
(2) Ericsson RRUS 12	В	From Face	3.00	0.0000	105.00	No Ice	3.67	1.46	0.06
(ATT)			0.00			1/2" Ice	3.92	1.64	0.08
			0.00			1" Ice	4.19	1.84	0.11
(2) Ericsson RRUS A2	В	From Face	3.00	0.0000	105.00	No Ice	1.87	0.50	0.03
(ATT)			0.00			1/2" Ice	2.05	0.62	0.04
, ,			0.00			1" Ice	2.24	0.75	0.05
Ericsson RRUS-E2	В	From Face	3.00	0.0000	105.00	No Ice	3.67	1.49	0.06
(ATT)			0.00			1/2" Ice	3.93	1.67	0.08
• /			0.00			1" Ice	4.19	1.87	0.11
Ericsson RRUS-32	В	From Face	3.00	0.0000	105.00	No Ice	3.87	2.76	0.08
(ATT)			0.00			1/2" Ice	4.15	3.02	0.10
` ,			0.00			1" Ice	4.44	3.29	0.14
(4) CCI HPA-65R-BUU-H8	C	From Face	3.00	0.0000	105,00	No Ice	13.62	9.18	0.10

tnxTower	Job	110-ft Monopole - MFP #40914-015	Page 3 of 6
Michael F. Plahovinsak, P.E. 18301 State Route 161 W	Project	CT1182 Woodstock	Date 18:44:48 02/10/14
Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Client	Florida Tower Partners	Designed by Mike

Description	Face or Leg	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	Placement	umacan mendilik Palastika berasilan pendilik palastika berasilan pendilik palastika berasilan pendilik pendili	C _A A _A Front	C₄A₄ Side	Weight
			Vert ft ft ft	٥	ft		ft²	ft²	K
w/ mount pipe (ATT)	***************************************	**************************************	0.00 0.00	***************************************		1/2" Ice 1" Ice	14.35 15.09	10.58 11.83	0.19 0.29
(3) Ericsson RRUS-11 (ATT)	С	From Face	3.00 0.00 0.00	0.0000	105.00	No Ice 1/2" Ice 1" Ice	2.55 2.77 2.99	0.92 1.07 1.23	0.05 0.06 0.08
(2) Ericsson RRUS 12 (ATT)	C	From Face	3.00 0.00	0.0000	105.00	No Ice 1/2" Ice	3.67 3.92	1.46 1.64	0.06 0.08
(2) Ericsson RRUS A2 (ATT)	С	From Face	0.00 3.00 0.00	0.0000	105.00	1" Ice No Ice 1/2" Ice	4.19 1.87 2.05	1.84 0.50 0.62	0.11 0.03 0.04
Ericsson RRUS-E2 (ATT)	C	From Face	0.00 3.00 0.00	0.0000	105.00	1" Ice No Ice 1/2" Ice	2.24 3.67 3.93	0.75 1.49 1.67	0.05 0.06 0.08
Ericsson RRUS-32 (ATT)	С	From Face	0.00 3.00 0.00	0.0000	105.00	1" Ice No Ice 1/2" Ice	4.19 3.87 4.15	1.87 2.76 3.02	0.11 0.08 0.10
4) Raycap DC6-48-60-18-8F Supressor	C.	None	0.00	0.0000	105.00	1" Ice No Ice I/2" Ice 1" Ice	4.44 1.47 1.67	3.29 1.47 1.67	0.14 0.03 0.05
(ATT) 12' Low Profile Platform (ATT)	С	None		0.0000	105.00	No Ice 1/2" Ice 1" Ice	1.88 15.70 16.00 18.00	1.88 15.70 16.00 18.00	0.07 1.10 1.70 2.30

Load Combinations

Comb.	Description
No.	-
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice
3	0.9 Dead+1.6 Wind 0 deg - No Ice
4	1.2 Dead+1.6 Wind 90 deg - No Ice
5	0.9 Dead+1.6 Wind 90 deg - No Ice
6	1.2 Dead+1.6 Wind 180 deg - No Ice
7	0.9 Dead+1.6 Wind 180 deg - No Ice
8 .	1.2 Dead+1.0 Ice+1.0 Temp
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
12	Dead+Wind 0 deg - Service
13	Dead+Wind 90 deg - Service
14	Dead+Wind 180 deg - Service
(4)(02:0:4)(03:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:	

tnx	7	0	w	ei
* * * * * *	-		6.6	- F

Michael F. Plahovinsak, P.E. 18301 State Route 161 W

Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

Job		Page
	110-ft Monopole - MFP #40914-015	4 of 6
Project		Date
	CT1182 Woodstock	18:44:48 02/10/14
Client		Designed by
	Florida Tower Partners	Mike

															F					

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Туре		Load		Moment	Moment
				Comb.	K	kip-ft	kip-ft
L1	110 - 57	Pole	Max Tension	6	0.00	0.00	0.00
			Max. Compression	8	-29.21	0.00	0.00
			Max. Mx	4	-9,60	-517.22	0.00
			Max, My	2	-9.60	0.00	517.22
			Max. Vy	4	14.40	-517.22	0.00
			Max. Vx	2	-14.40	0.00	517.22
L2	57 - 13.61	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-44.84	0.00	0.00
			Max. Mx	4	-19.11	-1230.50	0.00
			Max. My	6	-19.11	0.00	-1230.50
			Max. Vy	4	19.55	-1230.50	0.00
			Max. Vx	6	19.55	0.00	-1230.50
L3	13.61 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-54.65	0.00	0.00
			Max. Mx	4	-25.44	-1657.84	0.00
			Max, My	2	-25.44	0.00	1657.84
			Max. Vy	4	22.51	-1657.84	0.00
			Max. Vx	2	-22.51	0.00	1657.84

Maximum Tower Deflections - Service Wind

Section	Elevation	Horz.	Gov,	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	٥
L1	110 - 57	5.546	13	0.4080	0,0000
L2	62.5 - 13.61	1.921	13	0.2738	0.0000
L3	20.36 - 0	0.228	13	0.0932	0.0000

Critical Deflections and Radius of Curvature - Service Wind

	Obtáncia trum memercen menor en manor a mante en esta presenta presenta en consente en consente en consente en					iskistatiotiinisvaksisiataisiitaisiista kalainaan kasiin een kasiin kasiin ka
Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	•	•	ft
105.00	(4) CCI HPA-65R-BUU-H8 w/	13	5.127	0.3958	0.0000	118992
	mount pipe					

Maximum Tower Deflections - Design Wind

Speciment the summer subtraction of the summ	may recompany and recommendation means a Nichmonto Actor and and a Complete recipion of the fe				A CONTRACTOR CONTRACTO
Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	o	٥
LI	110 - 57	27.616	2	2,0320	0.0000
L2	62.5 - 13.61	9.566	2	1.3635	0.0000
L3	20.36 - 0	1.134	2	0.4640	0.0000

tnxTower	Job	110-ft Monopole - MFP #40914-015	Page 5 of 6
Michael F. Plahovinsak, P.E. 18301 State Route 161 W	Project	CT1182 Woodstock	Date 18:44:48 02/10/14
Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Client	Florida Tower Partners	Designed by Mike

			dius of		

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	0	ft
105.00	(4) CCI HPA-65R-BUU-H8 w/	2	25.529	1.9713	0.0000	23939
	mount pipe					

