

KENNETH C. BALDWIN

One State Street Hartford, CT 06103 Main (860) 275-8200 Fax (860) 275-8299 kbaldwin@rc.com Direct (860) 275-8345

Also admitted in Massachusetts

December 4, 2025

Via Federal Express

Melanie A. Bachman, Esq. Executive Director/Staff Attorney Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Re: Docket No. 533 – Application of Tarpon Towers III, LLC and Cellco Partnership d/b/a Verizon Wireless for a Certificate of Environmental Compatibility and Public Need for the Construction, Maintenance and Operation of a Wireless Telecommunications Facility at 161 Conrad Street, Naugatuck, Connecticut

Development and Management Plan Submission

Dear Attorney Bachman:

Enclosed please find fifteen (15) copies of the following:

- 1. Development and Management ("D&M") Plans prepared by All Points
 Technologies Corporation for the approved telecommunications facility at 161
 Conrad Street in Naugatuck, Connecticut incorporating the Council's conditions
 of approval. Also enclosed are two (2) full size (24" x 36") sets of D&M plans.
 In accordance with Condition 2(d), the tower has been relocated to south and east
 away from the properties at 171 Conrad Street and 134 Craig Circle.
- 2. Tower and Foundation Design from TAPP dated October 28, 2025. In accordance with Condition 2(e) of the Council's Decision and Order, the tower design incorporates a yield point that ensures the tower setback radius remains within the boundaries of the host parcel.
- 3. Geotechnical Study prepared by Welti Geotechnical, P.C. dated October 16, 2025.
- 4. Letter from Verizon Wireless confirming its commitment to share the approved tower.

33413252-v1

Robinson+Cole

Melanie A. Bachman, Esq. December 4, 2025 Page 2

We respectfully request that this information be reviewed, and this matter be placed on the next available Siting Council agenda for approval. Please feel free to contact me if you have any questions or require additional information. Thank you.

Sincerely,

Kenneth C. Baldwin

Enclosures Copy to:

N. Warren "Pete" Buss, Mayor Borough of Naugatuck Parties and Intervenors of Record



CT1239 CONRAD ST **161 CONRAD STREET** NAUGATUCK, CT 06770

DRAWING INDEX

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SP-1 ABUTTERS & MUNICIPALITY NOTIFICATION MAP

SP-2 SITE PLAN

CP-1 COMPOUND PLAN & ELEVATION

C-1 CELLCO PARTNERSHIP EQUIPMENT PLAN & DETAILS

C-2 CELLCO PARTNERSHIP ANTENNA PLAN & DETAILS

C-3 T-MOBILE EQUIPMENT PLAN & DETAILS

C-4 T-MOBILE ANTENNA PLAN & DETAILS

C-5 AT&T EQUIPMENT PLAN & DETAILS

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C-7 BOROUGH OF NAUGATUCK EQUIPMENT PLAN & DETAILS

C-8 SITE DETAILS

EC-1 EROSION CONTROL, PLANTING & ENVIRONMENTAL NOTES & **DETAILS**

N-1 NOTES & SPECIFICATIONS

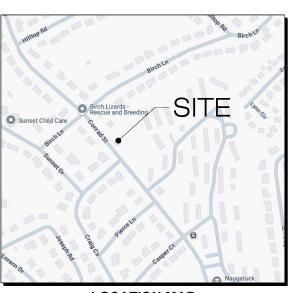
SITE DIRECTIONS

START: 20 ALEXANDER DRIVE

WALLINGFORD, CONNECTICUT 06492

END: 161 CONRAD STREET NAUGATUCK, CT 06770

1.	HEAD NORTH ON ALEXANDER DRIVE	0.3 MI
2.	TURN RIGHT ONTO BARNES INDUSTRIAL ROAD S	0.1 MI
3.	TURN LEFT ONTO BARNES RD. (CT-68 W)	4.4 MI
4.	TURN LEFT ONTO CT-68 W/ CT-70 W	1.2 MI
5.	TURN LEFT ONTO CT-68 W/ CT-70W / MAIN ST.	1.7 MI
6.	TURN LEFT ONTO CT-68 W	7.7 MI
7.	TURN RIGHT ONTO CHURCH ST.	0.5 MI
8.	TURN LEFT ONTO FIELD ST.	0.9 MI
9.	TURN LEFT ONTO CONRAD ST., DESTINATION WILL ON THE LEFT	0.3 MI



LOCATION MAP
SCALE: 1" = 200"

VZ SITE NAME: NAUGATUCK 5 VZ MDG LOCATON CODE: 616755202 VZ PSLC: 470709

VZ FUZE ID: 17453771 T-MOBLE SITE NAME: TARPON TOWERS MONOPOLE NAUGATUCK

T-MOBILE SITE ID: CTNH488A AT&T SITE NAME: S3458G AT&T SITE ID: SICT002531 LOCATION: 161 CONRAD STREET

NAUGATUCK CT 06770

PROJECT SCOPE: BAWLAND SITE W/ GROUND FQUIPMENT WITHIN A 6.737+ S.E. TELECOMMUNICATIONS EQUIPMENT COMPOUND W/ NEW 150'± AGL MONOPOLE.

COORDINATES & GROUND

FLEVATION INDICATED HEREIN

WERE ESTABLISHED FROM A FAA 1-A SURVEY CERTIFICATION

INC., DATED AUGUST 18, 2025

ASSESSORS TAX I.D: 014-9020

LATITUDE: 41° 29' 53.03" N (41.49806)

LONGITUDE: 73° 04' 12.18" W (73.07005)

GROUND ELEVATION: 514.9'± AMSL

PROPERTY OWNER: BOROUGH OF NAUGATUCK 229 CHURCH STREET NAUGATUCK, CT 06770

> APPLICANT: TARPON TOWERS III, LLC 8916 77th TERRACE EAST SUITE 103 LAKEWOOD RANCH, FL 34202

CO APPLICANTS: CELLCO PARTNERSHIP d/b/a VERIZON WIRELESS

20 ALEXANDER DRIVE WALLINGFORD, CT 06492

T-MOBILE NORTHEAST, LLC 15COMMERCE WAY NORTON, MA 02766

550 COCHITUATE ROAD FRAMINGHAM, MA 01701

LEGAL/REGULATORY COUNSEL: ROBINSON & COLE, LLP KENNETH C. BALDWIN, ESQ. ONE STATE STREET HARTFORD, CT 06103

ENGINEER CONTACT: ALL-POINTS TECHNOLOGY CORPORATION, P.C.

