

March 23, 2026

Via Federal Express

Melanie A. Bachman, Esq.
Executive Director/Staff Attorney
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

**Re: Docket No. 536 – Application of Tarpon Towers III, LLC and Cellco Partnership
d/b/a Verizon Wireless for a Certificate of Environmental Compatibility and Public
Need for the Construction, Maintenance and Operation of a Wireless
Telecommunications Facility at 78 Goshen East Street, Norfolk, Connecticut
Development and Management Plan Submission**

Dear Attorney Bachman:

Enclosed please find the original and fifteen (15) copies of the following:

1. Development and Management (“D&M”) Plans prepared by ProTerra Design Group, LLC for the approved telecommunications facility at 78 Goshen East Street in Norfolk, Connecticut incorporating the Council’s conditions of approval. Also enclosed are two (2) full size (24” x 36”) sets of D&M plans.
2. Tower and Foundation Design Report, containing the lattice tower and foundation design, prepared by CASE International LLC dated February 20, 2026.
3. Geotechnical Investigation Report prepared by Welti Geotechnical, P.C. dated February 9, 2026.

Please note that during the preparation of the D&M Plans Cellco Partnership d/b/a Verizon Wireless (“Cellco”) determined it required limited additional equipment installed on the tower. The limited additional equipment includes three (3) additional antennas and antennas and remote radio heads with updated technology. While these changes are modest, Cellco updated

34089881-v1

Melanie A. Bachman, Esq.
March 23, 2026
Page 2

the Far Field Emissions Calculations provided with the original Application (Attachment 14) to reflect the updated and additional equipment. A copy of the updated Far Field Emissions Calculations is enclosed. As these updated calculations demonstrate, the maximum permissible exposure level for all of Cellco's frequencies would be 12.4% of the FCC's Standard. Actual RF emissions levels from the proposed tower site would still be far below these "worst-case" calculations.

We respectfully request that this information be reviewed and this matter be placed on the next available Siting Council agenda for approval. Please feel free to contact me if you have any questions or require additional information. Thank you.

Sincerely,



Jonathan H. Schaefer

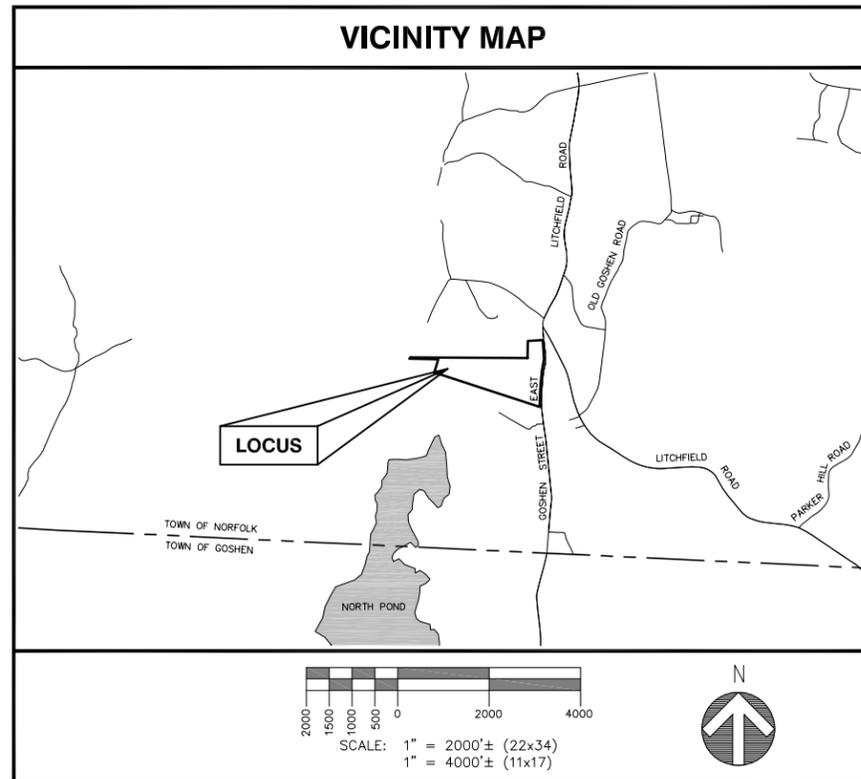
Enclosures

Copy to:

Henry Tirrell, First Selectman, Town of Norfolk



TARPON TOWERS III, LLC
SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058



DRAWING INDEX		
SHEET	DESCRIPTION	REVISION
T-1	TITLE SHEET	0
GN-1	GENERAL NOTES	0
RN-1	RESOURCE PROTECTION NOTES	0
C-1	ABUTTERS PLAN	A
C-2	EXISTING CONDITIONS	A
A-1	COMPILED PLOT PLAN	0
A-2	COMPOUND & KEY PLAN	0
A-3	TOWER ELEVATION	0
P-1 TO P-2	DRIVEWAY PLAN & PROFILES	0
D-1 TO D-2	DETAILS	0
CA-1 TO CA-3	TENANT DETAILS	0
E-1 TO E-3	ELECTRICAL & GROUNDING DETAILS	0
EC-1 TO EC-2	EROSION CONTROL PLAN & DETAILS	0

- GENERAL NOTES**
- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE ENGINEER & TARPON TOWERS III, LLC REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
 - THESE DEVELOPMENT AND MANAGEMENT PLANS TO BE FILED WITH THE CONNECTICUT SITING COUNCIL ARE FOR PERMITTING PURPOSES ONLY. NOT FOR FINAL CONSTRUCTION.
 - ALL WORK TO BE PERFORMED IN ACCORDANCE WITH TARPON TOWERS III, LLC CONSTRUCTION GUIDELINES.
 - NEW CONSTRUCTION SHALL CONFORM TO ALL APPLICABLE CODES AND ORDINANCES INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
 BUILDING CODE: CONNECTICUT STATE BUILDING CODE (2022) (IBC 2021 WITH AMENDMENTS)
 ELECTRICAL CODE: NEC 2020 WITH AMENDMENTS
 - ALL WORK TO CONFORM WITH THE CONNECTICUT SITING COUNCIL DOCKET NO. 536 DECISION ORDER ISSUED OCTOBER 30, 2025.
 - THESE PLANS ARE INTENDED FOR SITE CONSTRUCTION PURPOSES. REFER TO TOWER & FOUNDATION PLANS PREPARED BY CASE INTERNATIONAL LLC., TOWER DESIGN DATED FEBRUARY 20, 2026, (DRAWING NO. E-1) AND FOUNDATION DESIGN DATED FEBRUARY 20, 2026 (DRAWING NO. 26138-FD01 & 26138-FD02) FOR TOWER AND FOUNDATION PLANS.
 - HOURS OF CONSTRUCTION OPERATION SHALL BE LIMITED MONDAY THROUGH SATURDAY, 7:00AM TO 6:00PM.
 - ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND EXISTING PLANS OF RECORD. THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK. CALL DIG-SAFE (888) 344-7233 72-HOURS PRIOR TO ANY EXCAVATION.
 - THIS SHEET SET WAS ORIGINALLY PRINTED TO ANSI D (22"x34") WITH 1" MARGINS. PRINTING TO ANSI B (11"x17") WILL RESULT IN A HALF-SCALE (1:2) SHEET SET WITH 1/2" MARGINS. CONFIRM ALL SCALED DISTANCES WITH GRAPHICAL SCALES SHOWN HEREIN.

ANCHOR TENANT INFORMATION



SITE NAME: NORFOLK SOUTH CT
 MDG LOCATION ID: 5000941034
 FUZE PROJECT ID: 15322899
 78 GOSHEN EAST STREET
 CELCO PARTNERSHIP
 dba VERIZON WIRELESS
 20 ALEXANDER DRIVE
 WALLINGFORD, CT 06492

PROJECT INFORMATION

SITE TYPE:	RAW LAND
SCOPE OF WORK:	PROPOSED 186' TALL SELF-SUPPORT WITHIN 73'x73' FENCED COMPOUND AND 75'x75' LEASE AREA.
SITE NAME:	NORFOLK SOUTH CT
SITE NUMBER:	CT1238
SITE ADDRESS:	78 GOSHEN EAST STREET NORFOLK, CT 06058
ASSESSOR'S TAX ID#:	7-04-17
ZONING DISTRICT:	RURAL RESIDENTIAL (RR)
LATITUDE:	41° 55' 33.08"± N (RECORD 1A)
LONGITUDE:	73° 13' 06.02"± W (RECORD 1A)
(P) ELEVATION AT TOWER CENTER:	1499.1'± AMSL (0' AGL)
(P) TOP OF TOWER:	1685.1'± AMSL (186.0'± AGL)
(P) HIGHEST APPURTENANCE: (LIGHTNING ROD)	1691.1'± AMSL (192.0'± AGL)
DATUM:	NAD83/NAVD88
PROPERTY OWNER:	PAUL CHAPINSKY, SR. 78 GOSHEN EAST STREET NORFOLK, CT 06058
TOWER OWNER:	TARPON TOWERS III, LLC 8916 77TH TERRACE EAST STE. 103 LAKEWOOD RANCH, FL 34202
SITE ENGINEER:	PROTERRA DESIGN GROUP, LLC 4 BAY ROAD BLDG A; SUITE 200 HADLEY, MA 01035 (413) 320-4918
SURVEYOR:	NORTHEAST SURVEY CONSULTANTS 3 FERRY STREET STUDIO 1 EAST EASTHAMPTON, MA 01027 (413) 203-5144
WETLAND SCIENTIST:	ALL-POINTS TECHNOLOGY GROUP 567 VAUXHALL STREET EXTENSION SUITE 311 WATERFORD, CT 06385



4 Bay Road
 Building A; Suite 200
 Hadley, MA 01035
 Ph: (413)320-4918

CONSULTANTS:

REVISIONS

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT:
TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202



STAMP:

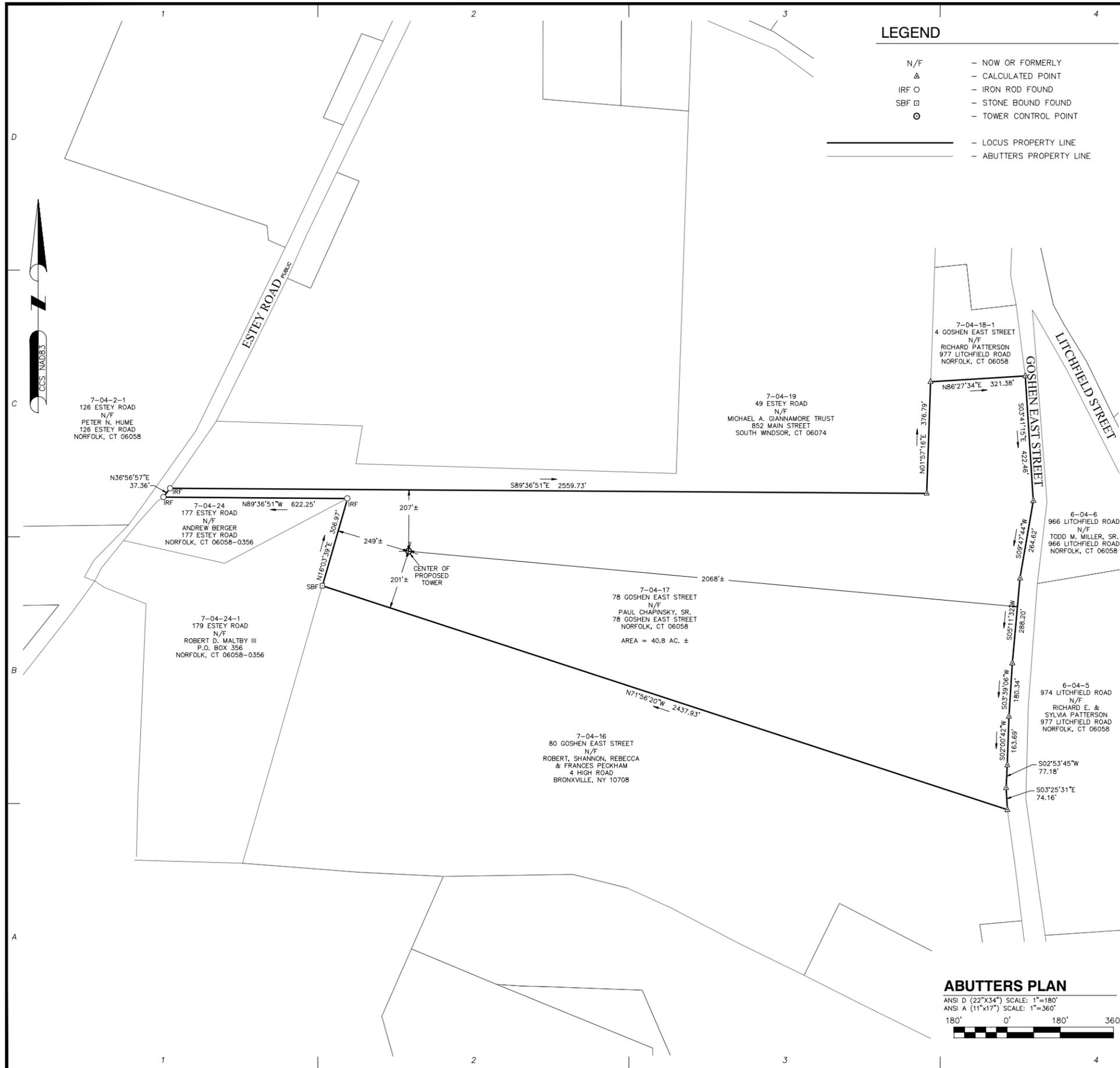


DATE: 03/16/2026
 DRAWN: JEB/STZ/PN
 CHECK: JMM/TEJ
 SCALE: SEE PLAN
 JOB NO.: 24-024
 SHEET TITLE:

TITLE SHEET

T-1

DEVELOPMENT & MANAGEMENT PLAN



LEGEND

- N/F - NOW OR FORMERLY
- △ - CALCULATED POINT
- IRF ○ - IRON ROD FOUND
- SBF □ - STONE BOUND FOUND
- - TOWER CONTROL POINT
- - LOCUS PROPERTY LINE
- - - - - ABUTTERS PROPERTY LINE

FAA 1-A CERTIFICATION

I HEREBY CERTIFY THAT THE LATITUDE, LONGITUDE, AND ELEVATION PRESENTED HEREON MEET THE REQUIREMENTS OF THE FAA WITH THE FOLLOWING ACCURACIES:

THREE (3) FEET VERTICALLY
TWENTY (20) FEET HORIZONTALLY

Charles G. Gidman
CHARLES G. GIDMAN, P.L.S. #70103

10-15-2024
DATE

SURVEY NOTES

- SITE DETAIL SHOWN ON THIS SURVEY IS BASED ON FIELD DATA COLLECTED ON 9-3-2024 & 10-1-2024. BOUNDARY INFORMATION IS COMPILED FROM RECORD DOCUMENTS AND IS NOT TO BE CONSTRUED AS HAVING BEEN OBTAINED AS THE RESULT OF A FIELD BOUNDARY SURVEY, AND IS SUBJECT TO CHANGE AS AN ACCURATE FIELD SURVEY MAY DISCLOSE.
- THE PURPOSE OF THIS SURVEY IS TO SUPPORT THE DESIGN AND CONSTRUCTION OF A TELECOMMUNICATION FACILITY. USE OF THIS SURVEY BY ANYONE OTHER THAN TARPON TOWERS III, LLC, AND USE OF THIS SURVEY FOR ANY PURPOSE NOT RELATED TO THE DESIGN OF THE INTENDED FACILITY IS STRICTLY PROHIBITED.
- PROPERTY LINES AND BOUNDARY DIMENSIONS SHOWN ON THIS SURVEY ARE BASED ON A COMPILATION OF RECORDED DEEDS AND ARE NOT INTENDED TO REPRESENT DEFINITIVE BOUNDARY LINES OR BE USED FOR THE PURPOSES OF CONVEYANCE. PROPERTY RELATED INFORMATION HAS NOT BEEN CONFIRMED BY FIELD SURVEY OBSERVATIONS AND NO ASSESSMENT OF LAND OCCUPATION HAS BEEN CONDUCTED IN THE PREPARATION OF THIS SURVEY. A PROPERTY LINE RETRACEMENT SURVEY HAS NOT BEEN CONDUCTED.
- THE PROPERTY LINES SHOWN ON THIS PLAN ARE THE LINES DIVIDING EXISTING OWNERSHIP'S, AND THE LINES OF STREETS AND WAYS SHOWN ARE THOSE OF PUBLIC OR PRIVATE STREETS OR WAYS ALREADY ESTABLISHED, AND NO NEW LINES FOR DIVISION OF EXISTING OWNERSHIP OR FOR NEW WAYS ARE SHOWN.
- PRIMARY GEODETIC SURVEY CONTROL WAS ESTABLISHED FROM AN ON THE GROUND SURVEY USING THE GLOBAL POSITIONING SYSTEM (GPS) ON 9-3-2024. THE GRID COORDINATES ARE BASED ON THE CONNECTICUT STATE PLANE COORDINATE SYSTEM OF 1983 (2011). ELEVATIONS SHOWN ARE ASSUMED AND ARE APPROXIMATED TO THE NAVD 88 VERTICAL DATUM. VERTICAL AND HORIZONTAL INFORMATION SHOWN MEETS THE STANDARD CRITERIA FOR AN FAA 1A CERTIFICATION (20'± HORIZONTAL AND 3'± VERTICAL).
- IN THE EVENT THAT BENCHMARKS (TBM'S), ESTABLISHED FOR THIS PROJECT AND PUBLISHED ON THIS SURVEY, ARE DESTROYED, NOT RECOVERABLE OR A DISCREPANCY IS FOUND, THE USER SHOULD NOTIFY THIS FIRM IN WRITING PRIOR TO COMMENCING OR CONTINUING ANY WORK.
- THE LOCUS PARCEL IS IDENTIFIED AS 7-04-17 IN THE TOWN OF NORFOLK TAX ASSESSOR'S DATABASE.
- THE PROJECT AREA IS LOCATED IN FLOOD ZONE "X" (NO SHADING) (AREAS OF MINIMAL FLOOD HAZARD) ACCORDING TO FLOOD INSURANCE RATE MAP COMMUNITY PANEL NUMBER 090181 0015 A, DATED 12/3/1987.
- ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND EXISTING PLANS OF RECORD. THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK. CALL THE FOLLOWING FOR ALL PRE-CONSTRUCTION NOTIFICATION 72-HOURS PRIOR TO ANY EXCAVATION ACTIVITY: CALL BEFORE YOU DIG 1-800-922-4455
- WETLANDS SHOWN HEREON WERE DELINEATED BY ALL-POINTS TECHNOLOGY CORPORATION IN JULY 2024.

PROPERTY OWNER: PAUL CHAPINSKY, SR.
78 GOSHEN EAST STREET
NORFOLK, CT 06058

LOCUS DEED REFERENCE: BOOK 85 PAGE 725

PLAN REFERENCES: PLAN #839, PLAN #840. PLAN #1051

THIS SURVEY HAS BEEN PREPARED PURSUANT TO THE REGULATIONS OF CONNECTICUT STATE AGENCIES SECTIONS 20-300B-1 THROUGH 20-300B-20 AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS INC. ON SEPTEMBER 26, 1997.

TYPE OF SURVEY: IMPROVEMENT LOCATION SURVEY
BOUNDARY SURVEY CATEGORY: DEPENDENT RESURVEY
CLASS OF ACCURACY: HORIZONTAL CLASS D
VERTICAL CLASS V-2
TOPOGRAPHIC CLASS T-2
PURPOSE OF SURVEY: PROPOSED CELLULAR UTILITIES

THIS DOCUMENT AND COPIES THEREOF ARE VALID ONLY IF THEY BEAR THE LIVE SIGNATURE AND EMBOSSED SEAL OF THE DESIGNATED PROFESSIONAL. UNAUTHORIZED ALTERATIONS RENDER ANY DECLARATION NULL AND VOID.

TO THE BEST OF MY KNOWLEDGE AND BELIEF, THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON.

Charles G. Gidman
CHARLES G. GIDMAN, P.L.S. #70103

ABUTTERS PLAN

ANSI D (22"x34") SCALE: 1"=180'
ANSI A (11"x17") SCALE: 1"=360'

4 Bay Road, Bldg A,
Suite 200
Hadley, MA 01035
Ph: (413)320-4918

CONSULTANTS:
NORTHEAST SURVEY
CONSULTANTS

3 Ferry Street
Studio 1 East
Easthampton, MA 01027
(413) 203-5144
northeastsurvey.com

NO.	DATE	REVISIONS
A	10/15/24	ISSUED FOR REVIEW

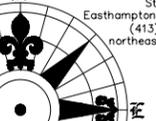
TITLE: SITE NAME: NORFOLK SOUTH CT
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT: TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
SUITE 103
TOWERS III, LLC LAKEWOOD RANCH, FL 34202



DATE: 10/15/2024
DRAWN: JDG
CHECK: BCF
SCALE: 1"=180'
JOB NO.: 24-234
SHEET TITLE:

ABUTTERS PLAN



NO.	DATE	REVISIONS
A	10/15/24	ISSUED FOR REVIEW

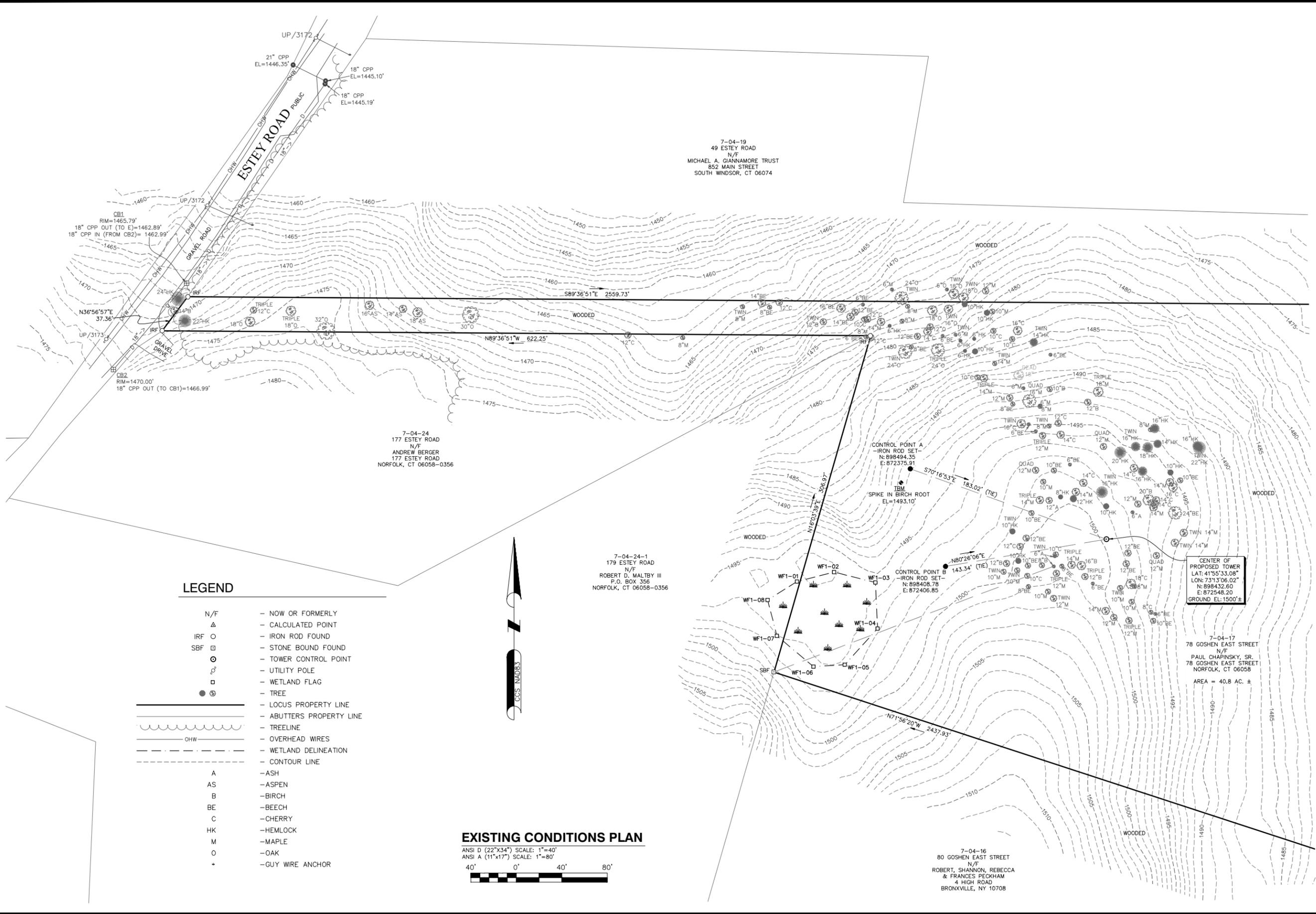
TITLE: SITE NAME: NORFOLK SOUTH CT
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT:
TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
SUITE 103
TOWERS III, LLC LAKEWOOD RANCH, FL 34202

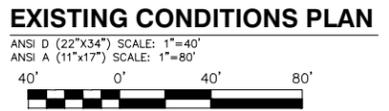


DATE: 10/15/2024
DRAWN: JDG
CHECK: BCF
SCALE: 1"=40'
JOB NO.: 24-234
SHEET TITLE:

EXISTING CONDITIONS



- LEGEND**
- N/F - NOW OR FORMERLY
 - △ - CALCULATED POINT
 - IRF ○ - IRON ROD FOUND
 - SBF □ - STONE BOUND FOUND
 - - TOWER CONTROL POINT
 - ⊙ - UTILITY POLE
 - - WETLAND FLAG
 - - TREE
 - - LOCUS PROPERTY LINE
 - - ABUTTERS PROPERTY LINE
 - - TREELINE
 - - OHW - OVERHEAD WIRES
 - - WETLAND DELINEATION
 - - CONTOUR LINE
 - A - ASH
 - AS - ASPEN
 - B - BIRCH
 - BE - BEECH
 - C - CHERRY
 - HK - HEMLOCK
 - M - MAPLE
 - O - OAK
 - - GUY WIRE ANCHOR



7-04-19
49 ESTEY ROAD
N/F
MICHAEL A. GIANNAMORE TRUST
852 MAIN STREET
SOUTH WINDSOR, CT 06074

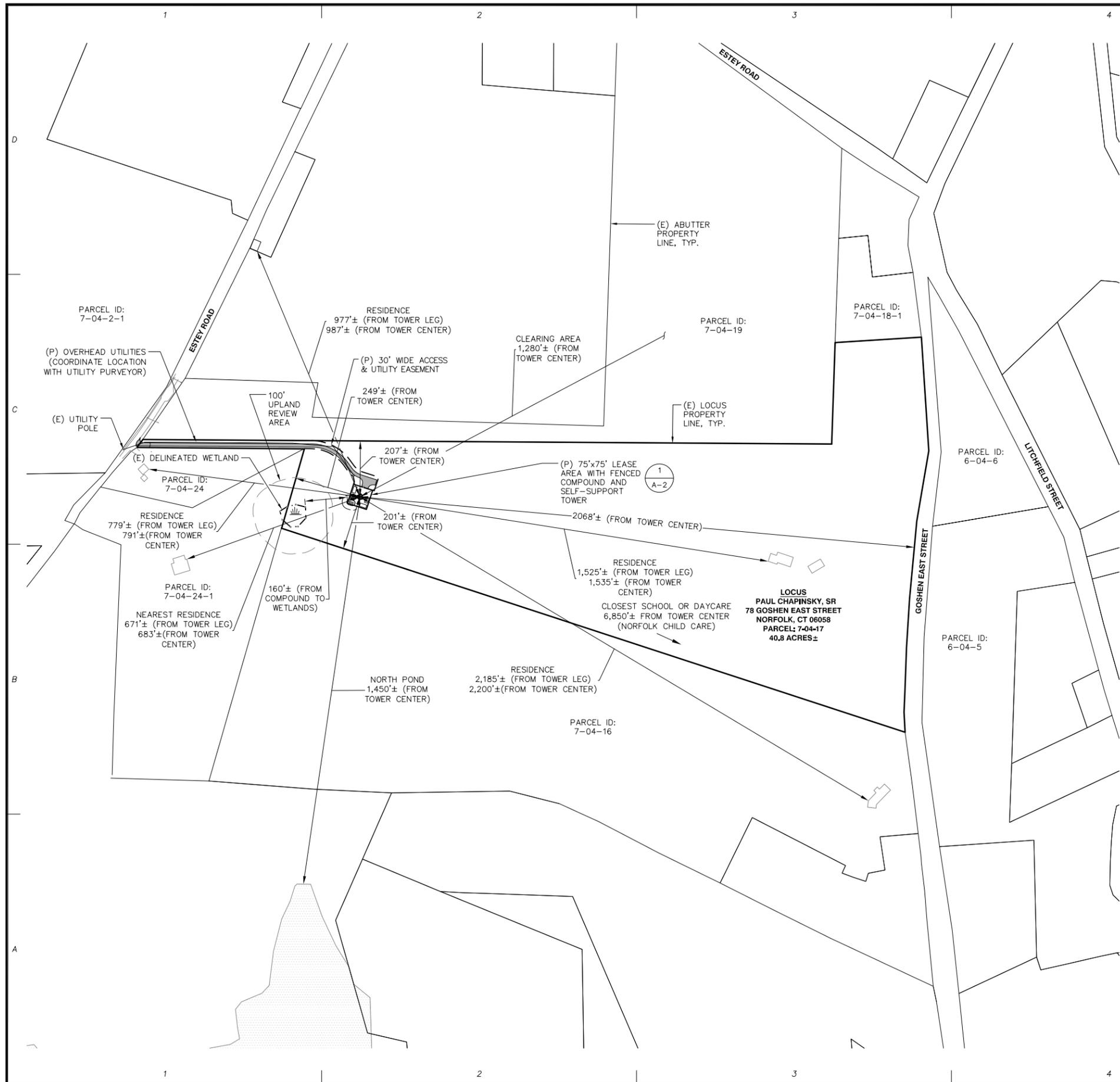
7-04-24
177 ESTEY ROAD
N/F
ANDREW BERGER
177 ESTEY ROAD
NORFOLK, CT 06058-0356

7-04-24-1
179 ESTEY ROAD
N/F
ROBERT D. MALTBY III
P.O. BOX 356
NORFOLK, CT 06058-0356

7-04-16
80 GOSHEN EAST STREET
N/F
ROBERT, SHANNON, REBECCA
& FRANCES PECKHAM
4 HIGH ROAD
BRONXVILLE, NY 10708

CENTER OF
PROPOSED TOWER
LAT: 41°55'33.08"
LON: 73°13'06.02"
N: 898432.60
E: 872548.20
GROUND EL: 1500'±

7-04-17
78 GOSHEN EAST STREET
N/F
PAUL CHARPINSKY, SR.
78 GOSHEN EAST STREET
NORFOLK, CT 06058
AREA = 40.8 AC. ±



GENERAL NOTES

1. THE TYPE, DIMENSIONS, MOUNTING HARDWARE, AND POSITIONS OF ALL PROJECT OWNER'S EQUIPMENT ARE SHOWN IN ILLUSTRATIVE FASHION. ACTUAL HARDWARE DETAILS AND FINAL LOCATIONS MAY DIFFER SLIGHTLY FROM WHAT IS SHOWN.
2. THE PROJECT OWNER'S PCS FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
3. THE DESIGN OF THE TOWER, FOUNDATION AND ANTENNA MOUNTING HARDWARE WILL MEET THE ANSI/EIA/TIA-222-H STANDARDS FOR STRUCTURAL STEEL ANTENNA SUPPORTING STRUCTURES AND 2022 CT STATE BUILDING CODE REQUIREMENTS. DETAILED CONSTRUCTION DRAWINGS AND STRUCTURAL CALCULATIONS WILL BE PREPARED BY A REGISTERED PROFESSIONAL ENGINEER AND SUBMITTED WITH A BUILDING PERMIT APPLICATION FOR REVIEW AND APPROVAL BY THE LOCAL BUILDING CODE ENFORCEMENT OFFICIAL.
4. ONCE THE FACILITY BECOMES FULLY OPERATIONAL, NORMAL AND ROUTINE MAINTENANCE BY TOWER OWNER'S AND CARRIER'S TECHNICIANS WILL BE PERFORMED. THE ESTIMATED VEHICULAR TRAFFIC GENERATED BY THESE VISITS IS PREDICTED TO BE LESS THAN THE TYPICAL TRAFFIC GENERATED BY A SINGLE-FAMILY DWELLING.

REFERENCES

PROPERTY LINE, TOPOGRAPHY AND EXISTING FEATURES FROM SURVEY PREPARED BY NORTHEAST SURVEY CONSULTANTS.

ZONING DISTRICTS - ZONING MAP FOR THE TOWN OF NORFOLK, CT FOUND IN TOWN OF NORFOLK ZONING REGULATIONS.

FLOODPLAIN - FLOOD INSURANCE RATE MAP (FIRM) MAP NUMBER 090181 0015 A EFFECTIVE DATE DECEMBER 3, 1987 PREPARED BY FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA), US DEPARTMENT OF HOMELAND SECURITY. ENTIRE AREA IS WITHIN ZONE "X" UNSHADED: AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL FLOOD CHANCE FLOODPLAIN.

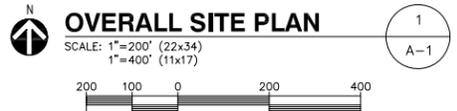
CSC DIMENSIONS

ASSESSORS ID: 7-04-17
(P) USE: WIRELESS COMMUNICATION FACILITY

SELF-SUPPORT TOWER HEIGHT (HIGHEST APPURTENANCE)	186'± (192'±)
EXISTING SITE ACCESS LENGTH	N/A
PROPOSED SITE ACCESS LENGTH	940'±
NUMBER OF TREES OVER 6" DBH TO BE REMOVED	89
NUMBER OF TREES OVER 14" DBH TO BE REMOVED	36 (INCLUDED IN THE 89 TOTAL NUMBER OF TREES OVER 6" TO BE REMOVED)
NUMBER OF RESIDENCES WITHIN 1000 FEET	3
CLOSEST TOWN OR CITY TO LOCUS PARCEL	3,180'± (GOSHEN)
NEAREST WETLANDS (FROM COMPOUND)	160'±
FOLLOWING DISTANCES FROM TOWER CENTER	
NORTHERN PROPERTY BOUNDARY	207'±
SOUTHERN PROPERTY BOUNDARY	201'±
WESTERN PROPERTY BOUNDARY	249'±
EASTERN PROPERTY BOUNDARY	2,068'±
CLOSEST RESIDENCE	680'± (7-04-24-1)
CLOSEST SCHOOL OR DAYCARE	6,850'± (NORFOLK CHILD CARE)

ESTEY ROAD IMPACT NOTE:
APPLICANT WILL WORK WITH THE TOWN AND ITS SITE CONTRACTOR IN ADVANCE OF CONSTRUCTION TO ENSURE THAT IMPACTS TO ESTEY ROAD ARE MINIMIZED AND IF DAMAGE OCCURS, PROMPTLY REPAIRED.

EXISTING WALL NOTE:
IT IS NOT ANTICIPATED ANY STONE WALLS WILL BE DISTURBED. WE WOULD PROPOSE THAT THEY WOULD BE MAINTAINED IN THEIR CURRENT CONDITION AND ANY SUCH STONE WALLS FOR ACCESS WOULD BE DISASSEMBLED AND RETURNED TO THEIR ORIGINAL LOCATION AS CLOSE AS POSSIBLE TO THEIR ORIGINAL OR AN IMPROVED STATE AFTER CONSTRUCTION.