			Pol	e Des	sign [Data				
Section	Elevation	Size	$L^{}$	$L_{ u}$	K1/r	A	P_u	ϕP_n	Ratio	
No.	ft		ft	ft		in^2	K	K	$\frac{P_u}{\Phi P_u}$	
L1	110 - 57 (1)	TP39.39x29.4x0.25	53.00	0.00	0.0	30.2350	-9.60	1951.42	0.005	
L2	57 - 13.61 (2)	TP46.87x37.8533x0.375	48.89	0.00	0.0	53,8589	-19.11	3794.70	0.005	
1.3	13.61 - 0.03	TP48 5v44 8751v0 375	20.36	በ ብበ	0.0	57 2808	-25 44	3953 98	0.006	

		Po	ole Ben	ding De	sign I	Data			
Section	Elevation	size	M_{ux}	ϕM_{nx}	Ratio	M_{uy}	$\phi M_{\pi y}$	Ratio '	
No.					M_{ux}			M_{yy}	
	ft		kip-ft	kip-ft	ϕM_{nx}	kip-ft	kip-fi	ϕM_{ny}	
L1	110 - 57 (1)	TP39.39x29.4x0.25	517.22	1530.97	0.338	0.00	1530.97	0.000	
L2	57 - 13.61 (2)	TP46.87x37.8533x0.375	1230.50	3529.43	0.349	0.00	3529.43	0.000	
1.2	13.61 - 0.63)	TPAR 5vAA 8751v0 375	1657.84	3013 15	0.424	0.00	3013 15	ብ በስስ	

Pole Shear Design Data

Section	Elevation	Size	Actual	ϕV_n	Ratio	Actual	ϕT_n	Ratio
No.			V_u		V_{u}	T_u		T_u
	ft		K	K	ϕV_n	kip-ft	kip-ft	$\overline{\phi T_n}$
LI	110 - 57 (1)	TP39.39x29.4x0.25	14.40	968.89	0.015	0.00	3065.68	0.000
L2	57 - 13.61 (2)	TP46.87x37.8533x0.375	19.55	1886.59	0.010	0.00	7067.50	0.000
L3	13.61 - 0 (3)	TP48,5x44.8751x0.375	22.51	1971.97	0.011	0.00	7835.87	0.000

Pole Interaction Design Data

Section No.	Elevation	Ratio P _u	Ratio M _{ux}	Ratio M _{uy}	Ratio V _u	Ratio T _u	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
	ft	ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n	Rano	Nano	
L1	110 - 57 (1)	0.005	0.338	0.000	0.015	0.000	0.343	1.000	4.8.2
L2	57 - 13.61 (2)	0.005	0.349	0.000	0.010	0.000	0.354	1.000	4.8.2
L3	13.61 - 0 (3)	0.006	0.424	0.000	0.011	0.000	0.430	1.000	4.8.2

tnxTower	Job	110-ft Monopole - MFP #40914-015	Page 6 of 6
Michael F. Plahovinsak, P.E. 18301 State Route 161 W	Project	CT1182 Woodstock	Date 18:44:48 02/10/14
Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com	Client	Florida Tower Partners	Designed by Mike

			Section Cap	acity T	able			
Section No.	Elevation ft	Component Type	Size	Critical Element	$P \ K$	øP _{allów} K	% Capacity	Pass Fail
L1	110 - 57	Pole	TP39.39x29.4x0.25	1	-9.60	1951.42	34.3	Pass
L2	57 - 13.61	Pole	TP46.87x37.8533x0.375	2	-19.11	3794.70	35.4	Pass
L3	13.61 - 0	Pole	TP48.5x44.8751x0.375	3	-25.44	3953.98	43.0	Pass
							Summary	
						Pole (L3)	43.0	Pass
						RATING =	43.0	Pass

Michael F. Plahovinsak, P.E.	Јо ь 110-ft monopole - MFP #40914-015	Page BP-G	
Plain City, OH 43064 Phone: 614-398-6250	Project CT1182 Woodstock	Date 02/10/2014	
email: mike@mfpeng.com	Client FLORIDA TOWER PARTNERS	Designed by Mike	

Anchor Rod and Base Plate Calculation

ANSI/TIA-222-G-2

Factored Base Reactions:		Pole Shape: Anchor Rods:		Base Plate:
Moment:	1658 ft-kips	18-Sided	(16) 2.25 in. A615 GR. 75	3 in. x 61.5 in. Round
Shear:	23 kips	Pole Dia. (D_f) :	Anchor Rods Evenly Spaced	fy = 50 ksi
Axial:	25 kips	48.50 in	On a 55.5 in Bolt Circle	·

Anchor Rod Calculation According to TIA-222-G section 4.9.9

$$\begin{array}{lll} \pmb{\varphi} = & 0.80 \text{ TIA 4.9.9} & \textit{The following Interation Equation Shall Be Satisfied:} \\ \pmb{I_{bolts}} = & 6160.50 \text{ in}^2 \text{ Momet of Inertia} \\ \pmb{P_u} = & 90 \text{ kips Tension Force} \\ \pmb{V_u} = & 1 \text{ kips Shear Force} \\ \pmb{R_{nt}} = & 325.00 \text{ kips Nominal Tensile Strength} \\ \pmb{\eta} = & 0.50 \text{ for detail type (d)} & 0.356 \leq 1 \end{array}$$

Base Plate Calculation According to TIA-222-G

φ =	0.90 TIA 4.7		
$\mathbf{M_{PL}} =$	216.6 in-kip Plate Moment		
$\mathbf{L} =$	9.5 in Section Length	Calculated Moment vs Factor	red Resistance
$\mathbf{Z} =$	21.4 Plastic Section Modulus	216.56 in-kip ≤	964 in-kip
$\mathbf{M}_{\mathbf{P}} =$	1071.3 in-kip Plastic Moment		
ф М =	964.2 in-kip Factored Resistance		