567 VAUXHALL STREET EXT., - SUITE 311 WATERFORD, CT 06385



8916 77th TERRACE EAST, SUITE 103



Cellco Partnership d/b/a



\mathbf{T} --Mobile

15 COMMERCE WAY SUITE B NORTON, MA 02766



550 COCHITUATE ROAD FRAMINGHAM, MA 01701

	D&M DOCUMENTS		
NO	DATE	REVISION	
0	09/12/25	FOR REVIEW: RCB	
1	10/06/25	CARRIER REVISIONS: RCB	
2	10/29/25	ADD TOWER INFO: RCB	
3	11/20/25	FINAL: RCB	
4	12/02/25	AT&T EQUIP REVISIONS: RCB	
5			
6			

DESIGN PROFESSIONALS OF RECORD

PROF: ROBERT C. BURNS. P.E. PROF: ROBERT C. BURNS, P.E.
COMP: ALL-POINTS TECHNOLOGY
CORPORATION, P.C.
ADD: 567 VAUXHALL STREET EXT.
SUITE 311
WATERFORD, CT 06385

OWNER: BOROUGH OF NAUGATU ADDRESS: 229 CHURCH STREET NAUGATUCK, CT 06770

CT1239 CONRAD ST

APT FILING NUMBER: CT752130

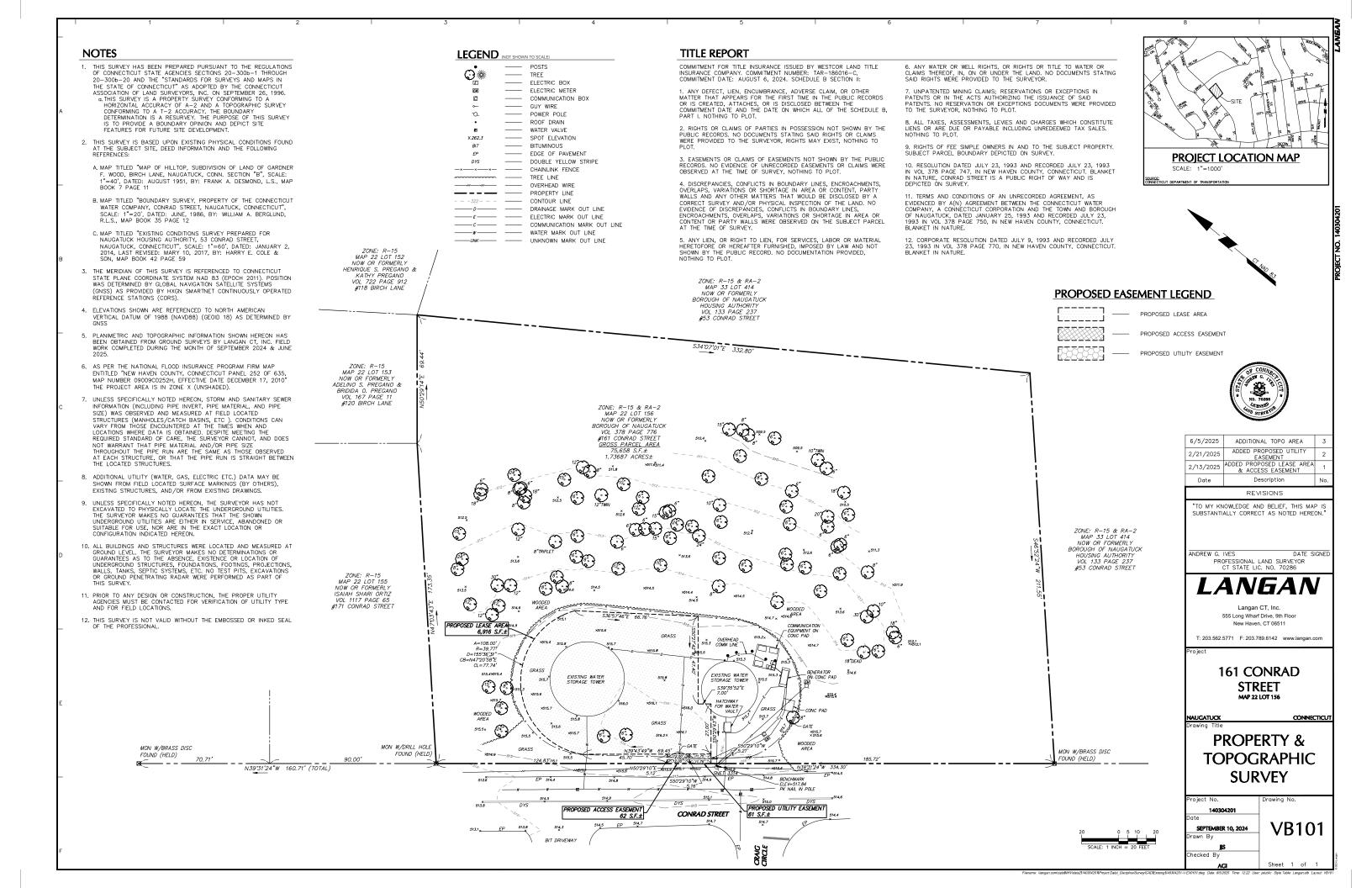
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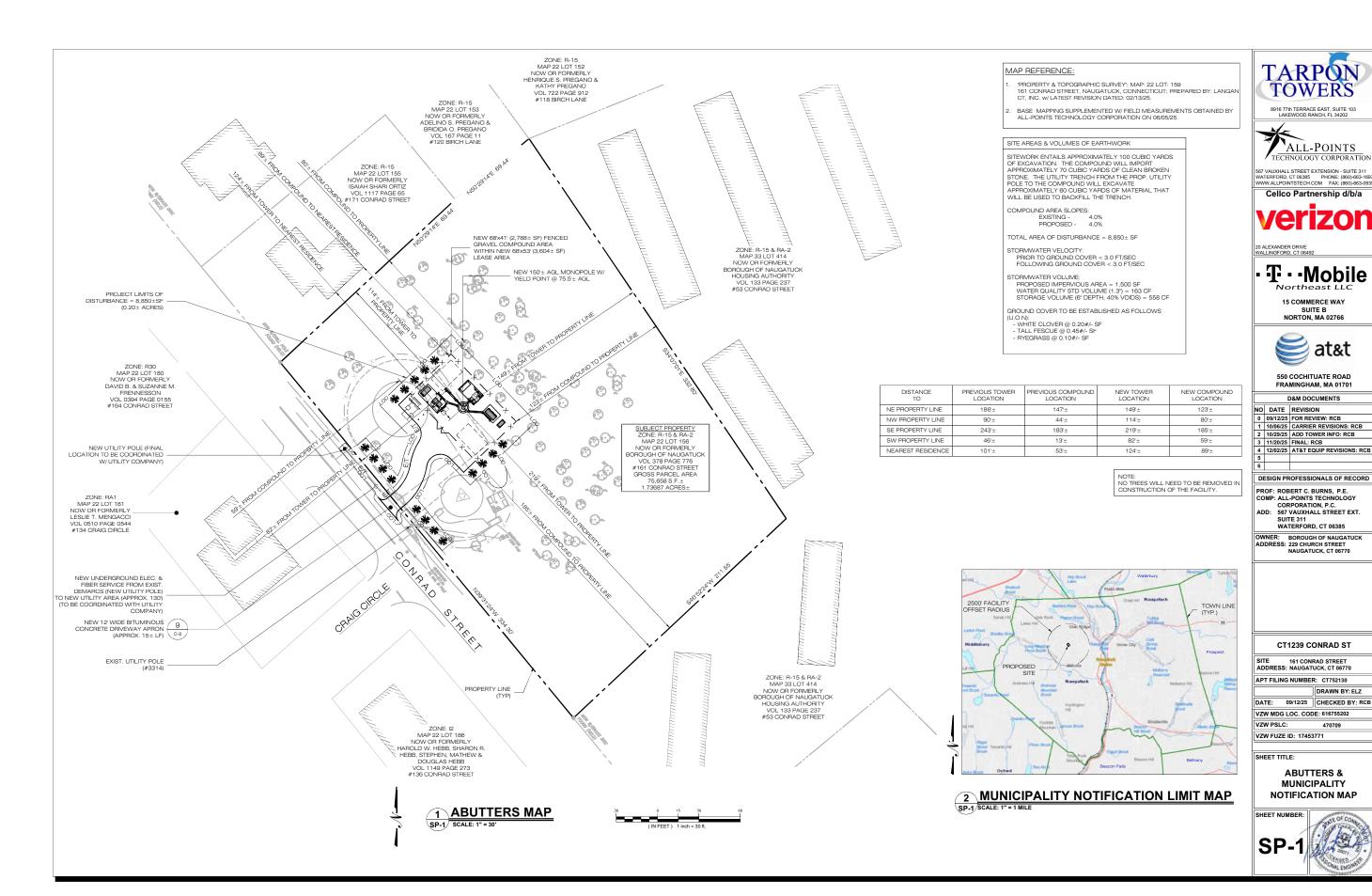
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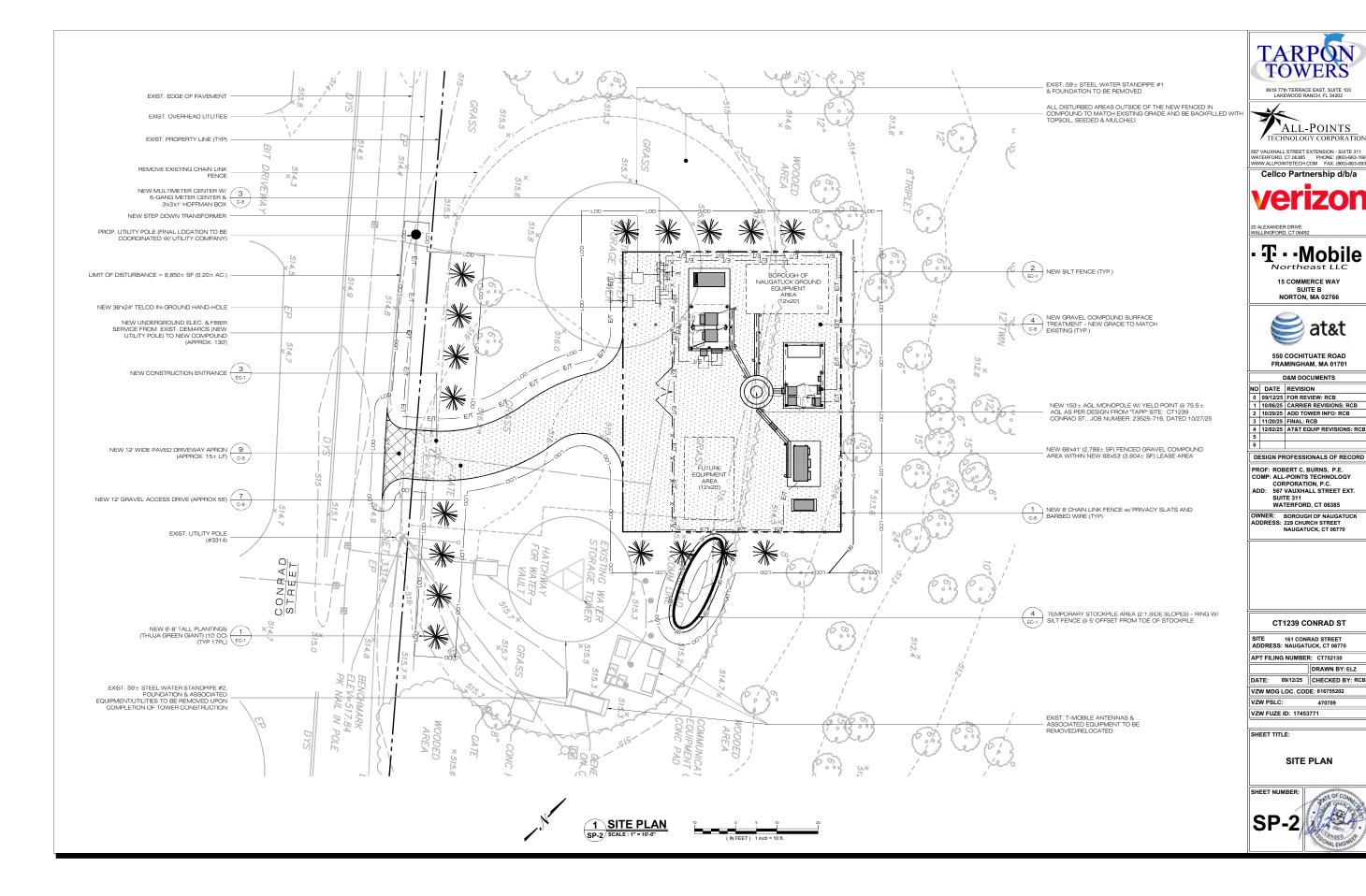
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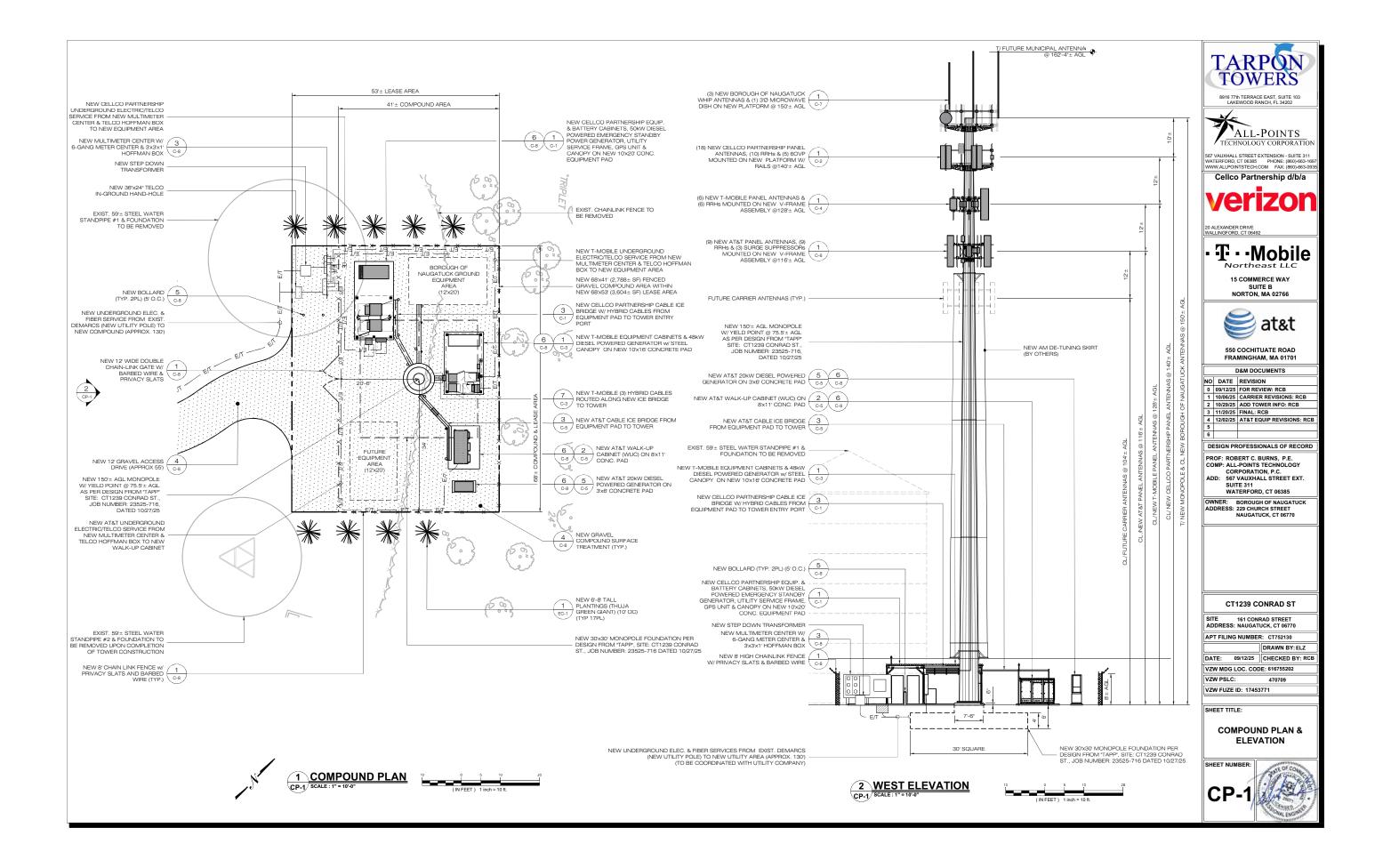
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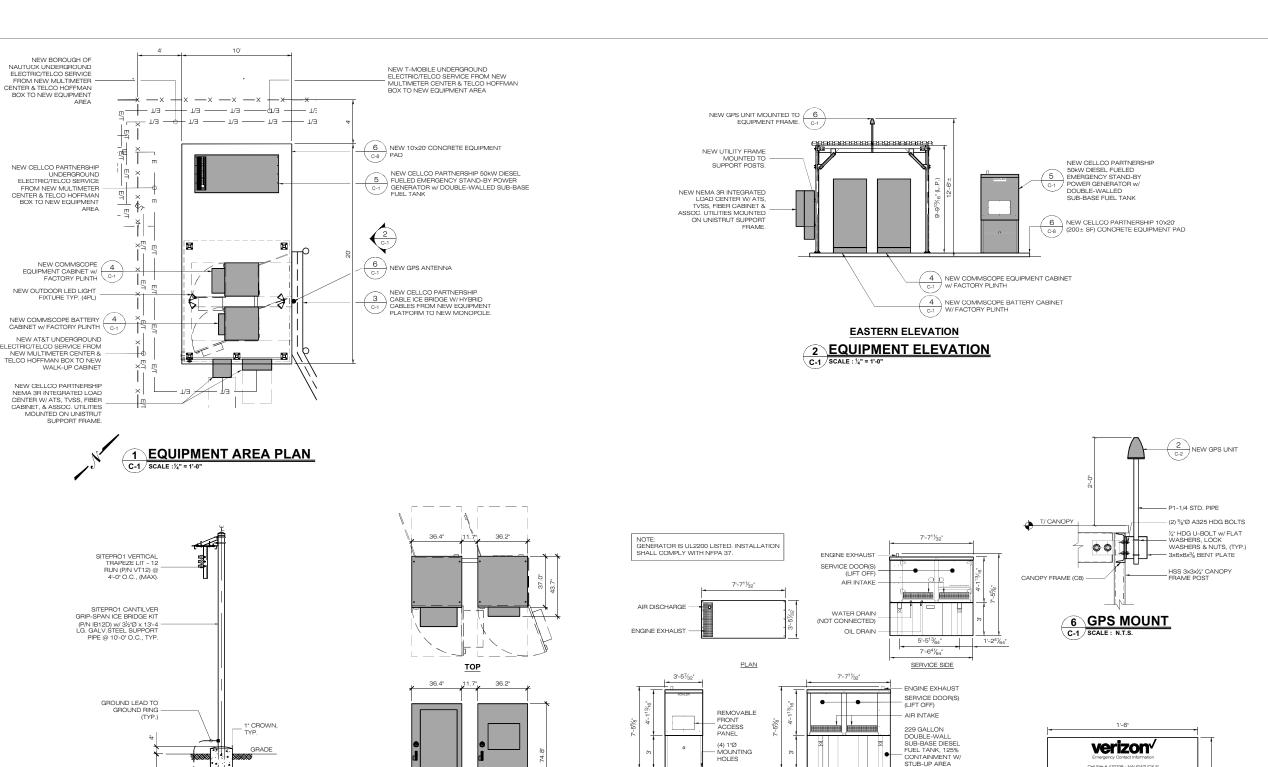