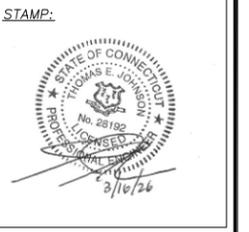


CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
C	03/16/26	ISSUED FOR D&M

TITLE: **SITE NAME: NORFOLK SOUTH CT**
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT: **TARPON TOWERS III, LLC**
8916 77TH TERRACE EAST
STE. 103
LAKELAND RANCH, FL 34202



DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

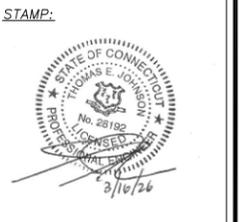
OVERALL SITE PLAN

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: **SITE NAME: NORFOLK SOUTH CT**
SITE NUMBER: CT1238
ADDRESS: 78 COSHEN EAST STREET
NORFOLK, CT 06058

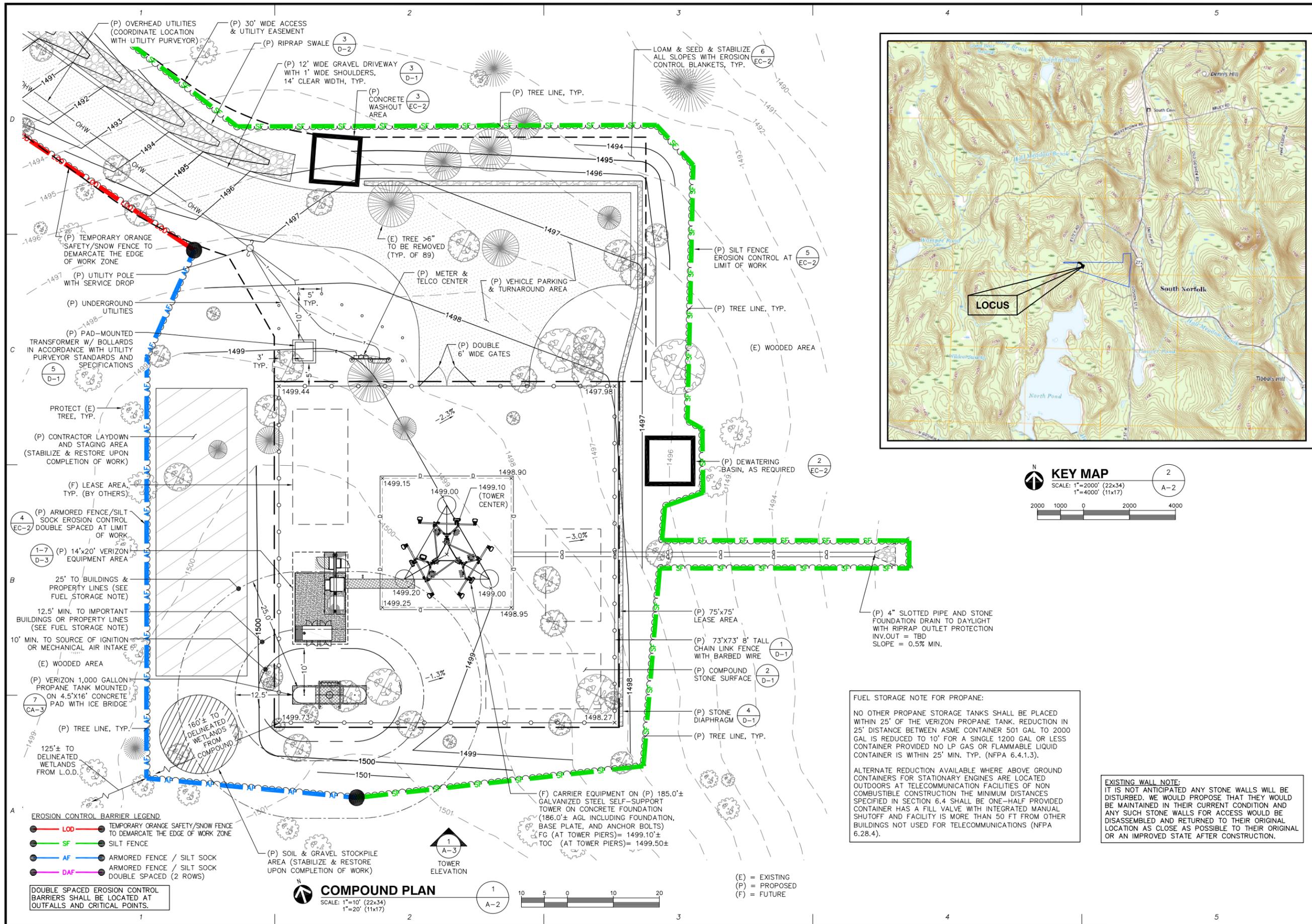
APPLICANT: **TARPON TOWERS III, LLC**
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202



DATE: 03/16/2026
 DRAWN: JEB/STZ/PN
 CHECK: JMM/TEJ
 SCALE: SEE PLAN
 JOB NO.: 24-024

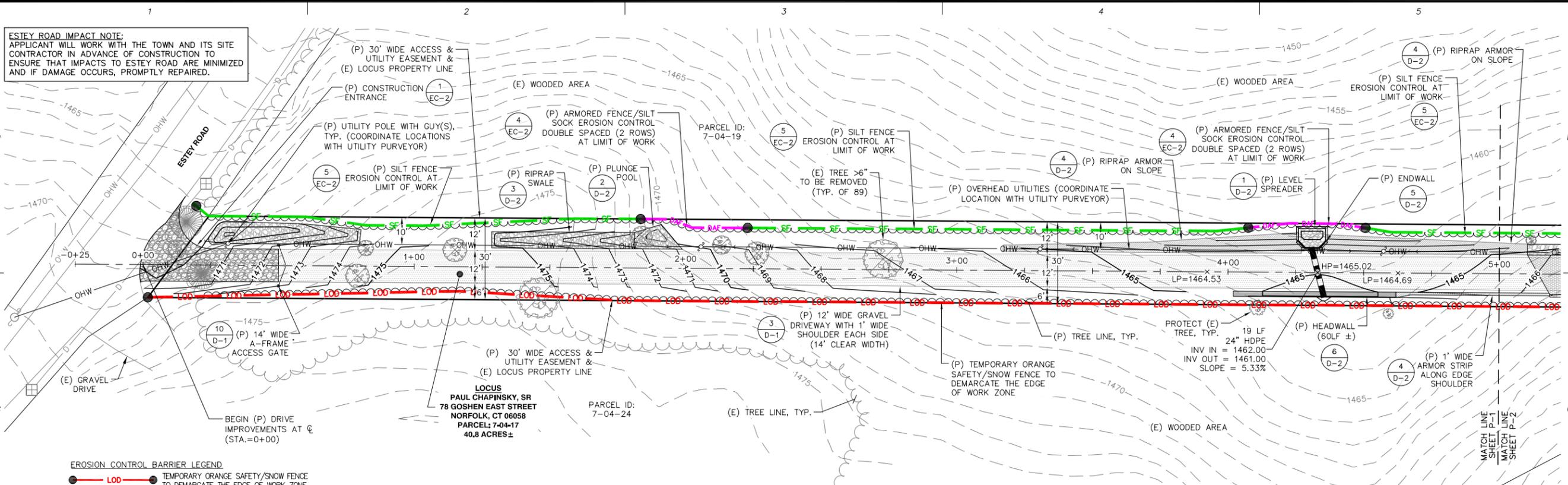
SHEET TITLE:
COMPOUND PLAN
& KEY MAP

A-2



(E) = EXISTING
 (P) = PROPOSED
 (F) = FUTURE

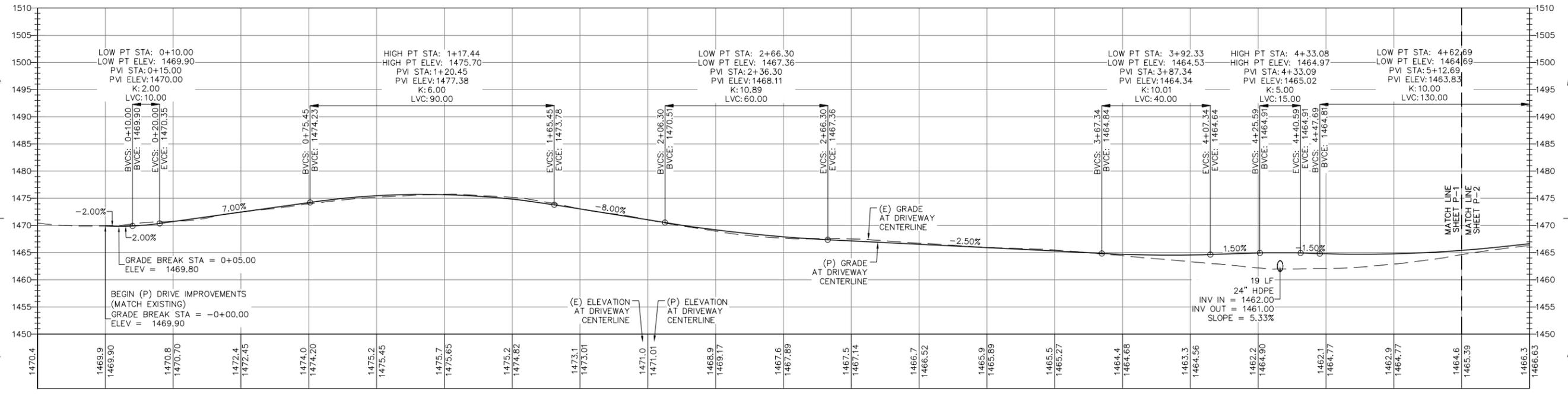
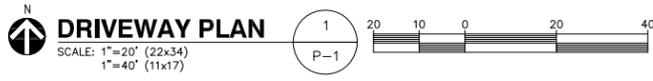
ESTEY ROAD IMPACT NOTE:
 APPLICANT WILL WORK WITH THE TOWN AND ITS SITE CONTRACTOR IN ADVANCE OF CONSTRUCTION TO ENSURE THAT IMPACTS TO ESTEY ROAD ARE MINIMIZED AND IF DAMAGE OCCURS, PROMPTLY REPAIRED.



- EROSION CONTROL BARRIER LEGEND**
- LOD — TEMPORARY ORANGE SAFETY/SNOW FENCE TO DEMARCAT THE EDGE OF WORK ZONE
 - SF — SILT FENCE
 - AF — ARMORED FENCE / SILT SOCK
 - DAF — ARMORED FENCE / SILT SOCK DOUBLE SPACED (2 ROWS)

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

EXISTING WALL NOTE:
 IT IS NOT ANTICIPATED ANY STONE WALLS WILL BE DISTURBED. WE WOULD PROPOSE THAT THEY WOULD BE MAINTAINED IN THEIR CURRENT CONDITION AND ANY SUCH STONE WALLS FOR ACCESS WOULD BE DISASSEMBLED AND RETURNED TO THEIR ORIGINAL LOCATION AS CLOSE AS POSSIBLE TO THEIR ORIGINAL OR AN IMPROVED STATE AFTER CONSTRUCTION.



DRIVEWAY PROFILE
 SCALE (22x34): 1"=20' HORIZONTAL, 1"=10' VERTICAL
 SCALE (11x17): 1"=40' HORIZONTAL, 1"=20' VERTICAL

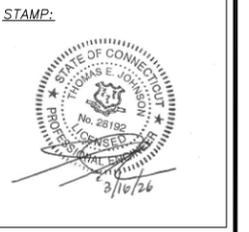
4 Bay Road
 Building A, Suite 200
 Hadley, MA 01035
 Ph: (413)320-4918

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
 NORFOLK, CT 06058

APPLICANT:
TARPON TOWERS III, LLC
 8916 77TH TERRACE EAST
 STE. 103
 LAKEWOOD RANCH, FL 34202



DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

SHEET TITLE:
DRIVEWAY PLAN & PROFILE
P-1

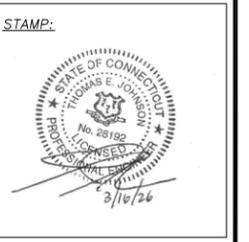
CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT:
TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202



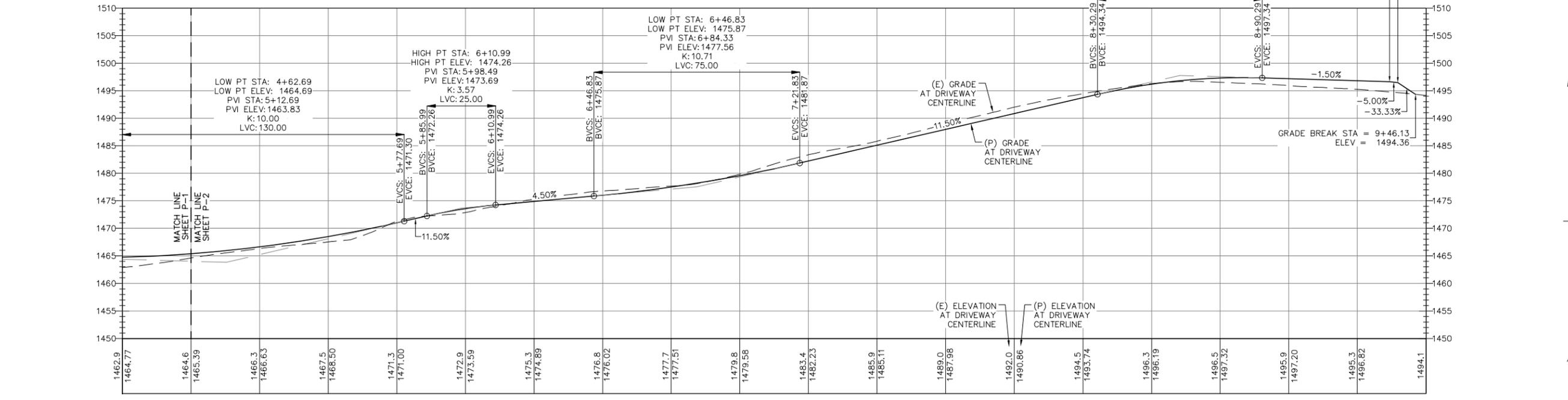
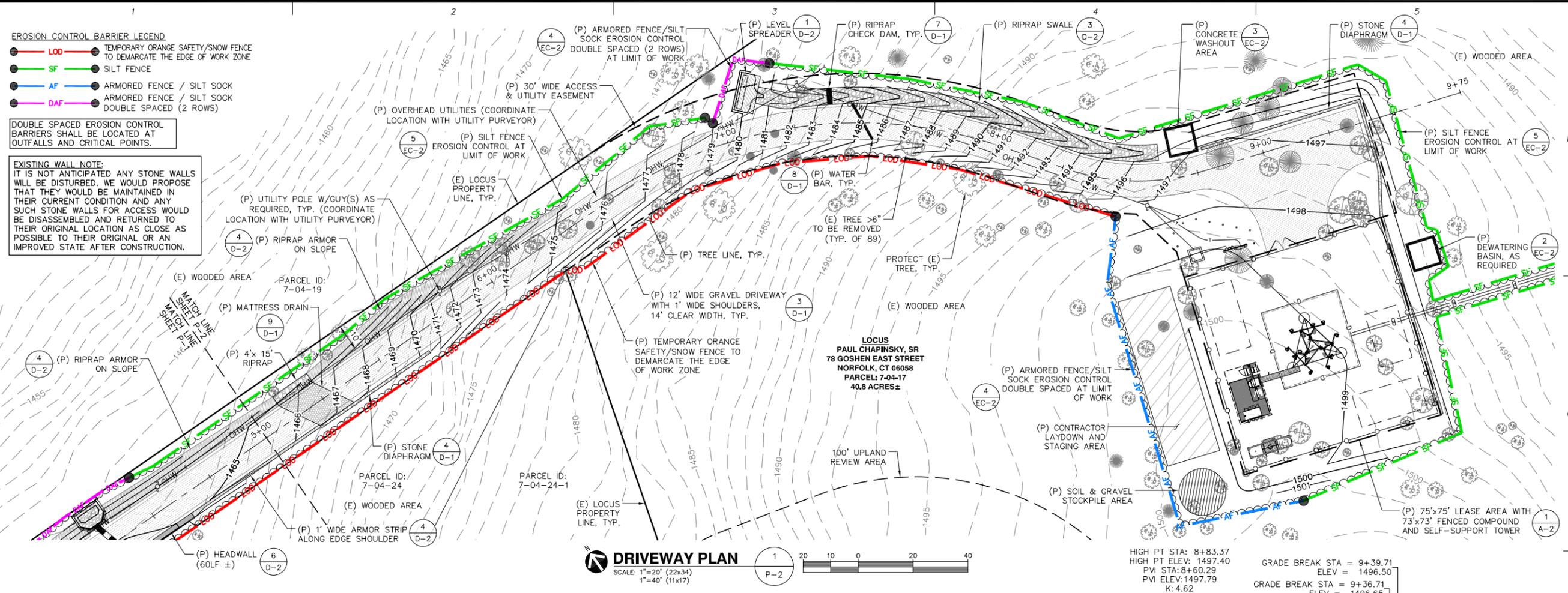
DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

SHEET TITLE:
DRIVEWAY PLAN & PROFILE
P-2

- EROSION CONTROL BARRIER LEGEND**
- LOD: TEMPORARY ORANGE SAFETY/SNOW FENCE TO DEMARCAT THE EDGE OF WORK ZONE
 - SF: SILT FENCE
 - AF: ARMORED FENCE / SILT SOCK
 - DAF: ARMORED FENCE / SILT SOCK DOUBLE SPACED (2 ROWS)

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

EXISTING WALL NOTE:
IT IS NOT ANTICIPATED ANY STONE WALLS WILL BE DISTURBED. WE WOULD PROPOSE THAT THEY WOULD BE MAINTAINED IN THEIR CURRENT CONDITION AND ANY SUCH STONE WALLS FOR ACCESS WOULD BE DISASSEMBLED AND RETURNED TO THEIR ORIGINAL LOCATION AS CLOSE AS POSSIBLE TO THEIR ORIGINAL OR AN IMPROVED STATE AFTER CONSTRUCTION.



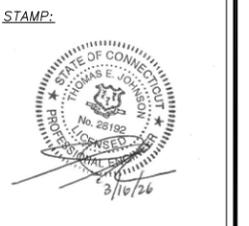
DRIVEWAY PROFILE
SCALE (22x34): 1"=20' HORIZONTAL
1"=10' VERTICAL
SCALE (11x17): 1"=40' HORIZONTAL
1"=20' VERTICAL

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: **SITE NAME: NORFOLK SOUTH CT**
SITE NUMBER: CT1238
ADDRESS: 78 COSHEN EAST STREET
NORFOLK, CT 06058

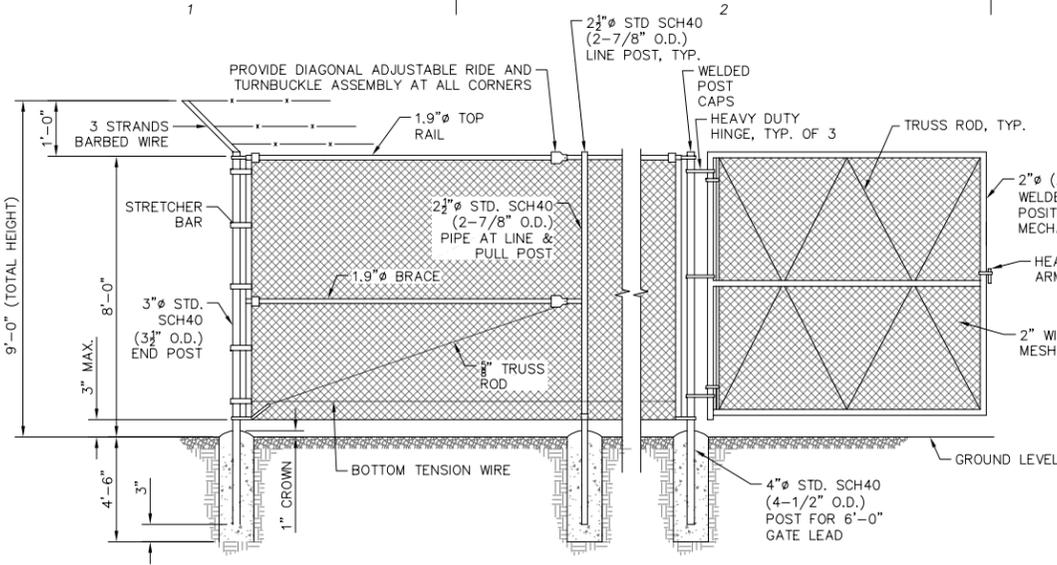
APPLICANT: **TARPON TOWERS III, LLC**
8916 77TH TERRACE EAST
STE. 103
LAKELWOOD RANCH, FL 34202



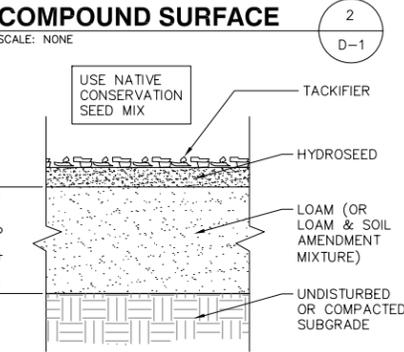
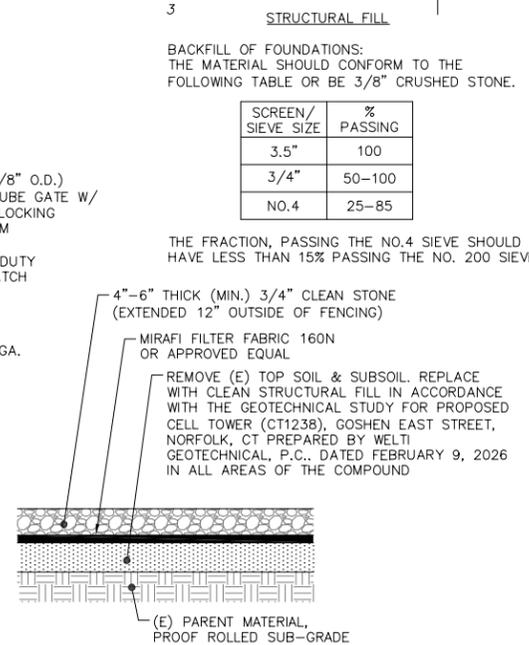
DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024
SHEET TITLE:

DETAILS

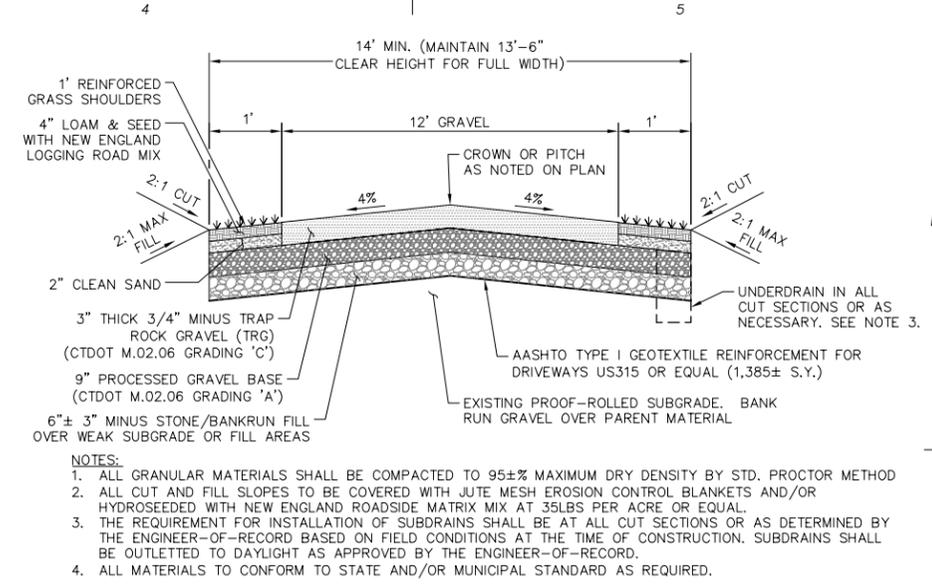
D-1



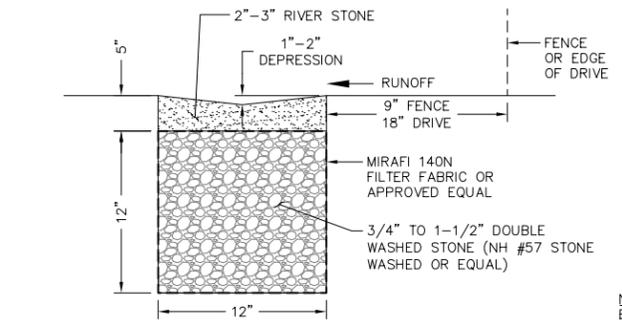
CHAIN LINK FENCE
SCALE: NONE



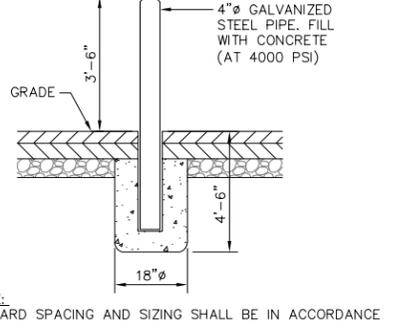
COMPOUND SURFACE
SCALE: NONE



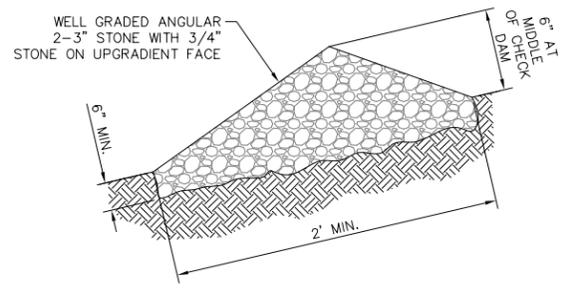
DRIVEWAY SECTION
SCALE: NONE



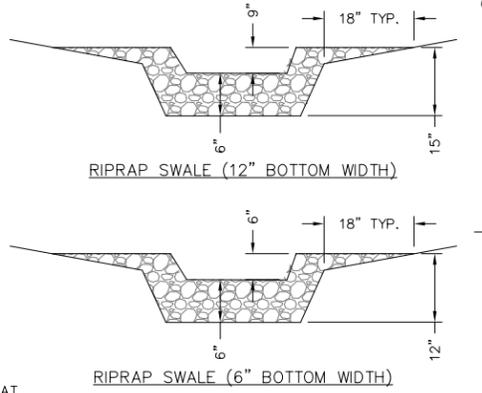
STONE DIAPHRAGM
SCALE: NONE



BOLLARD
SCALE: NONE



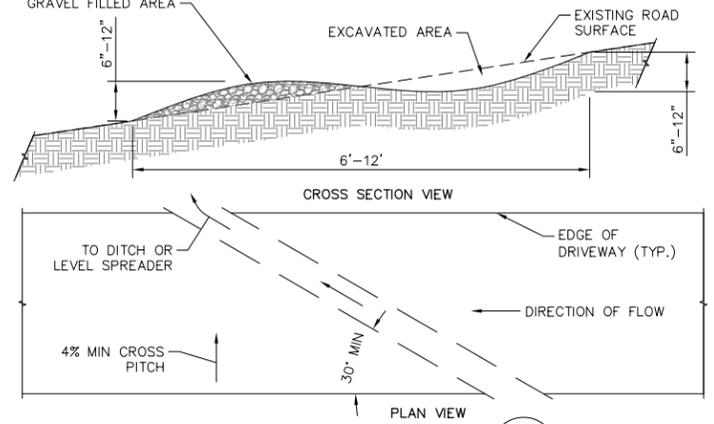
RIP RAP CHECK DAM
SCALE: NONE



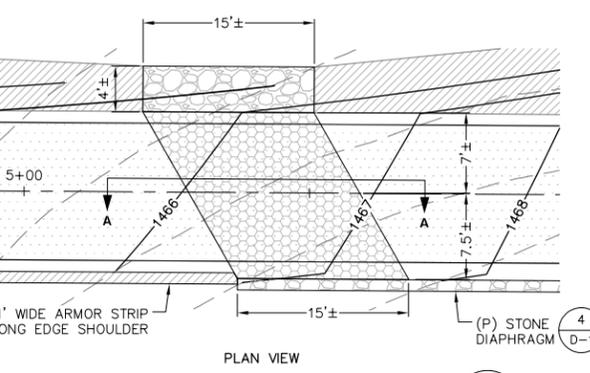
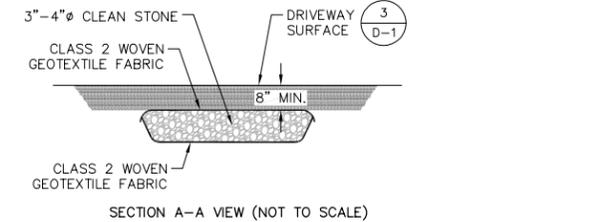
RIP RAP SWALE
SCALE: NONE

SPACING NEEDED BETWEEN WATER BARS	
SLOPE	DIVERSION SPACING (FT)
< 5%	NONE
5-10%	200
10-20%	150
20-35%	100

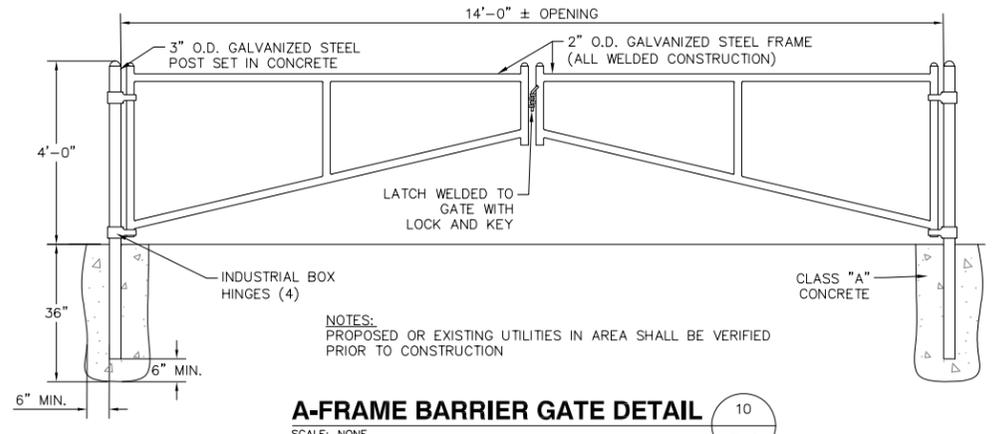
WATER BAR OR WATER DEFLECTOR MAY BE USED INTERCHANGEABLY BASED UPON FIELD CONDITIONS



WATER BAR
SCALE: NONE



MATTRESS DRAIN
SCALE: NONE



A-FRAME BARRIER GATE DETAIL
SCALE: NONE

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 COSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT: TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202

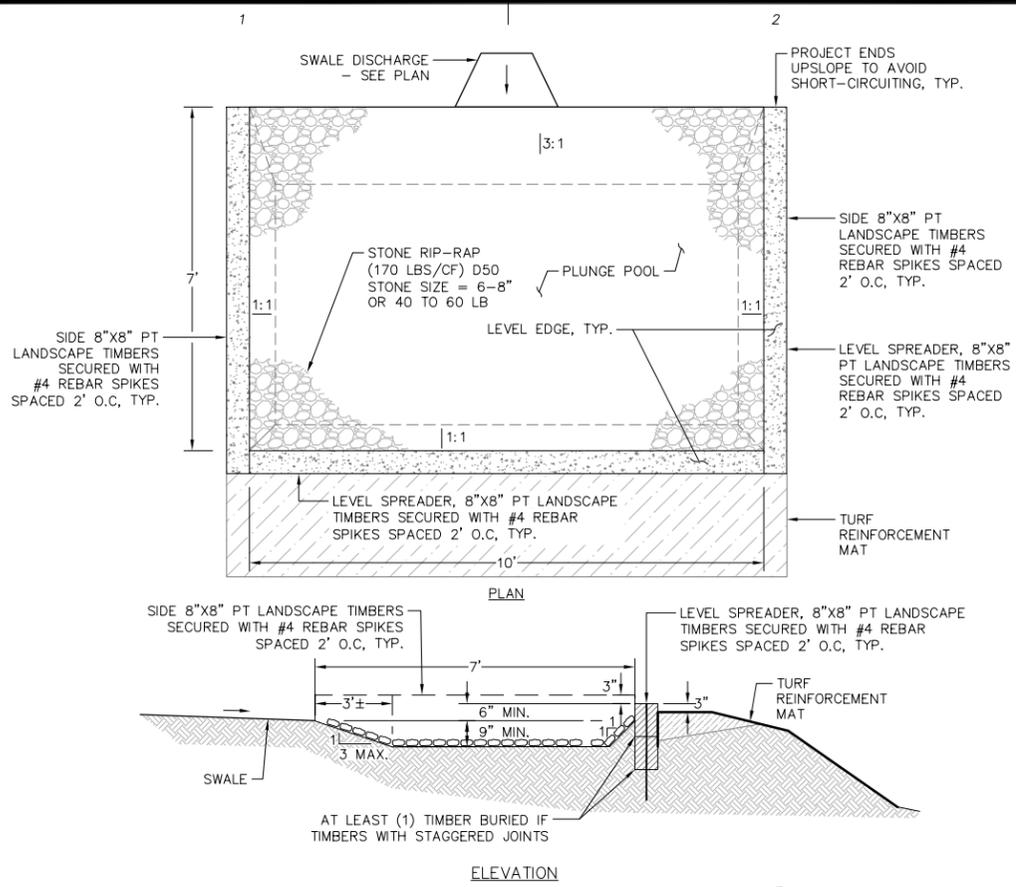
STAMP:



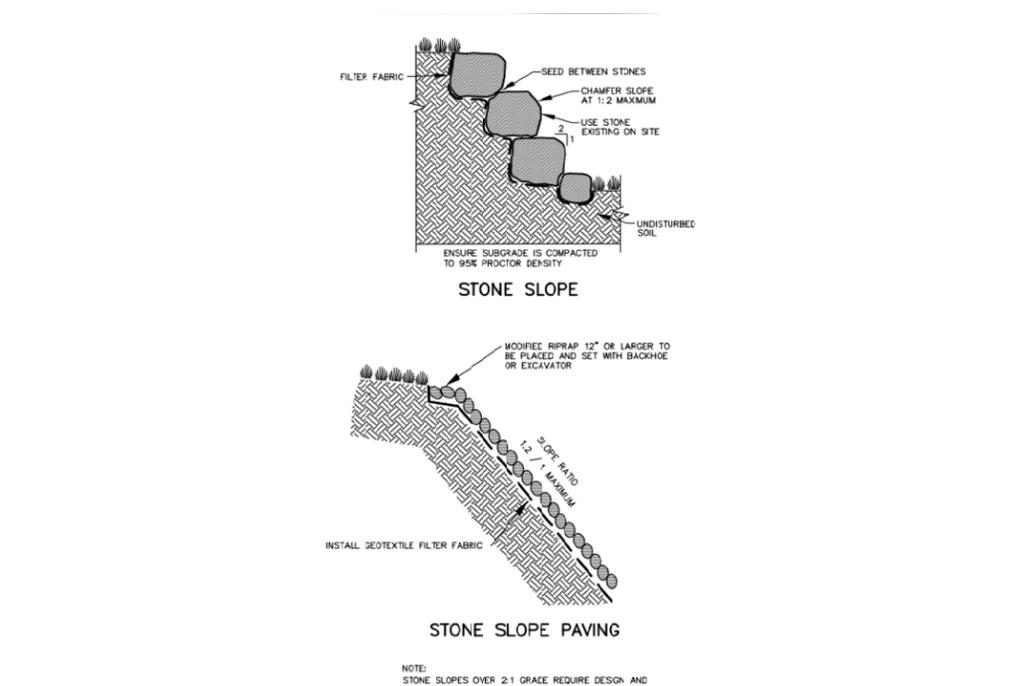
DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024
SHEET TITLE:

