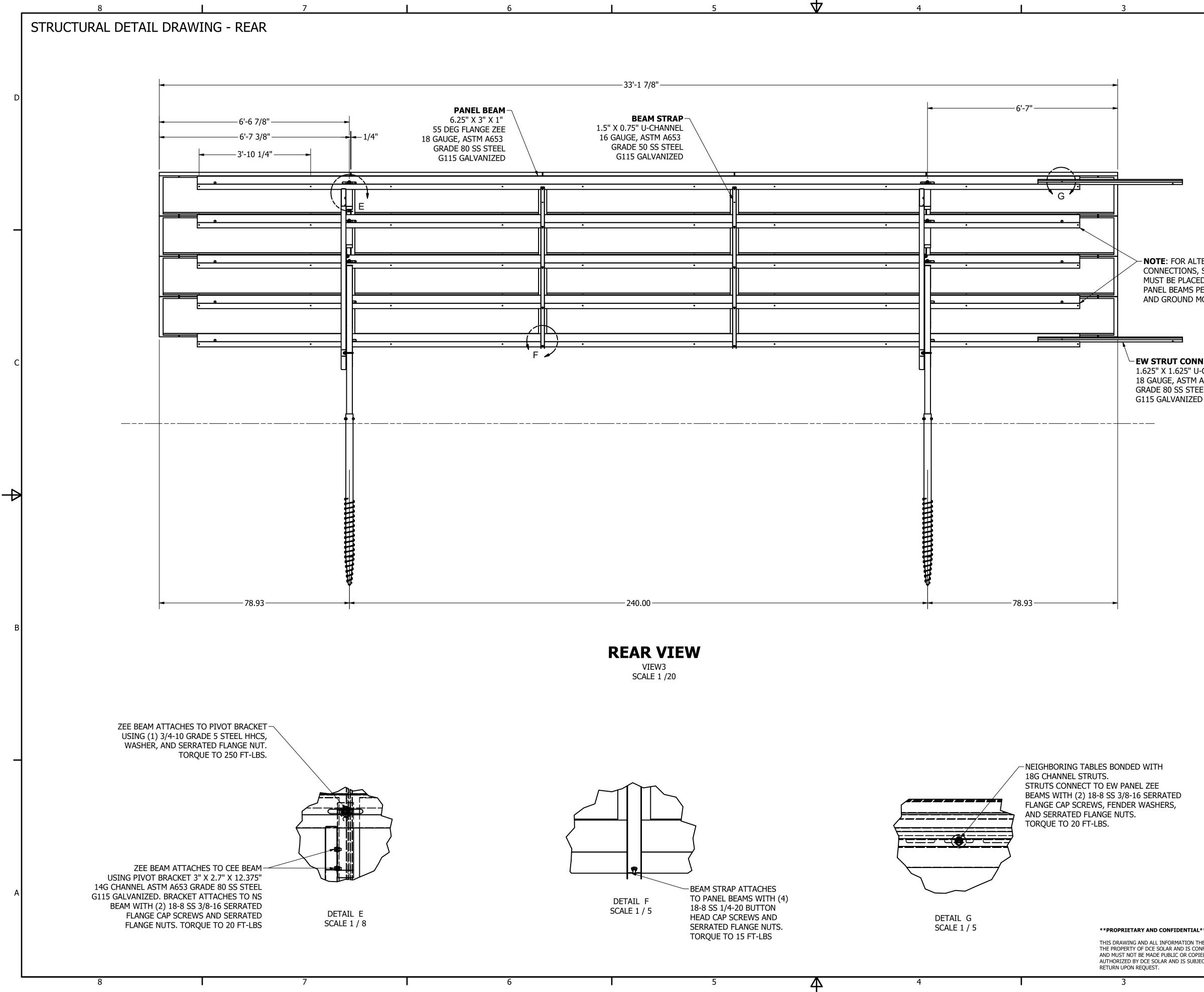
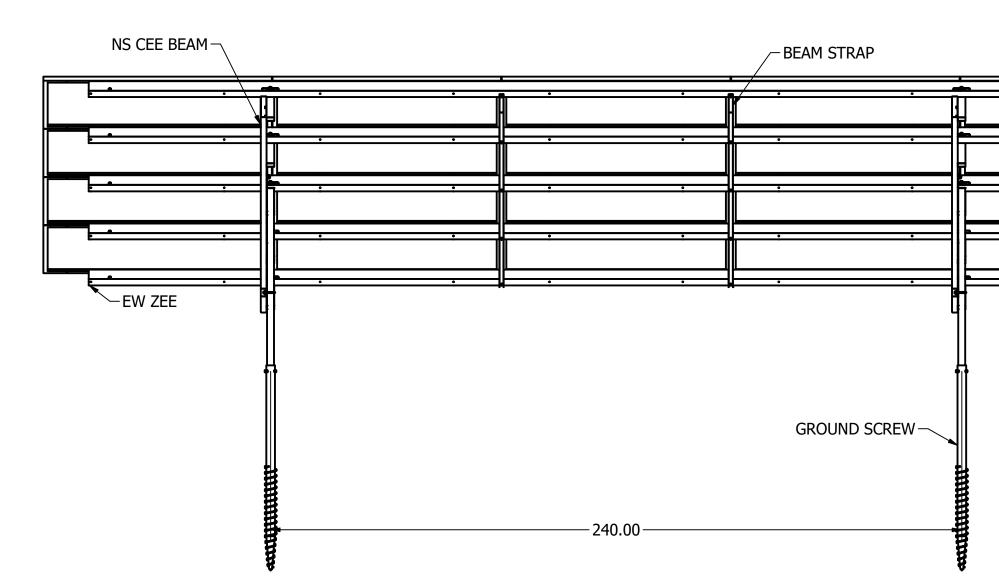


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	PROJECT INFORMATION INSTALLATION ADDRESS: LOT 33 PROSPECT STREET, BURLINGTON, CT 06013
	Structural General Notes
	1. The contractor will be solely responsible for all construction means, methods, techniques, sequences and procedures and shall at all times take reasonable precautions for the safety of its employees on the project, and shall comply with all applicable provisions of federal, state, and municipal safety laws and building construction codes.