CMC74-36EE OUTDOOR CMC74-36B OUTDOOR

COMMSCOPE

BATTERY CABINET Hx80.8"Wx36.2"Dx43.7

FRONT

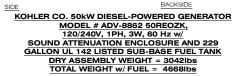
COMMSCOPE

EQUIPMENT CABINET HxWxD= 80.8"x36.4"x51.1"

12'Ø CONCRETE SONOTUBE PIER FOOTING (4000PSI MIN), TYP.

3 ICE BRIDGE DETAIL

C-1 SCALE : ½" = 1'-0"



3'

FACTORY PLINTH, TYP.

3'-431/32"

1'-24'/64' 5'-5'3/64"

7'-641/64

(MODEL #

GM100747-MA4)





TYPICAL 7 EMERGENCY NOTICE C-1 SCALE : N.T.S.



at&t

550 COCHITUATE ROAD FRAMINGHAM, MA 01701

D&M DOCUMENTS NO DATE REVISION 0 09/12/25 FOR REVIEW: RCB 1 10/06/25 CARRIER REVISIONS: RCB 2 10/29/25 ADD TOWER INFO: RCB 3 11/20/25 FINAL: RCB 4 12/02/25 AT&T EQUIP REVISIONS: RCE DESIGN PROFESSIONALS OF RECORD PROF: ROBERT C. BURNS, P.E. COMP: ALL-POINTS TECHNOLOGY CORPORATION, P.C. ADD: 567 VAUXHALL STREET EXT. SUITE 311 WATERFORD, CT 06385