DETAILS

D-2

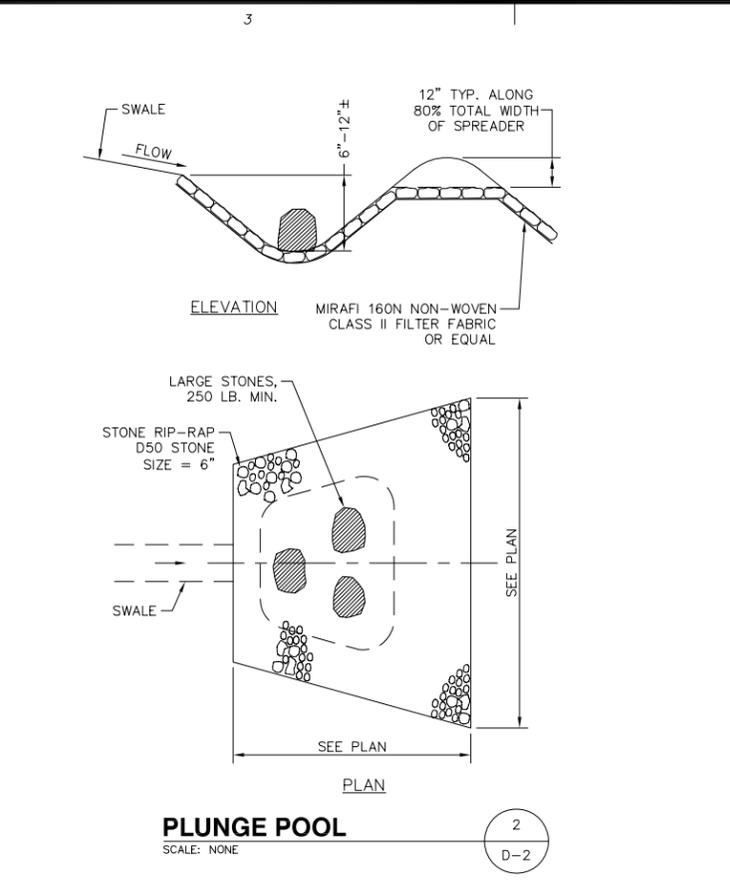


1
D-2

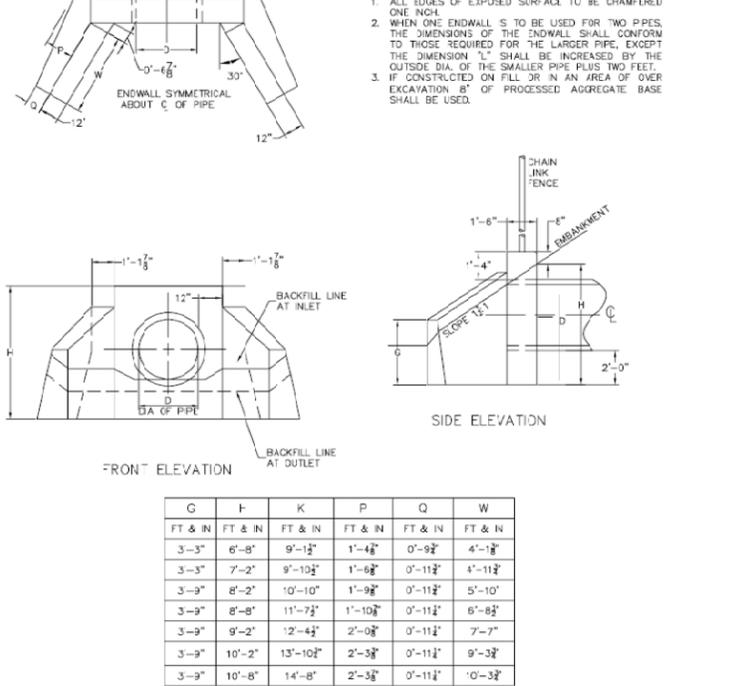


4
D-2

RIPRAP ARMOUR DETAIL
SCALE: NONE

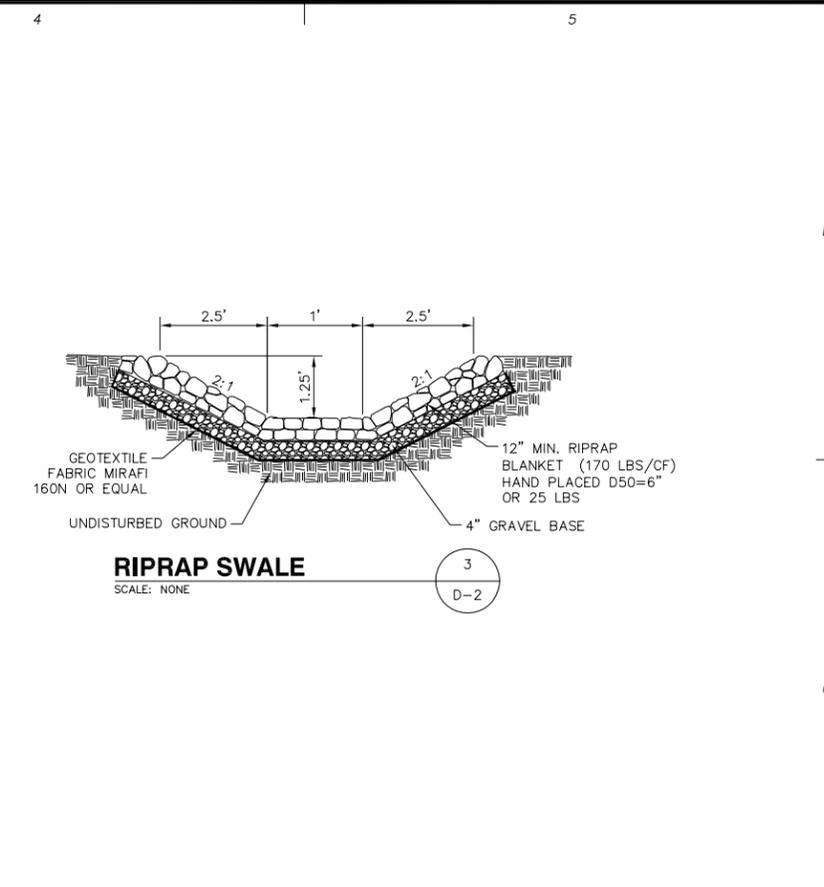


2
D-2

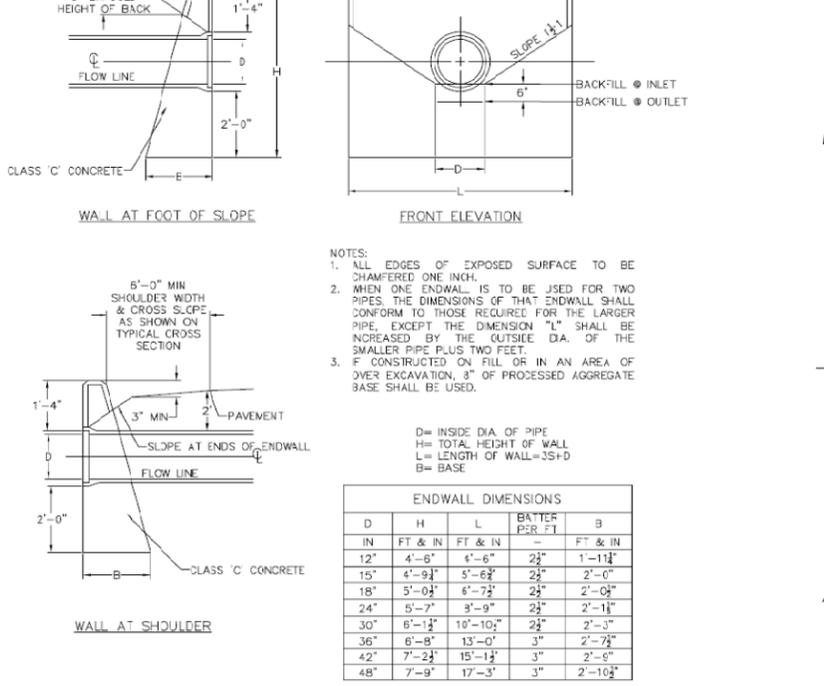


5
D-2

WINGWALL
SCALE: NONE



3
D-2



6
D-2

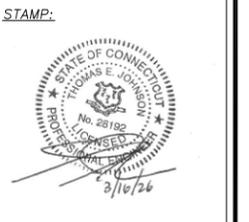
ENDWALL
SCALE: NONE

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

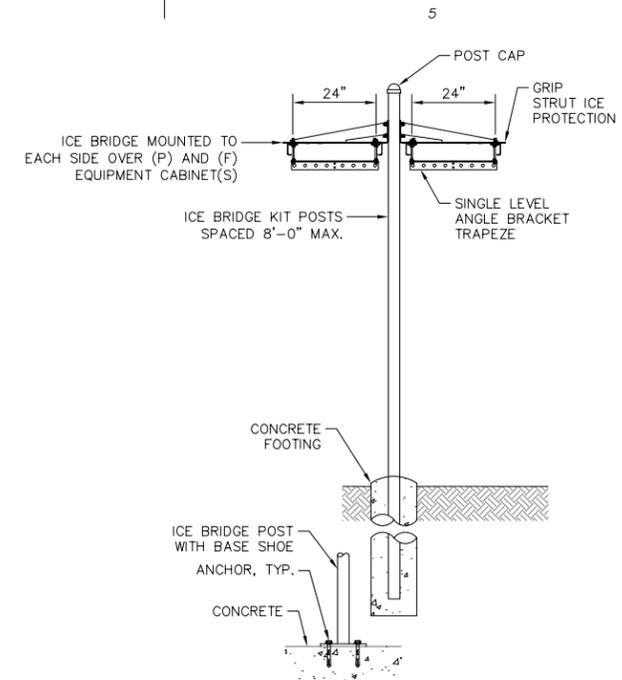
TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
 NORFOLK, CT 06058

APPLICANT: TARPON TOWERS III, LLC
 8916 77TH TERRACE EAST
 STE. 103
 LAKEWOOD RANCH, FL 34202

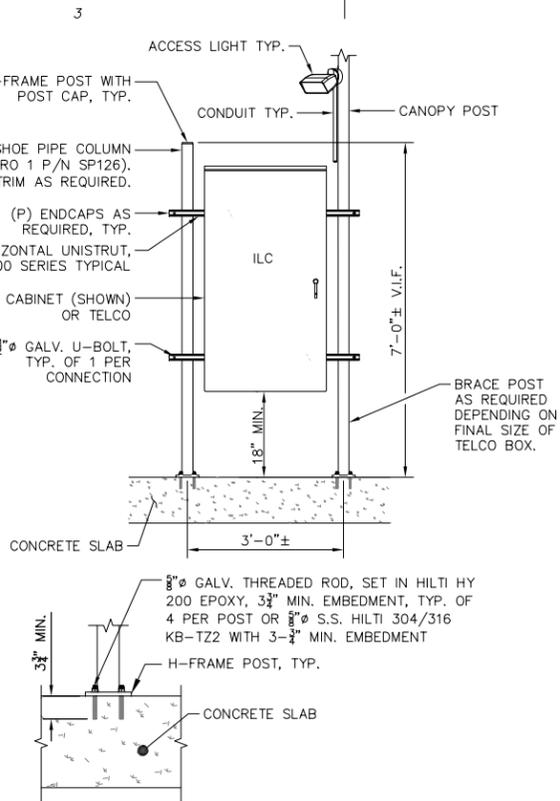


DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

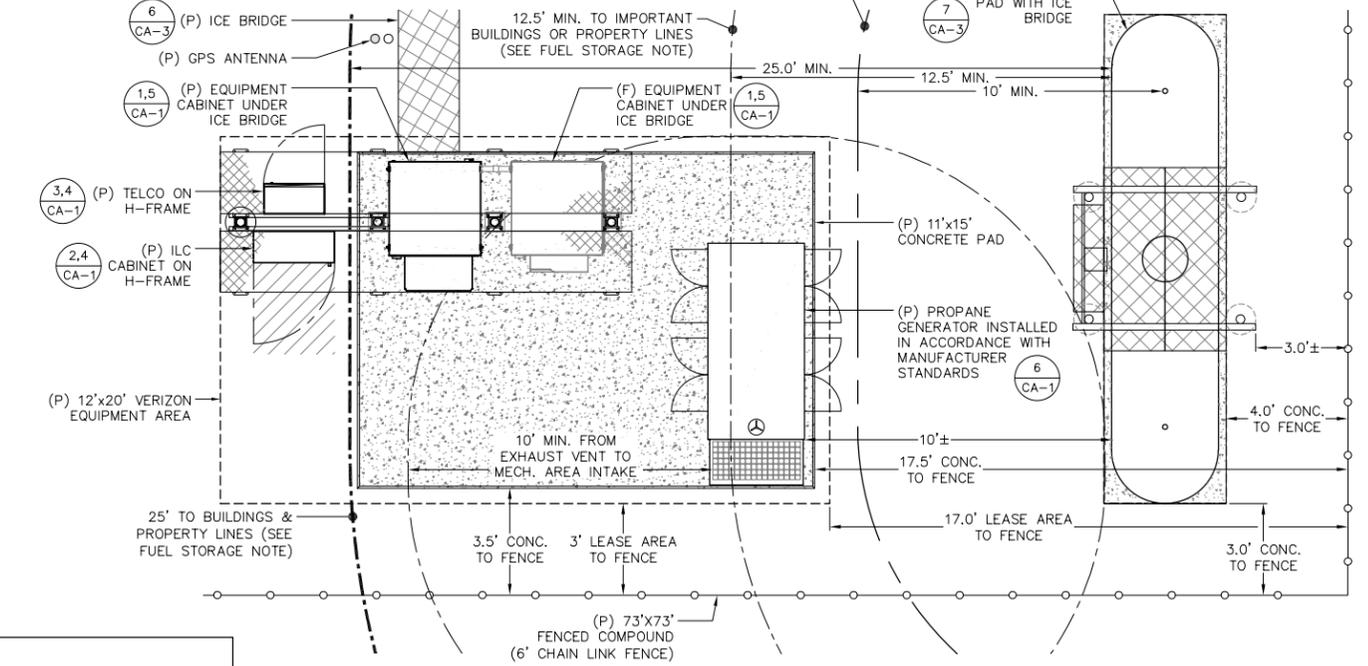
SHEET TITLE:
TENANT DETAILS
CA-1



EQUIPMENT CABINETS ICE BRIDGE
SCALE: NONE



UTILITY CABINET MOUNTING DETAIL
SCALE: NONE

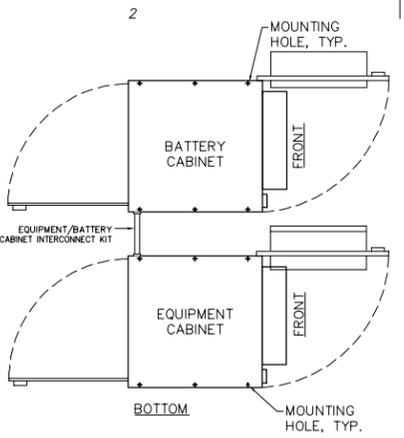


GROUND EQUIPMENT LAYOUT PLAN
SCALE: 1"=3' (22x34)
1"=6' (11x17)

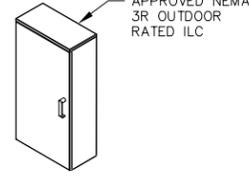
ANCHOR TENANT INFORMATION

SITE NAME: NORFOLK SOUTH CT
 MDG LOCATION ID: 5000941034
 FUZE PROJECT ID: 15322899
 78 GOSHEN EAST STREET

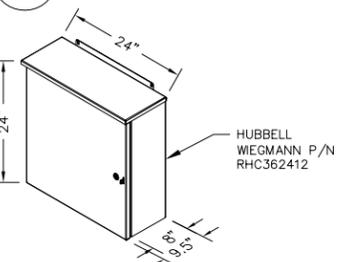
CELLCO PARTNERSHIP
 dba VERIZON WIRELESS
 20 ALEXANDER DRIVE
 WALLINGFORD, CT 06492



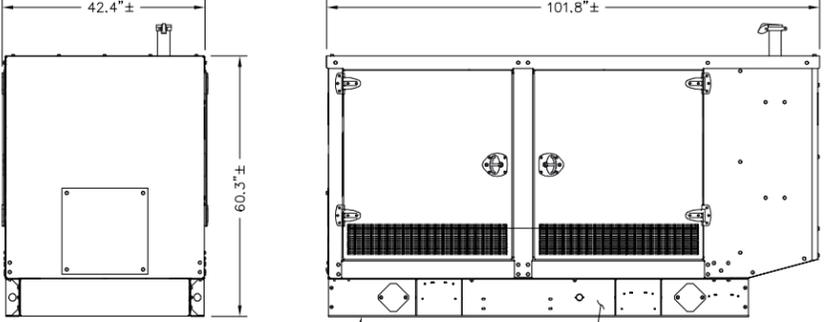
EQUIPMENT & BATTERY CABINET DETAIL
SCALE: NONE



INTEGRATED LOAD CENTER (ILC) CABINET
SCALE: NONE



TELCO CABINET
SCALE: NONE



50kW PROPANE GENERATOR
SCALE: NONE

- PROPANE MONITOR SHALL BE INSTALLED.
- CONTRACTOR TO VERIFY GAS & ELECTRICAL CONDUIT STUB-UP AREAS WITH THE MANUFACTURER OR CONSTRUCTION MANAGER PRIOR TO PLACEMENT OF CONCRETE SLAB.
- OPERATION OF THE EMERGENCY POWER GENERATOR FOR TESTING OR MAINTENANCE PURPOSES SHALL BE LIMITED TO A MAXIMUM OF ONE HOUR PER WEEK AND SHALL OCCUR ONLY ON WEEKDAYS (MONDAY-FRIDAY) AT A TIME WHEN, IN THE OPINION OF THE BUILDING COMMISSIONER, SUCH OPERATION WILL HAVE THE LEAST IMPACT ACOUSTICALLY ON THE SURROUNDING NEIGHBORHOOD.
- GENERATOR TO BE INSTALLED WITH AN EMERGENCY DISCONNECT AS REQUIRED BY NEC 445.18; DISCONNECTING MEANS AND SHUTDOWN OF PRIME MOVER.
- AN EXTERNAL EMERGENCY DISCONNECT MAY ALSO BE REQUIRED FOR GENERATORS GREATER THAN 15KW. CONFIRM WITH THE AUTHORITY HAVING JURISDICTION PRIOR TO INSTALLATION.

FUEL STORAGE NOTE FOR PROPANE:

NO OTHER PROPANE STORAGE TANKS SHALL BE PLACED WITHIN 25' OF THE VERIZON PROPANE TANK. REDUCTION IN 25' DISTANCE BETWEEN ASME CONTAINER 501 GAL TO 2000 GAL IS REDUCED TO 10' FOR A SINGLE 1200 GAL OR LESS CONTAINER PROVIDED NO LP GAS OR FLAMMABLE LIQUID CONTAINER IS WITHIN 25' MIN. TYP. (NFPA 6.4.1.3).

ALTERNATE REDUCTION AVAILABLE WHERE ABOVE GROUND CONTAINERS FOR STATIONARY ENGINES ARE LOCATED OUTDOORS AT TELECOMMUNICATION FACILITIES OF NON COMBUSTIBLE CONSTRUCTION THE MINIMUM DISTANCES SPECIFIED IN SECTION 6.4 SHALL BE ONE-HALF PROVIDED CONTAINER HAS A FILL VALVE WITH INTEGRATED MANUAL SHUTOFF AND FACILITY IS MORE THAN 50 FT FROM OTHER BUILDINGS NOT USED FOR TELECOMMUNICATIONS (NFPA 6.28.4).

ANCHOR TO CONCRETE PAD WITH 5/8"Ø HDG HILTI-KWIK BOLT 3 (LENGTH PER ENCLOSURE MANUFACTURER) WITH 3/4" MIN. EMBEDMENT, (TYP. OF 6 PER CABINET) CONFIRM RECOMMENDED ANCHOR BOLT SIZE WITH MANUFACTURER PRIOR TO CONSTRUCTION

VERIFY WITH CONSTRUCTION MANAGER PRIOR TO INSTALLATION OF CABINETS IF OPTIONAL CABINET PLINTH BASES AND TOPHATS HAVE BEEN ORDERED FOR USE IN LIEU OF OR IN ADDITION TO A STEEL CABINET SKID.

(P) GENERATOR BASE FRAME (VERIFY BASE DIMENSIONS & ANCHOR LOCATIONS WITH MANUFACTURER).
 CONFIRM LOCATION OF GAS LINE CONNECTION WITH MANUFACTURER PRIOR TO INSTALL.

CONTRACTOR TO VERIFY CONDUIT STUB-UP PLACEMENT WITH MANUFACTURER

PROVIDE EXTERNAL EMERGENCY DISCONNECT SWITCH WITH COVER (SEE NOTES 4 & 5)

700/AWS/PCS LTE AND 850 LTE/NR ANTENNA INFORMATION TABLE

SECTOR	PANEL ANTENNAS				FEEDER	SURGE SUPPRESSOR	FROM SURGE SUPPRESSOR	REMOTE RADIO UNIT		FROM REMOTE RADIO UNIT				
	AZIMUTH	QTY.	MAKE & MODEL	RAD. CENTER (AGL)				DOWN TILT	6x12 HYBRID CABLE	QUANTITY	1-PAIR HYBRID CABLE LENGTH	COAX JUMPER QTY.	COAX JUMPER SIZE	COAX CABLE LENGTH
ALPHA	20°	2	COMMSCOPE NHH-65C-R2B	180'	2 PROPOSED 235 LF±	1 PROPOSED 12-OVP	15'±	1	SAMSUNG B5/B13 RRH ORAN (RF4461d-13A)	4	1/2" ∅	15'±		
							15'±	1	SAMSUNG B2/B66g RRH ORAN (RF4439d-25A)	8	1/2" ∅	15'±		
BETA	140°	2	COMMSCOPE NHH-65C-R2B	180'					15'±	1	SAMSUNG B5/B13 RRH ORAN (RF4461d-13A)	4	1/2" ∅	15'±
							15'±	1	SAMSUNG B2/B66g RRH ORAN (RF4439d-25A)	8	1/2" ∅	15'±		
GAMMA	260°	2	COMMSCOPE NHH-65C-R2B	180'					15'±	1	SAMSUNG B5/B13 RRH ORAN (RF4461d-13A)	4	1/2" ∅	15'±
							15'±	1	SAMSUNG B2/B66g RRH ORAN (RF4439d-25A)	8	1/2" ∅	15'±		

BASED UPON RFDS DATED 04-10-2025

POST MODIFICATION INSPECTION (PMI) REQUIREMENTS:

A POST MODIFICATION INSPECTION (PMI) REPORT IS REQUIRED FOR THE NEW MOUNT AT THIS SITE, CONTRACTOR TO PROVIDE THE FOLLOWING INFORMATION, DOCUMENTS & PHOTOS TO CONFIRM THE MOUNT HAS BEEN COMPLETED PER THE MOUNT MANUFACTURER'S SPECIFICATIONS. ALL PHOTOS AND DOCUMENTS SHALL FOLLOW THE STANDARD VERIZON NAMING CONVENTIONS AND ORGANIZATION TREES.

PURPOSE - TO PROVIDE THE ENGINEER OF RECORD THE PROPER DOCUMENTATION IN ORDER TO COMPLETE THE REQUIRED MOUNT DESKTOP REVIEW OF THE POST MODIFICATION INSPECTION REPORT.

- CONTRACTOR IS RESPONSIBLE FOR MAKING CERTAIN THE PHOTOS PROVIDED AS NOTED BELOW PROVIDE CONFIRMATION THAT THE MODIFICATION/INSTALLATION WAS COMPLETED IN ACCORDANCE WITH THE MOUNT MANUFACTURER'S SPECIFICATIONS.
- CONTRACTOR SHALL RELAY ANY DATA THAT CAN IMPACT THE PERFORMANCE OF THE MOUNT OR THE MOUNT MODIFICATION, THIS INCLUDES SAFETY ISSUES.

BASE REQUIREMENTS:

- PROVIDE "AS BUILT DRAWINGS" SHOWING CONTRACTOR'S NAME, PREPARER'S SIGNATURE, AND DATE. ANY DEVIATIONS FROM THE DRAWINGS (PROPOSED MODIFICATION) MUST BE SHOWN.
- NOTATION THAT ALL HARDWARE WAS PROPERLY INSTALLED, AND THE EXISTING HARDWARE WAS INSPECTED FOR ANY ISSUES.
- VERIFICATION THAT LOADING IS AS COMMUNICATED IN THE DRAWINGS. NOTE: IF LOADING IS DIFFERENT THAN WHAT IS CONVEYED IN THE DRAWING CONTACT THE ENGINEER OF RECORD IMMEDIATELY.
- EACH PHOTO SHOULD BE DATED AND TIME STAMPED.
- PHOTOS SHOULD BE HIGH RESOLUTION AND SUBMITTED IN A .ZIP FILE AND SHOULD BE ORGANIZED IN THE FILE STRUCTURE AS DEPICTED IN THE VERIZON NETWORK STANDARDS PROCESS NSTD446.
- ANY SPECIAL PHOTOS OUTSIDE OF THE STANDARD REQUIREMENTS WILL BE INDICATED ON THE DRAWINGS.
- CONTRACTOR SHALL ENSURE THAT THE SAFETY CLIMB SYSTEM IS SUPPORTED AND NOT ADVERSELY IMPACTED BY THE INSTALLATION OF THE MOUNT. THIS MAY INVOLVE THE INSTALLATION OF WIRE GUIDES, OR OTHER ITEMS TO PROTECT THE SAFETY CLIMB.
- THE PHOTOS IN THE FILE STRUCTURE SHALL BE FORWARDED TO INFO@PROTERRA-DESIGN.COM

MATERIAL CERTIFICATION:

- MATERIALS UTILIZED MUST BE AS PER SPECIFICATION ON THE DRAWINGS OR THE EQUIVALENT AS VALIDATED BY THE ENGINEER OF RECORD.
- SUBMISSION OF SPECIFICATIONS, INVOICES CERTIFYING, AND/OR ENGINEER OF RECORD APPROVAL OF AN "EQUIVALENT" MUST BE SUBMITTED TO THE NOTED EMAIL BOX BY THE PMI CONTRACTOR.
- THE CONTRACTOR MUST CERTIFY THAT THE MATERIALS MEET THESE SPECIFICATIONS BY ONE OF THE METHODS BELOW AND SHALL BE SUBMITTED TO THE NOTED EMAIL BOX BY THE PMI CONTRACTOR:

THE MATERIAL UTILIZED WAS AS SPECIFIED ON THE CONSTRUCTION DRAWINGS.

THE MATERIAL UTILIZED WAS AN "EQUIVALENT" AND INCLUDED AS PART OF THE PMI ARE THE CERTIFICATIONS FROM THE ENGINEER OF RECORD, INVOICES, AND/OR SPECIFICATIONS VALIDATING ACCEPTED STATUS.

CERTIFYING INDIVIDUAL: COMPANY _____
NAME _____
SIGNATURE _____

PHOTO REQUIREMENTS:

BASE AND DURING INSTALLATION PHOTOS:

- BASE PICTURES INCLUDE
 - PHOTO OF GATE SIGNS SHOWING THE TOWER OWNER, SITE NAME, AND NUMBER
 - PHOTO OF CARRIER SHELTER SHOWING THE CARRIER SITE NAME AND NUMBER IF AVAILABLE
 - PHOTOS OF THE GALVANIZING COMPOUND AND/OR PAINT USED (IF APPLICABLE), CLEARLY SHOWING THE LABEL AND NAME DURING INSTALLATION PHOTOS IF PROVIDED - MUST BE PLACED ONLY IN THIS FOLDER

PHOTOS TAKEN AT GROUND LEVEL:

- OVERALL TOWER STRUCTURE BEFORE AND AFTER INSTALLATION OF THE MOUNT
- PHOTOS OF THE APPROPRIATE MOUNT BEFORE AND AFTER INSTALLATION; IF THE MOUNTS ARE AT DIFFERENT RAD ELEVATIONS, PICTURES MUST BE PROVIDED FOR ALL ELEVATIONS THAT THE MODIFICATIONS WERE INSTALLED

PHOTOS TAKEN AT MOUNT ELEVATION

- PHOTOS SHOWING EACH INDIVIDUAL SECTOR BEFORE AND ALSO AFTER INSTALLATION. EACH ENTIRE SECTOR MUST BE IN ONE PHOTO TO SHOW IN THE INTER-CONNECTION OF MEMBERS.
- CLOSE-UP PHOTOS OF EACH INSTALLED MOUNT COMPONENT PER THE MANUFACTURER'S INSTALLATION DRAWINGS; PICTURES SHOULD ALSO INCLUDE CONNECTION HARDWARE (U-BOLTS, BOLTS, NUTS, ALL-THREADED RODS, ETC.)
- PHOTOS SHOWING THE MEASUREMENTS OF THE INSTALLED MODIFICATION MEMBER SIZES (I.E. LENGTHS, WIDTHS, DEPTHS, DIAMETERS, THICKNESSES)
- PHOTOS SHOWING THE ELEVATION OR DISTANCES OF THE INSTALLED COMPONENTS FROM THE APPROPRIATE REFERENCE LOCATIONS SHOWN IN THE MANUFACTURER'S INSTALLATION DRAWINGS AND/OR THE CONSTRUCTION DRAWINGS SUCH AS:

- LOCATION OF TIE-BACKS ON SECTOR FRAMES INCLUDING SPACING FROM THE KNUCKLE AT THE STANDOFF ARM;
- SPACING BETWEEN ANTENNAS ON THE FACE;
- LOCATION OF RAD CENTER IN RELATION TO UPPER AND LOWER CROSS BARS;
- PHOTOS OF OTHER CRITICAL DIMENSIONS DEPICTED IN THE CONSTRUCTION DRAWINGS MAY BE REQUIRED.
- PHOTOS SHOWING THE INSTALLED COMPONENTS ONTO THE TOWER WITH TAPE DROP MEASUREMENTS (IF APPLICABLE) (I.E. RING/COLLAR MOUNTS, TIE-BACKS, V-BRACING KITS, ETC.); IF THE EXISTING MOUNT ELEVATION NEEDS TO BE CHANGED ACCORDING TO THE CONSTRUCTION DRAWINGS, A TAPE DROP MEASUREMENT SHALL BE PROVIDED BEFORE THE ELEVATION CHANGE
- PHOTOS SHOWING THE SAFETY CLIMB WIRE ROPE ABOVE AND BELOW THE MOUNT PRIOR TO MODIFICATION.
- PHOTOS SHOWING THE SAFETY CLIMB WIRE ROPE ABOVE AND BELOW THE MOUNT POST MODIFICATION.

(E) EXISTING
(P) PROPOSED
(F) FUTURE

NO MOUNT SUBSTITUTIONS WITHOUT PRIOR APPROVAL BY ENGINEER

ALL WORK TO BE COMPLETED IN ACCORDANCE WITH LATEST RF DESIGN SHEET DATED 04-10-2025

STRUCTURAL ANALYSIS REPORT TO BE COMPLETED PRIOR TO CONSTRUCTION

ANCHOR TENANT INFORMATION



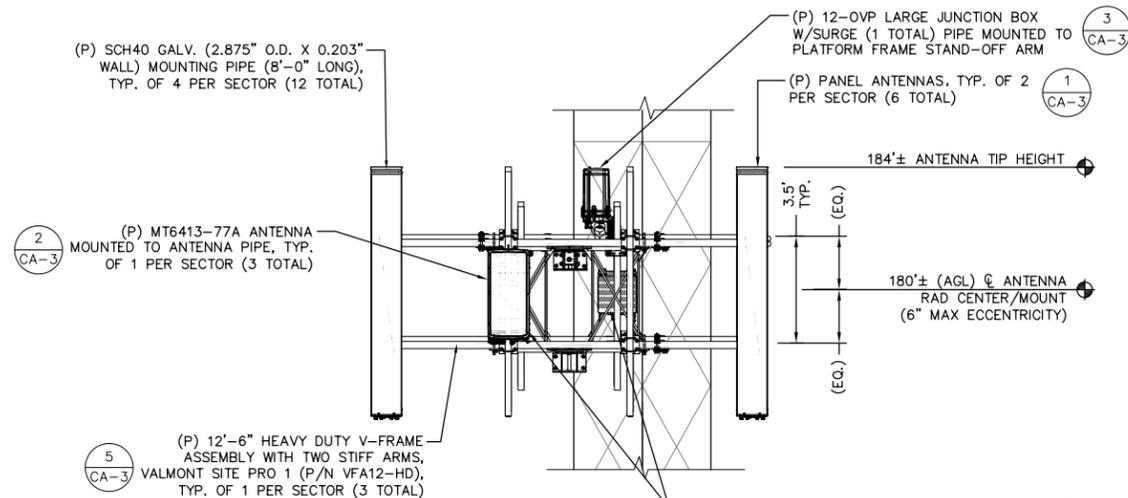
SITE NAME: NORFOLK SOUTH CT
MDG LOCATION ID: 5000941034
FUZE PROJECT ID: 15322899
78 GOSHEN EAST STREET

CELLCO PARTNERSHIP
dba VERIZON WIRELESS
20 ALEXANDER DRIVE
WALLINGFORD, CT 06492

L-SUB6/NR ANTENNA INFORMATION TABLE

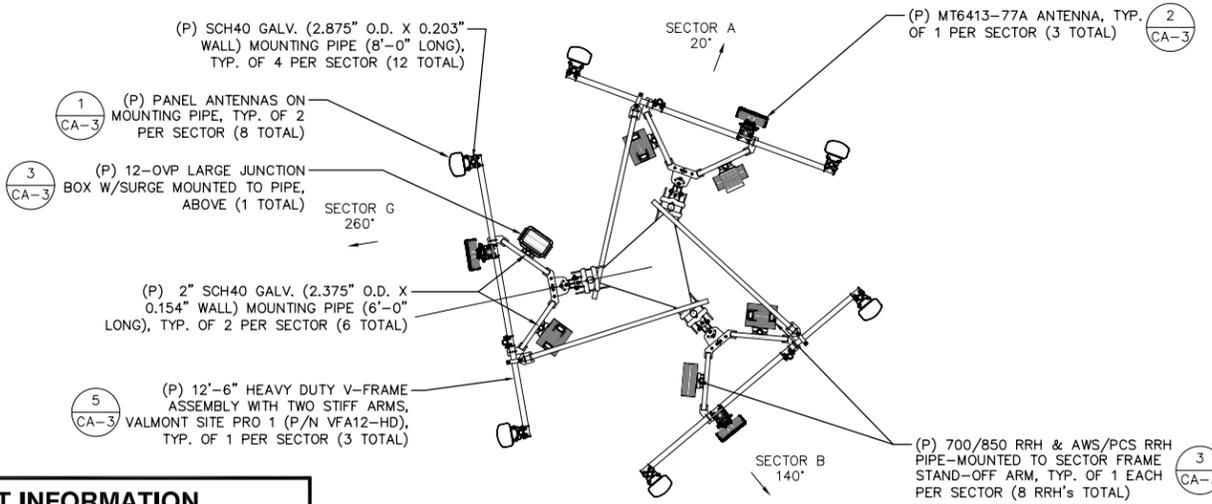
SECTOR	PANEL ANTENNAS				FEEDER	SURGE SUPPRESSOR	FROM SURGE SUPPRESSOR	
	AZIMUTH	QTY.	MAKE & MODEL	RAD. CENTER (AGL)				DOWN TILT
ALPHA	20°	1	SAMSUNG MT6413-77A	180'	0M/1°E	2 PROPOSED 235 LF± (SHARED)	1 PROPOSED 12-OVP (SHARED)	15'±
BETA	140°	1	SAMSUNG MT6413-77A	180'	0M/1°E			15'±
GAMMA	260°	1	SAMSUNG MT6413-77A	180'	0M/1°E			15'±

BASED UPON RFDS DATED 04-10-2025



RF ANTENNA ELEVATION

SCALE: 1"=3' (22x34)
1"=6' (11x17)



ANTENNA PLAN

SCALE: 1"=3' (22x34)
1"=6' (11x17)



CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06068

APPLICANT: TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202

TARPON TOWERS

STAMP:



DATE: 03/16/2026

DRAWN: JEB/STZ/PN

CHECK: JMM/TEJ

SCALE: SEE PLAN

JOB NO.: 24-024

SHEET TITLE:

**TENANT
DETAILS**

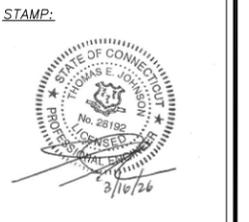
CA-2

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06068

APPLICANT: TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202



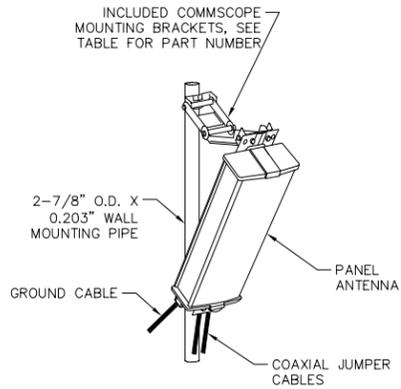
DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

SHEET TITLE:
TENANT DETAILS
CA-3

ANCHOR TENANT INFORMATION

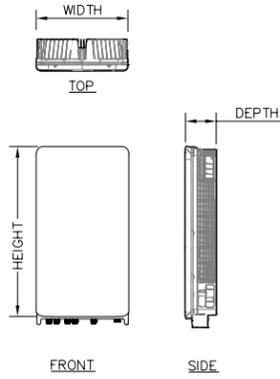
SITE NAME: NORFOLK SOUTH CT
MDG LOCATION ID: 5000941034
FUZE PROJECT ID: 15322899
78 GOSHEN EAST STREET

CELLCO PARTNERSHIP
dba VERIZON WIRELESS
20 ALEXANDER DRIVE
WALLINGFORD, CT 06492



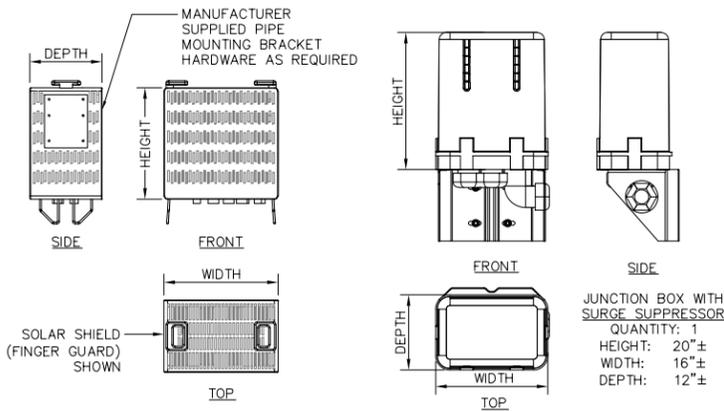
MODEL	HEIGHT	WIDTH	DEPTH	WEIGHT
COMMSCOPE NHH-65C-R2B	96.0"	11.9"	7.1"	51±lbs.