Anchor Rods Are Adequate	35.6%	V
Base Plate is Adequate	22.5%	図

Monopole Spread Footing Calculation

ANSI/TIA-222-G-2

Factored Base	Reactions:	Footing Dimensions:		Concrete:
Moment:	1658 ft-kips	23 ft x 23 ft	6.5 ft Square Pier	f'c = 4000 psi
Shear:	23 kips	x 2.25 ft thick	w/6 in Reveal	Steel fy = 60 ksi
Axial:	25 kips	Bearing 6.5 ft B.G.	51.5 Yd3 Concrete	f = 0.75
Soil Backfill	110 pcf	Ultimate Bearing:	6000 psf	Water Table n/a
Foundation We	eight			
	ght of Pole	25.0 kips		
Weight	of Concrete	208.64063 kips		
Wei	ght of Soil	227.55563 kips		
Bouya	ncy of Water	0.0 kips		
	Total	461.2 kips		
Overturning Re	esistance:			
Overturnir	ng Moment (M _u)	1819 ft-kips	1658 ft-k	ips + (23 kips x 7 ft)
Resisting	g Moment (R _s)	5303.7569 ft-kips	461.1962	5 kips x 23 ft / 2
фх	$R_s > M_u$	$M_{overturning} / f M_{resist}$	45.7	% OK
Soil Bearing Pr	eccure.			
_	ntricity (e)	3.94 ft	1819 ft-k	ips / 461.19625 kips
Leec	6(e)	23.7 ft >	23.0 ft	6e > 23
Maximu	m Soil Bearing	2197.3593 psf		d across corners
	Overburden	-715 psf	Outoulate	a deloss comers
	oil Bearing	1482.3593 psf		
	Soil Bearing (R _s)	6000 psf		
-	Bearing $< \phi \times R_s$	Net Bearing / f R _s	32.9	% OK
	5 1 3	,		
Bending Mome	nt in Pier:			
Bendi	ng Moment	1767.25 ft-kips	1658 ft-k	ips + (23 kips x 4.75 ft)
Pier Steel	Req'd (Loads)	21.96 in^2		
Min.	Pier Steel	23.9 in^2	1/2%	
Bending Mome	ent in Footing:			
Max Ber	nding Moment	1216.7952 ft-kips	Σ Momen	ts about pier face
Min. F	ooting Steel	$0.58 \text{ in}^2/\text{ft}$	0.18%	

From:

ALECSANDRU, RADU <RA9161@att.com>

Sent:

Friday, February 14, 2014 2:57 PM

To:

Chris Policinski

Cc:

Peter Lamontagne

Subject:

RE: AT&T NSB // CT2067 - Woodstock // Power Density Calcs Request // FA 10141310 // Project 2051002388

As requested...

Transmission Mode	Centerline Ht (feet)	Frequency (MHz)	Number of Channels	Power Per Channel (Watts)	Power Density (mW/cm²)	Standard Limits (mW/cm²)	Percent of Limit
AT&T UMTS	105	800 Band	2	500	0.0326	0.5867	5.56
AT&T UMTS	105	1900 Band	1	500	0.0163	1.0000	1.63
AT&T LTE	105	700 Band	1	500	0.0163	0.4667	3.49
AT&T LTE	105	1900 Band	1	500	0.0163	1.0000	1.63
AT&T LTE	105	2300 Band	1	500	0.0163	1.0000	1.63
Total	STATE AND THE STATE OF THE STAT		Carlot and the Carlot				13.9%

Please let me know if there are any questions.

Radu Alecsandru RF Engineer at&t mobility office 860-513-7598 mobile 860-965-6685

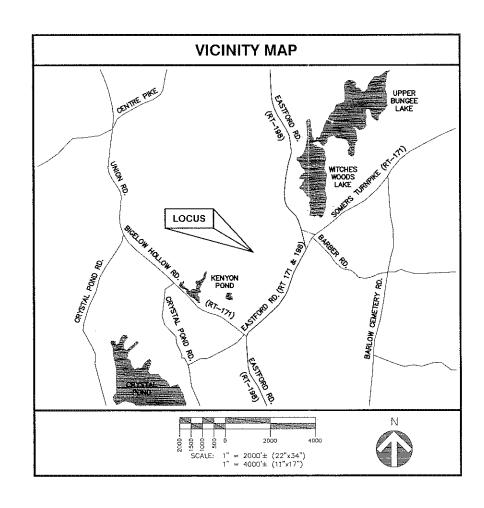


SITE NAME: WOODSTOCK RT 198 SITE NUMBER: CT2067 **ADDRESS: ROUTE 198** WOODSTOCK, CT 06282

DRAWING INDEX				
SHEET	DESCRIPTION .	REVISION		
T-1	TITLE SHEET	A		
GN-1	GENERAL NOTES	A		
A1	PLOT PLAN & COMPOUND PLAN	A		
A-2	ELEVATION & DETAILS	Α		
D-1	DETAILS	A		
S-1	STRUCTURAL DETAILS	Α		
E-1	ELECTRICAL & GROUNDING DETAILS	Α		
E-2	GROUNDING DETAILS & NOTES	Α		

GENERAL NOTES

- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER & AT&T REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
- ALL WORK TO BE PERFORMED IN ACCORDANCE WITH THE LATEST AT&T CONSTRUCTION GUIDELINES.
- ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND EXISTING PLANS OF RECORD, THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK CALL DIG-SAFE (888) 344-7233 72-HOURS PRIOR TO ANY EXCAVATION. THIS SHEET SET WAS ORIGINALLY PRINTED TO ANSI D (22"x34") WITH 1" MARGINS. PRINTING TO ANSI B (11"x17") WILL RESULT IN A HALF-SCALE (1:2) SHEET SET WITH 1/2" MARGINS. CONFIRM ALL SCALED DISTANCES WITH GRAPHICAL SCALES SHOWN HEREIN.



PROJECT INFORMATION

SITE TYPE: CO-LOCATION MONOPOLE

PROPOSED PANEL ANTENNAS & RF EQUIPMENT ON SECTOR FRAME MOUNTED ON (E) MONOPOLE. SHELTER & SCOPE OF WORK:

GENERATOR PROPOSED AT GROUND LEVEL

SITE NAME: WOODSTOCK RT 198

SITE NUMBER: CT2067

SITE ADDRESS: ROUTE 198 WOODSTOCK, CT 06282

ASSESSOR'S TAX ID#: MAP 5789 BLOCK 37 LOT 24

COMMUNITY DISTRICT ZONING DISTRICT:

LATITUDE: 41° 56' 21.97" N (RECORD PLANS) 72' 04' 55.26" W (RECORD PLANS)

DATUM:

LONGITUDE:

PROPERTY OWNER: N/F WOODSTOCK TOWER PARTNERS, LLC

35 KENNEDY DRIVE PUTNAM, CT 06260

PROTERRA DESIGN GROUP, LLC ENGINEER: NORTHAMPTON, MA 01060

DRIVING DIRECTIONS

DRIVING DIRECTIONS FROM AT&T IN ROCKY HILL, CT: TAKE 1-91 NORTH TO EXIT 29, MERGE ONTO CT-15/US-5 NORTH TOWARDS I-84, CONTINUE CNTO CT-15, MERGE ONTO I-84 EAST, TAKE EXIT 73 FOR CT-190 TOWARDS UNION, TURN RIGHT ONTO CT-190 E/BUCKLEY HIGHWAY, TURN RIGHT ONTO CT-171 E/BIGELOW HOLLOW ROAD, TURN LEFT ONTO CT-171 E/: CT-198 N / EASTFORD ROAD, ACCESS ROAD 0.6 MILES ON LEFT.

CONSTRUCTIO

ProTerra DESIGN GROUP, LLC i Short Street Suite 3 Northampton, MA 01060 Ph: (413)320-- 4918 Fox: (413)320-- 4917

NEW CINCULAR WHRELESS FCS, LLC "ATAT" SOO ENTERPRISE DRIVE ROCKY HILL, CT 08067

at&t

STAMP:

1/8/14 DRAWN: TBD CHECK: JMM/TEJ SCALE: SEE PLAN JOB NO.: 13-063

TITLE SHEET

SHEET TITLE:

T-1

SUBCONTRACTOR - GENERAL CONTRACTOR (CONSTRUCTION)

DEM - ORIGINAL EQUIPMENT MANUFACTURER

2. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.