OWNER: BOROUGH OF NAUGATUCK ADDRESS: 229 CHURCH STREET NAUGATUCK, CT 06770

CT1239 CONRAD ST

SITE 161 CONRAD STREET ADDRESS: NAUGATUCK, CT 06770

APT FILING NUMBER: CT752130 DRAWN BY: ELZ

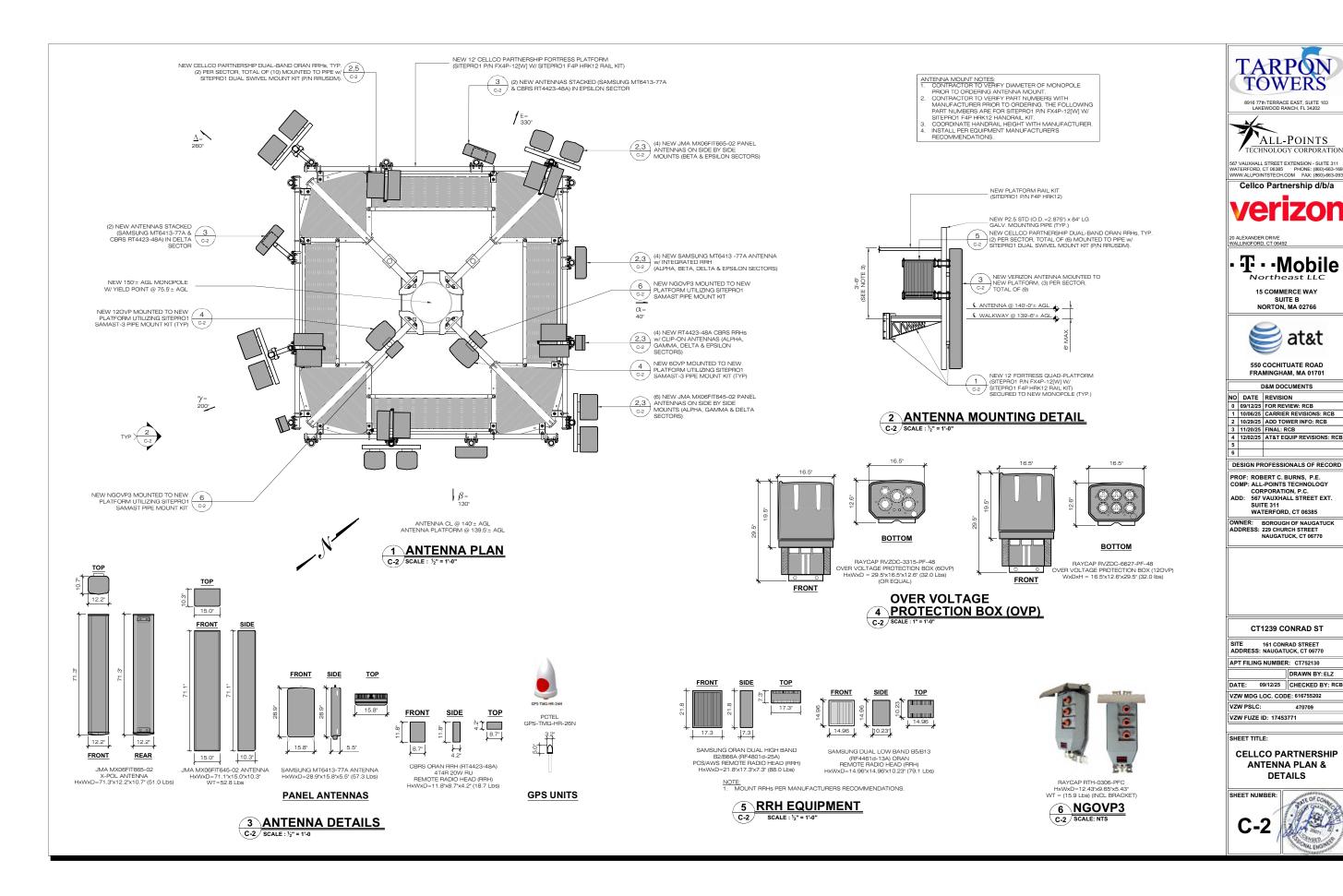
DATE: 09/12/25 CHECKED BY: RCE VZW MDG LOC. CODE: 616755202 VZW PSLC: 470709

VZW FUZE ID: 17453771

CELLCO PARTNERSHIP EQUIPMENT PLAN & DETAILS







8916 77th TERRACE EAST, SUITE 103 LAKEWOOD RANCH, FL 34202

'ALL-POINTS

*r*erizon

Northeast LLC

15 COMMERCE WAY SUITE B NORTON, MA 02766

550 COCHITUATE ROAD

FRAMINGHAM, MA 01701 D&M DOCUMENTS

CT1239 CONRAD ST

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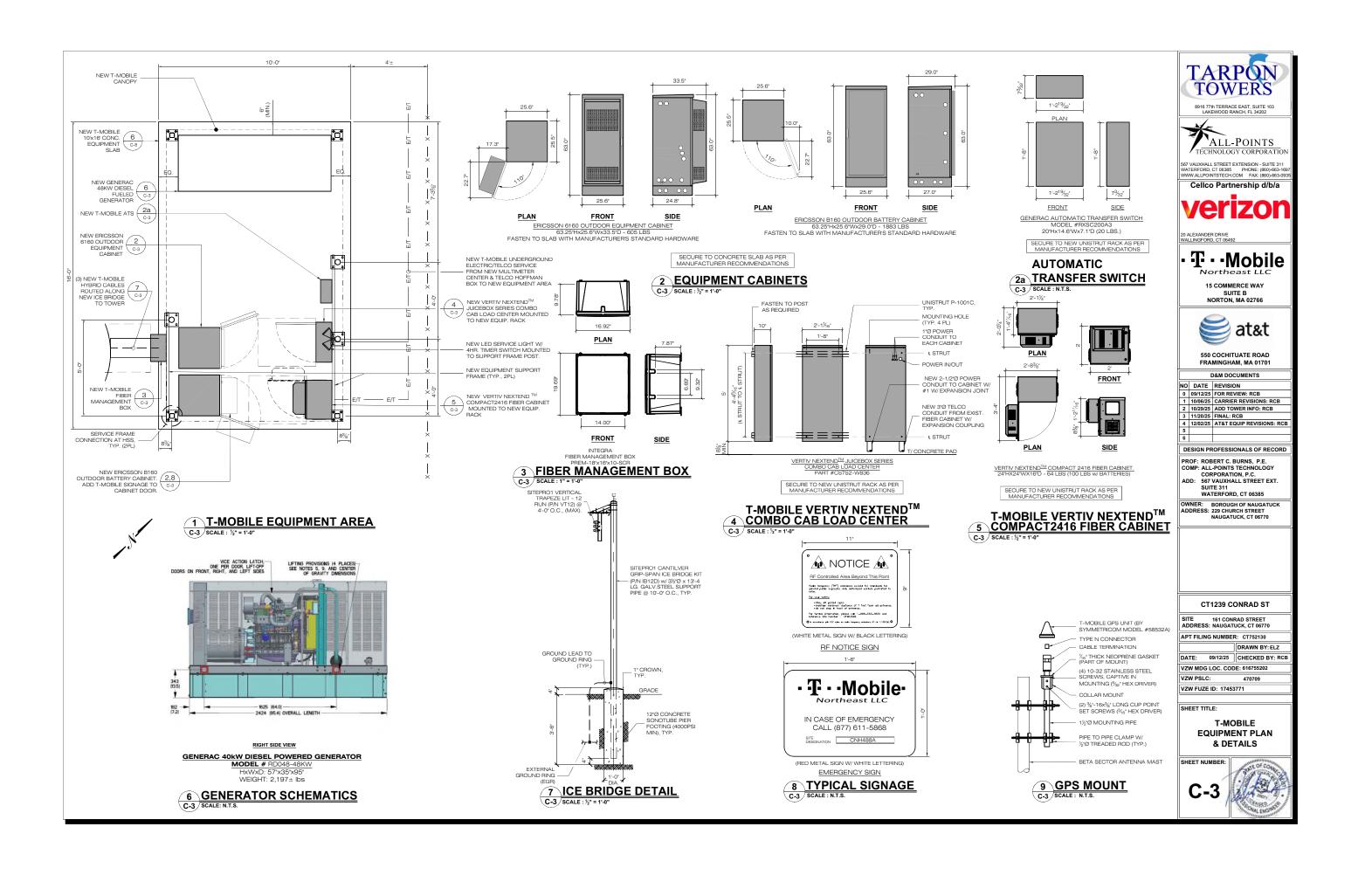
09/12/25 CHECKED BY: RCE