PANEL ANTENNA DETAIL 1
SCALE: NONE CA-3



MODEL	HEIGHT	WIDTH	DEPTH	WEIGHT
MT6413-77A - ANTENNA	28.9"±	15.8"±	5.5"±	57±lbs.

L-SUB6 ANTENNA DETAIL 2
SCALE: NONE CA-3



MODEL	HEIGHT	WIDTH	DEPTH	WEIGHT
SAMSUNG ORAN RRH (RF4461d-13A)	14.96"	14.96"	10.24"	79±lbs.
SAMSUNG ORAN RRH (RF4439d-25A)	14.96"	14.96"	10.04"	74±lbs.

RRH & OVP DETAIL 3
SCALE: NONE CA-3

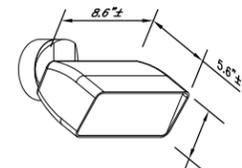


INTERMATIC FF2H

TIME CYCLE: 2 HOURS
SWITCH: SPST
HOLD: NO

OR APPROVED EQUIVALENT
20 AMP, 125 VAC

TIMER SWITCH DETAIL



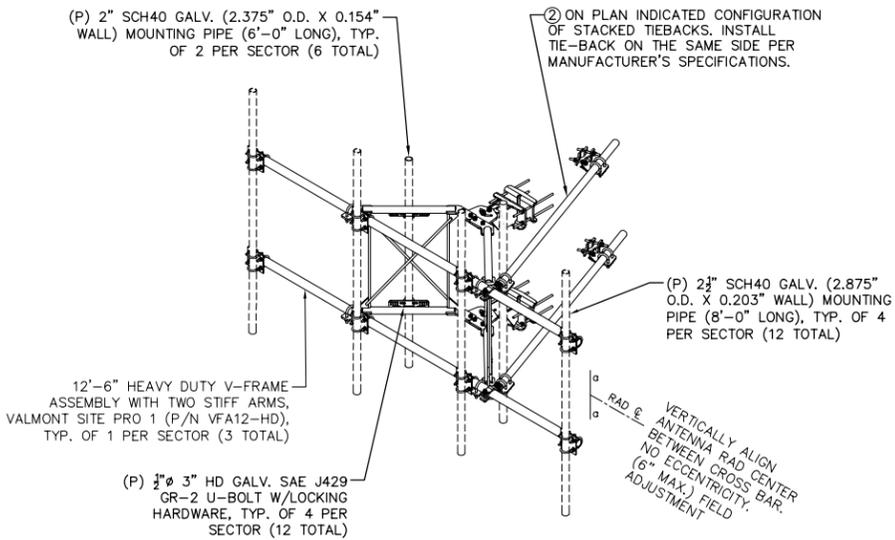
RAB LIGHTING - FFLED18N
18W
BRONZE

MOUNT PER
MANUFACTURER'S SPECIFICATION
125 VAC

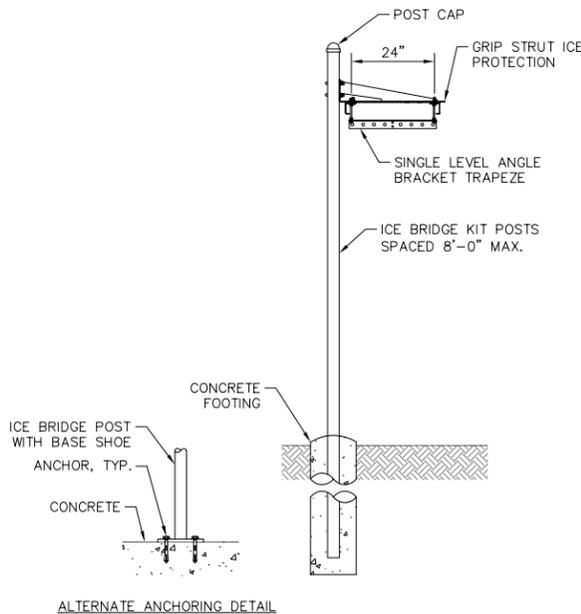
LED FLOOD LIGHT DETAIL

FLOOD LIGHT & SWITCH DETAIL 4
SCALE: NONE CA-3

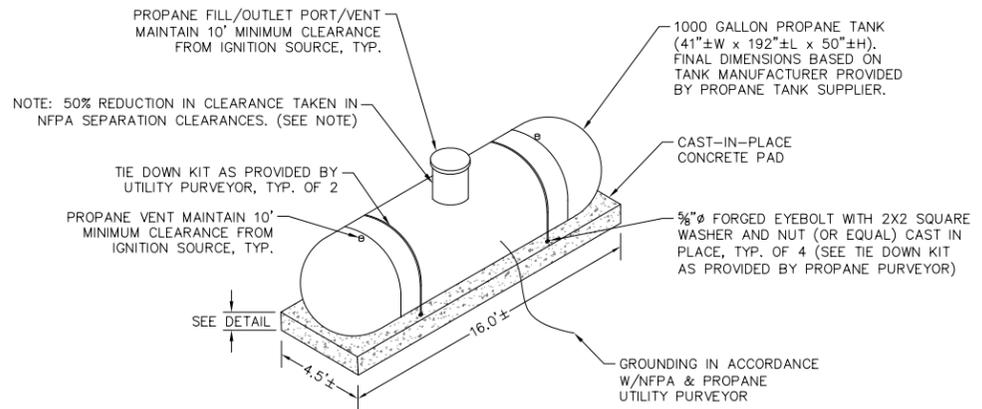
PMI NOTE: A POST MODIFICATION INSPECTION (PMI) REPORT IS REQUIRED FOR THE MOUNT MODIFICATIONS AT THIS SITE. CONTRACTOR TO PROVIDE THE INFORMATION, DOCUMENTS & PHOTOS AS NOTED IN THE PMI NOTES ON SHEET D-3 TO VERIZON AND TO THE ENGINEER OF RECORD FOR THE DESKTOP MOUNT MODIFICATION INSPECTION REVIEW.



SECTOR FRAME DETAIL 5
SCALE: NONE CA-3



ICE BRIDGE 6
SCALE: NONE CA-3

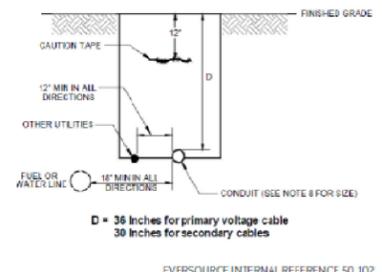
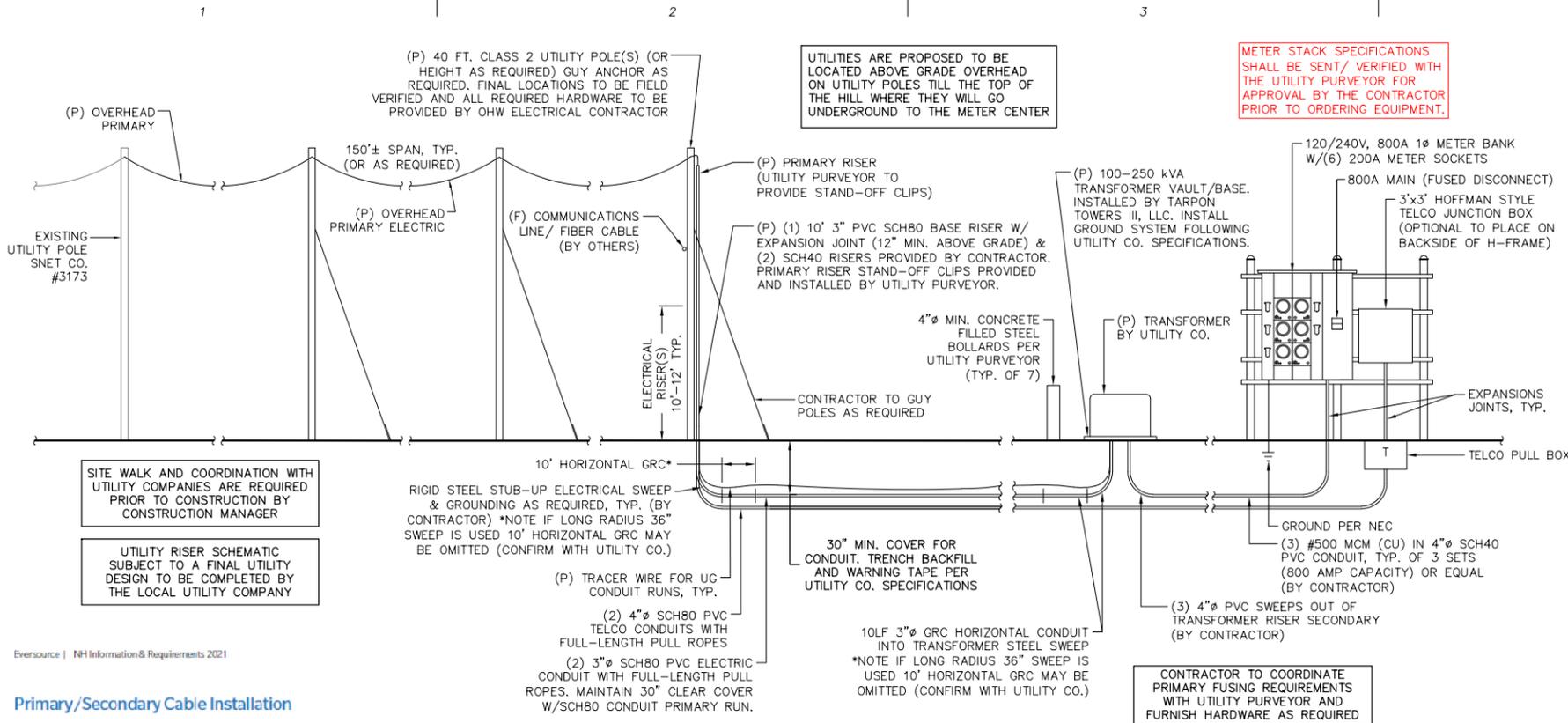


PROPANE NOTES:

NO OTHER PROPANE STORAGE TANKS SHALL BE PLACED WITHIN 25' OF THE VERIZON PROPANE TANK. REDUCTION IN 25' DISTANCE BETWEEN ASME CONTAINER 501 GAL TO 2000 GAL IS REDUCED TO 10' FOR A SINGLE 1200 GAL OR LESS CONTAINER PROVIDED NO LP GAS OR FLAMMABLE LIQUID CONTAINER IS WITHIN 25' MIN. TYP. (NFPA 6.4.1.3).

ALTERNATE REDUCTION AVAILABLE WHERE ABOVE GROUND CONTAINERS FOR STATIONARY ENGINES ARE LOCATED OUTDOORS AT TELECOMMUNICATION FACILITIES OF NON COMBUSTIBLE CONSTRUCTION THE MINIMUM DISTANCES SPECIFIED IN SECTION 6.4 SHALL BE ONE-HALF PROVIDED CONTAINER HAS A FILL VALVE WITH INTEGRATED MANUAL SHUTOFF AND FACILITY IS MORE THAN 50 FT FROM OTHER BUILDINGS NOT USED FOR TELECOMMUNICATIONS (NFPA 6.28.4).

PROPANE TANK ON CONCRETE PAD 7
SCALE: NONE CA-3



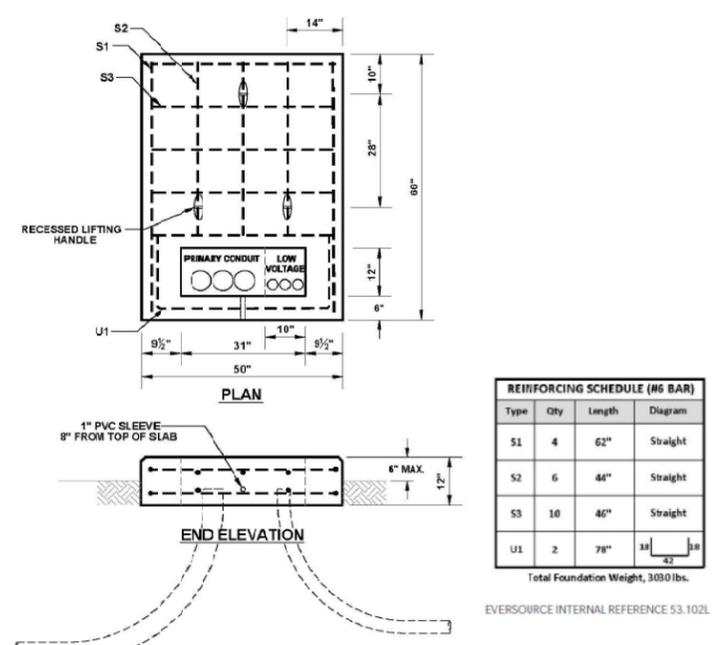
- NOTES**
- All non-metallic conduit and fittings shall be electrical grade, schedule 40 PVC, shall conform to the applicable sections of NEMA and be UL Listed. Only gray-colored conduit will be accepted. Any PVC conduit not having the proper NEMA and UL markings will not be accepted. All steel conduits shall conform to ASTM A123 and be rigid galvanized steel. All PVC conduit joints must be cemented. Steel fittings shall be sealed with compound.
 - All 90-degree sweeps will be made using rigid galvanized steel with a minimum radius of 24" for 3" conduit, 36" for 4" conduit and 5" conduit, and 48" for 6" conduit. All steel sweeps within 18" of surface shall be properly grounded.
 - A 10' horizontal section of rigid galvanized steel conduit will be required at each sweep for primary.
 - The conduit should cross paved areas at approximately 90 degrees.
 - Backfill may be made with excavated material or comparable; unless material is deemed unsuitable by the Company. Backfill shall be free of frozen lumps, rocks or stones 2" maximum in dimension, debris, and rubbish. Organic material shall not be used as backfill. Backfill shall be thoroughly compacted in six-inch layers.
 - A suitable pulling string, capable of 200 pounds of pull, must be installed in the conduit before the Company is notified to install cable. The string should be blown into the conduit after the run is assembled to avoid bonding the string to the conduit.
 - Routing of the conduit and inspection prior to backfill will be provided by the Company. Installation of the conduit will be done by the contractor. A Company Representative must be notified two business days prior to backfilling the trench. If a cable cannot be successfully pulled through the completed conduit system due to a construction error; it will be the contractor's responsibility to locate and repair the involved conduit. The contractor will be responsible for all resulting expenses.
 - Normal conduit sizes for the Company are 3" for single phase primary and secondary voltage cables, 4" for three phase secondary, and 5" for three phase primary.
 - All conduit installations must conform to the National Electrical Safety Code, state and local codes and ordinances, and where applicable, the National Electrical Code.
 - Other utilities shall maintain a minimum distance of 12" in all directions from the Company conduit.

BURIED CONDUIT SECTION
SCALE: NONE

4
E-1

ONE-LINE SCHEMATIC
SCALE: NONE

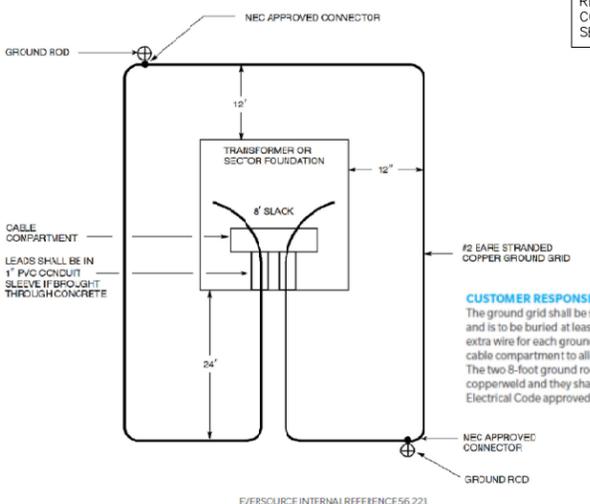
Single Phase Transformer Foundation 100-250 KVA



TRANSFORMER PAD AND GROUNDING DETAILS
SCALE: NONE

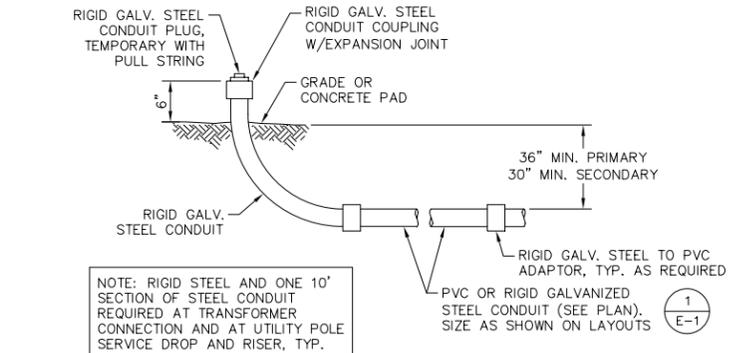
5
E-1

Pad-Mount Equipment Grounding Grid



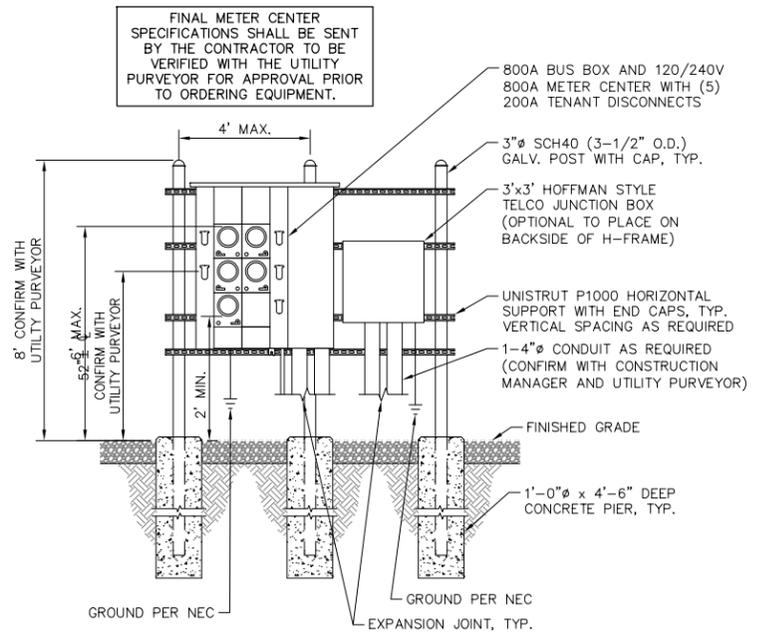
STUB-UP CONDUIT DETAIL
SCALE: NONE

3
E-1



TENANT METER CENTER
SCALE: NONE

2
E-1



Evsources | NH Information & Requirements 2021

Primary/Secondary Cable Installation

B

A

4 Bay Road
Building A, Suite 200
Hadley, MA 01035
Ph: (413)320-4918

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 COSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT: TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202

TARPON TOWERS

STAMP:

STATE OF CONNECTICUT
Professional Engineer
No. 28192
3/16/26

FOR SCHEMATIC ONLY

DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

SHEET TITLE:
ELECTRICAL DETAILS

E-1

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 COSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT:
TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202

STAMP:



FOR SCHEMATIC ONLY

DATE: 03/16/2026

DRAWN: JEB/STZ/PN

CHECK: JMM/TEJ

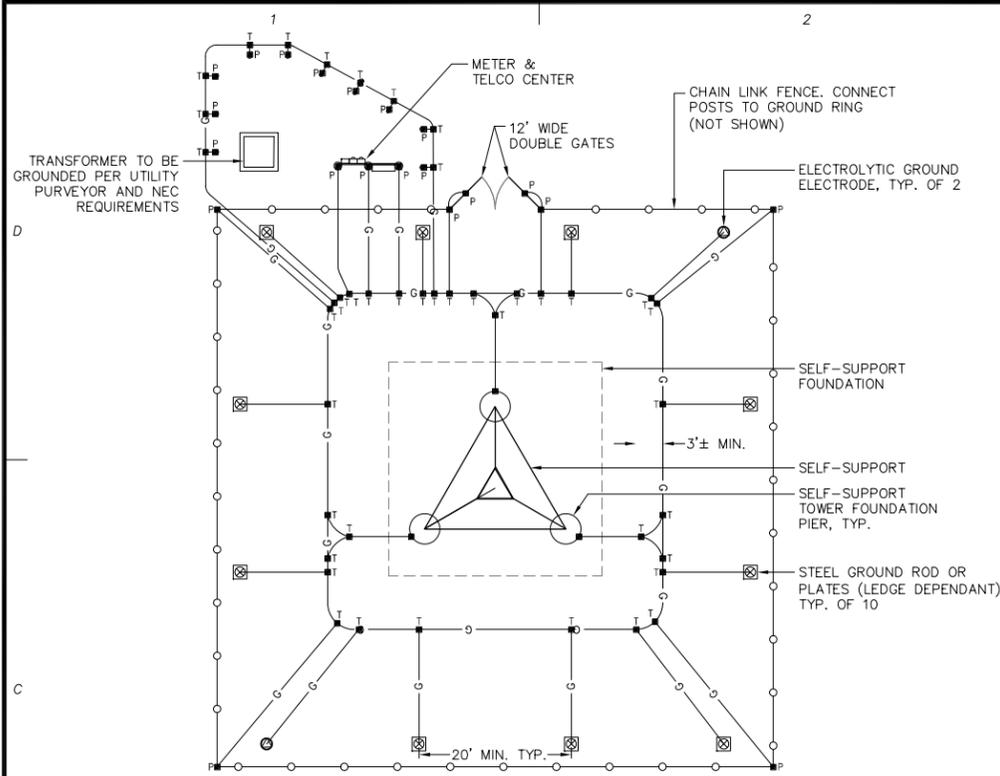
SCALE: SEE PLAN

JOB NO.: 24-024

SHEET TITLE:

**ELECTRICAL
DETAILS**

E-3

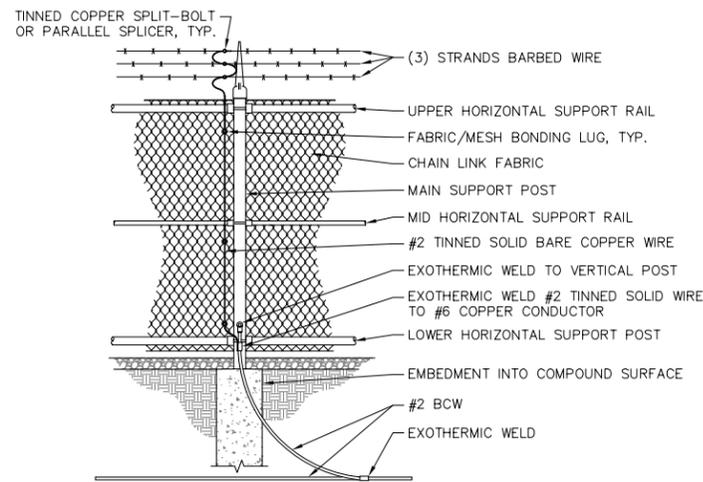


LEGEND

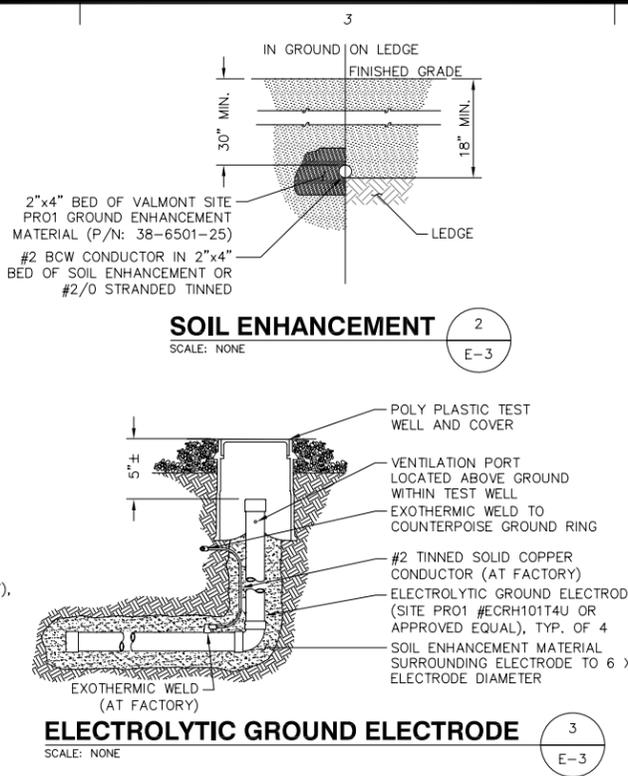
- ELECTROLYTIC GROUND ELECTRODE, TYP. OF 2 MIN. (SEE NOTE)
- ⊗ 5/8"Øx10' COPPER CLAD STEEL GROUND ROD DRIVE AT 45° (MAX.) OR 18"x18"x1/8" COPPER GROUND PLATE IN GROUND ENHANCEMENT MATERIAL, TYP. OF 10
- ⊕ EXOTHERMIC WELD #2 BCW TO #2 BCW CONDUCTOR PARALLEL
- ⊖ EXOTHERMIC WELD TO #2 BCW CONDUCTOR TO VERTICAL PIPE
- #2 BCW CONDUCTOR IN 2"x4" BED OF SOIL ENHANCEMENT OR #2/0 STRANDED TINNED

NOTE:
SITE GROUNDING SYSTEM IS A BASIC DESIGN. THE ACTUAL RESISTANCE TO GROUND CANNOT BE CONFIRMED WITHOUT A FIELD TEST. CONTRACTOR TO INSTALL AND PROVIDE RESISTANCE DOCUMENTATION OF 10 OHMS OR BETTER IN THE FORM OF A 3 POINT TEST OR EQUAL AT CLOSEOUT.

GROUNDING PLAN SCHEMATIC
SCALE: 1"=12'
E-3

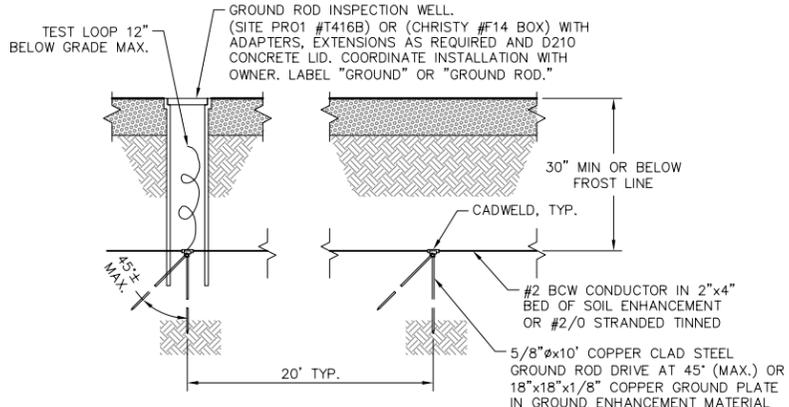


FENCE GROUNDING
SCALE: NONE
E-3

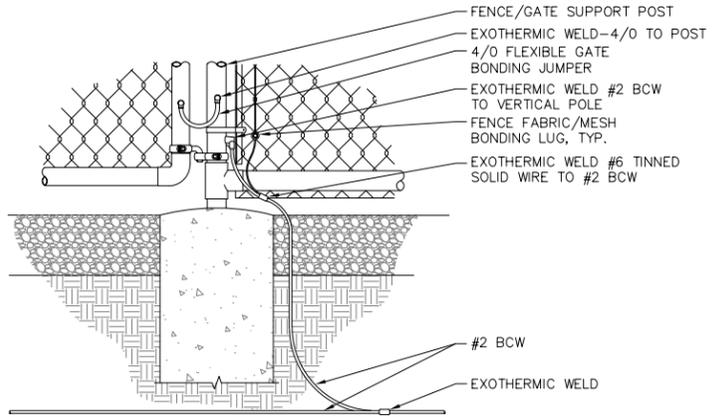


SOIL ENHANCEMENT
SCALE: NONE
E-3

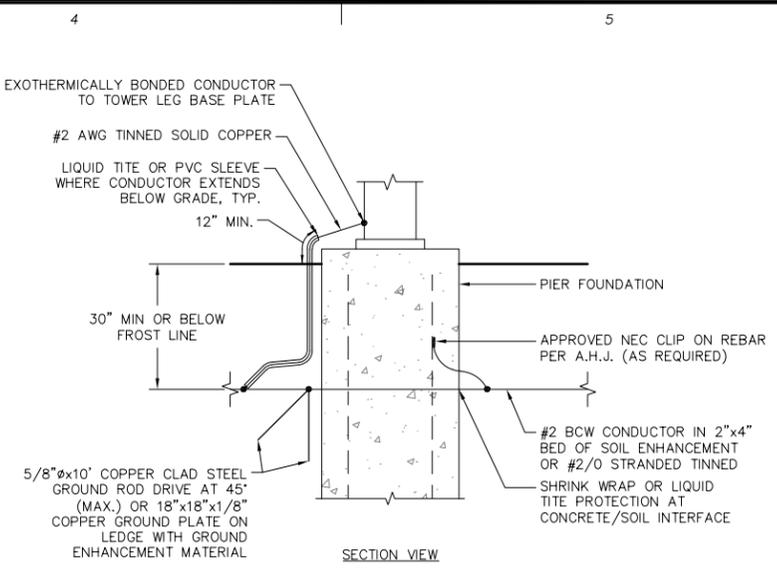
ELECTROLYTIC GROUND ELECTRODE
SCALE: NONE
E-3



GROUND RING WITH ROD
SCALE: NONE
E-3



FENCE GATE GROUNDING
SCALE: NONE
E-3



TOWER GROUNDING
SCALE: NONE
E-3

GROUNDING NOTES

- GENERAL**
- GROUNDING SHALL COMPLY WITH ARTICLE (250) OF THE NATIONAL ELECTRIC CODE, EIA/TIA-222 & MOTOROLA R-56 AS REQUIRED.
 - ALL GROUNDING DEVICES SHALL BE U.L. APPROVED OR LISTED FOR THEIR INTENDED USE.
 - THE CONTRACTOR SHALL SECURE A COPY OF ANY SOIL RESISTIVITY AND/OR SITE RESISTANCE TO EARTH TESTING PREVIOUSLY PERFORMED. IF NO RECORDS EXIST THAN A FIELD SOIL RESISTIVITY TEST SHALL BE PERFORMED TO ASSURE THE GROUNDING SYSTEM PROVIDES 10 OHMS OR LESS IN ACCORDANCE WITH OWNER SPECIFICATIONS.