3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES. RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK.

WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL, STATE AND FEDERAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS

5. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.

UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

THE SPECIFIED FOUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CONTRACTOR.

SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER, TI CABLES AND GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR AND/OR LANDLORD PRIOR TO CONSTRUCTION.

10 THE SURCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES, ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE OWNER.

11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY.

SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION AND RETURN DISTURBED AREAS TO ORIGINAL

13. THE SUBCONTRACTOR SHALL SUPERVISE AND DIRECT THE PROJECT DESCRIBED HEREIN, THE SUBCONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT.

14. SUBCONTRACTOR SHALL NOTIFY PROTERRA DESIGN GROUP, LLC 48 HOURS IN ADVANCE OF POURING CONCRETE OR BACKFILLING TRENCHES, SEALING ROOF AND WALL PENETRATIONS AND POST DOWNS, FINISHING NEW WALLS OR FINAL ELECTRICAL CONNECTIONS FOR ENGINEERING REVIEW.

15. CONSTRUCTION SHALL COMPLY WITH ALL SBA STANDARDS AND SPECIFICATIONS.

16, SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK, ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH

17. THE EXISTING CELL SITES ARE IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN OW TRAFFIC PERIODS AFTER MIDNIGHT.

18. IF THE EXISTING CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

SITE WORK GENERAL NOTES:

1. THE SUBCONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES PRIOR TO THE START OF CONSTRUCTION.

ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC, AND OTHER UTILITIES. WHERE ENCOUNTERED IN THE WORK SHALL BE PROTECTED AT ALL TIMES, AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY ENGINEERS, EXTREME CAUTION SHOULD BE USED BY THE SUBCONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. SUBCONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO A) FALL PROTECTION B) CONFINED SPACE C) ELECTRICAL SAFETY D) TRENCHING AND EXCAVATION

3. ALL SITE WORK SHALL BE AS INDICATED ON THE DRAWINGS AND PROJECT SPECIFICATIONS.

4. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE

THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE BIS EQUIPMENT AND TOWER

6. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANKMENT.

THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED

8. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF ENGINEERING, OWNER AND/OR LOCAL UTILITIES.

9. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE AND STABILIZED TO PREVENT EROSION AS SPECIFIED IN THE PROJECT SPECIFICATIONS.

SHACONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION, EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.

11. THE SUBCONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE SBA SPECIFICATION FOR SITE

CONCRETE & REINFORCING STEEL NOTES:

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST—IN—PLACE CONCRETE.

2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS, UNLESS NOTED OTHERWISE, A HIGHER STRENGTH (4000PSI) MAY BE USED. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 381 CODE REQUIREMENTS

3. REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60. DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE. SPLICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD, UNO.

4. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN

#6 AND LARGER #5 AND SMALLER & WWF1½ IN.
CONCRETE NOT EXPOSED TO EARTH OR WEATHER OR NOT CAST AGAINST THE GROUND SLAB AND WALL BEAMS AND COLUMNS

A CHAMFER 3/" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE WITH ACI 301

6. INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHORS SHALL BE PER MANUFACTURER'S WRITTEL RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO THE MANUFACTURERS RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWNINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR CONTRACTOR APPROVAL WHEN DRILLING HOLES IN CONCRETE. SPECIAL INSPECTIONS, REQUIRED BY GOVERN CODES, SHALL BE PERFORMED IN ORDER TO MAINTAIN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION WEDGE AND ORDER TO MIGHTEN MANUFACTURER'S MAXIMUM ALLOWABLE LOADS. ALL EXPANSION WEDGE AND ANCHORS SHALL BE STAINLESS STEEL OR HOT DIPPED GALVANIZED, EXPANSION BOLTS SHALL BE PROVIDED BY SIMPSON OR APPROVED EQUAL

CONCRETE CYLINDER TESTS ARE NOT REQUIRED FOR SLAB ON GRADE WHEN CONCRETE IS LESS THAN 50 CUBIC A CONCRETE CHINDER 1835 ARE NOT RECORDED FOR SEASON GROOT STALL BE PROVIDED BY THE CONCRETE SUPPLIER:

(A) RESULTS OF CONCRETE CYLINDER TEST PERFORMED AT THE SUPPLIERS PLANT.

(B) CERTIFICATION OF MINIMUM COMPRESSIVE STRENGTH FOR THE CONCRETE GRADE SUPPLIED.

FOR GREATER THAN 50 CUBIC YARDS THE GC SHALL PERFORM THE CONCRETE CYLINDER TEST.

. AS AN ALTERNATIVE TO ITEM 7. TEST CYLINDERS SHALL BE TAKEN INITIALLY AND THEREAFTER FOR EVERY 50 YARDS OF CONCRETE FROM EACH DIFFERENT SATCH PLANT.

E SOURPMENT SHALL NOT BY PLACED ON NEW PARS FOR SYVEN DAYS AFTER PAD IS POURED, UNLESS IT IS VERIFIED BY CYLINDER TESTS THAT COMPRESSIVE STRENGTH HAS BEEN ATTAINED.

STRUCTURAL STEEL NOTES:

1. ALL STEEL WORK SHALL BE PAINTED OR GALVANIZED IN ACCORDANCE WITH THE DRAWINGS AND SBA SPECIFICATIONS UNLESS OTHERWISE NOTED. STRUCTURAL STEEL SHALL BE ASTM-A-36 UNLESS OTHERWISE NOTED ON THE SITE SPECIFIC DRAWINGS. STEEL DESIGN, INSTALLATION AND BOLTING SHALL BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "MANUAL OF STEEL CONSTRUCTION"

2. ALL WELDING SHALL BE PERFORMED USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND AWS DII. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION", 9TH EDITION, PAINTED SURFACES SHALL BE TOUCHED UP.

5. BOLTED CONNECTIONS SHALL USE BEARING TYPE ASTM A325 BOLTS (¾"ø) AND SHALL HAVE MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE GALVANIZED OR STAINLESS STEEL. NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE %" DIA. ASTM A 307 BOLTS (GALV) UNLESS

CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ENGINEER REVIEW & APPROVAL ON PROJECTS REQUIRING STRUCTURAL STEEL

6 ALL STRUCTURAL STEEL WORK SHALL BE DONE IN ACCORDANCE WITH AISC SPECIFICATIONS.

SOIL COMPACTION NOTES:

1. EXCAVATE AS REQUIRED TO REMOVE VEGETATION AND TOPSOIL TO EXPOSE NATURAL SUBGRADE AND PLACE CRUSHED STONE AS REQUIRED.

COMPACTION CERTIFICATION: AN INSPECTION AND WRITTEN CERTIFICATION BY A QUALIFIED GEOTECHNICAL TECHNICIAN OR ENGINEER IS ACCEPTABLE.

3 AS AN ALTERNATE TO INSPECTION AND WRITTEN CERTIFICATION, THE "UNDISTURBED SOIL" BASE SHALL BE COMPACTED WITH "COMPACTION EQUIPMENT". LISTED BELOW, TO AT LEAST 90% MODIFIED PROCTOR MAXIMUM DENSITY PER ASTM D 1557 METHOD C.