ANTENNA PLAN &

DETAILS

470709

at&t



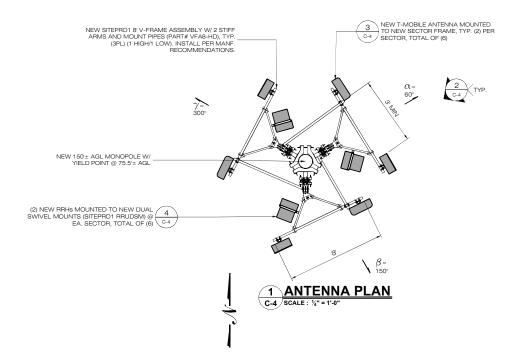


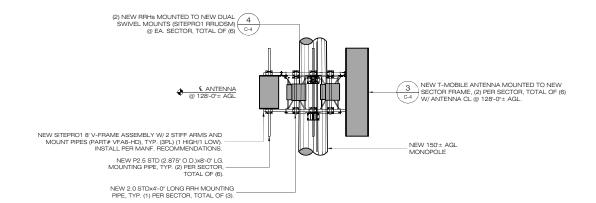
- ANTENNA MOUNT NOTES:

 1. CONTRACTIOR TO VERIEY DIAMETER OF MONOPOLE PRIOR TO ORDERING ANTENNA MOUNT.

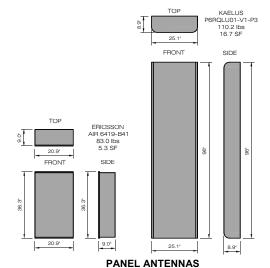
 2. CONTRACTOR TO VERIEY PART NUMBERS WITH MANUFACTURER PRIOR TO ORDERING.

 3. INSTALL PER EQUIPMENT MANUFACTURERS RECOMMENDATIONS.

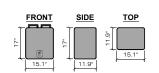




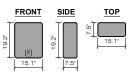
2 ANTENNA MOUNTING DETAIL
C-4 SCALE: 1/4" = 1'-0"



3 ANTENNA DETAIL C-4 SCALE: ½" = 1'-0"



ERICSSON RADIO-4460 B25/B66 (OR EQUAL) REMOTE RADIO UNIT (RRU) WxDxH=15.1"x11.9"x17.0" (108± Lbs)



ERICSSON RADIO-4480 B71/B85 (OR EQUAL) REMOTE RADIO UNIT (RRU) WxDxH=15.1"x7.5"x19.2" (93± Lbs)

4 RRU EQUIPMENT C-4 SCALE : ½" = 1'-0"



8916 77th TERRACE EAST, SUITE 103 LAKEWOOD RANCH, FL 34202



Cellco Partnership d/b/a



• T • • Mobile

15 COMMERCE WAY SUITE B NORTON, MA 02766



550 COCHITUATE ROAD FRAMINGHAM, MA 01701

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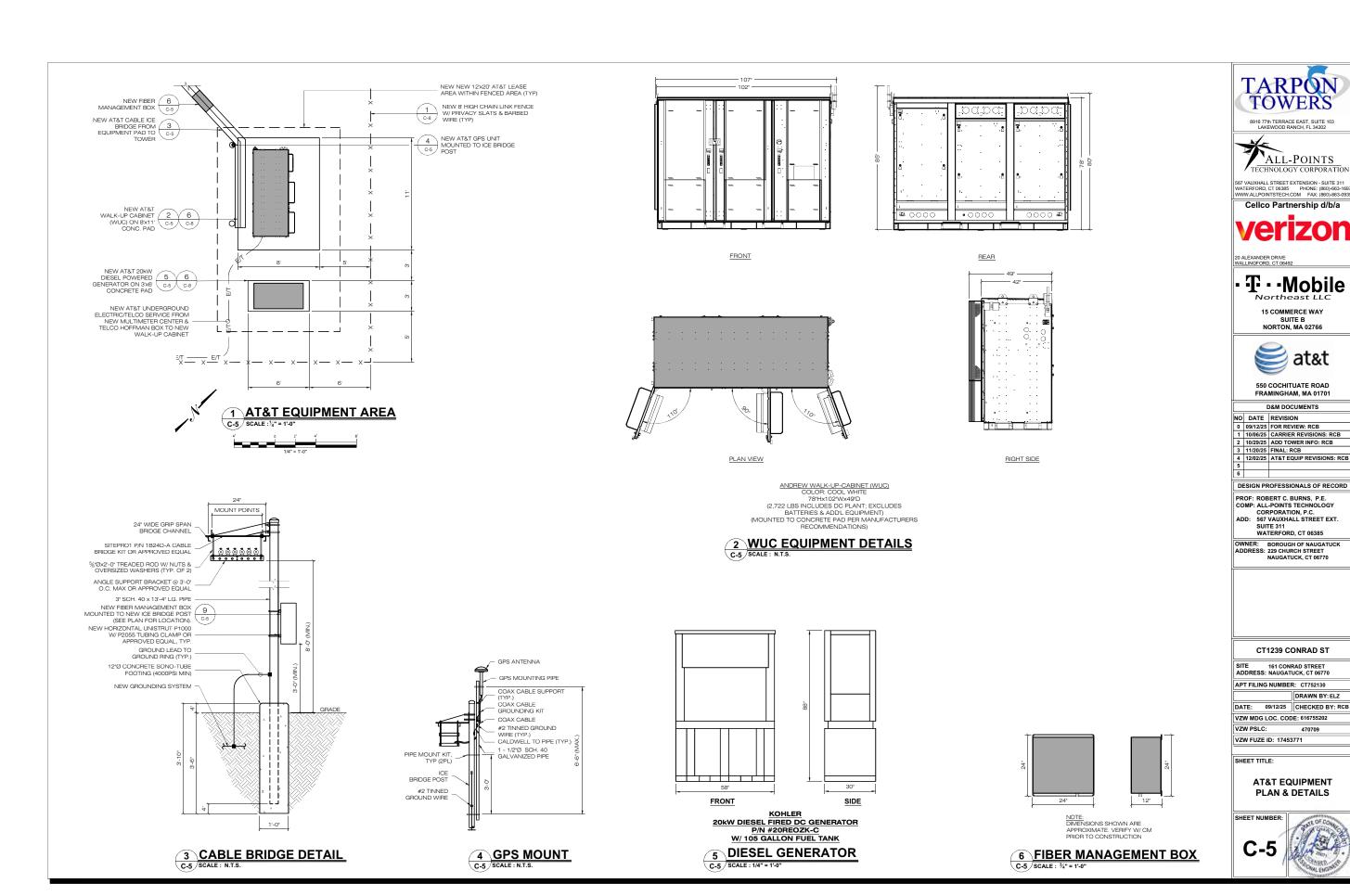
VZW FUZE ID: 17453771