- GROUND RODS**
- RODS SHALL BE 5/8" DIAMETER, 10' LONG COPPER CLAD STEEL OR SOLID COPPER
 - SHALL BE BURIED 30" (MIN.) OR BELOW PERMANENT MOISTURE LEVEL PENETRATING BELOW FROST LINE
 - RODS SEPARATED 20" (MIN.) TO OTHER GROUND RODS OR ELECTRODES
 - SEPARATION BETWEEN GROUND RODS IN SAME GROUNDING SYSTEM SHALL BE GREATER THAN SUM OF RESPECTIVE LENGTHS
 - GROUND RODS SHALL NOT BE SHORTENED BY CUTTING OR DEFORMED BY DRIVING MACHINERY
 - WHERE CONDITIONS REQUIRE, RODS MAY BE DRIVEN AT ANGLES UP TO 45 DEGREES OR HORIZONTAL ORIENTED PERPENDICULAR TO GROUND RING

- ELECTROLYTIC GROUND RODS**
- WHERE CONDITIONS REQUIRE, 10' LONG ELECTROLYTIC GROUND ELECTRODES (ALLTEC TERRADYNE TG-10L, LYNCOLE XIT K2L-10CS, OR EQUIVALENT) MAY BE USED. INSTALL PER MANUFACTURER'S SPECIFICATIONS
 - L-SHAPED ELECTROLYTIC RODS SHALL BE INSTALLED PERPENDICULAR TO GROUND RING
 - BACKFILL WITH GROUNDING ENCASEMENT MATERIALS (ALLTEC TERRAFILL, LYNCOLE LYCONITE, OR EQUIVALENT)

- GROUND PLATES**
- ONLY TO BE USED WHERE CONDITIONS PROHIBIT USE OF GROUND RODS
 - 1/16" (MIN.) THICKNESS WITH 2 SQUARE FEET (MIN.) AREA UNPAINTED COPPER CLAD STEEL OR SOLID COPPER, UNLESS NOTED OTHERWISE.
 - TOP EDGE BURIED 30" (MIN.) OR BELOW PERMANENT MOISTURE LEVEL
 - PLATES SHALL BE INSTALLED VERTICALLY
 - BACKFILL WITH GROUNDING ENCASEMENT MATERIALS 6" MINIMUM ON ALL SIDES

- RADIAL GROUNDING CONDUCTORS**
- #2 BARE COPPER WIRE IN BED OF GROUND ENHANCEMENT MATERIAL OR #2/0 STRANDED TINNED.
 - CONDUCTORS SHALL RADIATE FROM TOWER CENTER
 - SHALL BE BONDED TO GROUND RING AND DIRECTLY TO TOWER
 - WHERE NOT POSSIBLE TO BOND DIRECTLY TO TOWER, ADDITIONAL #2 CONDUCTORS SHALL BE BONDED TO RING TO OBTAIN EQUIVALENT.
 - BURIED 30" WHERE POSSIBLE, 18" (MIN.)
 - EACH RADIAL CONDUCTOR SHALL BE 25' (MIN.), 80' (MAX.) IN LENGTH
 - WHERE MULTIPLE RADIALS ARE USED, VARY CONDUCTOR LENGTHS

- GROUNDING ENCASEMENT MATERIALS**
- PRE-PACKAGED MATERIALS SHALL BE USED
 - ACCEPTABLE MATERIALS: BENTONITE, BENTONITE CONTAINING MATERIALS, CONCRETE, CONDUCTIVE CONCRETE, CEMENT WITH GRADED GRANULAR CARBONACEOUS AGGREGATE IN PLACE OF SAND OR GRAVEL

- CONDUCTORS**
- #2 BCW CONDUCTOR IN GROUND ENHANCEMENT MATERIAL OR #2/0 STRANDED TINNED.
 - #2 OR #6 AWG BCW OR STRANDED COPPER WHERE ABOVE GROUND AS NOTED
 - SPLICES SHALL BE EXOTHERMICALLY WELDED
 - 8" (MIN.) BENDING RADIUS FOR #2 OR SMALLER. 90 DEGREES (MIN.) BEND. ALL BENDS TOWARDS GROUND LOCATION.

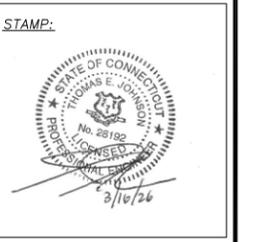
- CONNECTORS**
- GROUNDING CONNECTIONS SHALL BE EXOTHERMIC UNLESS OTHERWISE NOTED.
 - EXOTHERMIC WELDS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.
 - PRIOR TO INSTALLING LUGS ON GROUND WIRES, APPLY THOMAS & BETTS KOPR-SHIELD OR EQUAL
 - PREPARE ALL BONDING SURFACES FOR GROUNDING CONNECTIONS BY REMOVING ALL PAINT AND CORROSION DOWN TO SHINY METAL
 - FOLLOWING CONNECTION, APPLY APPROPRIATE CONDUCTIVE ANTI-OXIDIZING PAINT.
 - MECHANICAL CONNECTIONS SHALL BE 3 CRIMP STYLE COMPRESSION FIT CRIMPED WITH HYDRAULIC CRIMPING TOOLS OR EQUAL. NO SLIP BOLTS ARE ACCEPTABLE.

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: **SITE NAME: NORFOLK SOUTH CT**
SITE NUMBER: CT1238
ADDRESS: 78 GOSHEN EAST STREET
NORFOLK, CT 06058

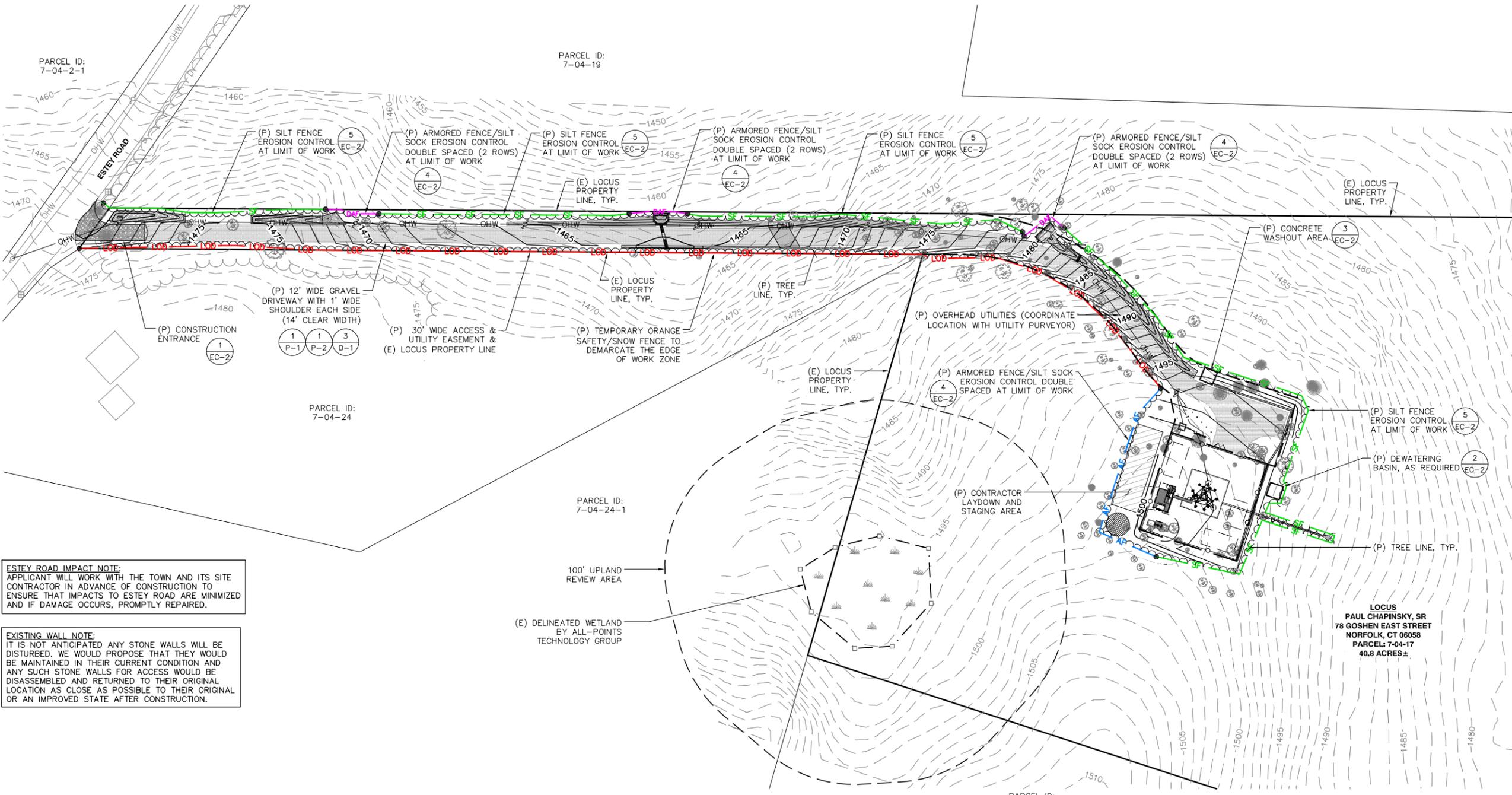
APPLICANT: **TARPON TOWERS III, LLC**
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202



STAMP:

DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024
SHEET TITLE:

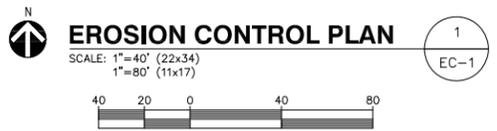
EC-1



ESTEY ROAD IMPACT NOTE:
APPLICANT WILL WORK WITH THE TOWN AND ITS SITE CONTRACTOR IN ADVANCE OF CONSTRUCTION TO ENSURE THAT IMPACTS TO ESTEY ROAD ARE MINIMIZED AND IF DAMAGE OCCURS, PROMPTLY REPAIRED.

EXISTING WALL NOTE:
IT IS NOT ANTICIPATED ANY STONE WALLS WILL BE DISTURBED. WE WOULD PROPOSE THAT THEY WOULD BE MAINTAINED IN THEIR CURRENT CONDITION AND ANY SUCH STONE WALLS FOR ACCESS WOULD BE DISASSEMBLED AND RETURNED TO THEIR ORIGINAL LOCATION AS CLOSE AS POSSIBLE TO THEIR ORIGINAL OR AN IMPROVED STATE AFTER CONSTRUCTION.

100' UPLAND REVIEW AREA
(E) DELINEATED WETLAND BY ALL-POINTS TECHNOLOGY GROUP

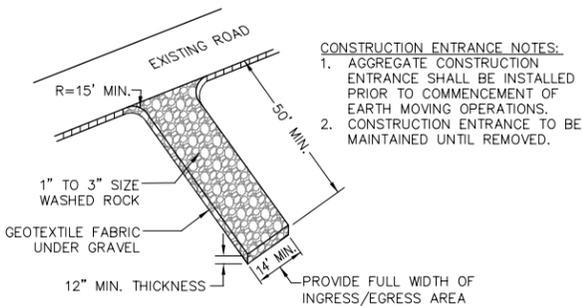


DISTURBANCE SUMMARY	
ITEM	AREA (SF)
TREE CLEARING	39,510
GRAVEL SURFACE (COMPOUND & DRIVEWAY)	17,830
RIPRAP (DRAINAGE)	3,640
LOAM & SEED	18,490
TOTAL EARTH DISTURBANCE	39,960

EROSION CONTROL BARRIER LEGEND

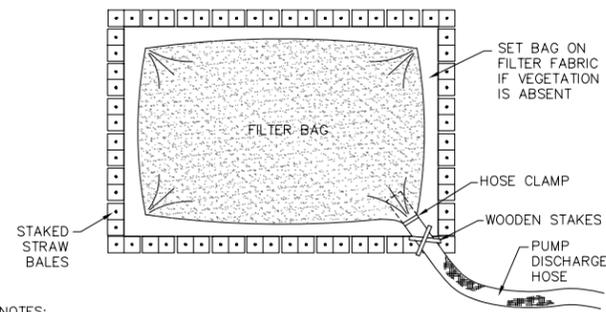
- TEMPORARY ORANGE SAFETY/SNOW FENCE TO DEMARCATATE THE EDGE OF WORK ZONE
- SF SILT FENCE
- AF ARMORED FENCE / SILT SOCK
- DAF ARMORED FENCE / SILT SOCK DOUBLE SPACED (2 ROWS)

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.



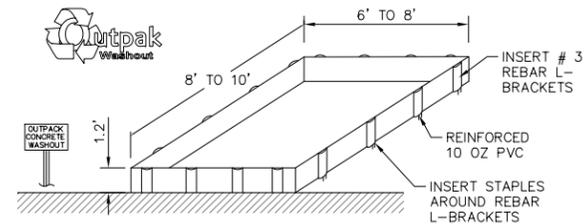
- CONSTRUCTION ENTRANCE NOTES:**
1. AGGREGATE CONSTRUCTION ENTRANCE SHALL BE INSTALLED PRIOR TO COMMENCEMENT OF EARTH MOVING OPERATIONS.
 2. CONSTRUCTION ENTRANCE TO BE MAINTAINED UNTIL REMOVED.

CONSTRUCTION ENTRANCE 1
SCALE: NONE EC-2



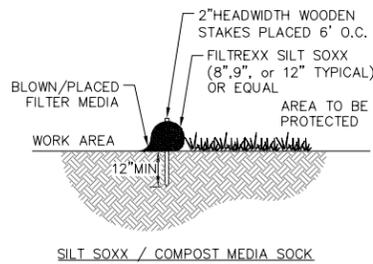
- NOTES:**
1. ENSURE DISCHARGE AREA IS COVERED BY STABLE VEGETATION.
 2. USE DIFFUSER NOZZLE OR LOW DISCHARGE RATE TO PREVENT SCOURING.
 3. TO BE PLACED AT AN UPLAND LOCATION THAT WILL ALLOW WATER TO DRAIN TO THE GROUND.
 4. SIZE OF STRAW BALE ENCLOSURE TO BE 10'x10' ADJUSTED TO WATER VOLUME.
 5. ADDITIONAL STRAW BALES MAY BE USED TO INCREASE RETENTION & FILTERING.

DEWATERING BASIN 2
SCALE: NONE EC-2



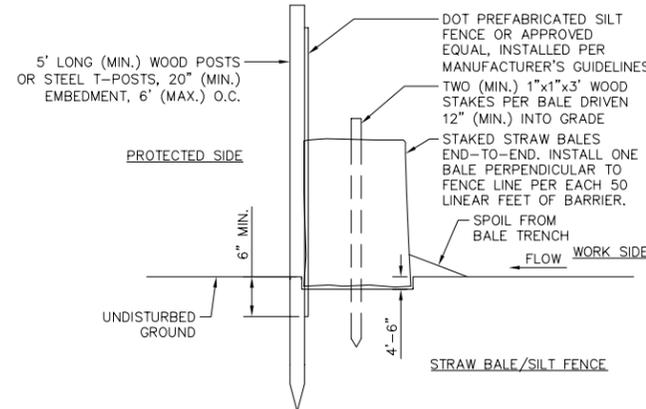
- NOTES:**
1. THE CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON THIS PROJECT.
 2. SIGNS SHALL BE PLACED AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CONCRETE WASHOUT.
 3. THE CONCRETE WASHOUT AREA WILL BE REPLACED AS NECESSARY TO MAINTAIN CAPACITY FOR WASTE CONCRETE AND OTHER LIQUID WASTE.
 4. WASHOUT RESIDUE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT AN APPROVED WASTE SITE.
 5. DO NOT MIX EXCESS AMOUNTS OF FRESH CONCRETE OR CEMENT ON-SITE.
 6. DO NOT WASH OUT CONCRETE TRUCKS INTO STORM DRAINS, OPEN DITCHES, STREETS, OR STREAMS.
 7. AVOID DUMPING EXCESS CONCRETE IN NON-DESIGNATED DUMPING AREAS.
 8. LOCATE WASHOUT AREA AT LEAST 50' (15 METERS) FROM STORM DRAINS, OPEN DITCHES, OR WATERBODIES.
 9. WASH OUT WASTES INTO THE OUTPACK WASHOUT AS SHOWN WHERE THE CONCRETE CAN SET, BE BROKEN UP, AND THEN DISPOSED OF PROPERLY.

CONCRETE WASHOUT AREA 3
SCALE: NONE EC-2

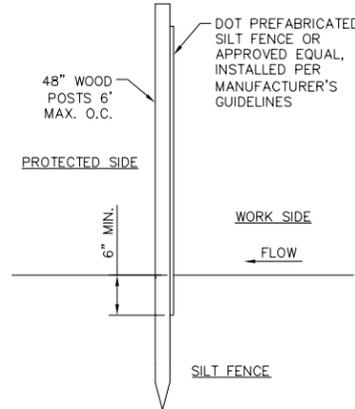


- NOTES:**
1. USE SILT SOXX WHERE CONDITIONS DO NOT ALLOW STAKES TO BE DRIVEN.
 2. SILT SOXX FILL TO MEET FILTREXX SPECIFICATIONS AND APPLICATION REQUIREMENTS.
 3. SILT SOXX COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER.
 4. FILTER MEDIA/STRAW BALES TO BE INERT AND FREE FROM INVASIVE WEEDS AND NON-NATIVE SPECIES.
 5. MAY STAKE BEHIND SOCK AT SLIGHT ANGLE (6" O.C.).

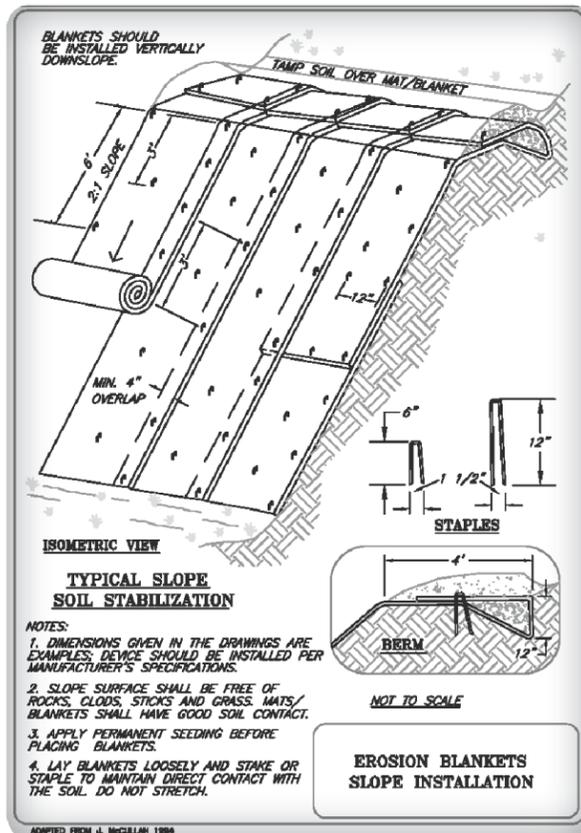
SILT SOXX / COMPOST MEDIA SOCK



EROSION CONTROL BARRIER 4
SCALE: NONE EC-2



SILT FENCE 5
SCALE: NONE EC-2



SLOPE TREATMENT 6
SCALE: NONE EC-2

EROSION CONTROL NOTES

1. APPLICANT PROPOSES TO CONSTRUCT A CELLULAR TELECOMMUNICATIONS FACILITY CONSISTING OF A FENCED COMPOUND, DRIVEWAY AND UTILITY WORK WITHIN A LEASE AREA AND EASEMENTS.
2. ALL WORK SHALL CONFORM TO THE CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL AS AMENDED.
3. TEMPORARY SILT FENCE EROSION CONTROL BARRIER SHALL BE MAINTAINED THROUGHOUT SITE CONSTRUCTION. STOCK PILE ON SITE 100 FT. OF SILT FENCE FOR EMERGENCY USE. TEMPORARY EROSION BARRIERS SHALL REMAIN IN PLACE UNTIL PERMANENT VEGETATIVE GROUND COVER IS ESTABLISHED.
4. THE CONTRACTOR SHALL CHIP ALL BRUSH AND SLASH CUTTINGS ON SITE AND STOCKPILE THE CHIPS TO BE USED ON ALL UNSTABLE, DISTURBED AREAS DURING CONSTRUCTION AS TEMPORARY STABILIZATION MULCH. NO BURNING WILL BE ALLOWED ON SITE.
5. TEMPORARY STABILIZATION MUST BE PROVIDED TO ANY DISTURBED EARTH THAT IS OPENED UP IN ANY ONE LOCATION FOR MORE THAN 14 DAYS. CHIPS FROM LAND CLEARING, EROSION CONTROL BLANKETS, OR FAST GROWING RYE GRASSES MAY BE USED FOR TEMPORARY STABILIZATION AS REQUIRED.
6. STRIPPED TOPSOIL SHALL BE STOCKPILED AND PROTECTED WITH STRAW MULCH. ALL STOCKPILES SHALL HAVE AN APPROVED SILTATION BARRIER TOTALLY SURROUNDING THE PILE. THE PILE SHALL BE IN AN APPROVED UPLAND AREA A MINIMUM OF TWENTY-FIVE FEET FROM ALL RESOURCE AREAS.
7. THE PHASING AND SEQUENCING OF THE WORK FOR THE SITE PREPARATION FOR THE TELECOMMUNICATIONS EQUIPMENT INSTALLATION CONSISTS OF INSTALLING TEMPORARY EROSION AND SEDIMENTATION CONTROL BARRIERS; CLEARING AND ROUGH GRADING OF COMPOUND; FOUNDATION WORK; BACK FILL FOUNDATIONS; FENCED COMPOUND CONSTRUCTION; INSTALLATION OF TOWER AND GROUND EQUIPMENT; INSTALLATION OF UTILITIES; GROUNDING AND LIGHTNING PROTECTION; EQUIPMENT TESTING; FINAL GRADING AND STABILIZATION OF DISTURBED AREAS; LOAM AND SEED DISTURBED AREAS OUTSIDE COMPOUND; FINAL CLEANUP. THE ESTIMATED TIME FOR COMPLETION OF THE WORK IS APPROXIMATELY SIXTEEN (16) WEEKS.
8. THE COMPOUND ENCLOSURE IS SURFACED WITH CRUSHED STONE UNDERLAIN BY A WEED-BLOCK SYNTHETIC FILTER FABRIC, DRAINAGE PATTERNS, RUNOFF VOLUMES AND PEAK FLOW RATES WILL NOT BE ALTERED BY THE PROPOSED CONSTRUCTION.
9. IF REQUIRED, TEMPORARY DEWATERING OF THE TRENCH EXCAVATIONS SHALL BE DIVERTED INTO A TEMPORARY STILLING BASIN. INFILTRATION IN THE STILLING BASIN AND FLOW THROUGH THE CRUSHED STONE CONTAINMENT BERM WILL RESULT IN DIFFUSE, NON-POINT SOURCE RUNOFF OVER VEGETATED AREAS.
10. ALL DISTURBED AREAS OUTSIDE THE LIMITS OF THE FENCED COMPOUND AND ROADWAY SHALL BE PERMANENTLY ESTABLISHED WITH A NATIVE VEGETATIVE GROUND COVER AT THE CONCLUSION OF CONSTRUCTION. GRADED AREAS SHALL BE PROTECTED WITH STRAW MULCH UNTIL A GOOD VEGETATIVE COVER IS ESTABLISHED.
11. THE TOTAL IMPACT AREA OF THE DISTURBED TOWER & COMPOUND CONSTRUCTION SITE IS BOUNDED BY THE "LIMIT OF WORK" AS SHOWN HEREON. THE MAXIMUM AREA OF DISTURBANCE WITHIN THE LIMIT OF WORK IS APPROXIMATELY 39,960 SF. THE PROJECT IMPACT AREA MEETS THE EXEMPTION THRESHOLD OF 43,560 SQUARE FEET (<1 ACRE) AND THEREFORE IS NOT REQUIRED TO OBTAIN A CONNECTICUT GENERAL PERMIT UNDER THE CGP PROGRAM OR A CONNECTICUT COMMERCIAL STORMWATER PERMIT FROM CT DEEP.
12. THE PROJECT OWNER'S GENERAL CONTRACTOR SHALL CONDUCT ALL SITE DEVELOPMENT WORK IN A MANNER THAT DOES NOT EXCEED THE LIMITS OF WORK SHOWN ON THE PLANS. ADDITIONALLY, THE PROJECT OWNER'S GENERAL CONTRACTOR SHALL CONDUCT ALL CONSTRUCTION ACTIVITIES IN A MANNER THAT DOES NOT RESULT IN STORM WATER DISCHARGES WITH AN ADVERSE IMPACT ON ANY RESOURCE AREAS OR DOWNSTREAM PROPERTIES.
13. A CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON THIS PROJECT. SIGNS SHALL BE PLACED AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CONCRETE WASHOUT. WASHOUT RESIDUE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT AN APPROVED WASTE SITE.
14. UPON COMPLETION OF THE PROJECT, THE CONTRACTOR SHALL REMOVE ALL ACCUMULATED SILT FROM BEHIND SILTATION BARRIERS AND DISPOSE OF SILT EVENLY IN UPLAND AREAS. REMOVE ALL EROSION CONTROL DEVICES WHEN A GOOD VEGETATIVE COVER IS ESTABLISHED.

CONSULTANTS:

NO.	DATE	REVISIONS
A	11/19/25	ISSUED FOR REVIEW
B	03/06/26	ISSUED FOR REVIEW
D	03/16/26	ISSUED FOR D&M

TITLE: SITE NAME: NORFOLK SOUTH CT
SITE NUMBER: CT1238
ADDRESS: 78 COSHEN EAST STREET
NORFOLK, CT 06058

APPLICANT: TARPON TOWERS III, LLC
8916 77TH TERRACE EAST
STE. 103
LAKEWOOD RANCH, FL 34202

TARPON TOWERS

STAMP:



DATE: 03/16/2026
DRAWN: JEB/STZ/PN
CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO.: 24-024

SHEET TITLE:
EROSION CONTROL DETAILS

EC-2

DALEY JOB NO. 59335
185' SELF-SUPPORTING TOWER
TARPON TOWERS
CT1238 NORFOLK SOUTH SITE
LITCHFIELD COUNTY, CONNECTICUT
LATITUDE: 41.925856 N
LONGITUDE: 73.218340 W

TOWER AND FOUNDATION DESIGN
REVISION 0

CASE JOB NO. 26138

PREPARED FOR:

Daley Tower Service, Inc.
601 Hector Connolly Road
Carencro, Louisiana 70520

PREPARED BY:

CASE International LLC

P.O. Box 4825
Lafayette, LA 70502
www.casengr.com



Michael F. Plahovinsak, P.E.
Sole Proprietor - Independent Engineer
18301 SR 161, Plain City, Ohio
614-398-6250 / mike@mfpeng.com
MFP Project #40926-012

Michael Plahovinsak 2026.02.20 16:18:29
-05'00'

TOWER CALCULATIONS

DESIGNED APPURTENANCE LOADING

TYPE	ELEVATION	TYPE	ELEVATION
Lightning Rod	185	RTH 0306-PFC	160
L-864 Beacon	185	RF4461D-13A	160
42,000 sqin EPA(A) Carrier (4,000 lbs)	180	RF4461D-13A	160
(2) NN-65C-HG-R1B	170	RF4461D-13A	160
(2) NN-65C-HG-R1B	170	RF4801D-25A	160
(2) NN-65C-HG-R1B	170	RF4801D-25A	160
MT6413-77A	170	RF4801D-25A	160
MT6413-77A	170	Sector Mount - EPA 13sqft, 800 lbs	160
MT6413-77A	170	Sector Mount - EPA 13sqft, 800 lbs	160
RTH 0306-PFC	170	Sector Mount - EPA 13sqft, 800 lbs	160
RTH 0306-PFC	170	(2) NN-65C-HG-R1B	150
RF4461D-13A	170	(2) NN-65C-HG-R1B	150
RF4461D-13A	170	(2) NN-65C-HG-R1B	150
RF4801D-25A	170	MT6413-77A	150
RF4801D-25A	170	MT6413-77A	150
RF4801D-25A	170	MT6413-77A	150
Sector Mount - EPA 13sqft, 800 lbs	170	RTH 0306-PFC	150
Sector Mount - EPA 13sqft, 800 lbs	170	RF4461D-13A	150
Sector Mount - EPA 13sqft, 800 lbs	170	RF4461D-13A	150
(2) NN-65C-HG-R1B	160	RF4461D-13A	150
(2) NN-65C-HG-R1B	160	RF4801D-25A	150
(2) NN-65C-HG-R1B	160	RF4801D-25A	150
MT6413-77A	160	RF4801D-25A	150
MT6413-77A	160	Sector Mount - EPA 13sqft, 800 lbs	150
MT6413-77A	160	Sector Mount - EPA 13sqft, 800 lbs	150
RTH 0306-PFC	160	Sector Mount - EPA 13sqft, 800 lbs	150

SYMBOL LIST

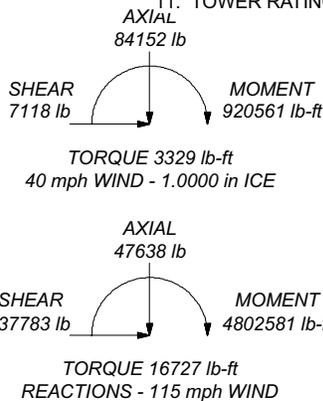
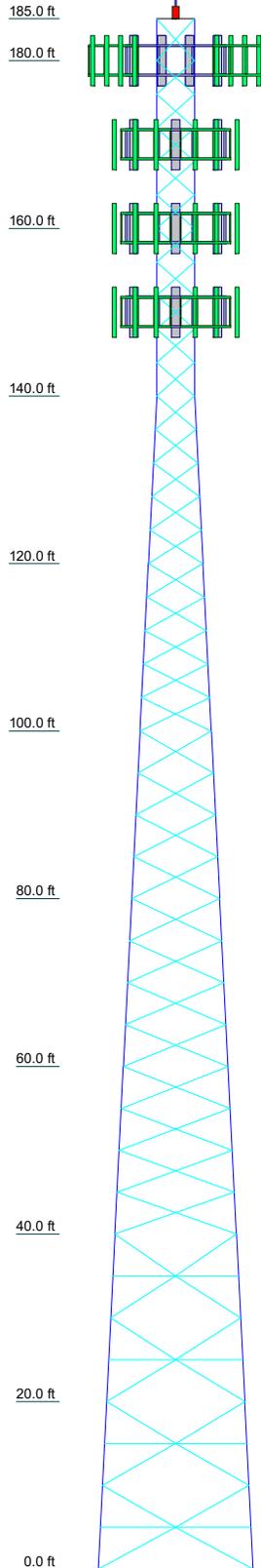
MARK	SIZE	MARK	SIZE
A	SR 1 3/4	B	L 1 3/4x1 3/4x3/16

MATERIAL STRENGTH

GRADE	Fy	Fu	GRADE	Fy	Fu
A529-50	50 ksi	65 ksi			

TOWER DESIGN NOTES

1. Tower is located in Litchfield County, Connecticut.
2. Tower designed for Exposure C to the TIA-222-H Standard.
3. Tower designed for a 115 mph basic wind in accordance with the TIA-222-H Standard.
4. Tower is also designed for a 40 mph basic wind with 1.00 in ice. Ice is considered to increase in thickness with height.
5. Deflections are based upon a 60 mph wind.
6. Tower Risk Category II.
7. Topographic Category 1 with Crest Height of 0.00 ft
8. Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications.
9. Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
10. Tower obstruction lighting is based on FAA Style E. CASE is not responsible for the determination of the FAA lighting requirements. If a different style is required, CASE should be notified and the tower design should be updated.
11. TOWER RATING: 97.8%



This design may not be used without the written consent of CASE, Inc.

CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job: 59335 Final Design Rev 0		
	Project: 185' SST CT1238 Norfolk South Site, CT		
	Client: Tarpon Towers	Drawn by: MJG	App'd:
	Code: TIA-222-H	Date: 02/20/26	Scale: NTS
	Path:	Dwg No. E-1	

Section	T1	T2	T3	T4	T5	T6	T7	T8	T9	T10
Legs		SR 2	SR 2 3/4	SR 3	SR 3 1/4	SR 3 1/4	SR 3 1/2	SR 3 1/2	SR 3 3/4	SR 3 3/4
Leg Grade						A529-50				
Diagonals						L2x2x3/16				
Diagonal Grade						A529-50				
Top Chords						N.A.				
Horizontals						N.A.				
Inner Bracing						N.A.				
Face Width (ft)	18.5					14.5				
# Panels @ (ft)						16.5				
Weight (lb)	21657.4					3673.2				
						3045.8				
						2868.3				
						2700.0				
						2327.7				
						2086.4				
						1942.6				
						1767.1				
						1080.9				
						4.5				
						1 @ 5				
						249.5				
						20 @ 4				
						6.5				
						8.5				
						10.5				
						12.5				
						20 @ 5				
						14.5				
						16.5				
						3045.8				
						2868.3				
						2700.0				
						2327.7				
						2086.4				
						1942.6				
						1767.1				
						1080.9				
						4.5				
						1 @ 5				
						249.5				

Feed Line Plan 20'

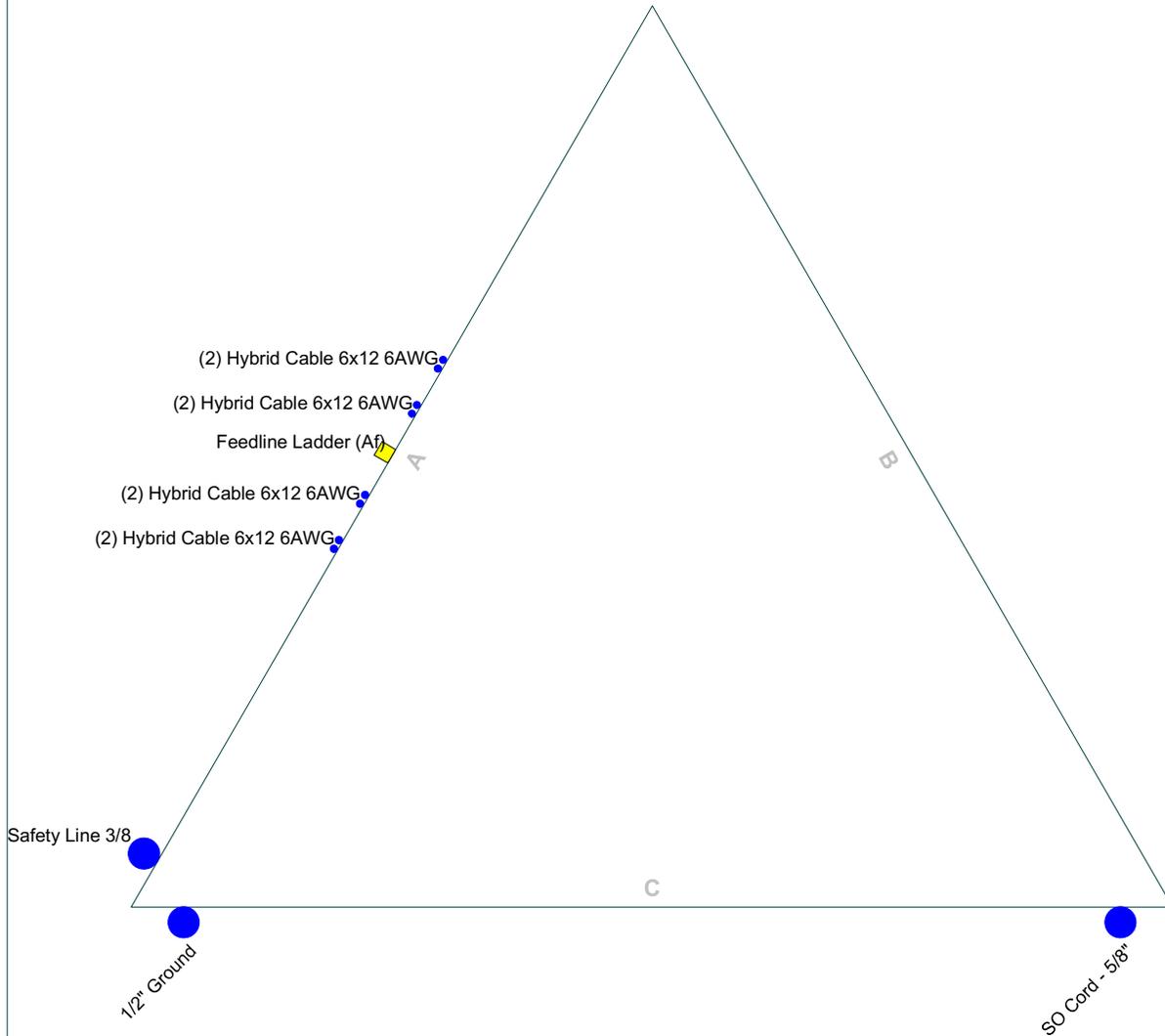
Round

Flat

App In Face

App Out Face

Section @ 20'



CASE International LLC
P.O. Box 4825
Lafayette, LA 70502
Phone: 337-232-3336
FAX: ashpley@casengr.com

Job: **59335 Final Design Rev 0**

Project: **185' SST CT1238 Norfolk South Site, CT**

Client: Tarpon Towers

Drawn by: AS

App'd:

Code: TIA-222-H

Date: 02/20/26

Scale: NTS

Path:

Dwg No. E-7

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job 59335 Final Design Rev 0	Page 1 of 24
	Project 185' SST CT1238 Norfolk South Site, CT	Date 14:41:36 02/20/26
	Client Tarpon Towers	Designed by MJG

Tower Input Data

The main tower is a 3x free standing tower with an overall height of 185.00 ft above the ground line.