	2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.
	3. These drawings and notes are for this specific project and no other use is authorized.
	4. Structure designed in accordance with the 2018 Connecticut State Building Code, ASCE 7-10, AISC 360-10 (14th Edition), and AISI S100-12: ASD Snow Loads: -Ground Snow Load pg = 40 psf
	-Importance Factor Is=0.8 -Exposure Factor Ce=1.0 -Slope Snow Load ps= 30 psf
5 BEAM X 2" X 0.6"	Wind Loads: -Basic Wind Speed V= 110 mph (MRI = 0.93 or 25 year) -Iw = 1 -Exposure = C -Wind Design performed in accordance with the requirements of ASCE -
GAUGE, ASTM A653 ADE 80 SS STEEL 15 GALVANIZED	Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.
	Seismic Loads: -SS = $0.182.g$, S1 = $0.065g$ -Site Class = D
	-SDS = 0.190g, SD1 = 0.100g -Seismic Design Category = B -Ordinary Steel Cantilever Column System
I GLE BRACE 5" X 1.75" U-CHANNEL GAUGE, ASTM A653 ADE 50 SS STEEL	5. Material strengths: -Hot-rolled structural steel ASTM A992 GR50 w/80 ksi yield. -Cold Formed Steel Sections comply w/ASTM A1003, structural grade,
15 GALVANIZED	galvanized to Grade as noted. -Formed Steel Brackets - ASTM A653 Grade 50 SS, G115 HDG -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85 -Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.
	6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.
	7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.
	8. For the purposes of this project, all arrays are classified as Exterior Arrays.
TO LEG ADAPTER USING (1) 3/8-16 U-BOLT AND (2) SERRATED FLANGE NUTS. TORQUE TO 20 FT-LBS	9. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.
	Engineer of Record
	~~~///////////////////////////////////
DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED TOLERANCES ARE AS FOLLOWS:	Material: Weight: 1752.640 lbmass CT-BF-DB, RISEN SOLAR RSM144-6-380BMDG, 4x5 25 DEG BURLINGTON SOLAR ONE -
$X = \pm 0.050" (1.27mm)$ $XX = \pm 0.015" (0.38mm)$ $XXX = \pm 0.005" (0.127mm)$ ANGLE = $\pm 5^{\circ}$	4x5, 25 DEG, BURLINGTON SOLAR ONE -       A         Description:       BIFACIAL - RISEN SOLAR, VEROGY         Project:       BURLINGTON SOLAR ONE - BIFACIAL - RISEN SOLAR
MIN. BREAK = 0.012" (0.3mm) SURFACE FINISH = 63 (US)	Drawn:         JSCOTT         Date:         7/15/2020           Scale:         Sheet:         1 of 3
HERE IN IS NFIDENTIAL LED UNLESS ECT TO	19410 Jetton Rd, Ste 220 Cornelius, NC, 28031 www.dcesolar.comFormat:Part NumberRev:046350
Elevating the Future for Solar Made in America	Phone:1-704-659-7474



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		INS 060	TALLATION ADDRESS:		INFORMATION PROSPECT STREET, BURLINGTO	N, CT	
			Ictural General Notes				
		1. T met take proj	he contractor will be s hods, techniques, seque reasonable precaution fect, and shall comply v	uences ans for the with all a	ponsible for all construction mear nd procedures and shall at all tim e safety of its employees on the pplicable provisions of federal, st ding construction codes.	ies	D
			f existing conditions m sult DCE Solar before p		cessary to revise structural detail ng with any change.	s,	
			hese drawings and no uthorized.	tes are fo	or this specific project and no oth	er use	
		Buil ASD	ding Code, ASCE 7-10,		e with the 2018 Connecticut State 60-10 (14th Edition), and AISI S10		
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STRUT CON	NNECTORS	-Im	portance Factor Is=0.8	-			
D ON 2ND 8 ER INSTALL	& 4TH EW .ATION MANUAL	•	posure Factor Ce= $1.0$ pe Snow Load ps= $30$	psf			
10UNT LAY(	TUC	510		201			
			d Loads: sic Wind Speed V= 11(	) mph (N	1RI = 0.93 or 25 year)		
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			oosure = C od Design performed i	a accord:	ance with the requirements of AS	CF -	
<b>NECTOR</b> -CHANNEL A653 EL		Win	• •	efer to V	Vind Tunnel Report by UWO BLW		С
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			= 0.182.g, S1 = 0.065 e Class = D	9 9			
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		5. №	laterial strengths:				
		-Ho	t-rolled structural steel		992 GR50 w/80 ksi yield.		A
			d Formed Steel Section anized to Grade as no		ly w/ASTM A1003, structural grad	le,	
		-For -I-B -Pla	med Steel Brackets - A	ASTM A6 lot Dip G Galvani			
					been designed for worst-case loa he array per the wind tunnel repo	-	
			oundation embedment State Licensed Geotech	•	are to be calculated and sealed b gineer.	y a	В
		8. F Arra	• •	s project,	all arrays are classified as Exterio	or	
		con	nection design, and de	terminat	ineer includes member design, ion of design base reactions only		
		obst	tructions, determinatio	n of site	do not conflict with existing site -specific foundation and geotechr - specifically noted is by others.	nical	
		Eng	ineer of Record				
			No. 27915			0	
			NO. 27915	101010°			
Γ							
	DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED	Material:					
Т	TOLERANCES ARE AS FOLLOWS:	Weight:	-		RSM144-6-380BMDG,		
	$X = \pm 0.050" (1.27mm)$ $XX = \pm 0.015" (0.38mm)$	Description	4x5, 25 DEG, BUR η: BIFACIAL - RISEN				А
	$XXX = \pm 0.005" (0.127mm)$ ANGLE = $\pm 5^{\circ}$	Project:		R ONE - E	BIFACIAL - RISEN SOLAR		
**	MIN. BREAK = 0.012" (0.3mm)	Drawn:	JSCOTT		Date: 7/15/2020		
HERE IN IS	SURFACE FINISH = 63 (US)	Scale:	10/10 1644-6 0.4 01 000	Format	Sheet: 2 of 3	Rev:	
ED UNLESS			19410 Jetton Rd, Ste 220 Cornelius, NC, 28031 www.dcesolar.com Phone:1-704-659-7474	D	4635	0. Rev.	