SHEET TITLE:

T-MOBILE **ANTENNA PLAN & DETAILS**

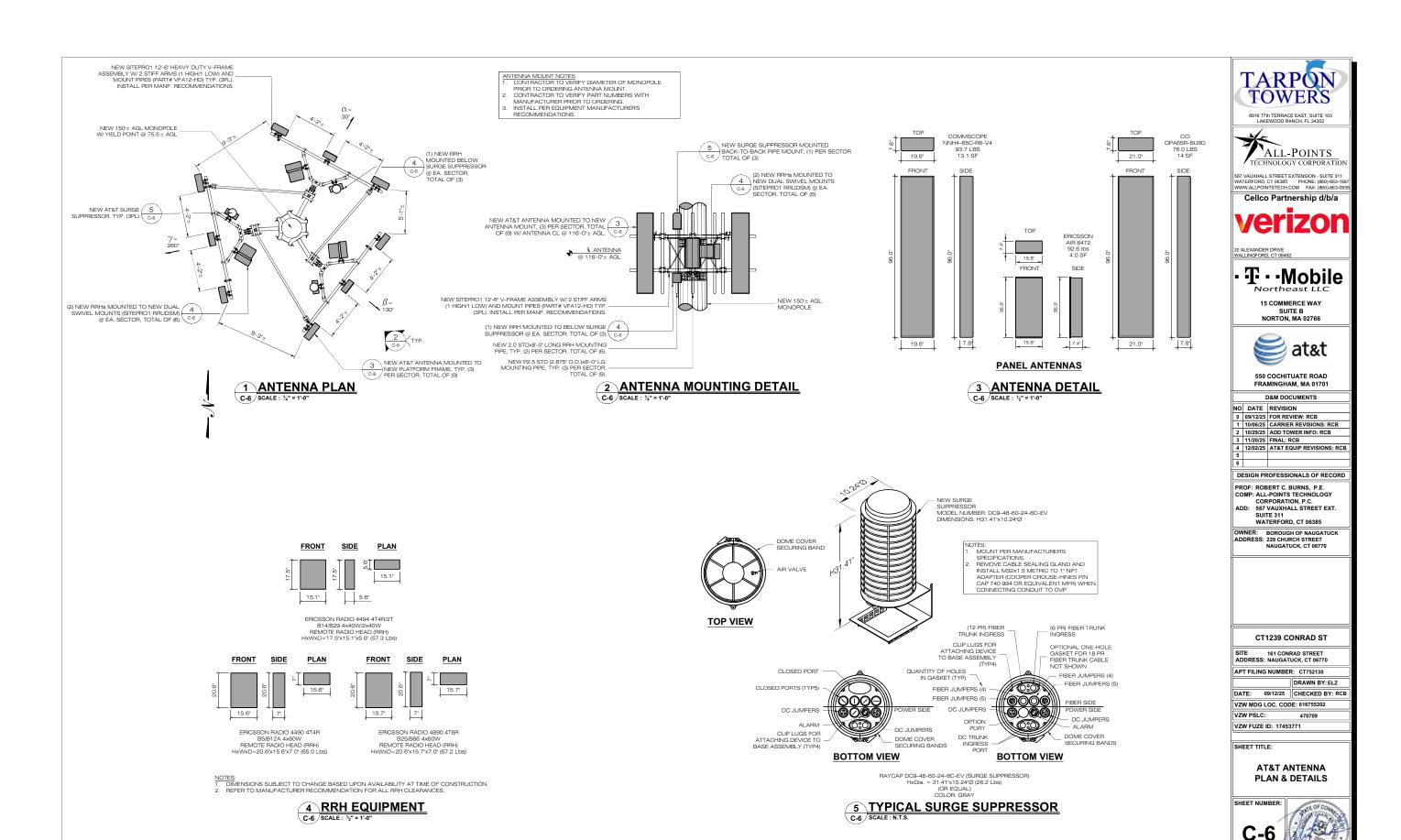
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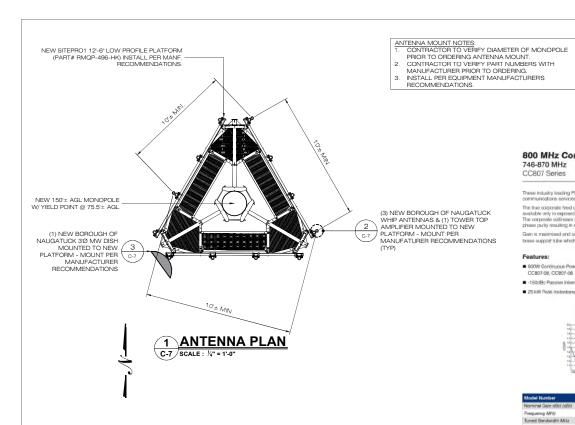




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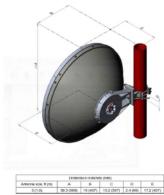
470709





VHLP3-11W-6WH/A

Antenna Dimensions and Mounting Information



Wind Forces at Wind Velocity Survival Ration

will broites at will divelocity 50	rvivarrauriy
Axial Force (FA)	2903 N 652,621 lbf
Angle a for MT Max	D."
Side Force (FS)	1439 N 323.5 lbf
Twisting Moment (MT)	1179 N·m 10,435.029 in
Zog without Ice	135 mm 5.315 in
Zog with 1/2 in (12 mm) Radial Ice	84 mm 3.307 in
Weight with 1/2 in (12 mm) Radial loe	46 kg 101.413 lb

COMMSCOPE"



800 MHz Corporate Collinear Antennas 746-870 MHz



These industry leading PIM and PIP rated collinear arrays allow site operature to combine, with complete integrity, a large number of communications services into a single, low profile collinear antenna array.