COMPACTED SUBBASE SHALL BE UNIFORM AND LEVELED, PROVIDE 6" MINIMUM CRUSHED STONE OR GRAVEL COMPACTED IN 3" LIFTS ABOVE COMPACTED SOIL, GRAVEL SHALL BE NATURAL OR CRUSHED WITH 100% PASSING #1

5. AS AN ALTERNATE TO ITEMS 2 AND 3, THE SUBGRADE SOILS WITH 5 PASSES OR A MEDIUM SIZED VIBRATORY PLATE COMPACTOR (SUCH AS BOMAG BPR 30/38) OR HAND-OPERATED SINGLE DRUM VIBRATORY ROLLER (SUCH AS BOMAG BW 55E). AND SOFT AREAS THAT ARE ENCOUNTERED SHOULD BE REMOVED AND REPLACED WITH A WELL-GRADED GRANULAR FILL AND COMPACTED AS STATED ABOVE.

COMPACTION EQUIPMENT NOTES:

1. HAND OPERATED DOUBLE DRUM, VIBRATORY ROLLER, VIBRATORY PLATE COMPACTOR OR JUMPING JACK

CONSTRUCTION NOTES:

SUBCONTRACTOR SHALL FIELD VERIFY SCOPE OF WORK, SBA ANTENNA PLATFORM LOCATION AND UTILITY TRENCHWORK.

SUBCONTRACTOR SHALL COORDINATE RF WORK AND PROCEDURES WITH CONTRACTOR.

SUBCONTRACTOR SHALL FURNISH AND INSTALL CABLE LADDER RACK, CABLE TRAY AND/OR ICE BRIDGE, AND CONDUIT AS REQUIRED TO SUPPORT CABLES TO THE NEW BTS LOCATION.

ELECTRICAL INSTALLATION NOTES:

1, WIRING, RACEWAY, AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE

2. SUBCONTRACTOR SHALL MODIFY OR INSTALL CABLE TRAY SYSTEM AS REQUIRED TO SUPPORT RF AND TRANSPORT CABLING TO THE NEW BTS EQUIPMENT, SUBCONTRACTOR SHALL SUBMIT MODIFICATIONS TO CONTRACTOR FOR

ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC AND

4 CARLES SHALL NOT BE ROUTED THROUGH LADDER-STYLE CABLE TRAY RUNGS.

5. EACH END OF EVERY POWER, GROUNDING, AND TI CONDUCTOR AND CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2 INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL), THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA, AND MATCH

6. POWER PHASE CONDUCTORS (I.E., HOTS) SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, ½ INCH PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). PHASE CONDUCTOR COLOR CODES SHALL CONFORM WITH THE NEC AND OSHA.

7. ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH ENGRAVED LAWACOID PLASTIC LABELS. ALL FOURDMENT SHALL BE LABELED WITH THEIR VOLTAGE RATING, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER AMPACITY RATING, AND BRANCH CIRCUIT ID NUMBERS (I.E., PANELBOARD AND CIRCUIT ID'S).

8. PANELBOARDS (ID NUMBERS) AND INTERNAL CIRCUIT BREAKERS (CIRCUIT ID NUMBERS) SHALL BE CLEARLY LABELED WITH ENGRAVED LAMACOID PLASTIC LABELS.

9. ALL TIE WRAPS SHALL BE CUT FLUSH WITH APPROVED CUTTING TOOL TO REMOVE SHARP EDGES.

10. POWER CONTROL, AND FOUIPMENT GROUND WIRING IN TUBING OF CONDUIT SHALL BE SINGLE CONDUCTOR (#34 AWG OR LARGER), 500 V, OIL RESISTANT THHN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR 9D C (WE'T AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED

11. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE CONDUCTOR (#6 AWG OR LARGER), 660 V, OIL RESISTANT THHN OR THWN-2 GREEN INSULATION, CLASS B STRANDED COPPER CABLE RATED FOR 90 °C (WET AND DRY) OPERATION; LISTED OR LABELED FOR THE LOCATION AND RACEWAY SYSTEM USED, UNLESS OTHERWISE SPECIFIED.

12. SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED OUTDOORS, OR BELOW GRADE, SHALL BE SINGLE CONDUCTOR #2 AWG SOLID TINNED COPPER CABLE, UNLESS OTHERWISE SPECIFIED.

13. POWER AND CONTROL WIRING, NOT IN TUBING OR CONDUIT, SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#34 AWG OR LARGER), 600 V. OIL RESISTANT THEN OR THWN-2, CLASS B STRANDED COPPER CABLE RATED FOR (WET AND DRY) OPERATION; WITH DUTER JACKET: LISTED OR LABELED FOR THE LOCATION USED, UNLESS OTHERWISE

14. ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRENUTS BY HARGER (OR EQUAL), LUGS AND WIRENUTS SHALL BE RATED FOR OPERATION AT NO LESS THAN 75°C (90°C IF AVAILABLE).

15, RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.

16 NEW RACEWAY OR CARLE TRAY WILL MATCH THE EXISTING INSTALLATION WHERE POSSIBLE

17. ELECTRICAL METALLIC TUBING (EMT) OR RIGID NONMETALLIC CONDUIT (i.E., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE BO FOR LOCATIONS SUBJECT TO PHYSICAL DAMAGE) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

18. ELECTRICAL METALLIC TUBING (EMT), ELECTRICAL NONMETALLIC TUBING (EMT), OR RIGID NONMETALLIC CONDUIT (RIGID PVC, SCHEDULE 40) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.

GALVANIZED STEEL INTERMEDIATE METALLIC CONDUIT (IMC) SHALL BE USED FOR OUTDOOR LOCATIONS ABOVE

20. RIGID NONMETALLIC CONDUIT (LE., RIGID PVC SCHEDULE 40 OR RIGID PVC SCHEDULE 80) SHALL BE USED UNDERGROUND; DIRECT BURIED, IN AREAS OF OCCASIONAL LIGHT VEHICLE TRAFFIC OR ENCASED IN REINFORCED CONCRETE IN AREAS OF HEAVY VEHICLE TRAFFIC.

LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE TION OCCURS OR FLEXIBILITY IS NEEDED

22. CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED, SETSCREW FITTINGS ARE NOT ACCEPTABLE.

23. CABINETS, BOXES AND WIREWAYS SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.

24. CABINETS, BOXES AND WIREWAYS TO MATCH THE EXISTING INSTALLATION WHERE POSSIBLE

25. WIREWAYS SHALL BE EPOXY-COATED (GRAY) AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARD; SHALL BE PANDUIT TYPE E (OR EQUAL); AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR

26. EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES, AND PULL BOXES SHALL BE CALVANIZED OR EPOXY-COATED SHEET STEEL, SHALL MEET OR EXCEED UL 50, AND RATED NEMA 1 (OR BETTER) INDOORS, OR NEMA 3R (OR BETTER) OUTDOORS.

27, METAL RECEPTACLE, SWITCH, AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED, OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.