Eleva	ting the Future for Solar Made in America	2	Phone:1-704-659-7474		1	5	

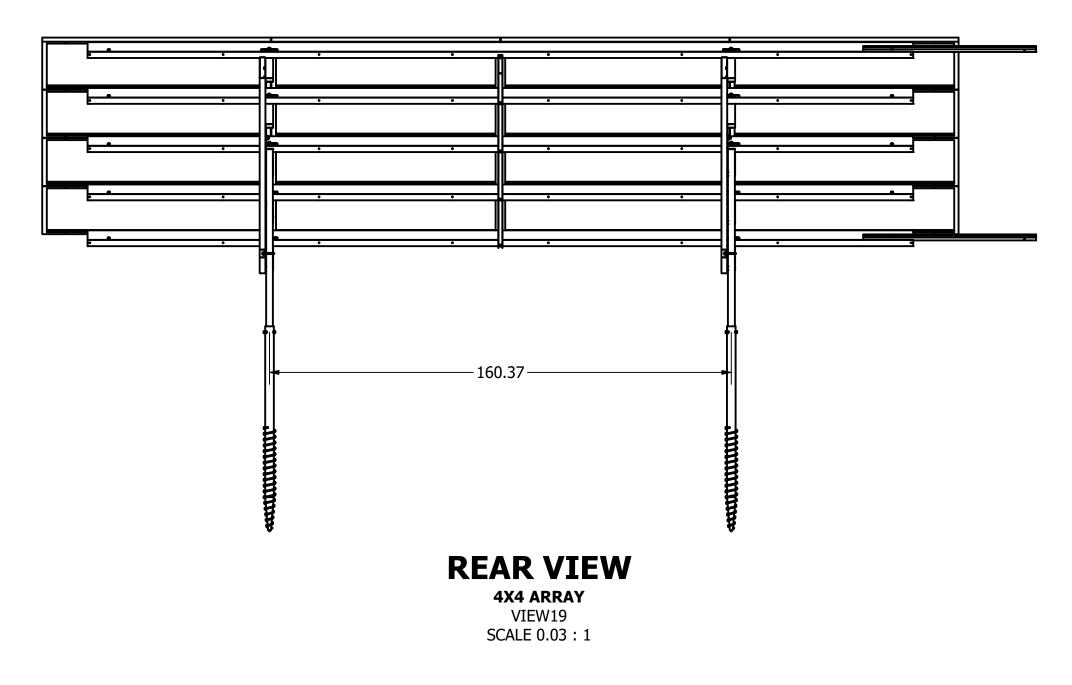


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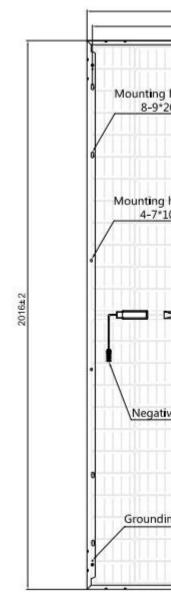
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**REAR VIEW 4X5 ARRAY** VIEW13 SCALE 0.03 : 1



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Control       Contro       Control       Control	PANEL BEAM	6.25Z3X1X55	DEG	A653 SS Gr80	18GA	and municipal safety laws and building construction codes.			
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Index Provided Bit Instruction of Preference December 2011       Media Loads       <		I				-Slope Snow Load ps= 30 psf			
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PANEL		TION IPTION			PROJECT INFORMATION INSTALLATION ADDRESS: LOT 33 PROSPECT STREET, BURLINGTON, CT
RISEN SOLAR TECHNOLOGY					06013
RSM144-6-380BMDG 2016					Structural General Notes
	998				1. The contractor will be calchy recommobile for all construction means
MATER	MATERIAL DESCRIPTION r MATERIAL DESCRIPTION t				1. The contractor will be solely responsible for all construction means,methods, techniques, sequences and procedures and shall at all timestake reasonable precautions for the safety of its employees on theproject, and shall comply with all applicable provisions of federal, state,
25Z3X1X55DEG		A653	3 SS Gr80	18GA	and municipal safety laws and building construction codes.
S2X0.625			3 SS Gr80	14GA	2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.
75CU1.75		A653	3 SS Gr50	14GA	
5CU0.75		A653	3 SS Gr50	16GA	3. These drawings and notes are for this specific project and no other use is authorized.