The base of the tower is set at an elevation of 0.00 ft above the ground line.

The face width of the tower is 4.50 ft at the top and 18.50 ft at the base.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower is located in Litchfield County, Connecticut.

Tower base elevation above sea level: 1499.00 ft.

Basic wind speed of 115 mph.

Risk Category II.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 40 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards..

Tower obstruction lighting is based on FAA Style E. CASE is not responsible for the determination of the FAA lighting requirements. If a different style is required, CASE should be notified and the tower design should be updated..

Non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

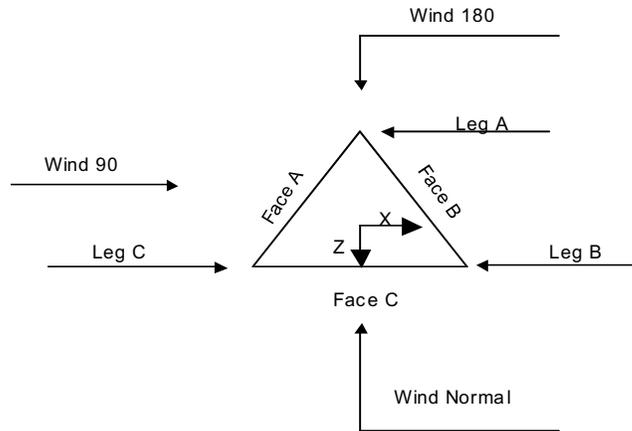
Stress ratio used in tower member design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Options

Consider Moments - Legs	Assume Legs Pinned	√ Calculate Redundant Bracing Forces
Consider Moments - Horizontals	Assume Rigid Index Plate	Ignore Redundant Members in FEA
Consider Moments - Diagonals	√ Use Clear Spans For Wind Area	√ SR Leg Bolts Resist Compression
Use Moment Magnification	√ Use Clear Spans For KL/r	√ All Leg Panels Have Same Allowable
√ Use Code Stress Ratios	√ Retension Guys To Initial Tension	Offset Girt At Foundation
√ Use Code Safety Factors - Guys	Bypass Mast Stability Checks	√ Consider Feed Line Torque
Escalate Ice	√ Use Azimuth Dish Coefficients	√ Include Angle Block Shear Check
Always Use Max Kz	√ Project Wind Area of Appurtenances	Use TIA-222-H Bracing Resist. Exemption
Kz In Exposure D Hurricane Region	Alternative Appurt. EPA Calculation	Use TIA-222-H Tension Splice Exemption
√ Include Bolts In Member Capacity	√ Autocalc Torque Arm Areas	Poles
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Include Shear-Torsion Interaction
√ Secondary Horizontal Braces Leg	Sort Capacity Reports By Component	Always Use Sub-Critical Flow
√ Use Diamond Inner Bracing (4 Sided)	√ Triangulate Diamond Inner Bracing	Use Top Mounted Sockets
√ SR Members Have Cut Ends	√ Treat Feed Line Bundles As Cylinder	Pole Without Linear Attachments
SR Members Are Concentric	Ignore KL/ry For 60 Deg. Angle Legs	Pole With Shroud Or No Appurtenances
Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules	Outside and Inside Corner Radii Are Known
Use Special Wind Profile		

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job 59335 Final Design Rev 0	Page 2 of 24
	Project 185' SST CT1238 Norfolk South Site, CT	Date 14:41:36 02/20/26
	Client Tarpon Towers	Designed by MJG



Triangular Tower

Tower Section Geometry

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	<i>ft</i>			<i>ft</i>		<i>ft</i>
T1	185.00-180.00			4.50	1	5.00
T2	180.00-160.00			4.50	1	20.00
T3	160.00-140.00			4.50	1	20.00
T4	140.00-120.00			4.50	1	20.00
T5	120.00-100.00			6.50	1	20.00
T6	100.00-80.00			8.50	1	20.00
T7	80.00-60.00			10.50	1	20.00
T8	60.00-40.00			12.50	1	20.00
T9	40.00-20.00			14.50	1	20.00
T10	20.00-0.00			16.50	1	20.00

Tower Section Geometry (cont'd)

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	<i>ft</i>	<i>ft</i>				<i>in</i>	<i>in</i>
T1	185.00-180.00	5.00	X Brace	No	No	0.0000	0.0000
T2	180.00-160.00	4.00	X Brace	No	No	0.0000	0.0000
T3	160.00-140.00	4.00	X Brace	No	No	0.0000	0.0000

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	3 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T4	140.00-120.00	4.00	X Brace	No	No	0.0000	0.0000
T5	120.00-100.00	4.00	X Brace	No	No	0.0000	0.0000
T6	100.00-80.00	5.00	X Brace	No	No	0.0000	0.0000
T7	80.00-60.00	5.00	X Brace	No	No	0.0000	0.0000
T8	60.00-40.00	5.00	X Brace	No	No	0.0000	0.0000
T9	40.00-20.00	5.00	Double K	No	Yes	0.0000	0.0000
T10	20.00-0.00	5.00	Double K	No	Yes	0.0000	0.0000

Tower Section Geometry (cont'd)

Tower Elevation	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
ft						
T1 185.00-180.00	Solid Round	1 3/4	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T2 180.00-160.00	Solid Round	2	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T3 160.00-140.00	Solid Round	2 3/4	A529-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A529-50 (50 ksi)
T4 140.00-120.00	Solid Round	3	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T5 120.00-100.00	Solid Round	3	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T6 100.00-80.00	Solid Round	3 1/4	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T7 80.00-60.00	Solid Round	3 1/2	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T8 60.00-40.00	Solid Round	3 1/2	A529-50 (50 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T9 40.00-20.00	Solid Round	3 1/2	A529-50 (50 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A529-50 (50 ksi)
T10 20.00-0.00	Solid Round	3 3/4	A529-50 (50 ksi)	Equal Angle	L3x3x3/16	A529-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
ft						
T1 185.00-180.00	Equal Angle	L1 3/4x1 3/4x3/16	A529-50 (50 ksi)	Solid Round		A529-50 (50 ksi)

Tower Section Geometry (cont'd)

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	4 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T9 40.00-20.00	None	Solid Round		A36 (36 ksi)	Equal Angle	L2 1/2x2 1/2x3/16	A529-50 (50 ksi)
T10 20.00-0.00	None	Solid Round		A36 (36 ksi)	Equal Angle	L3x3x3/16	A529-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Secondary Horizontal Type	Secondary Horizontal Size	Secondary Horizontal Grade	Inner Bracing Type	Inner Bracing Size	Inner Bracing Grade
T9 40.00-20.00	Solid Round		A36 (36 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)
T10 20.00-0.00	Solid Round		A36 (36 ksi)	Equal Angle	L2x2x3/16	A529-50 (50 ksi)

Tower Section Geometry (cont'd)

Tower Elevation ft	Gusset Area (per face) ft ²	Gusset Thickness in	Gusset Grade	Adjust. Factor A _f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
T1 185.00-180.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T2 180.00-160.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T3 160.00-140.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T4 140.00-120.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T5 120.00-100.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T6 100.00-80.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T7 80.00-60.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T8 60.00-40.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T9 40.00-20.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T10 20.00-0.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000

Tower Section Geometry (cont'd)

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	6 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 185.00-180.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)
T2 180.00-160.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)
T3 160.00-140.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)
T4 140.00-120.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)
T5 120.00-100.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)
T6 100.00-80.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)
T7 80.00-60.00	0.0000	0.75 (1)	0.0000	0.75 (1)	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75 (1)	0.0000	0.75 (1)
	0.0000	0.75 (2)	0.0000	0.75 (2)							0.0000	0.75 (2)	0.0000	0.75 (2)
	0.0000	0.75 (3)	0.0000	0.75 (3)							0.0000	0.75 (3)	0.0000	0.75 (3)
	0.0000	0.75 (4)	0.0000	0.75 (4)							0.0000	0.75 (4)	0.0000	0.75 (4)

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	8 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Elevation ft	Leg Connection Type	Leg Bolt Size in	Leg No.	Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
				Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.						
T7 80.00-60.00	Flange	1.0000	6	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.5000	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T8 60.00-40.00	Flange	1.0000	6	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.5000	0	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9 40.00-20.00	Flange	1.0000	6	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.5000	1	0.6250	0
		A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T10 20.00-0.00	Flange	1.2500	6	0.5000	1	0.6250	0	0.6250	0	0.6250	0	0.5000	1	0.6250	0
		F1554-105		A325N		A325N		A325N		A325N		A325N		A325N	

Tower Section Geometry (cont'd)

Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T1 185.00-180.00	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
	0.6250	0 (2)	0.6250	0 (2)							0.6250	0 (2)	0.6250	0 (2)
	A325N		A325N								A325N		A325N	
	0.6250	0 (3)	0.6250	0 (3)							0.6250	0 (3)	0.6250	0 (3)
T2 180.00-160.00	0.6250	0 (4)	0.6250	0 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N							A325N		A325N		
	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
	0.6250	0 (2)	0.6250	0 (2)							0.6250	0 (2)	0.6250	0 (2)
T3 160.00-140.00	0.6250	0 (3)	0.6250	0 (3)							0.6250	0 (3)	0.6250	0 (3)
	A325N		A325N							A325N		A325N		
	0.6250	0 (4)	0.6250	0 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N								A325N		A325N	
	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
T4 140.00-120.00	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
	0.6250	0 (2)	0.6250	0 (2)							0.6250	0 (2)	0.6250	0 (2)
	A325N		A325N								A325N		A325N	
	0.6250	0 (3)	0.6250	0 (3)							0.6250	0 (3)	0.6250	0 (3)
	A325N		A325N								A325N		A325N	
T5 120.00-100.00	0.6250	0 (4)	0.6250	0 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N								A325N		A325N	
	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
	0.6250	0 (2)	0.6250	0 (2)							0.6250	0 (2)	0.6250	0 (2)
T5 120.00-100.00	A325N		A325N								A325N		A325N	
	0.6250	0 (3)	0.6250	0 (3)							0.6250	0 (3)	0.6250	0 (3)
	A325N		A325N								A325N		A325N	

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job 59335 Final Design Rev 0	Page 9 of 24
	Project 185' SST CT1238 Norfolk South Site, CT	Date 14:41:36 02/20/26
	Client Tarpon Towers	Designed by MJG

Tower Elevation ft	Redundant Horizontal		Redundant Diagonal		Redundant Sub-Diagonal		Redundant Sub-Horizontal		Redundant Vertical		Redundant Hip		Redundant Hip Diagonal	
	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.	Bolt Size in	No.
T6 100.00-80.00	0.6250	0 (4)	0.6250	0 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N								A325N		A325N	
	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T7 80.00-60.00	0.6250	0 (2)	0.6250	0 (2)							0.6250	0 (2)	0.6250	0 (2)
	A325N		A325N								A325N		A325N	
	0.6250	0 (3)	0.6250	0 (3)							0.6250	0 (3)	0.6250	0 (3)
	A325N		A325N								A325N		A325N	
T8 60.00-40.00	0.6250	0 (4)	0.6250	0 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N								A325N		A325N	
	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
T9 40.00-20.00	0.6250	0 (2)	0.6250	0 (2)							0.6250	0 (2)	0.6250	0 (2)
	A325N		A325N								A325N		A325N	
	0.6250	0 (3)	0.6250	0 (3)							0.6250	0 (3)	0.6250	0 (3)
	A325N		A325N								A325N		A325N	
T10 20.00-0.00	0.6250	0 (4)	0.6250	0 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N								A325N		A325N	
	0.6250	0 (1)	0.6250	0 (1)	0.6250	0	0.6250	0	0.6250	0	0.6250	0 (1)	0.6250	0 (1)
	A325N		A325N		A325N		A325N		A325N		A325N		A325N	
	0.6250	1 (2)	0.6250	1 (2)							0.6250	0 (2)	0.6250	0 (2)
	A325N		A325N								A325N		A325N	
	0.6250	1 (3)	0.6250	1 (3)							0.6250	0 (3)	0.6250	0 (3)
	A325N		A325N								A325N		A325N	
	0.6250	1 (4)	0.6250	1 (4)							0.6250	0 (4)	0.6250	0 (4)
	A325N		A325N								A325N		A325N	

Feed Line/Linear Appurtenances - Entered As Round Or Flat

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	11 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Section	Tower Elevation ft	Face	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight lb
T6	100.00-80.00	C	0.000	0.000	2.250	0.000	15.20
		A	0.000	0.000	33.470	0.000	444.40
		B	0.000	0.000	0.000	0.000	0.00
T7	80.00-60.00	C	0.000	0.000	2.250	0.000	15.20
		A	0.000	0.000	33.470	0.000	444.40
		B	0.000	0.000	0.000	0.000	0.00
T8	60.00-40.00	C	0.000	0.000	2.250	0.000	15.20
		A	0.000	0.000	33.470	0.000	444.40
		B	0.000	0.000	0.000	0.000	0.00
T9	40.00-20.00	C	0.000	0.000	2.250	0.000	15.20
		A	0.000	0.000	33.470	0.000	444.40
		B	0.000	0.000	0.000	0.000	0.00
T10	20.00-0.00	C	0.000	0.000	2.250	0.000	15.20
		A	0.000	0.000	16.735	0.000	222.20
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	1.625	0.000	12.60

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft ²	A_F ft ²	C_{AA} In Face ft ²	C_{AA} Out Face ft ²	Weight lb
T1	185.00-180.00	A	1.187	0.000	0.000	5.061	0.000	87.44
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	2.936	0.000	31.01
T2	180.00-160.00	A	1.178	0.000	0.000	45.804	0.000	628.50
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	11.675	0.000	122.57
T3	160.00-140.00	A	1.163	0.000	0.000	79.503	0.000	993.30
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	11.558	0.000	120.02
T4	140.00-120.00	A	1.147	0.000	0.000	87.404	0.000	1074.82
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	11.426	0.000	117.14
T5	120.00-100.00	A	1.128	0.000	0.000	86.726	0.000	1062.24
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	11.273	0.000	113.83
T6	100.00-80.00	A	1.106	0.000	0.000	85.926	0.000	1047.52
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	11.094	0.000	109.93
T7	80.00-60.00	A	1.078	0.000	0.000	84.947	0.000	1029.68
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	10.875	0.000	105.15
T8	60.00-40.00	A	1.042	0.000	0.000	83.675	0.000	1006.76
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	10.589	0.000	98.94
T9	40.00-20.00	A	0.991	0.000	0.000	81.825	0.000	973.98
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	10.174	0.000	90.60
T10	20.00-0.00	A	0.887	0.000	0.000	39.079	0.000	455.48
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	6.950	0.000	60.68

Feed Line Center of Pressure

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	12 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Section	Elevation	CP _x	CP _z	CP _x	CP _z
	ft	in	in	Ice in	Ice in
T1	185.00-180.00	-1.6592	-0.2830	-2.5779	1.1411
T2	180.00-160.00	-2.8467	-1.3081	-3.9804	-0.2538
T3	160.00-140.00	-3.5432	-2.0414	-4.9223	-1.5212
T4	140.00-120.00	-4.6142	-2.4803	-6.2311	-1.8709
T5	120.00-100.00	-5.3881	-2.9321	-7.4426	-2.2525
T6	100.00-80.00	-6.4341	-3.5008	-8.8906	-2.7072
T7	80.00-60.00	-6.9324	-3.7992	-9.7012	-2.9995
T8	60.00-40.00	-7.4068	-4.0894	-10.3884	-3.2769
T9	40.00-20.00	-7.7776	-4.3232	-11.2195	-3.6468
T10	20.00-0.00	-4.0643	-1.9948	-6.7232	-1.4696

Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T1	2	Safety Line 3/8	180.00 - 185.00	0.6000	0.6000
T1	3	SO Cord - 5/8"	180.00 - 185.00	0.6000	0.6000
T1	4	1/2" Ground	180.00 - 185.00	0.6000	0.6000
T1	5	Feedline Ladder (Af)	180.00 - 185.00	0.6000	0.6000
T2	2	Safety Line 3/8	160.00 - 180.00	0.6000	0.6000
T2	3	SO Cord - 5/8"	160.00 - 180.00	0.6000	0.6000
T2	4	1/2" Ground	160.00 - 180.00	0.6000	0.6000
T2	5	Feedline Ladder (Af)	160.00 - 180.00	0.6000	0.6000
T2	9	Hybrid Cable 6x12 6AWG	160.00 - 180.00	0.6000	0.6000
T2	10	Hybrid Cable 6x12 6AWG	160.00 - 170.00	0.6000	0.6000
T3	2	Safety Line 3/8	140.00 - 160.00	0.6000	0.5947
T3	3	SO Cord - 5/8"	140.00 - 160.00	0.6000	0.5947
T3	4	1/2" Ground	140.00 - 160.00	0.6000	0.5947
T3	5	Feedline Ladder (Af)	140.00 - 160.00	0.6000	0.5947
T3	9	Hybrid Cable 6x12 6AWG	140.00 - 160.00	0.6000	0.5947
T3	10	Hybrid Cable 6x12 6AWG	140.00 - 160.00	0.6000	0.5947
T3	11	Hybrid Cable 6x12 6AWG	140.00 - 160.00	0.6000	0.5947
T3	12	Hybrid Cable 6x12 6AWG	140.00 - 150.00	0.6000	0.5947
T4	2	Safety Line 3/8	120.00 - 140.00	0.6000	0.6000
T4	3	SO Cord - 5/8"	120.00 -	0.6000	0.6000

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	13 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K_a No Ice	K_a Ice
			140.00		
T4	4	1/2" Ground	120.00 -	0.6000	0.6000
			140.00		
T4	5	Feedline Ladder (Af)	120.00 -	0.6000	0.6000
			140.00		
T4	9	Hybrid Cable 6x12 6AWG	120.00 -	0.6000	0.6000
			140.00		
T4	10	Hybrid Cable 6x12 6AWG	120.00 -	0.6000	0.6000
			140.00		
T4	11	Hybrid Cable 6x12 6AWG	120.00 -	0.6000	0.6000
			140.00		
T4	12	Hybrid Cable 6x12 6AWG	120.00 -	0.6000	0.6000
			140.00		
T5	2	Safety Line 3/8	100.00 -	0.6000	0.6000
			120.00		
T5	3	SO Cord - 5/8"	100.00 -	0.6000	0.6000
			120.00		
T5	4	1/2" Ground	100.00 -	0.6000	0.6000
			120.00		
T5	5	Feedline Ladder (Af)	100.00 -	0.6000	0.6000
			120.00		
T5	9	Hybrid Cable 6x12 6AWG	100.00 -	0.6000	0.6000
			120.00		
T5	10	Hybrid Cable 6x12 6AWG	100.00 -	0.6000	0.6000
			120.00		
T5	11	Hybrid Cable 6x12 6AWG	100.00 -	0.6000	0.6000
			120.00		
T5	12	Hybrid Cable 6x12 6AWG	100.00 -	0.6000	0.6000
			120.00		
T6	2	Safety Line 3/8	80.00 - 100.00	0.6000	0.6000
T6	3	SO Cord - 5/8"	80.00 - 100.00	0.6000	0.6000
T6	4	1/2" Ground	80.00 - 100.00	0.6000	0.6000
T6	5	Feedline Ladder (Af)	80.00 - 100.00	0.6000	0.6000
T6	9	Hybrid Cable 6x12 6AWG	80.00 - 100.00	0.6000	0.6000
T6	10	Hybrid Cable 6x12 6AWG	80.00 - 100.00	0.6000	0.6000
T6	11	Hybrid Cable 6x12 6AWG	80.00 - 100.00	0.6000	0.6000
T6	12	Hybrid Cable 6x12 6AWG	80.00 - 100.00	0.6000	0.6000
T7	2	Safety Line 3/8	60.00 - 80.00	0.6000	0.6000
T7	3	SO Cord - 5/8"	60.00 - 80.00	0.6000	0.6000
T7	4	1/2" Ground	60.00 - 80.00	0.6000	0.6000
T7	5	Feedline Ladder (Af)	60.00 - 80.00	0.6000	0.6000
T7	9	Hybrid Cable 6x12 6AWG	60.00 - 80.00	0.6000	0.6000
T7	10	Hybrid Cable 6x12 6AWG	60.00 - 80.00	0.6000	0.6000
T7	11	Hybrid Cable 6x12 6AWG	60.00 - 80.00	0.6000	0.6000
T7	12	Hybrid Cable 6x12 6AWG	60.00 - 80.00	0.6000	0.6000
T8	2	Safety Line 3/8	40.00 - 60.00	0.6000	0.6000
T8	3	SO Cord - 5/8"	40.00 - 60.00	0.6000	0.6000
T8	4	1/2" Ground	40.00 - 60.00	0.6000	0.6000
T8	5	Feedline Ladder (Af)	40.00 - 60.00	0.6000	0.6000
T8	9	Hybrid Cable 6x12 6AWG	40.00 - 60.00	0.6000	0.6000
T8	10	Hybrid Cable 6x12 6AWG	40.00 - 60.00	0.6000	0.6000
T8	11	Hybrid Cable 6x12 6AWG	40.00 - 60.00	0.6000	0.6000
T8	12	Hybrid Cable 6x12 6AWG	40.00 - 60.00	0.6000	0.6000
T9	2	Safety Line 3/8	20.00 - 40.00	0.6000	0.6000
T9	3	SO Cord - 5/8"	20.00 - 40.00	0.6000	0.6000
T9	4	1/2" Ground	20.00 - 40.00	0.6000	0.6000
T9	5	Feedline Ladder (Af)	20.00 - 40.00	0.6000	0.6000
T9	9	Hybrid Cable 6x12 6AWG	20.00 - 40.00	0.6000	0.6000
T9	10	Hybrid Cable 6x12 6AWG	20.00 - 40.00	0.6000	0.6000
T9	11	Hybrid Cable 6x12 6AWG	20.00 - 40.00	0.6000	0.6000
T9	12	Hybrid Cable 6x12 6AWG	20.00 - 40.00	0.6000	0.6000
T10	2	Safety Line 3/8	10.00 - 20.00	0.6000	0.6000

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	14 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	K _a No Ice	K _a Ice
T10	3	SO Cord - 5/8"	10.00 - 20.00	0.6000	0.6000
T10	4	1/2" Ground	0.00 - 20.00	0.6000	0.6000
T10	5	Feedline Ladder (Af)	10.00 - 20.00	0.6000	0.6000
T10	9	Hybrid Cable 6x12 6AWG	10.00 - 20.00	0.6000	0.6000
T10	10	Hybrid Cable 6x12 6AWG	10.00 - 20.00	0.6000	0.6000
T10	11	Hybrid Cable 6x12 6AWG	10.00 - 20.00	0.6000	0.6000
T10	12	Hybrid Cable 6x12 6AWG	10.00 - 20.00	0.6000	0.6000

Discrete Tower Loads

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			ft ft ft	°	ft	ft ²	ft ²	lb
Lightning Rod	A	From Leg	0.00 0.00 5.00	0.0000	185.00	No Ice 1.00 1/2" Ice 1.30 1" Ice 1.60	1.00 1.30 1.60	10.00 13.00 16.00
L-864 Beacon	A	From Leg	0.00 0.00 0.00	0.0000	185.00	No Ice 2.00 1/2" Ice 2.60 1" Ice 3.20	2.00 2.60 3.20	28.00 36.00 44.00

42,000 sqin EPA(A) Carrier (4,000 lbs)	C	None		0.0000	180.00	No Ice 292.00 1/2" Ice 380.00 1" Ice 468.00	292.00 380.00 468.00	4000.00 5200.00 6400.00

(2) NN-65C-HG-R1B	A	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 17.07 1/2" Ice 17.70 1" Ice 18.33	8.20 8.79 9.40	78.00 172.23 274.65
(2) NN-65C-HG-R1B	B	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 17.07 1/2" Ice 17.70 1" Ice 18.33	8.20 8.79 9.40	78.00 172.23 274.65
(2) NN-65C-HG-R1B	C	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 17.07 1/2" Ice 17.70 1" Ice 18.33	8.20 8.79 9.40	78.00 172.23 274.65
MT6413-77A	A	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 3.79 1/2" Ice 4.04 1" Ice 4.30	1.46 1.65 1.85	57.32 81.70 109.53
MT6413-77A	B	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 3.79 1/2" Ice 4.04 1" Ice 4.30	1.46 1.65 1.85	57.32 81.70 109.53
MT6413-77A	C	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 3.79 1/2" Ice 4.04 1" Ice 4.30	1.46 1.65 1.85	57.32 81.70 109.53
RTH 0306-PFC	B	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 1.00 1/2" Ice 1.13 1" Ice 1.26	0.56 0.67 0.78	15.87 24.57 35.24
RTH 0306-PFC	C	From Leg	3.00 0.00 0.00	0.0000	170.00	No Ice 1.00 1/2" Ice 1.13 1" Ice 1.26	0.56 0.67 0.78	15.87 24.57 35.24
RF4461D-13A	A	From Leg	3.00 0.00	0.0000	170.00	No Ice 3.87 1/2" Ice 4.12	1.58 1.77	58.00 83.47

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	15 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight	
			Horz	Vert						
			ft	ft	°	ft	ft ²	ft ²	lb	
RF4461D-13A	B	From Leg	0.00	3.00	0.0000	170.00	1" Ice	4.38	1.97	112.44
			0.00	3.00	0.0000	170.00	No Ice	3.87	1.58	58.00
			0.00	0.00			1/2" Ice	4.12	1.77	83.47
RF4461D-13A	C	From Leg	0.00	3.00	0.0000	170.00	1" Ice	4.38	1.97	112.44
			0.00	3.00	0.0000	170.00	No Ice	3.87	1.58	58.00
			0.00	0.00			1/2" Ice	4.12	1.77	83.47
RF4801D-25A	A	From Leg	0.00	3.00	0.0000	170.00	1" Ice	4.38	1.97	112.44
			0.00	3.00	0.0000	170.00	No Ice	3.87	1.58	58.00
			0.00	0.00			1/2" Ice	4.12	1.77	83.47
RF4801D-25A	B	From Leg	0.00	3.00	0.0000	170.00	1" Ice	4.38	1.97	112.44
			0.00	3.00	0.0000	170.00	No Ice	3.87	1.58	58.00
			0.00	0.00			1/2" Ice	4.12	1.77	83.47
RF4801D-25A	C	From Leg	0.00	3.00	0.0000	170.00	1" Ice	4.38	1.97	112.44
			0.00	3.00	0.0000	170.00	No Ice	3.87	1.58	58.00
			0.00	0.00			1/2" Ice	4.12	1.77	83.47
Sector Mount - EPA 13sqft, 800 lbs	A	From Leg	0.00	1.50	0.0000	170.00	1" Ice	4.38	1.97	112.44
			0.00	1.50	0.0000	170.00	No Ice	13.00	10.00	800.00
			0.00	0.00			1/2" Ice	16.90	13.00	1040.00
Sector Mount - EPA 13sqft, 800 lbs	B	From Leg	0.00	1.50	0.0000	170.00	1" Ice	20.80	16.00	1280.00
			0.00	1.50	0.0000	170.00	No Ice	13.00	10.00	800.00
			0.00	0.00			1/2" Ice	16.90	13.00	1040.00
Sector Mount - EPA 13sqft, 800 lbs	C	From Leg	0.00	1.50	0.0000	170.00	1" Ice	20.80	16.00	1280.00
			0.00	1.50	0.0000	170.00	No Ice	13.00	10.00	800.00
			0.00	0.00			1/2" Ice	16.90	13.00	1040.00

(2) NN-65C-HG-R1B	A	From Leg	0.00	3.00	0.0000	160.00	No Ice	17.07	8.20	78.00
			0.00	3.00	0.0000	160.00	1/2" Ice	17.70	8.79	172.23
			0.00	0.00			1" Ice	18.33	9.40	274.65
(2) NN-65C-HG-R1B	B	From Leg	0.00	3.00	0.0000	160.00	No Ice	17.07	8.20	78.00
			0.00	3.00	0.0000	160.00	1/2" Ice	17.70	8.79	172.23
			0.00	0.00			1" Ice	18.33	9.40	274.65
(2) NN-65C-HG-R1B	C	From Leg	0.00	3.00	0.0000	160.00	No Ice	17.07	8.20	78.00
			0.00	3.00	0.0000	160.00	1/2" Ice	17.70	8.79	172.23
			0.00	0.00			1" Ice	18.33	9.40	274.65
MT6413-77A	A	From Leg	0.00	3.00	0.0000	160.00	No Ice	3.79	1.46	57.32
			0.00	3.00	0.0000	160.00	1/2" Ice	4.04	1.65	81.70
			0.00	0.00			1" Ice	4.30	1.85	109.53
MT6413-77A	B	From Leg	0.00	3.00	0.0000	160.00	No Ice	3.79	1.46	57.32
			0.00	3.00	0.0000	160.00	1/2" Ice	4.04	1.65	81.70
			0.00	0.00			1" Ice	4.30	1.85	109.53
MT6413-77A	C	From Leg	0.00	3.00	0.0000	160.00	No Ice	3.79	1.46	57.32
			0.00	3.00	0.0000	160.00	1/2" Ice	4.04	1.65	81.70
			0.00	0.00			1" Ice	4.30	1.85	109.53
RTH 0306-PFC	B	From Leg	0.00	3.00	0.0000	160.00	No Ice	1.00	0.56	15.87
			0.00	3.00	0.0000	160.00	1/2" Ice	1.13	0.67	24.57
			0.00	0.00			1" Ice	1.26	0.78	35.24
RTH 0306-PFC	C	From Leg	0.00	3.00	0.0000	160.00	No Ice	1.00	0.56	15.87
			0.00	3.00	0.0000	160.00	1/2" Ice	1.13	0.67	24.57
			0.00	0.00			1" Ice	1.26	0.78	35.24
RF4461D-13A	A	From Leg	0.00	3.00	0.0000	160.00	No Ice	3.87	1.58	58.00
			0.00	3.00	0.0000	160.00	1/2" Ice	4.12	1.77	83.47
			0.00	0.00			1" Ice	4.38	1.97	112.44
RF4461D-13A	B	From Leg	0.00	3.00	0.0000	160.00	No Ice	3.87	1.58	58.00
			0.00	3.00	0.0000	160.00	1/2" Ice	4.12	1.77	83.47
			0.00	0.00			1" Ice	4.38	1.97	112.44
RF4461D-13A	C	From Leg	0.00	3.00	0.0000	160.00	No Ice	3.87	1.58	58.00

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	16 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft ²	ft ²	lb
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4801D-25A	A	From Leg	3.00	0.0000	160.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4801D-25A	B	From Leg	3.00	0.0000	160.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4801D-25A	C	From Leg	3.00	0.0000	160.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
Sector Mount - EPA 13sqft, 800 lbs	A	From Leg	1.50	0.0000	160.00	No Ice	13.00	10.00	800.00
			0.00			1/2" Ice	16.90	13.00	1040.00
			0.00			1" Ice	20.80	16.00	1280.00
Sector Mount - EPA 13sqft, 800 lbs	B	From Leg	1.50	0.0000	160.00	No Ice	13.00	10.00	800.00
			0.00			1/2" Ice	16.90	13.00	1040.00
			0.00			1" Ice	20.80	16.00	1280.00
Sector Mount - EPA 13sqft, 800 lbs	C	From Leg	1.50	0.0000	160.00	No Ice	13.00	10.00	800.00
			0.00			1/2" Ice	16.90	13.00	1040.00
			0.00			1" Ice	20.80	16.00	1280.00

(2) NN-65C-HG-R1B	A	From Leg	3.00	0.0000	150.00	No Ice	17.07	8.20	78.00
			0.00			1/2" Ice	17.70	8.79	172.23
			0.00			1" Ice	18.33	9.40	274.65
(2) NN-65C-HG-R1B	B	From Leg	3.00	0.0000	150.00	No Ice	17.07	8.20	78.00
			0.00			1/2" Ice	17.70	8.79	172.23
			0.00			1" Ice	18.33	9.40	274.65
(2) NN-65C-HG-R1B	C	From Leg	3.00	0.0000	150.00	No Ice	17.07	8.20	78.00
			0.00			1/2" Ice	17.70	8.79	172.23
			0.00			1" Ice	18.33	9.40	274.65
MT6413-77A	A	From Leg	3.00	0.0000	150.00	No Ice	3.79	1.46	57.32
			0.00			1/2" Ice	4.04	1.65	81.70
			0.00			1" Ice	4.30	1.85	109.53
MT6413-77A	B	From Leg	3.00	0.0000	150.00	No Ice	3.79	1.46	57.32
			0.00			1/2" Ice	4.04	1.65	81.70
			0.00			1" Ice	4.30	1.85	109.53
MT6413-77A	C	From Leg	3.00	0.0000	150.00	No Ice	3.79	1.46	57.32
			0.00			1/2" Ice	4.04	1.65	81.70
			0.00			1" Ice	4.30	1.85	109.53
RTH 0306-PFC	B	From Leg	3.00	0.0000	150.00	No Ice	1.00	0.56	15.87
			0.00			1/2" Ice	1.13	0.67	24.57
			0.00			1" Ice	1.26	0.78	35.24
RTH 0306-PFC	C	From Leg	3.00	0.0000	150.00	No Ice	1.00	0.56	15.87
			0.00			1/2" Ice	1.13	0.67	24.57
			0.00			1" Ice	1.26	0.78	35.24
RF4461D-13A	A	From Leg	3.00	0.0000	150.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4461D-13A	B	From Leg	3.00	0.0000	150.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4461D-13A	C	From Leg	3.00	0.0000	150.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4801D-25A	A	From Leg	3.00	0.0000	150.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	17 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C _{AA} Front	C _{AA} Side	Weight
			Horz	Vert					
			ft	ft	°	ft	ft ²	ft ²	lb
RF4801D-25A	B	From Leg	3.00	0.0000	150.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
RF4801D-25A	C	From Leg	3.00	0.0000	150.00	No Ice	3.87	1.58	58.00
			0.00			1/2" Ice	4.12	1.77	83.47
			0.00			1" Ice	4.38	1.97	112.44
Sector Mount - EPA 13sqft, 800 lbs	A	From Leg	1.50	0.0000	150.00	No Ice	13.00	10.00	800.00
			0.00			1/2" Ice	16.90	13.00	1040.00
			0.00			1" Ice	20.80	16.00	1280.00
Sector Mount - EPA 13sqft, 800 lbs	B	From Leg	1.50	0.0000	150.00	No Ice	13.00	10.00	800.00
			0.00			1/2" Ice	16.90	13.00	1040.00
			0.00			1" Ice	20.80	16.00	1280.00
Sector Mount - EPA 13sqft, 800 lbs	C	From Leg	1.50	0.0000	150.00	No Ice	13.00	10.00	800.00
			0.00			1/2" Ice	16.90	13.00	1040.00
			0.00			1" Ice	20.80	16.00	1280.00

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Wind 180 deg - No Ice
15	0.9 Dead+1.0 Wind 180 deg - No Ice
16	1.2 Dead+1.0 Wind 210 deg - No Ice
17	0.9 Dead+1.0 Wind 210 deg - No Ice
18	1.2 Dead+1.0 Wind 240 deg - No Ice
19	0.9 Dead+1.0 Wind 240 deg - No Ice
20	1.2 Dead+1.0 Wind 270 deg - No Ice
21	0.9 Dead+1.0 Wind 270 deg - No Ice
22	1.2 Dead+1.0 Wind 300 deg - No Ice
23	0.9 Dead+1.0 Wind 300 deg - No Ice
24	1.2 Dead+1.0 Wind 330 deg - No Ice
25	0.9 Dead+1.0 Wind 330 deg - No Ice
26	1.2 Dead+1.0 Ice+1.0 Temp
27	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
28	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
29	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
30	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
31	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	18 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