28 NONMETALLIC RECEPTACIF, SWITCH, AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2; AND RATED NEMA 1 (OR BETTER) INDOORS, OR WEATHER PROTECTED (WP OR BETTER) OUTDOORS.

29. THE SUBCONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CONTRACTOR BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.

30. THE SUBCONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD AGAINST LIFE AND

IS NOT BLOCKED

31. ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE LOCAL CODES. 32. CONDUIT ROUTINGS ARE SCHEMATIC, SUBCONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT

ProTerra

Northampton, MA 01060 Ph: (4\3)320~4918 Fox: (4\3)320~4917

CLIENT:



Salem, NH 03079



277 C RT 2067 198 3282 T.A.R. 8 SITE NAME: WOODSTO SITE NUMBER: C. ADDRESS: ROUT! WOODSTOCK, CT (

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at&t

STAMP:

1/8/14 DATE:

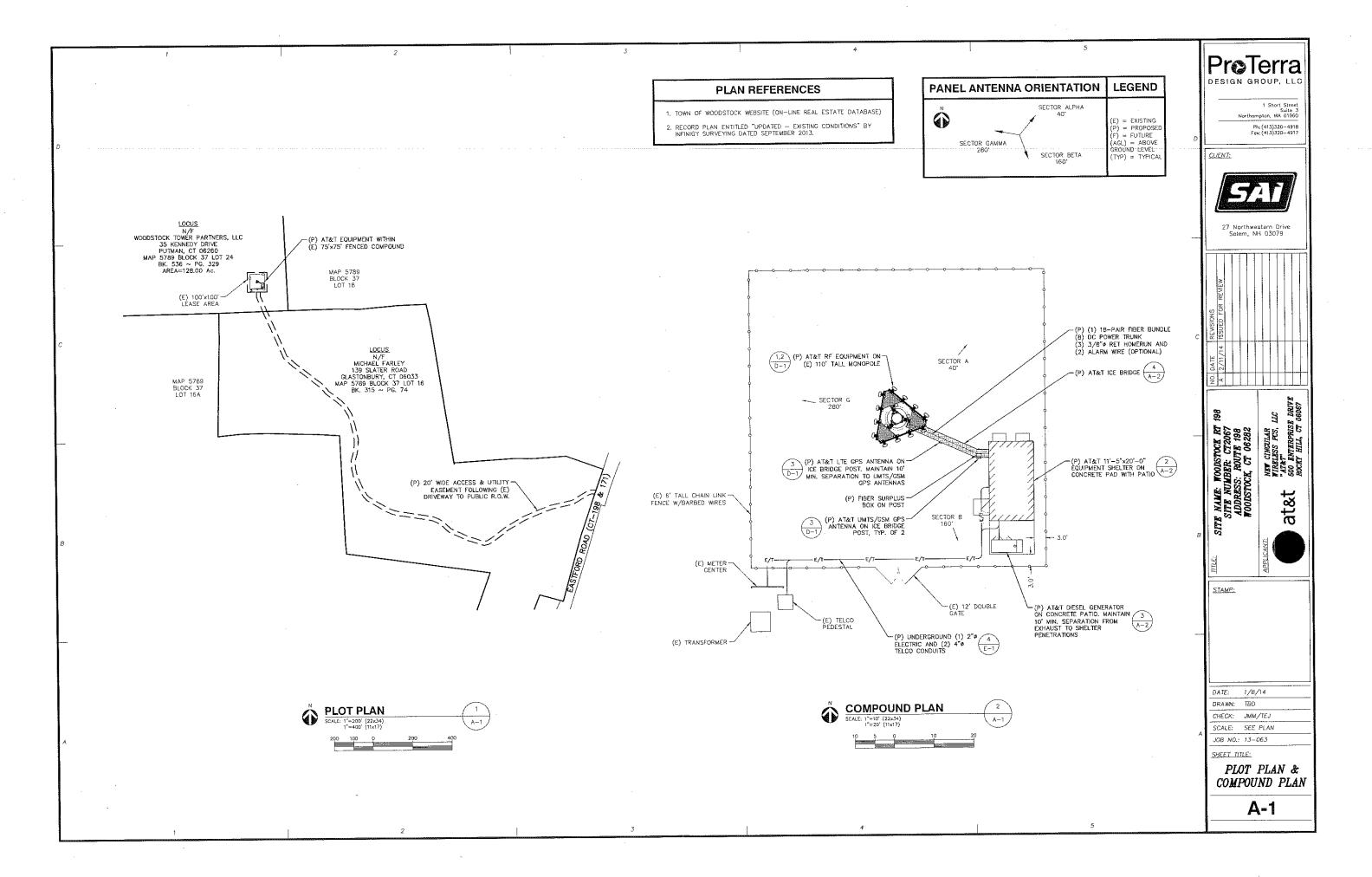
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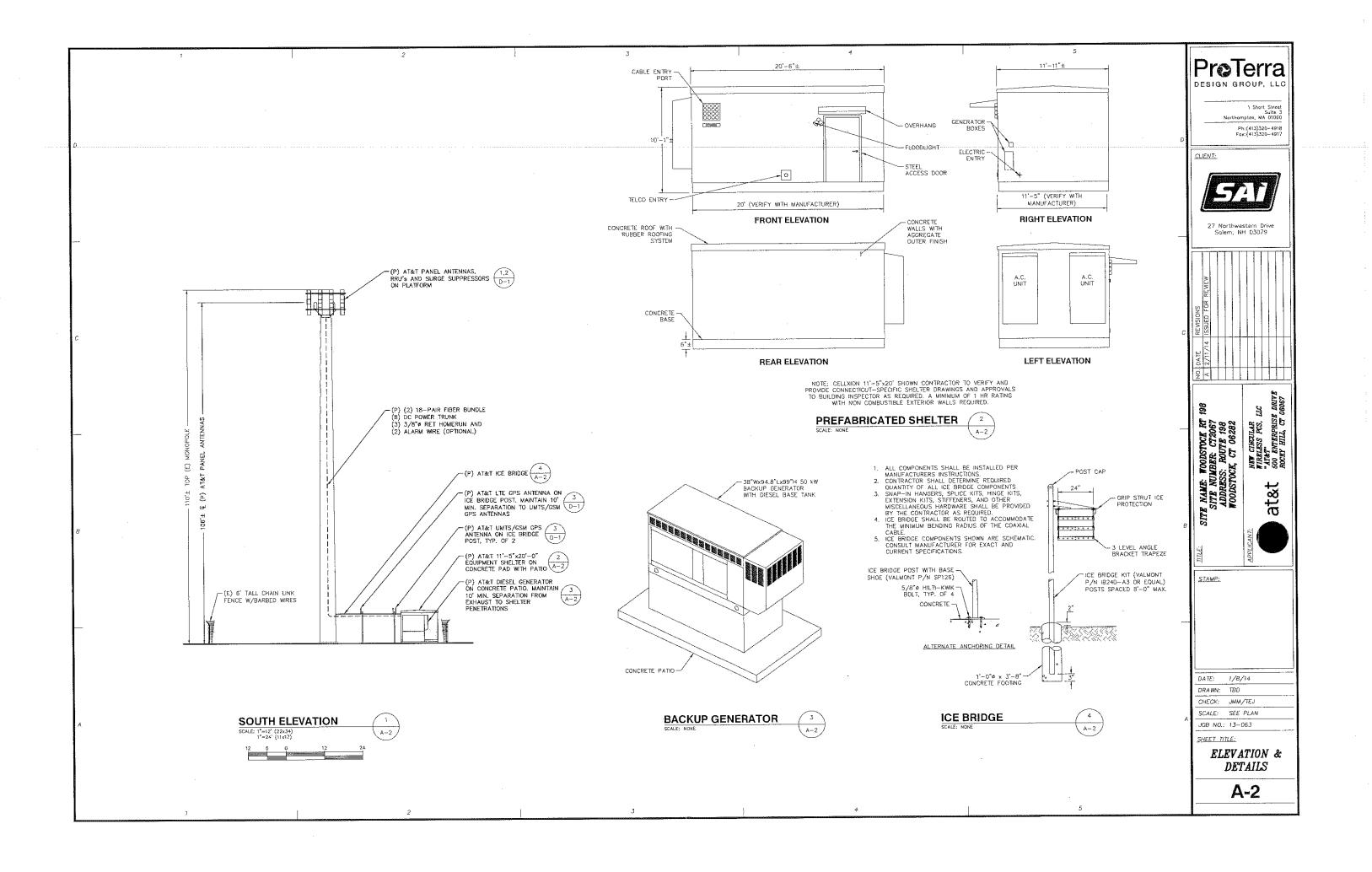
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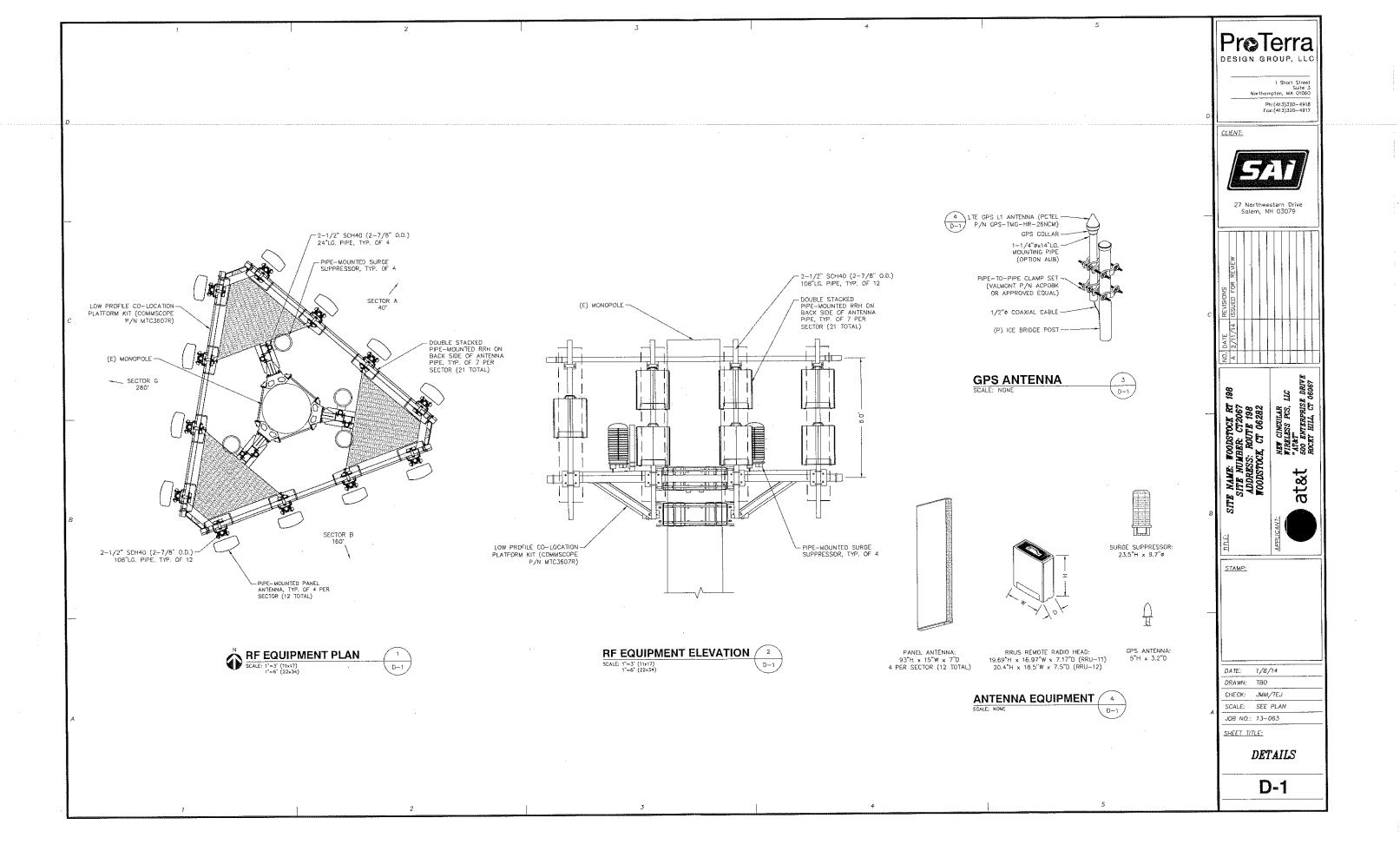
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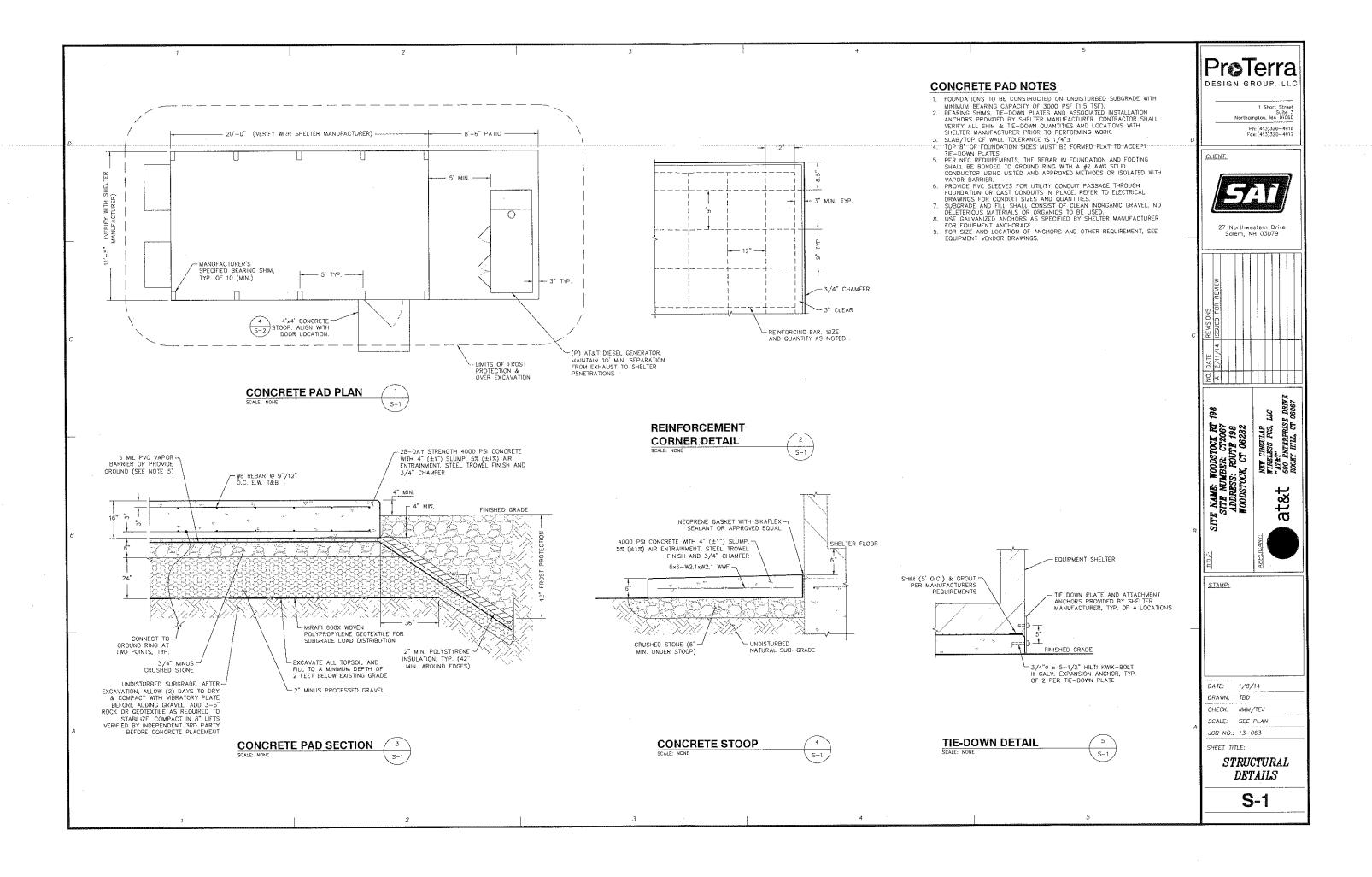
GENERAL NOTES