S2.375x0.154			GRADE C	-	4. Structure designed in accordance with the 2018 Connecticut State Building Code, ASCE 7-10, AISC 360-10 (14th Edition), and AISI S100-12:
PULL TEST L	OADS (GRO REAR (lbs		-	IT (LBS)	ASD
	2,300	-	2	150	Snow Loads: -Ground Snow Load pg = 40 psf
UPLIFT*	2,950 4,200		-	.800 .550	-Importance Factor Is=0.8
	900		1	100	-Exposure Factor Ce=1.0 -Slope Snow Load ps= 30 psf
	NOTES				
SUMED AS 70% ( . TEST IN CASE F					TO Wind Loads: -Basic Wind Speed V= 110 mph (MRI = 0.93 or 25 year)
IF NO REFUSAL					-Iw = 1
CASE OF REFUSA RAL FORCES USE					-Exposure = C -Wind Design performed in accordance with the requirements of ASCE -
	PILE REME				Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT
ON REQUIRING					Laboratory dated 12/11/14.
ALVANIZATION I		IDELINES			Seismic Loads:
NING ZINC DUS	T (IN ACCOF	RDANCE W	ITH "ASTM A 7	80-01"	-SS = 0.182.g, S1 = 0.065g -Site Class = D
ACCORDANCE V	VITH "ASTM	A 780-01"	SECTION A3)	ONE OF THE	
BE FOLLOWED	TO MAINTA	IN THE DC	E WARRANTY I	REQUIREMEN	NTSSeismic Design Category = B -Ordinary Steel Cantilever Column System
		998±	6		5. Material strengths: -Hot-rolled structural steel ASTM A992 GR50 w/80 ksi yield.
1	1			7	-Cold Formed Steel Sections comply w/ASTM A1003, structural grade,
	<b>.</b>				galvanized to Grade as noted. -Formed Steel Brackets - ASTM A653 Grade 50 SS, G115 HDG
	Mountir 8-9	9*20	Drainage holes		-I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85
				<b></b>	-Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.
	Mountin 4-7	ng holes /*10			6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.
	V				
~					7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.
2016±2				400±1 1,76±1 1,76±1	Б
		Namej	plate		8. For the purposes of this project, all arrays are classified as Exterior Arrays.
	$  \rangle$				9. Scope of work by Structural Engineer includes member design,
	\Neg.	ative(-)100mm	ositive(+)270mm		connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site
		<u></u>	ositive(+)270mmy		obstructions, determination of site-specific foundation and geotechnical
	, 				parameters, and all other work not specifically noted is by others.
	Groun	iding holes			Engineer of Record
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					ELOCONAL ENCINITATION
			A -		