<i>Comb. No.</i>	<i>Description</i>
32	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
33	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
34	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp
35	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp
36	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp
37	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp
38	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp
39	Dead+Wind 0 deg - Service
40	Dead+Wind 30 deg - Service
41	Dead+Wind 60 deg - Service
42	Dead+Wind 90 deg - Service
43	Dead+Wind 120 deg - Service
44	Dead+Wind 150 deg - Service
45	Dead+Wind 180 deg - Service
46	Dead+Wind 210 deg - Service
47	Dead+Wind 240 deg - Service
48	Dead+Wind 270 deg - Service
49	Dead+Wind 300 deg - Service
50	Dead+Wind 330 deg - Service

Maximum Tower Deflections - Service Wind

<i>Section No.</i>	<i>Elevation ft</i>	<i>Horz. Deflection in</i>	<i>Gov. Load Comb.</i>	<i>Tilt °</i>	<i>Twist °</i>
T1	185 - 180	8.196	47	0.4534	0.1101
T2	180 - 160	7.722	47	0.4538	0.1100
T3	160 - 140	5.834	47	0.4204	0.1027
T4	140 - 120	4.158	47	0.3477	0.0773
T5	120 - 100	2.859	47	0.2636	0.0502
T6	100 - 80	1.888	47	0.1920	0.0342
T7	80 - 60	1.175	47	0.1390	0.0233
T8	60 - 40	0.654	43	0.0986	0.0145
T9	40 - 20	0.287	43	0.0619	0.0069
T10	20 - 0	0.078	43	0.0278	0.0028

Critical Deflections and Radius of Curvature - Service Wind

<i>Elevation ft</i>	<i>Appurtenance</i>	<i>Gov. Load Comb.</i>	<i>Deflection in</i>	<i>Tilt °</i>	<i>Twist °</i>	<i>Radius of Curvature ft</i>
185.00	Lightning Rod	47	8.196	0.4534	0.1101	87485
180.00	42,000 sqin EPA(A) Carrier (4,000 lbs)	47	7.722	0.4538	0.1100	87485
170.00	(2) NN-65C-HG-R1B	47	6.769	0.4438	0.1083	63386
160.00	(2) NN-65C-HG-R1B	47	5.834	0.4204	0.1027	21197
150.00	(2) NN-65C-HG-R1B	47	4.954	0.3872	0.0916	14526

Maximum Tower Deflections - Design Wind

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	19 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	185 - 180	30.139	10	1.6651	0.4050
T2	180 - 160	28.401	10	1.6666	0.4044
T3	160 - 140	21.466	10	1.5443	0.3777
T4	140 - 120	15.307	10	1.2777	0.2845
T5	120 - 100	10.534	10	0.9690	0.1848
T6	100 - 80	6.959	10	0.7060	0.1258
T7	80 - 60	4.335	10	0.5114	0.0858
T8	60 - 40	2.411	10	0.3626	0.0534
T9	40 - 20	1.058	11	0.2278	0.0253
T10	20 - 0	0.286	11	0.1021	0.0105

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
185.00	Lightning Rod	10	30.139	1.6651	0.4050	24034
180.00	42,000 sqin EPA(A) Carrier (4,000 lbs)	10	28.401	1.6666	0.4044	24034
170.00	(2) NN-65C-HG-R1B	10	24.900	1.6301	0.3982	17397
160.00	(2) NN-65C-HG-R1B	10	21.466	1.5443	0.3777	5788
150.00	(2) NN-65C-HG-R1B	10	18.235	1.4226	0.3370	3961

Bolt Design Data

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load per Bolt lb	Ratio Load Allowable	Allowable Ratio	Criteria
T1	185	Leg	A325N	0.7500	4	226.36	30101.40	0.008	✓	1 Bolt Tension
		Diagonal	A325N	0.5000	1	1574.22	8835.73	0.178	✓	1 Bolt Shear
T2	180	Leg	A325N	0.7500	4	11488.90	30101.40	0.382	✓	1 Bolt Tension
		Diagonal	A325N	0.5000	1	6555.28	8835.73	0.742	✓	1 Bolt Shear
T3	160	Leg	A325N	1.0000	4	34586.20	54517.00	0.634	✓	1 Bolt Tension
		Diagonal	A325N	0.6250	1	11399.90	11654.30	0.978	✓	1 Member Block Shear
T4	140	Leg	A325N	1.0000	4	45264.90	54517.00	0.830	✓	1 Bolt Tension
		Diagonal	A325N	0.5000	1	5499.60	8835.73	0.622	✓	1 Bolt Shear
T5	120	Leg	A325N	1.0000	4	50975.90	54517.00	0.935	✓	1 Bolt Tension
		Diagonal	A325N	0.5000	1	3976.86	8835.73	0.450	✓	1 Bolt Shear
T6	100	Leg	A325N	1.0000	6	36893.50	54517.00	0.677	✓	1 Bolt Tension
		Diagonal	A325N	0.5000	1	3745.69	8835.73	0.424	✓	1 Bolt Shear
T7	80	Leg	A325N	1.0000	6	39473.40	54517.00	0.724	✓	1 Bolt Tension
		Diagonal	A325N	0.5000	1	3664.61	8835.73	0.415	✓	1 Bolt Shear
T8	60	Leg	A325N	1.0000	6	41839.90	54517.00	0.767	✓	1 Bolt Tension

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job 59335 Final Design Rev 0	Page 20 of 24
	Project 185' SST CT1238 Norfolk South Site, CT	Date 14:41:36 02/20/26
	Client Tarpon Towers	Designed by MJG

Section No.	Elevation ft	Component Type	Bolt Grade	Bolt Size in	Number Of Bolts	Maximum Load per Bolt lb	Allowable Load per Bolt lb	Ratio Load Allowable	Allowable Ratio	Criteria
T9	40	Diagonal	A325N	0.5000	1	3806.58	8835.73	0.431 ✓	1	Bolt Shear
		Leg	A325N	1.0000	6	43739.10	54517.00	0.802 ✓	1	Bolt Tension
T10	20	Diagonal	A325N	0.5000	1	4551.34	8835.73	0.515 ✓	1	Bolt Shear
		Horizontal	A325N	0.5000	1	5614.31	6946.88	0.808 ✓	1	Member Bearing
		Leg	F1554-105	1.2500	6	45812.70	90854.00	0.504 ✓	1	Bolt Tension
		Diagonal	A325N	0.5000	1	4745.50	8835.73	0.537 ✓	1	Bolt Shear
		Horizontal	A325N	0.5000	1	5653.25	6946.88	0.814 ✓	1	Member Bearing

Compression Checks

Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	185 - 180	1 3/4	5.00	5.00	137.1 K=1.00	2.4053	-1064.38	28890.80	0.037 ¹ ✓
T2	180 - 160	2	20.00	4.00	96.0 K=1.00	3.1416	-52693.80	72063.20	0.731 ¹ ✓
T3	160 - 140	2 3/4	20.00	4.00	69.8 K=1.00	5.9396	-152343.00	187145.00	0.814 ¹ ✓
T4	140 - 120	3	20.03	4.01	64.1 K=1.00	7.0686	-198105.00	235529.00	0.841 ¹ ✓
T5	120 - 100	3	20.03	4.01	64.1 K=1.00	7.0686	-223791.00	235529.00	0.950 ¹ ✓
T6	100 - 80	3 1/4	20.03	5.01	74.0 K=1.00	8.2958	-244105.00	250223.00	0.976 ¹ ✓
T7	80 - 60	3 1/2	20.03	5.01	68.7 K=1.00	9.6211	-262867.00	306641.00	0.857 ¹ ✓
T8	60 - 40	3 1/2	20.03	5.01	68.7 K=1.00	9.6211	-280493.00	306641.00	0.915 ¹ ✓
T9	40 - 20	3 1/2	20.03	5.01	68.7 K=1.00	9.6211	-295245.00	306641.00	0.963 ¹ ✓
T10	20 - 0	3 3/4	20.03	5.01	64.1 K=1.00	11.0447	-311759.00	368015.00	0.847 ¹ ✓

¹ P_u / φP_n controls

Diagonal Design Data (Compression)

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	21 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	185 - 180	L2x2x3/16	6.73	3.11	101.0 K=1.07	0.7150	-1561.23	19820.60	0.079 ¹ ✓
T2	180 - 160	L2x2x3/16	6.02	2.75	92.9 K=1.11	0.7150	-6555.28	22278.90	0.294 ¹ ✓
T3	160 - 140	L2 1/2x2 1/2x3/16	6.02	2.71	79.3 K=1.21	0.9020	-11677.70	32691.40	0.357 ¹ ✓
T4	140 - 120	L2x2x3/16	6.17	2.92	96.8 K=1.09	0.7150	-5499.60	21123.80	0.260 ¹ ✓
T5	120 - 100	L2x2x3/16	9.21	4.43	135.0 K=1.00	0.7150	-3601.86	11223.40	0.321 ¹ ✓
T6	100 - 80	L2x2x3/16	11.41	5.55	168.9 K=1.00	0.7150	-3629.68	7174.38	0.506 ¹ ✓
T7	80 - 60	L2x2x3/16	13.23	6.45	196.4 K=1.00	0.7150	-3664.61	5306.81	0.691 ¹ ✓
T8	60 - 40	L2x2x3/16	15.10	7.38	224.9 K=1.00	0.7150	-3806.58	4046.99	0.941 ¹ ✓
T9	40 - 20	KL/R > 200 (C) - 206 L2 1/2x2 1/2x3/16	9.65	9.19	222.7 K=1.00	0.9020	-4551.34	5206.19	0.874 ¹ ✓
T10	20 - 0	KL/R > 200 (C) - 236 L3x3x3/16 KL/R > 200 (C) - 275	10.52	10.05	202.3 K=1.00	1.0900	-4745.50	7624.61	0.622 ¹ ✓

¹ P_u / φP_n controls

Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T9	40 - 20	L2 1/2x2 1/2x3/16	16.00	7.75	187.9 K=1.00	0.9020	-5614.31	7313.90	0.768 ¹ ✓
T10	20 - 0	L3x3x3/16	18.00	8.74	176.0 K=1.00	1.0900	-5653.25	10075.60	0.561 ¹ ✓

¹ P_u / φP_n controls

Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	185 - 180	L1 3/4x1 3/4x3/16	4.50	4.35	139.8 K=0.92	0.6211	-955.35	9100.74	0.105 ¹ ✓

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job 59335 Final Design Rev 0	Page 22 of 24
	Project 185' SST CT1238 Norfolk South Site, CT	Date 14:41:36 02/20/26
	Client Tarpon Towers	Designed by MJG

¹ $P_u / \phi P_n$ controls

Inner Bracing Design Data (Compression)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L_u</i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in²</i>	<i>P_u</i> <i>lb</i>	ϕP_n <i>lb</i>	Ratio $\frac{P_u}{\phi P_n}$
T9	40 - 20	L2x2x3/16	8.00	8.00	243.7 K=1.00	0.7150	-5.46	3447.10	0.002 ¹ ✓
T10	20 - 0	L2x2x3/16	9.00	9.00	274.1 K=1.00	0.7150	-6.27	2723.63	0.002 ¹ ✓
KL/R > 250 (C) - 279									

¹ $P_u / \phi P_n$ controls

Tension Checks

Leg Design Data (Tension)

Section No.	Elevation <i>ft</i>	Size	<i>L</i> <i>ft</i>	<i>L_u</i> <i>ft</i>	<i>Kl/r</i>	<i>A</i> <i>in²</i>	<i>P_u</i> <i>lb</i>	ϕP_n <i>lb</i>	Ratio $\frac{P_u}{\phi P_n}$
T1	185 - 180	1 3/4	5.00	5.00	137.1	2.4053	905.45	108238.00	0.008 ¹ ✓
T2	180 - 160	2	20.00	4.00	96.0	3.1416	45955.80	141372.00	0.325 ¹ ✓
T3	160 - 140	2 3/4	20.00	4.00	69.8	5.9396	138345.00	267281.00	0.518 ¹ ✓
T4	140 - 120	3	20.03	4.01	64.1	7.0686	181060.00	318086.00	0.569 ¹ ✓
T5	120 - 100	3	20.03	4.01	64.1	7.0686	203904.00	318086.00	0.641 ¹ ✓
T6	100 - 80	3 1/4	20.03	5.01	74.0	8.2958	221361.00	373310.00	0.593 ¹ ✓
T7	80 - 60	3 1/2	20.03	5.01	68.7	9.6211	236840.00	432951.00	0.547 ¹ ✓
T8	60 - 40	3 1/2	20.03	5.01	68.7	9.6211	251040.00	432951.00	0.580 ¹ ✓
T9	40 - 20	3 1/2	20.03	5.01	68.7	9.6211	262613.00	432951.00	0.607 ¹ ✓
T10	20 - 0	3 3/4	20.03	5.01	64.1	11.0447	275092.00	497010.00	0.553 ¹ ✓

¹ $P_u / \phi P_n$ controls

Diagonal Design Data (Tension)

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	23 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	185 - 180	L2x2x3/16	6.73	3.11	63.3	0.4484	1574.22	21857.50	0.072 ¹
T2	180 - 160	L2x2x3/16	6.02	2.75	56.4	0.4484	6513.62	21857.50	0.298 ¹
T3	160 - 140	L2 1/2x2 1/2x3/16	6.02	2.71	44.1	0.5710	11399.90	27837.80	0.410 ¹
T4	140 - 120	L2x2x3/16	6.17	2.92	59.7	0.4484	5009.55	21857.50	0.229 ¹
T5	120 - 100	L2x2x3/16	7.80	3.73	75.5	0.4484	3801.55	21857.50	0.174 ¹
T6	100 - 80	L2x2x3/16	10.08	4.89	97.9	0.4484	3539.04	21857.50	0.162 ¹
T7	80 - 60	L2x2x3/16	13.23	6.45	128.2	0.4484	3519.16	21857.50	0.161 ¹
T8	60 - 40	L2x2x3/16	15.10	7.38	146.4	0.4484	3693.82	21857.50	0.169 ¹
T9	40 - 20	L2 1/2x2 1/2x3/16	9.65	9.19	146.2	0.5886	4336.96	28694.70	0.151 ¹
T10	20 - 0	L3x3x3/16	10.08	9.61	126.5	0.7296	4398.13	35568.50	0.124 ¹

¹ P_u / φP_n controls

Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T9	40 - 20	L2 1/2x2 1/2x3/16	16.00	7.75	121.1	0.5886	5614.31	28694.70	0.196 ¹
T10	20 - 0	L3x3x3/16	18.00	8.74	113.0	0.7296	5653.25	35568.50	0.159 ¹

¹ P_u / φP_n controls

Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L _u ft	Kl/r	A in ²	P _u lb	φP _n lb	Ratio $\frac{P_u}{\phi P_n}$
T1	185 - 180	L1 3/4x1 3/4x3/16	4.50	4.35	97.3	0.4658	940.59	22708.70	0.041 ¹

¹ P_u / φP_n controls

tnxTower CASE International LLC P.O. Box 4825 Lafayette, Louisiana 70502 Phone: 337-232-3336 FAX: mgranberry@casengr.com	Job	59335 Final Design Rev 0	Page	24 of 24
	Project	185' SST CT1238 Norfolk South Site, CT	Date	14:41:36 02/20/26
	Client	Tarpon Towers	Designed by	MJG

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P lb	ϕP_{allow} lb	% Capacity	Pass Fail
T1	185 - 180	Leg	1 3/4	3	-1064.38	28890.80	3.7	Pass
		Diagonal	L2x2x3/16	9	-1561.23	19820.60	7.9	Pass
							17.8 (b)	
T2	180 - 160	Top Girt	L1 3/4x1 3/4x3/16	4	-955.35	9100.74	10.5	Pass
		Leg	2	13	-52693.80	72063.20	73.1	Pass
		Diagonal	L2x2x3/16	27	6513.62	21857.50	29.8	Pass
							74.2 (b)	
T3	160 - 140	Leg	2 3/4	46	-152343.00	187145.00	81.4	Pass
		Diagonal	L2 1/2x2 1/2x3/16	60	11399.90	27837.80	41.0	Pass
							97.8 (b)	
T4	140 - 120	Leg	3	80	-198105.00	235529.00	84.1	Pass
		Diagonal	L2x2x3/16	110	-5499.60	21123.80	26.0	Pass
							62.2 (b)	
T5	120 - 100	Leg	3	113	-223791.00	235529.00	95.0	Pass
		Diagonal	L2x2x3/16	119	-3601.86	11223.40	32.1	Pass
							45.0 (b)	
T6	100 - 80	Leg	3 1/4	146	-244105.00	250223.00	97.6	Pass
		Diagonal	L2x2x3/16	152	-3629.68	7174.38	50.6	Pass
T7	80 - 60	Leg	3 1/2	173	-262867.00	306641.00	85.7	Pass
		Diagonal	L2x2x3/16	179	-3664.61	5306.81	69.1	Pass
T8	60 - 40	Leg	3 1/2	200	-280493.00	306641.00	91.5	Pass
		Diagonal	L2x2x3/16	206	-3806.58	4046.99	94.1	Pass
T9	40 - 20	Leg	3 1/2	227	-295245.00	306641.00	96.3	Pass
		Diagonal	L2 1/2x2 1/2x3/16	236	-4551.34	5206.19	87.4	Pass
		Horizontal	L2 1/2x2 1/2x3/16	229	-5614.31	7313.90	76.8	Pass
							80.8 (b)	
T10	20 - 0	Inner Bracing	L2x2x3/16	239	-5.45	3447.10	0.3	Pass
		Leg	3 3/4	266	-311759.00	368015.00	84.7	Pass
		Diagonal	L3x3x3/16	275	-4745.50	7624.61	62.2	Pass
		Horizontal	L3x3x3/16	268	-5653.25	10075.60	56.1	Pass
		Inner Bracing	L2x2x3/16	278	-6.26	2723.63	0.3	Pass
							81.4 (b)	
							Summary	
						Leg (T6)	97.6	Pass
						Diagonal (T3)	97.8	Pass
						Horizontal (T10)	81.4	Pass
						Top Girt (T1)	10.5	Pass
						Inner Bracing (T10)	0.3	Pass
						Bolt Checks	97.8	Pass
						RATING =	97.8	Pass

FOUNDATION DESIGN

Foundation Design

Mat Footing

Design Parameters:

Tower Info:

$FaceWidth := 18.5$ ft

$V_T := 37783$ lbf

$C_T := 47638$ lbf

$M_T := 4802581$ lbf·ft

$C_{Leg} := 315452$ lbf

$T_{Leg} := 277695$ lbf

$V_{Leg} := 21524$ lbf

Footing Info:

$Conc.Comp. := 4000$ (psi)

$Width := 28$ ft

$Length := 28$ ft

$SlabDepth := 1.5$ ft

$No.SlabReb := 33$

$SlabReb.Dia. := 1.0$ in

$SlabReb.Area := 0.79 \cdot in^2$

$SlabReb.Wt := 2.67$ plf

$Offset := 2.5$ ft

Pedestal Info:

$No.Ped := 3$

$Ped.Dia. := 4$ ft

$Ped.Depth := 5$ ft

$No.Ped.Reb := 16$

$Ped.Reb.Dia. := 1.0$ in

$Ped.Reb.Area := 0.79$ in²

$Ped.Reb.Wt := 2.67$ plf

$No.Couples := 7$

$Coup.Dist. := 28.9 \cdot in$

$Ped.TieWt := 0.376$ plf

Design Info:

$Ult.SoilBearing := 6000$ psf

$SafFact := 2$

Foundation Resistance Factor

$\phi_1 := 0.75$

Client Resistance Factor

$\phi_2 := 1.0$

Fall Zone Resistance Factor

$\phi_3 := 0.87$

Total Resistance Factor

$ResistanceFactor := \phi_1 \cdot \phi_2 \cdot \phi_3 = 0.653$

Soil Capacity Design:

$$M_{max} := M_T + \left((V_T) (Ped.Depth + SlabDepth) \right) + (C_T \cdot Offset) = 5167265.5 \text{ lbf} \cdot \text{ft}$$

$$C_{max} := C_T + \left((0.9) \left(150 \frac{\text{lbf}}{\text{ft}^3} \right) \left((Length) (Width) (SlabDepth) \downarrow + (No.Ped) (Ped.Dia.)^2 \left(\frac{\pi}{4} \right) (Ped.Depth) \right) \right) \downarrow$$

$$+ (0.9) \left(115 \frac{\text{lbf}}{\text{ft}^3} \right) \left((Length) (Width) (Ped.Depth - 1 \text{ ft}) \downarrow - (No.Ped) (Ped.Dia.)^2 \left(\frac{\pi}{4} \right) (Ped.Depth - 1 \text{ ft}) \right)$$

$$C_{max} = 540813 \text{ lbf}$$

$$e := \frac{M_{max}}{C_{max}} = 9.55 \text{ ft}$$

$$e_1 := \left(\frac{Length}{2} \right) - e = 4.45 \text{ ft}$$

Soil Capacity: $Soil := (ResistanceFactor) \cdot (SafFact) \cdot Ult.SoilBearing$

Maximum Bearing Pressure Applied: $q_{max} := \frac{(2 \cdot C_{max})}{3 (Length) (e_1)}$

Results: $q_{max} = 2897 \text{ psf}$ $Soil = 7830 \text{ psf}$

Soil Bearing Capacity Check: $q_{max} < Soil$ $Check_{q_{max}} = \text{"Pass"}$

Mat Design:

$$W_{max} := q_{max} \cdot Length = 81105 \frac{\text{lbf}}{\text{ft}}$$

$$l := \frac{C_{max}}{0.5 (W_{max})} = 13.34 \text{ ft}$$

$$\Delta Pressure := \frac{W_{max}}{l} = 6082 \text{ psf}$$

$$M_{at} := \left(\frac{Length}{2} \right) - \left(\frac{FaceWidth}{2} \right) \left(\frac{1.732}{3} \right) - Offset = 6.16 \text{ ft}$$

$$W_{at} := W_{max} - (\Delta Pressure) (M_{at}) = 43644 \frac{\text{lbf}}{\text{ft}}$$

$$M_{max} := (0.5) (W_{at}) (M_{at})^2 + \left((0.5) (W_{max} - W_{at}) (M_{at})^2 \left(\frac{2}{3} \right) \right)$$

$$\phi M_n := 0.9 \cdot (\phi_2 \cdot \phi_3) \cdot (No.SlabReb) (SlabReb.Area) 60 \text{ ksi} \left(\left(\left(\left(SlabDepth \cdot 12 \frac{\text{in}}{\text{ft}} \right) \downarrow \right) \downarrow \right) - 3 \text{ in} - (1.5 \cdot SlabReb.Dia.) \right) \left(\frac{(No.SlabReb \cdot SlabReb.Area \cdot 60 \text{ ksi})}{2 \cdot .85 \cdot 4 \text{ ksi} \cdot \left(Length \cdot 12 \frac{\text{in}}{\text{ft}} \right)} \right)$$

Results: $M_{max} = 15621 \text{ kip} \cdot \text{in}$ $\phi M_n = 15696 \text{ kip} \cdot \text{in}$

Moment Check: $M_{max} < \phi M_n$ $Check_{Mmax} = \text{"Pass"}$

Punching/Pullout

$$\phi V_{cc} := 0.75 \cdot (\phi_2 \cdot \phi_3) \cdot (4) \left(\sqrt{\text{Conc.Comp. psi}} \right) \left((\text{Ped.Dia.}) + (\text{SlabDepth}) \right) (\pi) \cdot (\text{SlabDepth})$$

$$\phi V_{ct} := 0.75 \cdot (\phi_2 \cdot \phi_3) (4) \left(\sqrt{\text{Conc.Comp. psi}} \right) \left(\left((\text{Ped.Dia.} - 6 \text{ in}) \downarrow \right) \left(\pi \right) (\text{SlabDepth} - 6 \text{ in}) \right) \left(\pi \right) (\text{SlabDepth} - 6 \text{ in})$$

Results: $\phi V_{cc} = 616080 \text{ lbf}$ $\phi V_{ct} = 336043 \text{ lbf}$

Punching/Pullout Check: $\phi V_{cc} > C_{Leg}$ $Check \phi V_{cc} = \text{"Pass"}$

$\phi V_{ct} > T_{Leg}$ $Check \phi V_{ct} = \text{"Pass"}$

Pedestal Design

$$M_{ped} := \frac{(V_{Leg} \cdot \text{Ped.Depth})}{1000 \frac{\text{lbf}}{\text{kip}}} = 1291 \text{ kip} \cdot \text{in}$$

$$\phi V_c := \text{ResistanceFactor} \cdot 2 \cdot \left(\sqrt{\text{Conc.Comp. psi}} \right) (\text{Ped.Dia.} - 6 \text{ in})^2 \left(\frac{\pi}{4} \right) = 114348 \text{ lbf}$$

$$\phi M_n := 0.9 \cdot (\phi_2 \cdot \phi_3) \cdot (\text{No.Couples}) (\text{Ped.Reb.Area}) (60 \text{ ksi}) (\text{Coup.Dist.}) = 7508 \text{ kip} \cdot \text{in}$$

$$\phi T_n := 0.9 \cdot (\phi_2 \cdot \phi_3) \cdot (\text{No.Ped.Reb}) (\text{Ped.Reb.Area}) (60000 \text{ psi}) = 593827.2 \text{ lbf}$$

$$\text{Combined} := \frac{T_{Leg}}{\phi T_n} + \frac{M_{ped}}{\phi M_n}$$

Results $\text{Combined} = 0.64$

Pedestal Check: $\text{Combined} < 1$ $Check_{comb.} = \text{"Pass"}$

Rebar Weight

Mat Rebar

Total Length of Mat Rebar

$$L_{matreb} := 2 \cdot No.SlabReb \cdot (Width - 6 \text{ in}) + 2 \cdot No.SlabReb \cdot (Length - 6 \text{ in}) = 3630 \text{ ft}$$

Total Weight of Mat Rebar

$$W_{matreb} := L_{matreb} \cdot SlabReb.Wt = 9692.1 \text{ lbf}$$

Pedestal Rebar

Total Length of Longitudinal Pedestal Rebar

$$L_{pedlong} := (Ped.Depth + SlabDepth + 3 \text{ in}) \cdot No.Ped.Reb \cdot No.Ped = 324 \text{ ft}$$

Total Length of Pedestal Ties

$$n_{ties} := \text{ceil} \left(\frac{Ped.Depth - 5 \text{ in}}{9 \text{ in}} + 1 \right) = 8$$

$$L_{pedtie} := n_{ties} \cdot (\pi \cdot (Ped.Dia. - 6 \text{ in}) + 12 \text{ in}) \cdot No.Ped = 287.9 \text{ ft}$$

Total Weight of Pedestal Rebar

$$W_{pedreb} := L_{pedlong} \cdot Ped.Reb.Wt + L_{pedtie} \cdot Ped.TieWt = 973.3 \text{ lbf}$$

Total Rebar Weight

$$W_{reb} := W_{matreb} + W_{pedreb} = 10665 \text{ lbf}$$

Concrete Volume

Mat Foundation

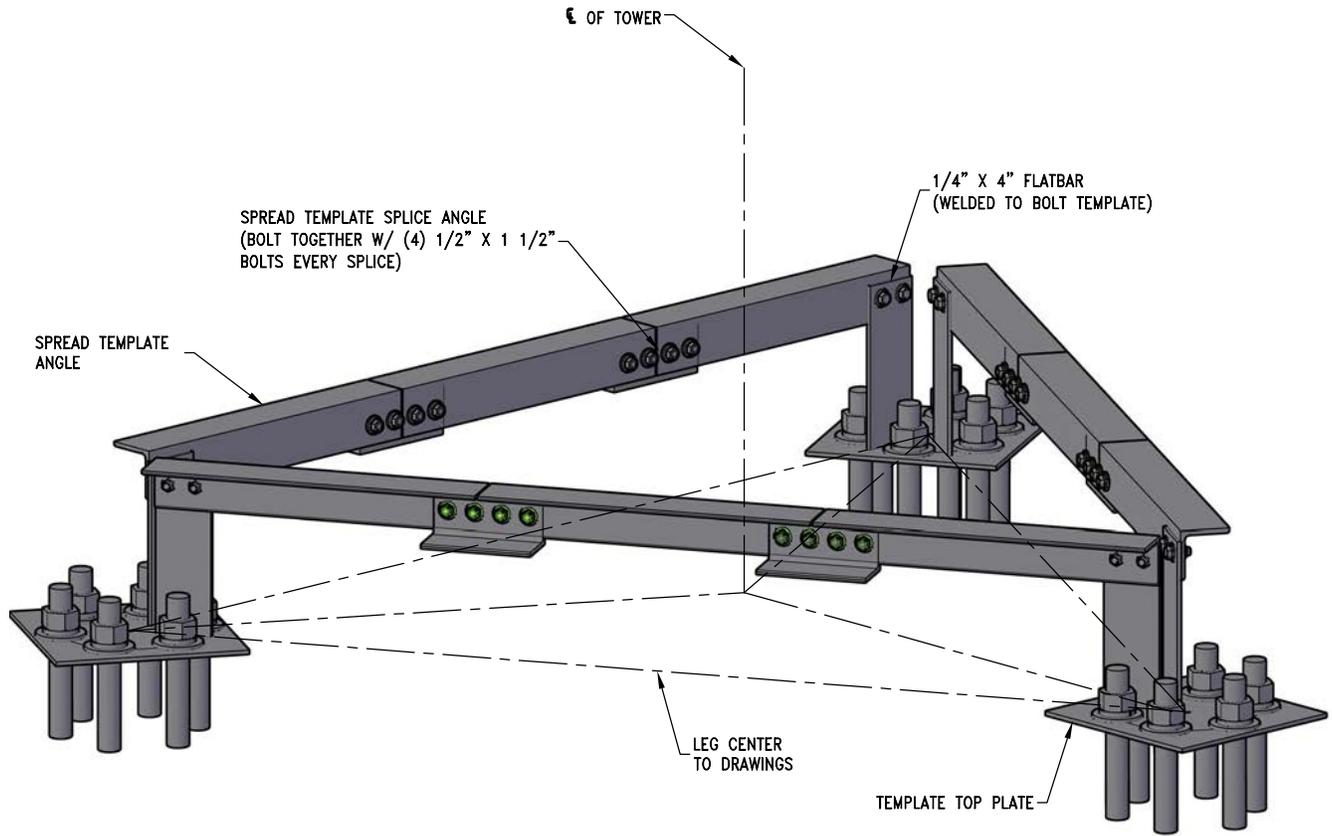
$$V_{mat} := Width \cdot Length \cdot SlabDepth = 43.556 \text{ yd}^3$$

Pedestals

$$V_{ped} := No.Ped \cdot \frac{\pi}{4} \cdot Ped.Dia.^2 \cdot Ped.Depth = 7 \text{ yd}^3$$

Total Concrete Volume

$$V_{conc} := V_{mat} + V_{ped} = 50.5 \text{ yd}^3$$



SPREAD TEMPLATE
SCALE: N.T.S.

GENERAL NOTES:

1. CONTRACTOR IS REQUIRED TO FIELD MEASURE ANCHOR BOLT CENTERS TO VERIFY CONFORMITY WITH DRAWINGS PRIOR TO POURING CONCRETE. DALEY TOWER IS NOT RESPONSIBLE IF INSTALLATION DOES NOT MATCH FOUNDATION DRAWINGS.

C.A.S.E.
INTERNATIONAL
LAFAYETTE, LOUISIANA
(337) 232-3336
WWW.CASENGR.COM

△							
△							
△							
△							
△							
△							
△	2-18-26	ISSUED FOR CONSTRUCTION	TPC	MJG			
NO.	DATE	REVISION DESCRIPTION	BY	APPR. BY			

DALEY TOWER SERVICE, INC. CARENCRO, LA DALEY JOB NO.: 59335			TARPON TOWERS 185' SELF SUPPORTER CT1238 NORFOLK SOUTH SITE, CT
ANCHOR BOLT SPREAD TEMPLATE			
DRAWN BY:	TPC	DRAWING NO.	26138-AB02
DESIGNED BY:	MJG		
CASE JOB NO.:	26138		
			REV. △

WELTI GEOTECHNICAL, P.C.

227 Williams Street · P.O. Box 397
Glastonbury, CT 06033-0397

(860) 633-4623 / FAX (860) 657-2514

February 9, 2026

Mr. Brett Buggeln
Tarpon Towers, LLC
8916 77th Terrace East, Ste 103
Lakewood Ranch, FL 34202

Ref: Geotechnical Study for Proposed Cell Tower (CT1238), Goshen East Street, Norfolk, CT

Dear Brett:

1.0 Herewith are the data from the test borings taken at the above referenced site. Four borings were drilled at the proposed tower foundation. The borings encountered bedrock at 11 to 11.5 feet below the existing grades and were cored 5 feet into the bedrock. A boring location plan is included with boring logs. *The boring was drilled by Clarence Welty Associates, Inc. and sampling was conducted by this firm solely to obtain indications of subsurface conditions as part of a geotechnical exploration program. No services were performed to evaluate subsurface environmental conditions.*

2.0 The **Subject Project** will include the construction of a 186-foot high, lattice type tower. The site layout plan indicates the 3 tower legs will be supported by piers atop a large mat foundation. The mat should on the plan is about 37' x 37'.

3.0 The **Soils Cross Section** from the borings is generally as follows:

Topsoil to 3" to 6"

Subsoils; fine SAND, some Silt, trace Roots and Gravel to 1.5 to 3.5 feet, loose to medium compact

Moraine; fine to coarse SAND, some Gravel, little to some Silt, few Cobbles and Boulders to the top of bedrock at 11 to 11.5 feet, dense

Bedrock; Granofels

Note: The rock cores had recoveries ranging from of 63% to 93% and RQD values ranging

from 42% to 80%

The **groundwater** was locally just above the bedrock at 10 to 10.5 feet below the existing grades at the completion of the boring

4.0 In general the criteria for tower support is that the foundation capacity would exceed the loads, which might collapse the tower. **Movements from strains in the soils should be limited to differential settlement (or lateral movements of less than 1/2").**

5.0 The **foundation for the tower** can be with a large mat designed to prevent overturning by gravity resistance of the weight of the mat and soil cover. The mat foundation can be placed on the natural inorganic soils at least 5 feet below the existing grade. There should be a minimum 6" layer of 3/8" crushed stone beneath foundations on the natural soils. The **Allowable Bearing Pressure** on the crushed stone atop the natural soils at 5+ feet below grade can be 3.0 Tons/sf. The **Allowable Bearing Pressure** on the crushed stone atop the natural soils at 8+ feet below grade can be 4.0 Tons/sf.

5.1 In **summary** the following soil properties and design values would apply to alternate 1.

Soil Property/Parameter	Value
Soil Unit Weight (Backfill)	125 pcf
Soil Unit Weight (Natural soils at 4+ feet below the exiting grades)	125 pcf
Soil Unit Weight Submerged (Natural)	63 pcf
Angle of Internal Friction (ϕ)	34°
Cohesion	0
Pull Out Angle from Vertical	30°
Sliding Coefficient	0.6
Frost Protection Depth (by code)	3.5 feet
Allowable Soil Bearing Pressure on the natural soil inorganic at 5+ feet below the existing grade	3.0 Tons/sf
Allowable Soil Bearing Pressure on the natural soil inorganic at 8+ feet below the existing grade	4.0 Tons/sf

6.0 Regarding **backfill of foundations**, the material should conform to the following or be 3/8" crushed stone.

Percent Passing	Sieve Size
100	3.5"
50 - 100	3/4"
25 - 85	No.4

The fraction, passing the No.4 sieve should have less than 15% passing the No. 200 sieve.

All backfill and fill must be compacted to at least 95% of modified optimum density in accordance with ASTM D-1557.

7.0 The soils at the subject site are generally in OSHA class C which would require excavations that are in excess of 5 feet to have slopes which are less than 34° (i.e., 1.5H to 1.0V).