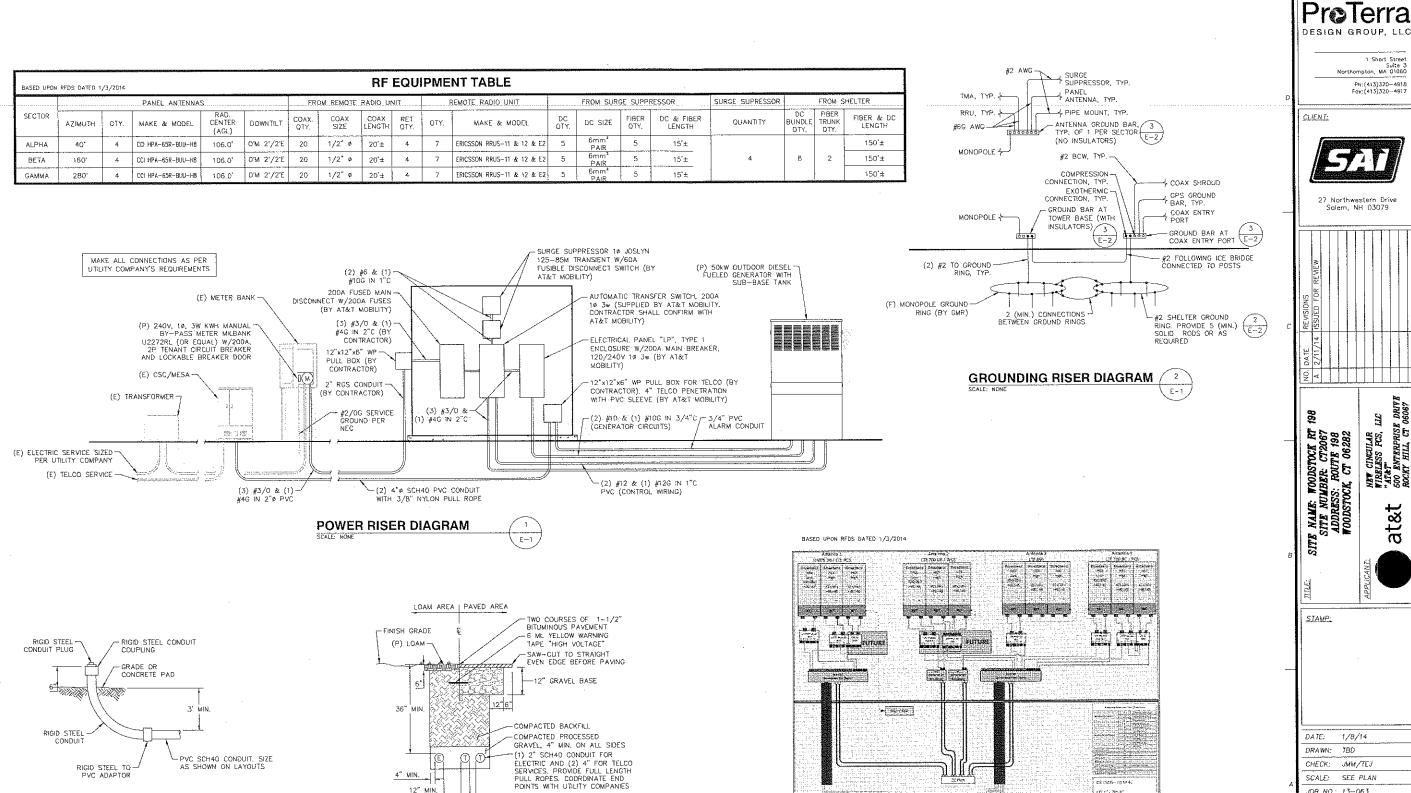
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RIGID STEEL TO -

STUB-UP CONDUIT DETAIL

4" MIN.

12" MIN.

NOTES

1. DETAIL AS SHOWN IS FOR SECONDARY ELECTRIC SERVICE. PRIMARY

HIGH VOLTAGE SERVICE REQUIRES 4" CONCRETE ENCASEMENT.
2. SCHBO PVC MAYBE REQUIRED IN AREAS UNDER THE INFLUENCE OF VEHICULAR TRAFFIC.

E-1

BURIED CONDUIT SECTION

1 Shart Street Suite 3 Northompton, MA 01060 Ph: (413)320-4918 Fax: (413)320-4917 CLIENT: 27 Northwestern Drive Salem, NH 03079 RISE DRIVE CT 06067 277 E NAME: WOODSTOCK RT 1 SITE NUMBER: CT2067 ADDRESS: ROUTE 198 WOODSTOCK, CT 06282 NEW CINCULAR WIRELESS PCS, "ATAT" 500 ENTERPRIS ROCKY HILL, CT at&t STAMP: DATE: 1/8/14 DRAWN: TBD CHECK: JMM/TEJ SCALE: SEE PLAN JOB NO.: 13-063 ELECTRICAL & GROUNDING

DETAILS

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SECTORS A,B, & G RF PLUMBING DIAGRAM

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