			DIMENSIONS AR		Material:
			UNLESS OTHERN TOLERANCES ARE		Weight: 1752.640 lbmass CT-BF-DB, RISEN SOLAR RSM144-6-380BMDG,
				50" (1.27mm)	4x5, 25 DEG, BURLINGTON SOLAR ONE -
			$.XX = \pm 0.01$ $.XXX = \pm 0.005$	" (0.12/mm)	Description: <b>BIFACIAL - RISEN SOLAR, VEROGY</b>
			ANGLE = MIN. BREAK = 0.0	±5°	Project:       BURLINGTON SOLAR ONE - BIFACIAL - RISEN SOLAR         Drawn:       JSCOTT         Date:       7/15/2020
**PROPRIETARY AN			SURFACE FINIS		Drawn:         JSCOTT         Date:         7/15/2020           Scale:         Sheet:         3         of         3
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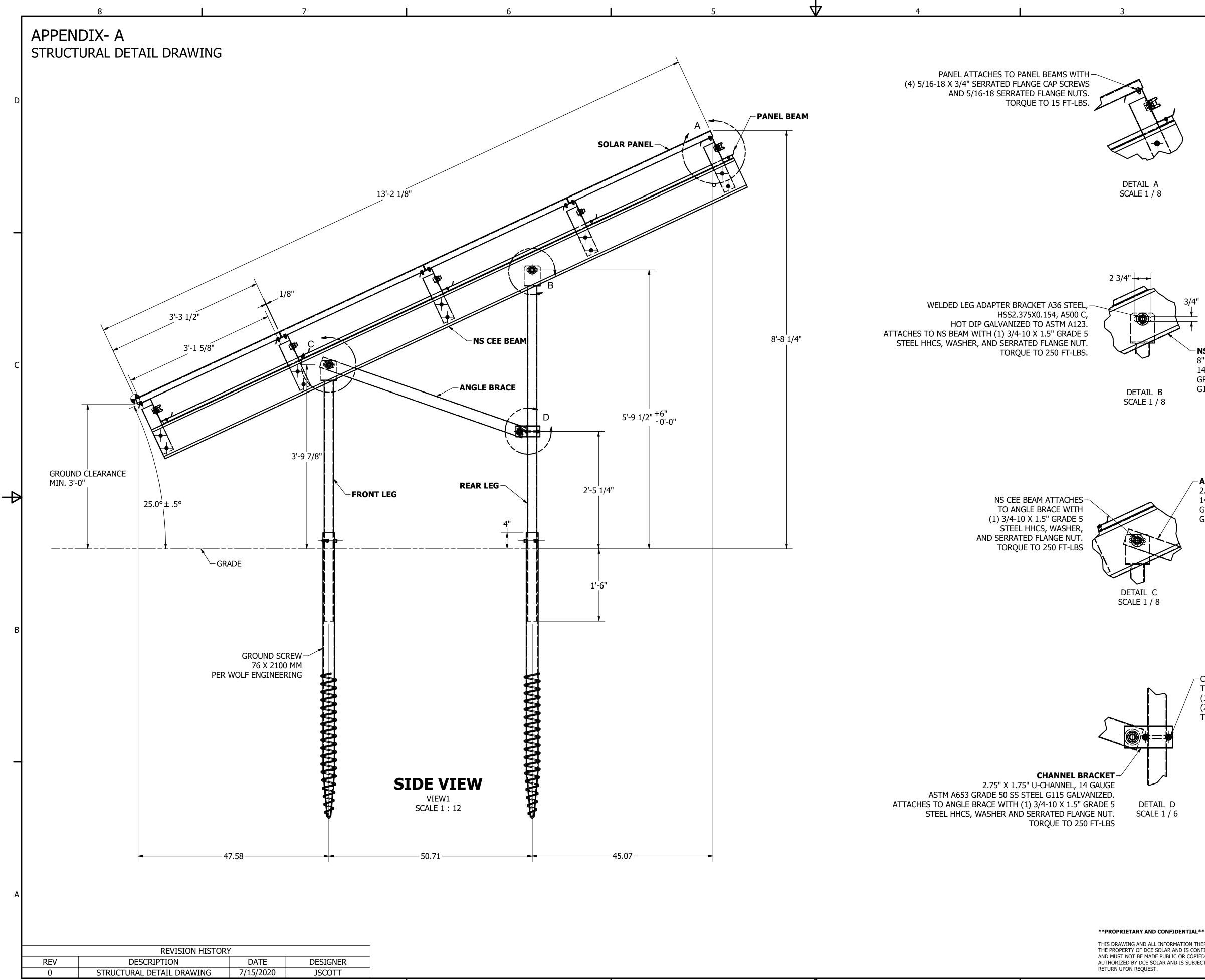
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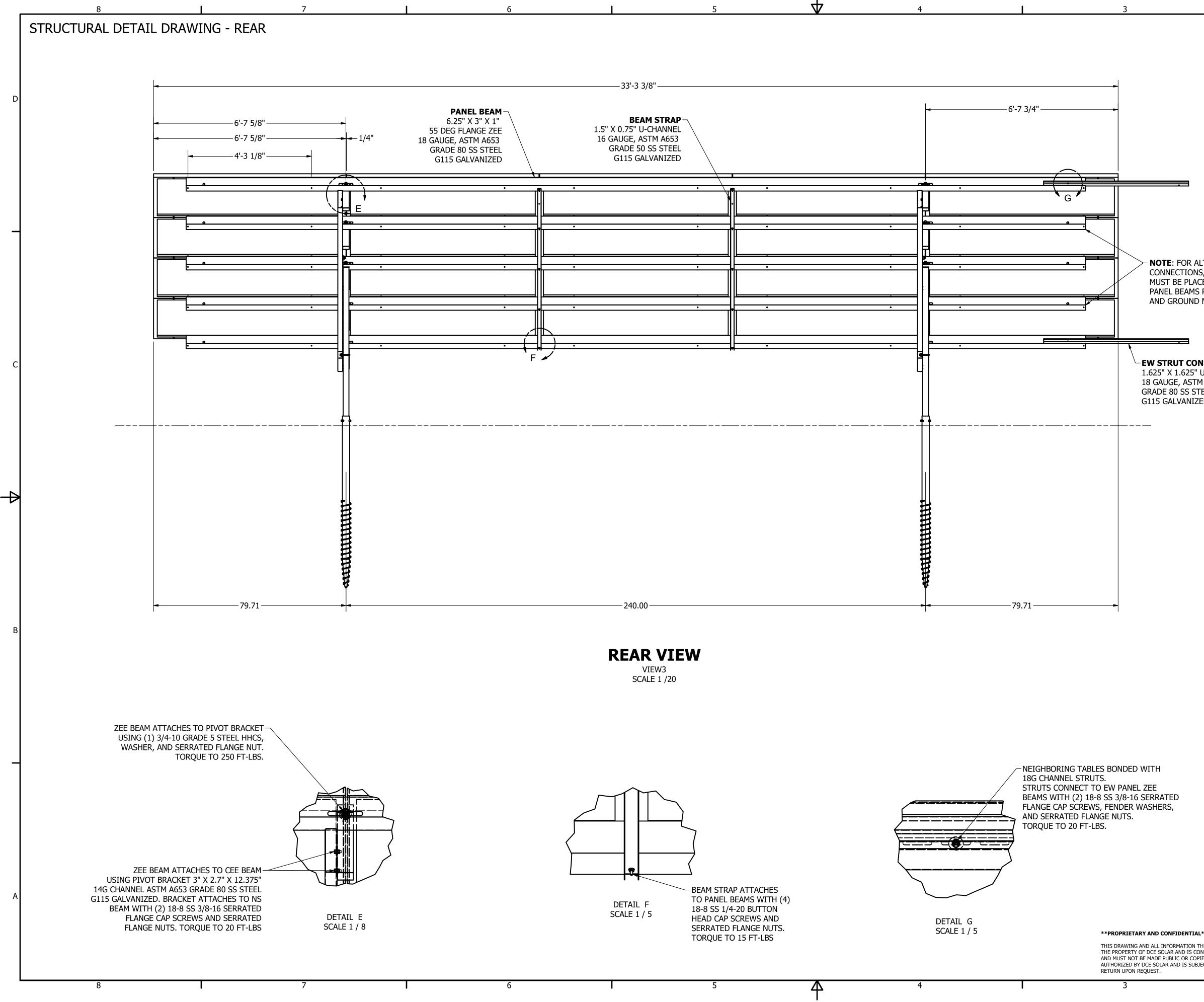
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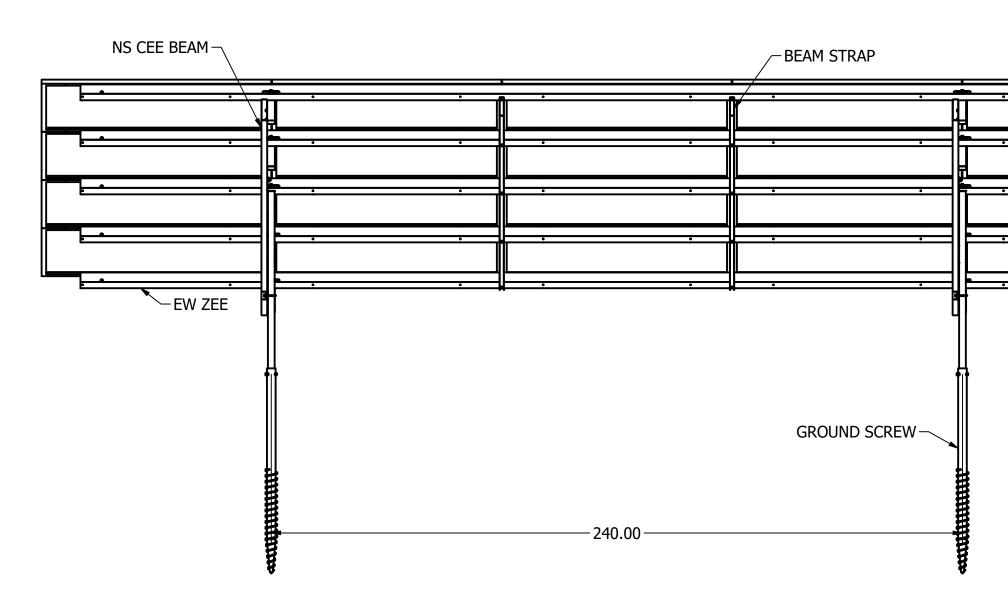
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			PROJECT INFORMATION INSTALLATION ADDRESS: LOT 33 PROSPECT STREET, BURLINGTON, CT 06013
			Structural General Notes
			1. The contractor will be solely responsible for all construction means, methods, techniques, sequences and procedures and shall at all times take reasonable precautions for the safety of its employees on the project, and shall comply with all applicable provisions of federal, state, and municipal safety laws and building construction codes.
			2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.
			3. These drawings and notes are for this specific project and no other use is authorized.
			4. Structure designed in accordance with the 2018 Connecticut State Building Code, ASCE 7-10, AISC 360-10 (14th Edition), and AISI S100-12: ASD Snow Loads: -Ground Snow Load pg = 40 psf
			-Importance Factor Is=0.8 -Exposure Factor Ce=1.0 -Slope Snow Load ps= 30 psf
			Wind Loads: -Basic Wind Speed V= 110 mph (MRI = 0.93 or 25 year) -Iw = 1 -Exposure = C
	0.6" , ASTM A653 SS STEEL		-Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.
			Seismic Loads: -SS = $0.182.g$ , S1 = $0.065g$ -Site Class = D
			-SDS = 0.190g, SD1 = 0.100g -Seismic Design Category = B -Ordinary Steel Cantilever Column System
14 gauge	S <b>RACE</b> .75" U-CHANNEL E, ASTM A653 D SS STEEL		5. Material strengths: -Hot-rolled structural steel ASTM A992 GR50 w/80 ksi yield. -Cold Formed Steel Sections comply w/ASTM A1003, structural grade,