8.0 This report has been prepared for specific application to the subject project in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. In the event that any changes in the nature, design and location of structures are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing.

The analyses and recommendations submitted in this report are based in part upon data obtained from referenced explorations. The extent of variations between explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

Welti Geotechnical, P.C., should perform a general review of the final design and specifications in order that geotechnical design recommendations may be properly interpreted and implemented as they were intended.

If you have any questions please call me.

Very truly yours,



Max Welti, P. E.
President, Welti Geotechnical, P.C.

APPENDIX

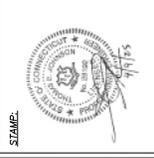
**TEST BORING LOCATION
+
TEST BORING DATA**

DATE	REVISIONS
10/17/24	ISSUED FOR REVIEW
04/02/25	PERMITTING REVISED
04/09/25	PERMITTING REVISED

TARPOON TOWERS III, LLC
8916 77TH TERRACE EAST
NORFOLK, CT 06068

TARPOON TOWERS III, LLC
LAKEMOOD RANCH, FL 34202

APPLICANT:

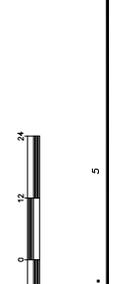
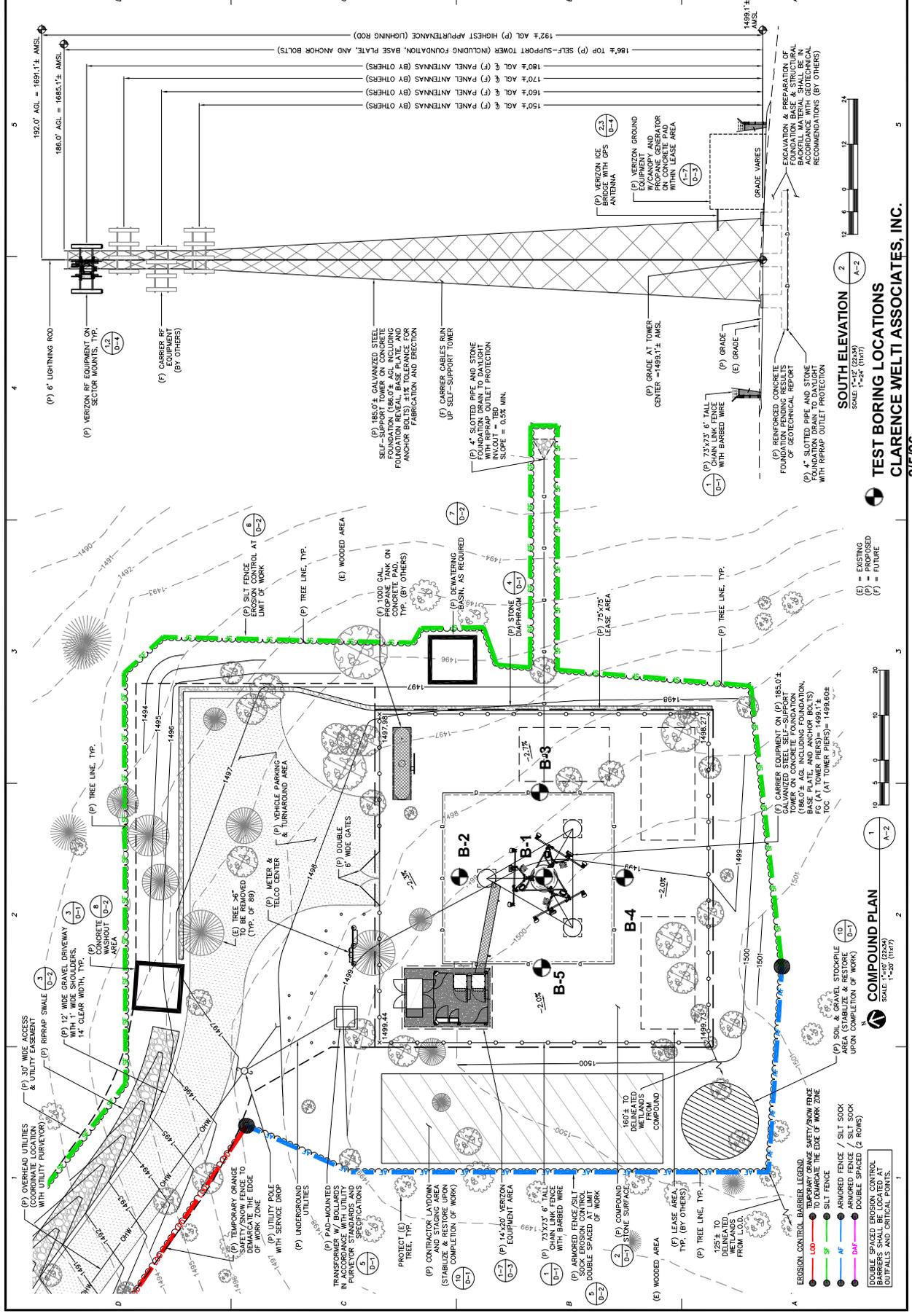


DATE: 04/09/2025
DRAWN: JEB

CHECK: JMM/TEJ
SCALE: SEE PLAN
JOB NO: 22-031
SHEET TITLE:

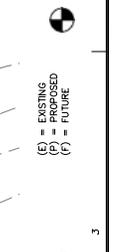
COMPOUND PLAN & ELEVATION

A-2



SOUTH ELEVATION 2
SCALE: 1"=20' (2234)
SCALE: 1"=20' (1187)

TEST BORING LOCATIONS
CLARENCE WELTI ASSOCIATES, INC.
2/15/26



EROSION CONTROL BARRIER LEGEND:

- Orange Safety/Snow Fence
- Silt Fence
- Silt Sock
- Double Spaced Erosion Control
- Double Spaced Erosion Control

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

COMPOUND PLAN
SCALE: 1"=20' (1187)

LEGEND:

- 1.00 - EXISTING
- 1.01 - PROPOSED
- 1.02 - FUTURE

CLARENCE WELTI ASSOC., INC. P.O. BOX 397 GLASTONBURY, CONN 06033				CLIENT TARPON TOWERS				PROJECT NAME PROPOSED CELL TOWER - CT1238			
								LOCATION 78 GOSHEN EAST STREET, NORFOLK, CT			
	AUGER	CASING	SAMPLER	CORE BAR.	OFFSET	SURFACE ELEV.		HOLE NO. B-1			
TYPE	HSA		SS	NQ	LINE & STA.	GROUND WATER OBSERVATIONS		START DATE 2/3/26			
SIZE I.D.	3.75"		1.375"	2.0"	LONGITUDE	AT none FT. AFTER 0 HOURS		FINISH DATE 2/3/26			
HAMMER WT.			140lbs		LATITUDE	AT FT. AFTER HOURS					
HAMMER FALL			30"								
DEPTH	SAMPLE			A	STRATUM DESCRIPTION + REMARKS	ELEV.					
	NO.	BLOWS/6"	DEPTH								
0	1	3-4-3-8	0.0'-2.0'		TOPSOIL BR.FINE SAND, SOME SILT, LITTLE GRAVEL	0.25					
	2	12-9-12-22	2.0'-4.0'		GREY/BR.FINE-CRS.SAND, SOME GRAVEL, LITTLE TO SOME SILT, FEW COBBLES & BOULDERS	3.0					
5	3	20-18-24-38	5.0'-7.0'								
10	4	10-60	10.0'-10.8'		CORED BEDROCK - GRANOFELS RUN #1 11.0' - 16.0' RECOVERED 56" RQD = 80%	11.0					
15											
					BOTTOM OF BORING @ 16.0'	16.0					
20											
25											
30											
35											
LEGEND: COL. A: SAMPLE TYPE: D=DRY A=AUGER C=CORE U=UNDISTURBED PISTON S=SPLIT SPOON PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOME=20-35% AND=35-50%						DRILLER: J. BREWER INSPECTOR:					
						SHEET 1 OF 1		HOLE NO. B-1			

CLARENCE WELTI ASSOC., INC. P.O. BOX 397 GLASTONBURY, CONN 06033				CLIENT TARPON TOWERS			PROJECT NAME PROPOSED CELL TOWER - CT1238		
							LOCATION 78 GOSHEN EAST STREET, NORFOLK, CT		
	AUGER	CASING	SAMPLER	CORE BAR.	OFFSET	SURFACE ELEV.		HOLE NO. B-3	
TYPE	HSA		SS	NQ	LINE & STA.	GROUND WATER OBSERVATIONS		START DATE 2/3/26	
SIZE I.D.	3.75"		1.375"	2.0"	LONGITUDE	AT 10.5 FT. AFTER 0 HOURS		FINISH DATE 2/3/26	
HAMMER WT.			140lbs		LATITUDE	AT FT. AFTER HOURS			
HAMMER FALL			30"						
DEPTH	SAMPLE			A	STRATUM DESCRIPTION + REMARKS	ELEV.			
	NO.	BLOWS/6"	DEPTH						
0	1	3-6-4-25	0.0'-2.0'		TOPSOIL	0.25			
					BR.FINE SAND AND SILT	1.5			
	2	9-21-30-35	2.0'-4.0'		GREY/BR.FINE-CRS.SAND, SOME GRAVEL, LITTLE TO SOME SILT, FEW COBBLES & BOULDERS				
5	3	20-25-38-25	5.0'-7.0'						
10	4	9-13-60	10.0'-11.3'		CORED BEDROCK - GRANOFELS	11.5			
15					RUN #1 11.5' - 16.5' RECOVERED 38" RQD = 42%				
20					BOTTOM OF BORING @ 16.5'	16.5			
25									
30									
35									
LEGEND: COL. A: SAMPLE TYPE: D=DRY A=AUGER C=CORE U=UNDISTURBED PISTON S=SPLIT SPOON PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOME=20-35% AND=35-50%						DRILLER: J. BREWER INSPECTOR:			
						SHEET 1 OF 1		HOLE NO. B-3	

CLARENCE WELTI ASSOC., INC. P.O. BOX 397 GLASTONBURY, CONN 06033				CLIENT TARPON TOWERS			PROJECT NAME PROPOSED CELL TOWER - CT1238		
							LOCATION 78 GOSHEN EAST STREET, NORFOLK, CT		
	AUGER	CASING	SAMPLER	CORE BAR.	OFFSET	SURFACE ELEV.		HOLE NO. B-4	
TYPE	HSA		SS	NQ	LINE & STA.	GROUND WATER OBSERVATIONS		START DATE 2/5/26	
SIZE I.D.	3.75"		1.375"	2.0"	LONGITUDE	AT 10.0 FT. AFTER 0 HOURS		FINISH DATE 2/5/26	
HAMMER WT.			140lbs		LATITUDE	AT FT. AFTER HOURS			
HAMMER FALL			30"						
DEPTH	SAMPLE			A	STRATUM DESCRIPTION + REMARKS	ELEV.			
	NO.	BLOWS/6"	DEPTH						
0	1	3-20-27-60	0.0'-1.6'		TOPSOIL BR.FINE SAND, SOME SILT	0.25			
5	2	60	5.0'-5.3'		GREY/BR.FINE-CRS.SAND, SOME GRAVEL, LITTLE TO SOME SILT, FEW COBBLES & BOULDERS	3.5			
10	3	14-60	10.0'-10.8'		CORED BEDROCK - GRANOFELS RUN #1 11.0' - 16.0' RECOVERED 52" RQD = 73%	11.0			
15									
20									
25									
30									
35					BOTTOM OF BORING @ 16.0'	16.0			
LEGEND: COL. A: SAMPLE TYPE: D=DRY A=AUGER C=CORE U=UNDISTURBED PISTON S=SPLIT SPOON PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOME=20-35% AND=35-50%						DRILLER: J. BREWER INSPECTOR:			
						SHEET 1 OF 1		HOLE NO. B-4	

CLARENCE WELTI ASSOC., INC. P.O. BOX 397 GLASTONBURY, CONN 06033				CLIENT TARPON TOWERS			PROJECT NAME PROPOSED CELL TOWER - CT1238		
							LOCATION 78 GOSHEN EAST STREET, NORFOLK, CT		
	AUGER	CASING	SAMPLER	CORE BAR.	OFFSET	SURFACE ELEV.		HOLE NO. B-5	
TYPE	HSA		SS	NQ	LINE & STA.	GROUND WATER OBSERVATIONS		START DATE 2/4/26	
SIZE I.D.	3.75"		1.375"	2.0"	LONGITUDE	AT none FT. AFTER 0 HOURS		FINISH DATE 2/4/26	
HAMMER WT.			140lbs		LATITUDE	AT FT. AFTER HOURS			
HAMMER FALL			30"						
DEPTH	SAMPLE			A	STRATUM DESCRIPTION + REMARKS	ELEV.			
	NO.	BLOWS/6"	DEPTH						
0	1	2-3-3-6	0.0'-2.0'	A	TOPSOIL BR.FINE SAND, SOME SILT, TRACE GRAVEL	0.25			
	2	8-7-33-36	2.0'-4.0'		GREY/BR.FINE-CRS.SAND, SOME GRAVEL, LITTLE TO SOME SILT, FEW COBBLES & BOULDERS	3.0			
5	3	36-32-35	5.0'-6.5'						
10	4	20-20-30	10.0'-11.5'		CORED BEDROCK - GRANOFELS	11.5			
					RUN #1 11.5' - 16.5' RECOVERED 55" RQD = 77%				
15									
					BOTTOM OF BORING @ 16.5'	16.5			
20									
25									
30									
35									
LEGEND: COL. A: SAMPLE TYPE: D=DRY A=AUGER C=CORE U=UNDISTURBED PISTON S=SPLIT SPOON PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOME=20-35% AND=35-50%						DRILLER: J. BREWER INSPECTOR:			
						SHEET 1 OF 1		HOLE NO. B-5	

Location	Norfolk South CT				
Date	3/18/2026				
Band	C-Band	AWS	PCS	850	700
Operating Frequency (MHz)	3,700	2,145	1,970	869	746
General Population MPE (mW/cm ²)	1	1	1	0.57933333	0.49733333
ERP Per Transmitter (Watts)	66,830	10,303	10,303	1,884	1,679
Number of Transmitters	2	2	2	4	4
Antenna Centerline (CL) (feet)	120	120	120	180	180
Total ERP (Watts)	133,660	20,606	20,606	7,536	6,716
Total ERP (dBm)	81	73	73	69	68
Maximum % of General Population Limit	12.40%				

*Guidelines adopted by the FCC on August 1, 1996, 47 CFR Section 1.1310 based on NCRP Report 86, 1986 and generally on ANSI/IEEE C95.1-1992

MHz = Megahertz
mW/cm² = milliwatts per square centimeter
ERP = Effective Radiated Power

Absolute worst case maximum values used, including the following assumptions:

1. closest accessible point is distance from antenna to base of pole;
2. continuous transmission from all available channels at full power for indefinite time period;
3. calculation takes into account a point of interest of 2m or 6.56ft

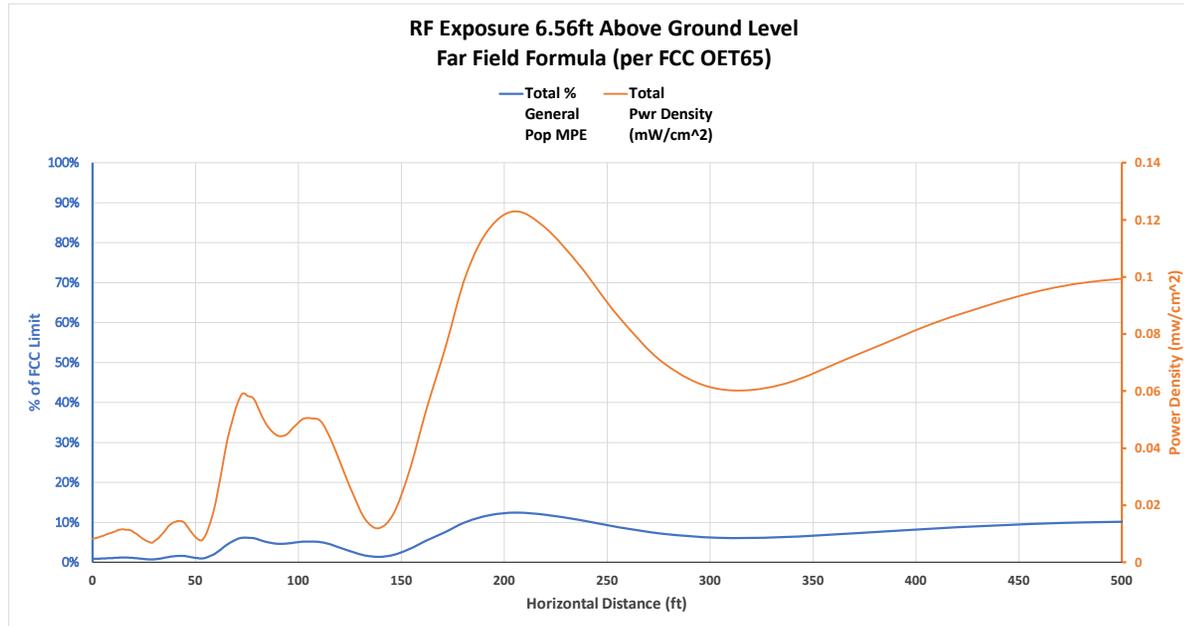
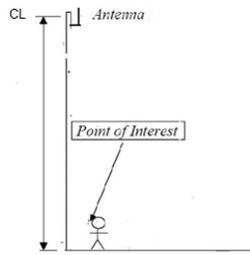
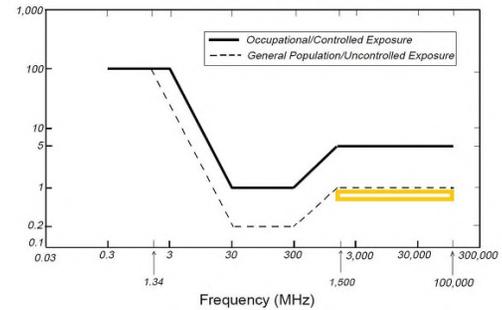


Figure 1. FCC Limits for Maximum Permissible Exposure (MPE) Plane-wave Equivalent Power Density



Angle Below Horizon	Power Density (mW/cm ²)					Percent of General Population MPE										Distance	Total Pwr Density (mW/cm ²)	Total % General Pop MPE
	C-Band	AWS	PCS	850	700 MHz	39GHz	28GHz	C-Band	CBRS	AWS	PCS	Cellular	CDMA	700 MHz				
90	0.002199444	0.002693426	0.0031645	6.09097E-06	3.347E-05	0.00%	0.00%	0.22%	0.00%	0.27%	0.32%	0.00%	0.00%	0.01%	0	0.008096931	0.81%	
89	0.002411443	0.002693204	0.003164239	6.67838E-06	2.91501E-05	0.00%	0.00%	0.24%	0.00%	0.27%	0.32%	0.00%	0.00%	0.01%	1.029848831	0.008304715	0.83%	
88	0.002643441	0.002692538	0.003163456	7.49247E-06	2.5386E-05	0.00%	0.00%	0.26%	0.00%	0.27%	0.32%	0.00%	0.00%	0.01%	2.0603254	0.008532314	0.86%	
87	0.002897279	0.002691427	0.003162151	8.21388E-06	2.21064E-05	0.00%	0.00%	0.29%	0.00%	0.27%	0.32%	0.00%	0.00%	0.00%	3.092058978	0.008781177	0.88%	
86	0.003174964	0.002689868	0.00316032	9.0041E-06	2.01563E-05	0.00%	0.00%	0.32%	0.00%	0.27%	0.32%	0.00%	0.00%	0.00%	4.125681905	0.009054313	0.91%	
85	0.003399496	0.002687861	0.003157961	1.00995E-05	1.92429E-05	0.00%	0.00%	0.34%	0.00%	0.27%	0.32%	0.00%	0.00%	0.00%	5.161831148	0.009274661	0.93%	
84	0.003724063	0.002685402	0.003155072	1.10696E-05	1.96834E-05	0.00%	0.00%	0.37%	0.00%	0.27%	0.32%	0.00%	0.00%	0.00%	6.201149881	0.00959529	0.96%	
83	0.003986074	0.002682487	0.003151647	1.21319E-05	2.20749E-05	0.00%	0.00%	0.40%	0.00%	0.27%	0.32%	0.00%	0.00%	0.00%	7.244289093	0.009854415	0.99%	
82	0.004265782	0.002679112	0.003147682	1.36049E-05	2.53317E-05	0.00%	0.00%	0.43%	0.00%	0.27%	0.31%	0.00%	0.00%	0.01%	8.291909247	0.010131513	1.02%	
81	0.004460421	0.002675273	0.003143171	1.49083E-05	3.04367E-05	0.00%	0.00%	0.45%	0.00%	0.27%	0.31%	0.00%	0.00%	0.01%	9.344681979	0.010324209	1.04%	
80	0.004771727	0.002670963	0.003138108	1.67158E-05	3.65676E-05	0.00%	0.00%	0.48%	0.00%	0.27%	0.31%	0.00%	0.00%	0.01%	10.40329186	0.010634081	1.07%	
79	0.005103835	0.002666176	0.003132484	1.83143E-05	4.393E-05	0.00%	0.00%	0.51%	0.00%	0.27%	0.31%	0.00%	0.00%	0.01%	11.46843824	0.01096474	1.10%	
78	0.005458048	0.002660906	0.003126292	2.10098E-05	5.15693E-05	0.00%	0.00%	0.55%	0.00%	0.27%	0.31%	0.00%	0.00%	0.01%	12.54083714	0.011317826	1.14%	
77	0.00573089	0.002655145	0.003119523	2.76704E-05	5.78077E-05	0.00%	0.00%	0.56%	0.00%	0.27%	0.31%	0.00%	0.00%	0.01%	13.62122328	0.011433235	1.15%	
76	0.005689455	0.002648884	0.003112167	3.72884E-05	6.33203E-05	0.00%	0.00%	0.57%	0.00%	0.26%	0.31%	0.01%	0.00%	0.01%	14.71035217	0.011551114	1.16%	
75	0.005545736	0.002642113	0.003104212	5.14154E-05	6.47236E-05	0.00%	0.00%	0.55%	0.00%	0.26%	0.31%	0.01%	0.00%	0.01%	15.80900235	0.011408201	1.15%	
74	0.005530436	0.002634824	0.003095648	7.08881E-05	6.31747E-05	0.00%	0.00%	0.55%	0.00%	0.26%	0.31%	0.01%	0.00%	0.01%	16.91797776	0.01139497	1.15%	

73	0.005388509	0.002627005	0.003086461	8.91279E-05	6.02537E-05	0.00%	0.00%	0.54%	0.00%	0.26%	0.31%	0.02%	0.00%	0.01%	18.03811021	0.011251356	1.14%
72	0.005129608	0.002618644	0.003076638	0.000107007	5.36269E-05	0.00%	0.00%	0.51%	0.00%	0.26%	0.31%	0.02%	0.00%	0.01%	19.17026208	0.010985524	1.11%
71	0.004556204	0.002609729	0.003066164	0.000119886	4.55762E-05	0.00%	0.00%	0.46%	0.00%	0.26%	0.31%	0.02%	0.00%	0.01%	20.31532918	0.01039756	1.05%
70	0.004045968	0.002600247	0.003055024	0.000122484	3.6987E-05	0.00%	0.00%	0.40%	0.00%	0.26%	0.31%	0.02%	0.00%	0.01%	21.47424382	0.00986071	1.00%
69	0.00343035	0.002590184	0.003043201	0.000119493	2.801E-05	0.00%	0.00%	0.34%	0.00%	0.26%	0.30%	0.02%	0.00%	0.01%	22.64797807	0.009211238	0.93%
68	0.00271361	0.002579525	0.003173509	0.000108782	2.02548E-05	0.00%	0.00%	0.27%	0.00%	0.26%	0.32%	0.02%	0.00%	0.00%	23.83754732	0.00859568	0.87%
67	0.001912694	0.002568253	0.003385616	9.90376E-05	1.27554E-05	0.00%	0.00%	0.19%	0.00%	0.26%	0.34%	0.02%	0.00%	0.00%	25.04401416	0.007969626	0.80%
66	0.001287147	0.002556351	0.003528747	6.38023E-05	8.03168E-06	0.00%	0.00%	0.13%	0.00%	0.26%	0.35%	0.01%	0.00%	0.00%	26.26849243	0.00744408	0.75%
65	0.000687846	0.002543803	0.003850201	2.77903E-05	5.05666E-06	0.00%	0.00%	0.07%	0.00%	0.25%	0.39%	0.00%	0.00%	0.00%	27.51215183	0.007114697	0.71%
64	0.000231862	0.002530588	0.004010712	1.23849E-05	4.60111E-06	0.00%	0.00%	0.02%	0.00%	0.25%	0.40%	0.00%	0.00%	0.00%	28.77622273	0.006790147	0.68%
63	1.74919E-05	0.003168321	0.004176661	7.10937E-06	9.37132E-06	0.00%	0.00%	0.00%	0.00%	0.32%	0.42%	0.00%	0.00%	0.00%	30.06200152	0.007378955	0.74%
62	5.62741E-05	0.00378705	0.00424914	6.77182E-06	2.5158E-05	0.00%	0.00%	0.01%	0.00%	0.38%	0.42%	0.00%	0.00%	0.01%	31.37085647	0.008124395	0.82%
61	0.000361109	0.004321463	0.004126965	1.81766E-05	5.12251E-05	0.00%	0.00%	0.04%	0.00%	0.43%	0.41%	0.00%	0.00%	0.01%	32.70423404	0.00887894	0.89%
60	0.000943669	0.00481739	0.004100264	6.28419E-05	9.73238E-05	0.00%	0.00%	0.09%	0.00%	0.48%	0.41%	0.01%	0.00%	0.02%	34.06366588	0.010021489	1.02%
59	0.001666652	0.005246106	0.003979578	0.00013706	0.000164771	0.00%	0.00%	0.17%	0.00%	0.52%	0.40%	0.02%	0.00%	0.03%	35.45077652	0.011194167	1.15%
58	0.002562752	0.005710825	0.003773105	0.000248596	0.000254368	0.00%	0.00%	0.26%	0.00%	0.57%	0.38%	0.04%	0.00%	0.05%	36.86729176	0.012549646	1.30%
57	0.003352716	0.006072801	0.003337252	0.000392638	0.000366406	0.00%	0.00%	0.34%	0.00%	0.61%	0.33%	0.07%	0.00%	0.07%	38.315048	0.013521814	1.42%
56	0.003907569	0.006164515	0.00288336	0.00055259	0.000503933	0.00%	0.00%	0.39%	0.00%	0.62%	0.29%	0.10%	0.00%	0.10%	39.79600249	0.014011967	1.49%
55	0.004151707	0.006112502	0.002793952	0.000725637	0.00064668	0.00%	0.00%	0.42%	0.00%	0.61%	0.28%	0.13%	0.00%	0.13%	41.31224475	0.014430479	1.56%
54	0.003929587	0.006058138	0.002769103	0.000889071	0.000810786	0.00%	0.00%	0.39%	0.00%	0.61%	0.28%	0.15%	0.00%	0.16%	42.86600915	0.014456686	1.59%
53	0.003390443	0.005864738	0.002743144	0.001016359	0.000970547	0.00%	0.00%	0.34%	0.00%	0.59%	0.27%	0.18%	0.00%	0.20%	44.45968896	0.013985231	1.57%
52	0.002322434	0.005295849	0.002716037	0.001084038	0.001109211	0.00%	0.00%	0.23%	0.00%	0.53%	0.27%	0.19%	0.00%	0.22%	46.09585196	0.012527569	1.44%
51	0.001234228	0.004564441	0.002687742	0.001054196	0.001210294	0.00%	0.00%	0.12%	0.00%	0.46%	0.27%	0.18%	0.00%	0.24%	47.77725796	0.010750902	1.27%
50	0.000299639	0.004023381	0.002658221	0.000978744	0.001290146	0.00%	0.00%	0.03%	0.00%	0.40%	0.27%	0.17%	0.00%	0.26%	49.50687824	0.009250131	1.13%
49	1.35375E-05	0.003232448	0.002627432	0.000828475	0.001343535	0.00%	0.00%	0.00%	0.00%	0.32%	0.26%	0.14%	0.00%	0.27%	51.28791753	0.008045427	1.00%
48	0.000769486	0.002313093	0.002595333	0.000701038	0.001335708	0.00%	0.00%	0.08%	0.00%	0.23%	0.26%	0.12%	0.00%	0.27%	53.12383861	0.007714658	0.96%
47	0.003023896	0.002180515	0.002561882	0.000592988	0.001297226	0.00%	0.00%	0.30%	0.00%	0.22%	0.26%	0.10%	0.00%	0.26%	55.01839008	0.009656506	1.14%
46	0.006833117	0.002150855	0.002527034	0.000549769	0.001202679	0.00%	0.00%	0.68%	0.00%	0.22%	0.25%	0.09%	0.00%	0.24%	56.97563771	0.013263453	1.49%
45	0.012255676	0.002119969	0.002490746	0.000612536	0.001064391	0.00%	0.00%	1.23%	0.00%	0.21%	0.25%	0.11%	0.00%	0.21%	59	0.018543318	2.01%
44	0.01869394	0.00208782	0.003088112	0.000765395	0.000941576	0.00%	0.00%	1.87%	0.00%	0.21%	0.31%	0.13%	0.00%	0.19%	61.09628851	0.025576844	2.71%
43	0.025391395	0.002054371	0.004706291	0.001048161	0.000832526	0.00%	0.00%	2.54%	0.00%	0.21%	0.47%	0.18%	0.00%	0.17%	63.26975389	0.034032744	3.56%
42	0.032156561	0.002019584	0.00668747	0.001434632	0.000735716	0.00%	0.00%	3.22%	0.00%	0.20%	0.67%	0.25%	0.00%	0.15%	65.52613837	0.043033963	4.48%
41	0.036259598	0.001983422	0.008657953	0.001831494	0.000680417	0.00%	0.00%	3.63%	0.00%	0.20%	0.87%	0.32%	0.00%	0.14%	67.87173603	0.049412884	5.14%
40	0.039004757	0.002133584	0.011458026	0.00231523	0.000643534	0.00%	0.00%	3.90%	0.00%	0.21%	1.15%	0.39%	0.00%	0.13%	70.31346196	0.055471424	5.77%
39	0.038222731	0.003469876	0.01413559	0.002594812	0.000636907	0.00%	0.00%	3.82%	0.00%	0.35%	1.41%	0.45%	0.00%	0.13%	72.85893224	0.059059915	6.16%
38	0.03334272	0.005140357	0.016255235	0.002879344	0.000629888	0.00%	0.00%	3.33%	0.00%	0.51%	1.63%	0.50%	0.00%	0.13%	75.51565653	0.058247545	6.10%
37	0.027109292	0.007097582	0.019548364	0.002979453	0.000622453	0.00%	0.00%	2.71%	0.00%	0.71%	1.95%	0.51%	0.00%	0.13%	78.29564448	0.057357143	6.01%
36	0.018734086	0.008722174	0.02190911	0.003010257	0.000560497	0.00%	0.00%	1.87%	0.00%	0.87%	2.19%	0.52%	0.00%	0.11%	81.206653331	0.052936124	5.57%
35	0.010507509	0.011207068	0.023414883	0.002901751	0.000481537	0.00%	0.00%	1.05%	0.00%	1.12%	2.34%	0.50%	0.00%	0.10%	84.2607324	0.048512747	5.11%
34	0.004165543	0.012813384	0.025566063	0.00273066	0.000351753	0.00%	0.00%	0.42%	0.00%	1.28%	2.56%	0.47%	0.00%	0.07%	87.47109714	0.045627403	4.80%
33	0.000558593	0.014291821	0.026612617	0.002508322	0.000177563	0.00%	0.00%	0.06%	0.00%	1.43%	2.66%	0.43%	0.00%	0.04%	90.85203287	0.044148916	4.61%
32	0.000265285	0.015911293	0.026406174	0.002197652	9.81587E-05	0.00%	0.00%	0.03%	0.00%	1.59%	2.64%	0.38%	0.00%	0.02%	94.41973721	0.044878563	4.66%
31	0.002814894	0.016123315	0.026758043	0.001879059	8.39287E-05	0.00%	0.00%	0.28%	0.00%	1.61%	2.68%	0.32%	0.00%	0.02%	98.19248946	0.047659241	4.91%
30	0.006672254	0.016682642	0.0252502	0.001429775	0.000105983	0.00%	0.00%	0.67%	0.00%	1.67%	2.53%	0.25%	0.00%	0.02%	102.19099976	0.050140855	5.13%
29	0.010424816	0.016071657	0.022701819	0.001013614	0.000237598	0.00%	0.00%	1.04%	0.00%	1.61%	2.27%	0.17%	0.00%	0.05%	106.4388176	0.050449504	5.14%
28	0.012324394	0.01544393	0.020359065	0.00061049	0.000596552	0.00%	0.00%	1.23%	0.00%	1.54%	2.04%	0.11%	0.00%	0.12%	110.9628615	0.049334431	5.04%
27	0.011022278	0.013812223	0.016992747	0.000226255	0.000965091	0.00%	0.00%	1.10%	0.00%	1.38%	1.70%	0.04%	0.00%	0.19%	115.7940198	0.043018594	4.42%
26	0.00712005	0.012035808	0.013197001	0.000102928	0.00126206	0.00%	0.00%	0.71%	0.00%	1.20%	1.32%	0.02%	0.00%	0.25%	120.9679267	0.033721994	3.51%
25	0.0027624	0.009756159	0.009756159	0.000117321	0.001476881	0.00%	0.00%	0.28%	0.00%	0.98%	0.98%	0.02%	0.00%	0.30%	126.5259083	0.02386892	2.54%
24	5.35248E-05	0.007023401	0.006259612	0.00019724	0.001496114	0.00%	0.00%	0.01%	0.00%	0.70%	0.63%	0.03%	0.00%	0.30%	132.5161697	0.015029892	1.67%
23	0.002158475	0.004593452	0.00348449	0.00036245	0.001285931	0.00%	0.00%	0.22%	0.00%	0.46%	0.35%	0.06%	0.00%	0.26%	138.9952896	0.011884797	1.34%
22	0.01166128	0.002376265	0.0001721452	0.000619397	0.00093743	0.00%	0.00%	1.12%	0.00%	0.24%	0.17%	0.11%	0.00%	0.19%	146.0301244	0.016820672	1.82%
21	0.028160661	0.000949806	0.001115925	0.00068072	0.00047091	0.00%	0.00%	2.82%	0.00%	0.09%	0.11%	0.12%	0.00%	0.09%	153.7002548	0.031378023	3.23%
20	0.050022376	0.000885435	0.001648757	0.000591599	0.000240989	0.00%	0.00%	5.00%	0.00%	0.09%	0.16%	0.10%	0.00%	0.05%	162.1011677	0.053389155	5.41%
19	0.070183544	0.000820781	0.003582841	0.000379222	0.00016938	0.00%	0.00%	7.02%	0.00%	0.08%	0.36%	0.07%	0.00%	0.03%	171.3484418	0.075135769	7.56%
18	0.091325045	0.002446708	0.006738793	0.000271189	0.000167202	0.00%	0.00%	9.13%	0.00%	0.24%	0.67%	0.05%	0.00%	0.03%	181.5833287	0.100948938	10.13%
17	0.100443218	0.005127585	0.010962582	0.000319759	0.000312458	0.00%	0.00%	10.04%	0.00%	0.51%	1.10%	0.06%	0.00%	0.06%	192.9803405	0.117165602	11.77%
16	0.097694192	0.0090753	0.015061249	0.00047113	0.000650295	0.00%	0.00%	9.77%	0.00%	0.91%	1.51%	0.08%	0.00%	0.13%	205.7574522	0.122952166</	