G115 GAL	VANIZED		galvanized to Grade as noted. -Formed Steel Brackets - ASTM A653 Grade 50 SS, G115 HDG -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85 -Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.
			6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.
			7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.
			8. For the purposes of this project, all arrays are classified as Exterior Arrays.
TO LEG A (1) 3/8-1( (2) SERR/	BRACKET ATTACHES DAPTER USING 6 U-BOLT AND ATED FLANGE NUTS. TO 20 FT-LBS		9. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.
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	DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED TOLERANCES ARE AS FOLLOWS:	Mater Weigl	
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*	ANGLE = $\pm 5^{\circ}$ MIN. BREAK = 0.012" (0.3mm)	Draw	Date: 7/15/2020
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		PROJECT INFORMATION INSTALLATION ADDRESS: LOT 33 PROSPECT STREET, BURLINGTON, CT
		06013 Structural Caparal Notae
		Structural General Notes 1. The contractor will be solely responsible for all construction means, methods, techniques, sequences and procedures and shall at all times take reasonable precautions for the safety of its employees on the project, and shall comply with all applicable provisions of federal, state, and municipal safety laws and building construction codes.
		2. If existing conditions make it necessary to revise structural details, consult DCE Solar before proceeding with any change.
		3. These drawings and notes are for this specific project and no other use is authorized.
		4. Structure designed in accordance with the 2018 Connecticut State Building Code, ASCE 7-10, AISC 360-10 (14th Edition), and AISI S100-12: ASD Snow Loads:
LTERNATE		-Ground Snow Load pg = 40 psf
•	CONNECTORS ND & 4TH EW	-Importance Factor Is=0.8 -Exposure Factor Ce=1.0
5 PER INST D MOUNT L	TALLATION MANUAL _AYOUT	-Slope Snow Load ps= 30 psf
		Wind Loads: -Basic Wind Speed V= 110 mph (MRI = 0.93 or 25 year) -Iw = 1 -Exposure = C
<b>NNECTOF</b> U-CHANN M A653 TEEL		-Wind Design performed in accordance with the requirements of ASCE - Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT Laboratory dated 12/11/14.
ΖED		Seismic Loads: -SS = 0.182.g, S1 = 0.065g -Site Class = D -SDS = 0.190g, SD1 = 0.100g
		-Seismic Design Category = B -Ordinary Steel Cantilever Column System
		<ul> <li>5. Material strengths:</li> <li>-Hot-rolled structural steel ASTM A992 GR50 w/80 ksi yield.</li> <li>-Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted.</li> </ul>
		-Formed Steel Brackets - ASTM A653 Grade 50 SS, G115 HDG -I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85 -Plate - A36 Steel, Hot Dip Galvanized -Connectors - Stainless Steel unless otherwise noted.
		6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.
		7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.
		8. For the purposes of this project, all arrays are classified as Exterior Arrays.
		9. Scope of work by Structural Engineer includes member design, connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.
		Engineer of Record
	<b>(</b>	SI CONAL ENGINE
	DIMENSIONS ARE IN INCHES UNLESS OTHERWISE NOTED TOLERANCES ARE AS FOLLOWS:	Material: Weight: 1758.293 lbmass CT-BF-DB, TRINA SOLAR TSM-DEG15MC.20(II),
	$X = \pm 0.050" (1.27mm)$ $XX = \pm 0.015" (0.38mm)$ $XXX = \pm 0.005" (0.127mm)$	4x5, 25 DEG, BURLINGTON SOLAR ONE -         Description:         BIFACIAL - TRINA SOLAR, VEROGY         Project:         BURLINGTON SOLAR ONE - BIFACIAL - TRINA SOLAR
L**	ANGLE = $\pm 5^{\circ}$ MIN. BREAK = 0.012" (0.3mm)	Drawn: JSCOTT Date: 7/15/2020
THERE IN IS ONFIDENTIAL	SURFACE FINISH = 63 (US)	Scale:     Sheet:     2     of     3       19410 letton Rd, Ste 220     Format:     Part Number     Rev:
PIED UNLESS BJECT TO		19410 Jetton Rd, Ste 220 Cornelius, NC, 28031 www.dcesolar.com Phone:1-704-659-7474FOrmal: Pormal: DPart NumberRev:46370
	Elevating the Future for Solar Made in America	2 1



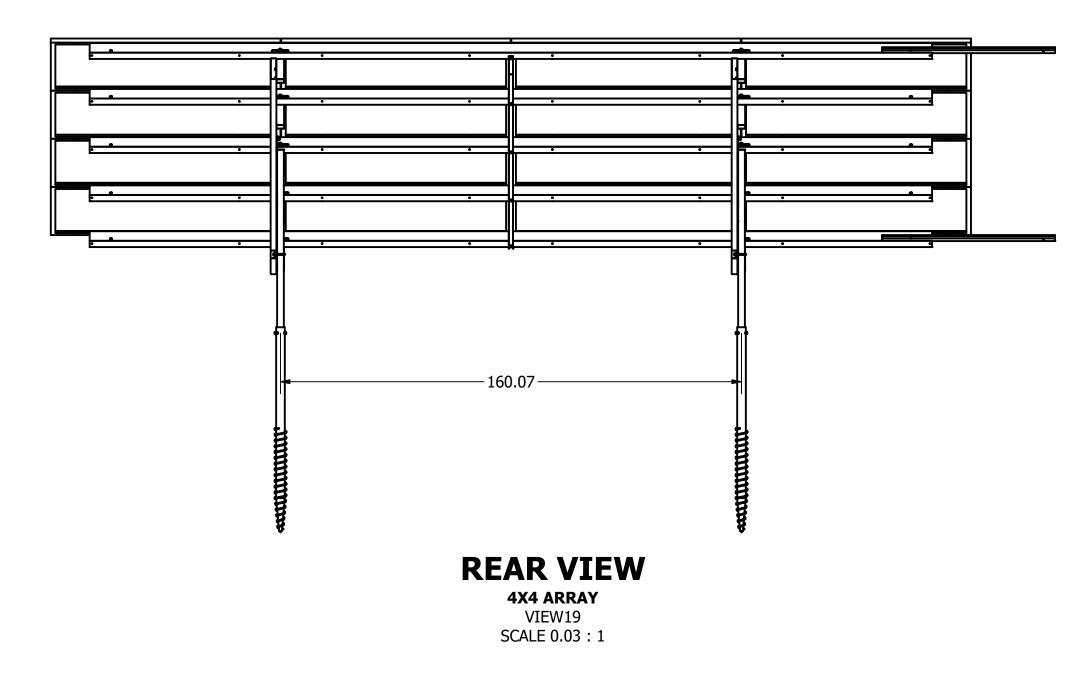
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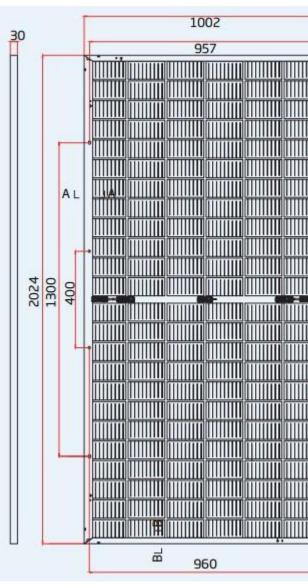
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**REAR VIEW 4X5 ARRAY** VIEW13 SCALE 0.03 : 1



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	PANEL S	DESCRIPTION			PROJECT INFORMATION INSTALLATION ADDRESS: LOT 33 PROSPECT STREET, BURLINGTON, CT
MANUFACTURER	NUFACTURER TRINA SOLAR				06013
MODEL ENGTH (mm)		TSM-DEG15MC.2 2024	20(11)		Structural General Notes
VIDTH (mm)		1002			1. The contractor will be calchy recognized for all construction means
HICKNESS (mm)	MATERIA	30 L DESCRIPTION			1. The contractor will be solely responsible for all construction means, methods, techniques, sequences and procedures and shall at all times
1EMBER	SHAPE		1ATERIAL	GAGE	take reasonable precautions for the safety of its employees on the project, and shall comply with all applicable provisions of federal, state,
ANEL BEAM	6.25Z3X1X55DEG	A65	53 SS Gr80	18GA	and municipal safety laws and building construction codes.
S CEE BEAM	8CS2X0.625	A65	53 SS Gr80	14GA	<ol> <li>If existing conditions make it necessary to revise structural details,</li> <li>consult DCE Solar before proceeding with any change.</li> </ol>
CKER BRACE	2.75CU1.75	A65	53 SS Gr50	14GA	
AM BRACE	1.5CU0.75		53 SS Gr50	16GA	is authorized.
RONT/REAR LEG	HSS2.375x0.154	A50 ADS (GROUNDSCRE	00 GRADE C	-	4. Structure designed in accordance with the 2018 Connecticut State Building Code, ASCE 7-10, AISC 360-10 (14th Edition), and AISI S100-12:
	F	REAR (lbs)		NT (LBS)	ASD
NFACTORED UPLIFT		2,300		450	Snow Loads: Ground Snow Load pg = 40 psf
NFACTORED ADJUS		2,950 4,200		1,800 2,550	-Importance Factor Is=0.8
NFACTORED LATER		900		100	-Exposure Factor Ce=1.0
	I	NOTES			-Slope Snow Load ps= 30 psf
DJUSTED UPLIFT I	S ASSUMED AS 70% OF	THE DOWNWARD	LOAD. IT'S RE(		
	PULL TEST IN CASE PUS		BE PERFORMED	)	-Basic Wind Speed V= 110 mph (MRI = 0.93 or 25 year)
	LIFT IF NO REFUSAL IS E IN CASE OF REFUSAL.				-Iw = 1 -Exposure = C
	ATERAL FORCES USE SA		1.5 AND 2, RES	SPECTIVELY.	-Wind Design performed in accordance with the requirements of ASCE -
	IN-FIELD PI	ILE REMEDIATION			Wind Tunnel Procedure. Refer to Wind Tunnel Report by UWO BLWT
	DIATION REQUIRING TH				Laboratory dated 12/11/14.
	TO GALVANIZATION DAI		IU CUAT AND	INCAL METALS	Seismic Loads:
USE PAINTS CO	NTAINING ZINC DUST (1		NITH "ASTM A 7	780-01"	-SS = 0.182.g, S1 = 0.065g
CTION A2) USE ZINC SPRA	Y (IN ACCORDANCE WIT	TH "Δςτμ δ 780-0	1" SECTION 43)	) ONF OF THE	-Site Class = D -SDS = $0.190g$ , SD1 = $0.100g$
	MUST BE FOLLOWED TO		-		
					-Ordinary Steel Cantilever Column System
30	1002 957		9×14		<ul> <li>-Hot-rolled structural steel ASTM A992 GR50 w/80 ksi yield.</li> <li>-Cold Formed Steel Sections comply w/ASTM A1003, structural grade, galvanized to Grade as noted.</li> <li>-Formed Steel Brackets - ASTM A653 Grade 50 SS, G115 HDG</li> <li>-I-Beams - A992, 50 ksi, Hot Dip Galvanized to ASTM 123 Grade 85</li> <li>-Plate - A36 Steel, Hot Dip Galvanized</li> <li>-Connectors - Stainless Steel unless otherwise noted.</li> </ul>
A L A L A L A L A L A L A L A L A L A L					<ul><li>6. Members and connections have been designed for worst-case loading associated with exterior zones of the array per the wind tunnel report.</li><li>7. Foundation embedment depths are to be calculated and sealed by a CT State Licensed Geotechnical engineer.</li></ul>
2024					8. For the purposes of this project, all arrays are classified as Exterior Arrays.
					9. Scope of work by Structural Engineer includes member design,
					connection design, and determination of design base reactions only. Layout of PV arrays such that they do not conflict with existing site
					obstructions, determination of site-specific foundation and geotechnical parameters, and all other work not specifically noted is by others.
					Engineer of Record
			irounding Hole		NUMBER OF CONVECTION
	<del>ط</del> 960	12.	-Drain Hole		
					□ ☆ 。 □ ☆ 。 ■ No. 27915 。 <u>A</u>
					ERSENSER OF THE TRANSPORT
					······································
			DIMENSIONS A		aterial:
			UNLESS OTHER TOLERANCES ARI	RWISE NOTED	eight: 1758.293 lbmass CT-BF-DB, TRINA SOLAR TSM-DEG15MC.20(II),
				050" (1.27mm)	4x5, 25 DEG, BURLINGTON SOLAR ONE -
			$.XX = \pm 0.0$ $.XXX = \pm 0.003$	)5" (0.127mm)	escription: BIFACIAL - TRINA SOLAR, VEROGY
			ANGLE =	= ± 5 ° –	oject: BURLINGTON SOLAR ONE - BIFACIAL - TRINA SOLAR
				1 ()1 )" / () 2mm · ·	
	**PROPRIETARY AND C	CONFIDENTIAL**	MIN. BREAK = $0$		rawn: JSCOTT Date: 7/15/2020
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