- 500W Continuous Power rating for CC807-11,
 Extraordinary bandwidth characteristics with superior partient control
- 25 kW Peak instantaneous (PIP) rating





Electrical Specifications

Model Number	C/C807-03-P	CC807-08-P	CC807-08-P	CC807-11-P
Nominal Gain d'Ed (dEl)	3 (5.1)	6 (8.1)	8 (10.1)	10.5 (12.6)
Frequency MHz		746	- 870	
Tuned Bandwidth MHz		Full	Band	
VSWR (Return Lote)		<1	5:1	
Downtilt* **	Not Offered	II 'Std -3',-5"	0 "Std, -1", -2	2", 3", 4", 5"
Vertical Beamwidth*	24	17	9	4.5
Horizontal Beamwidth ¹		Omni -	/- 0.5dB	
Input Power W	250		500	
Passive IM 3rd order (2x20W) dBc		-	50	
Peak Instantaneous Power KW			5	

Model Number		CC807-03-P	CC807-05-P	CC807-08-P	CC997-11-P
Construction			Sky blue fibre	glass radome	
Length mm (inches)		1202 (47)	1741 (69)	2817 (111)	5219 (205)
Radome Diameter mm (mote	va)		78	(9)	
Weight kg (Mx)		4 (9)	7 (16)	12(27)	22 (40)
Shipping Weight Ag (/be)		8 (18)	11 (26)	18 (40)	30 (66)
	Н		115	(4.9)	
Shipping Dimensions mm (Inches)	W		115	(4.9)	
and (contract)	L	1400 (95)	1900 (76)	3000 (118)	5600 (220)
Termination			4.3-10 to	od řemale	
Suggested Clamps (not include	ied)		2 x U	0-114	
Invertible Mounting			Yes	(1)	
Projected area cm ⁻ (ft ⁻)	No loe	806 (0.9)	1208 (1.4)	2320 (2.6)	4500 (4.9)
Euclieged steel gitt, (tt.)	With Ice	1048 (1.2)	1571 (1.7)	2983 (2.1)	5750 (5.2)
Lateral Thrust @180km/h N (1	00 mph (be)	96 (22)	100 (34)	276 (82)	540 (121)
Wind Gust Rating km/h (mph)	No lice		>240	(>150)	
Torque @ 160km/h Mm (180m	ph tHbs)	20 (15)	73 (54)	278 (20%)	1032 (761)

dbSpectra

700/800 MHz Antenna - Omnidirectional, Low-PIM/Hi-PIP, 8.8 dBd

Specifi	ications
Design Type	True Corporate Feed
Frequency Range	764-869 MHz
Passive Intermodulation – PIM (2 x 20W sources)	-150 dBc, 3rd Order
Bandwidth	105 MHz
Gain (average over BW)	8.8 dBd
Configuration	Single antenna
Beam Tilt (electrical downtilt)	(x) = - , 2, 3, 4, or 6 degrees
Vertical Beamwidth (E-Plane) typ.	6.2"
Impedance	50 ohms
VSWR / Return Loss	1.5:1 / 14 dB (min.)
Average Power Rating	500 W
Peak Instantaneous Power	25 kW
Polarization	Vertical
Lightning Protection	Direct Ground
Connector DS7C09P36U(x)D DS7C09P36U(x)M Equivalent Flat-Plate Area	7/16 DIN (F) 4.3-10 (F) 2.35 sq. ft.
Lateral Windload Thrust @100mph	99 lbf.
Rated Wind Speed	175 mph (without ice) 149 mph (with \s'' radial ice)
Total Length	14.2 feet
Mounting Mast Length	35 inches
Mounting Hardware (included)	DSH3V3N
Mast O.D.	2.5 inches
Radome color	Horizon Blue



Features and Benefits

Tested to stringent Peak Instantaneous Power (PIP) levels of 25 KW using dbSpectra's multi-channel P25 PIP test bed. High PIP level is demanded by today's digital systems.

PIM-rated Design - 3rd-Order performance better than -150 dBc!

Sturdy Construction - Heavy-wall fiberglass radome minimizes tip deflection. Excellent Lightning Protection -

conductor DC ground. **Radiation Patterns:**





Vertical (No Tilt)









Cellco Partnership d/b/a



T--Mobile

15 COMMERCE WAY SUITE B NORTON, MA 02766



550 COCHITUATE ROAD FRAMINGHAM, MA 01701

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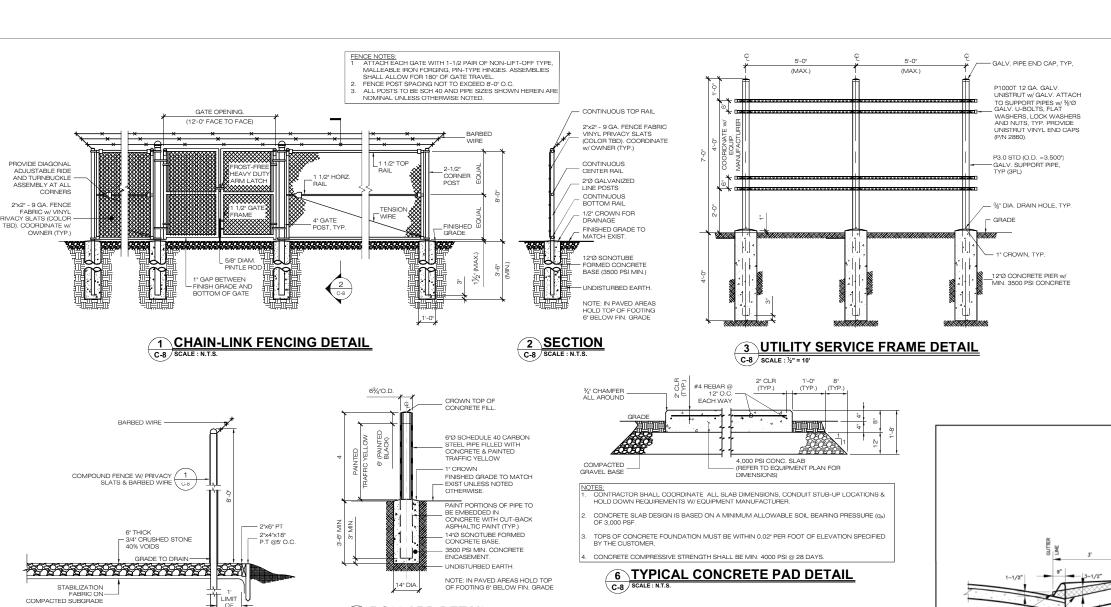
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BOROUGH OF NAUGATUCK **EQUIPMENT PLAN &** DETAILS

SHEET NUMBER:





EXISTING COMPACTED SUBBASE OR GRANULAR FILL

5 BOLLARD DETAIL

4 COMPOUND DETAIL
C-8 SCALE : N.T.S.

BOUGH GRADE

BENCHING REQUIRED IF NATURAL SLOPE IS GREATER THAN 1:4

EXISTING GRADE

12' MIN

7 TYPICAL ACCESS DWY CROSS SECTION C.8 SCALE: N.T.S.

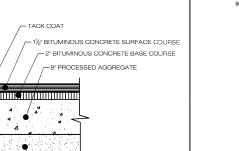
GEOTEXTILE 150 MIL THICK

GRADE ATER REMOVAL OF TOPSOIL

* CROSS SLOPE GRADE SHALL BE 1-2% AS SHOWN ON PROPOSED GRADING

* WHERE CUT OR FILL EMBANKMENTS ARE STEEPER THAN 3:1 USE A STAPLED IN PLACE, BIODEGRADABLE EROSION CONTROL BLANKET OR A BONDED FIBER MATRIX HYDROSEED APPLICATION.

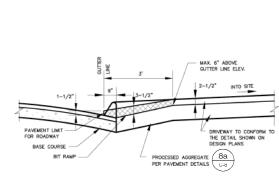
SCARIFY AND COMPACT TOP 6" OF EXISTING



EXIST. COMPACTED SUBBASE OR GRANULAR FILL

. SUBBASE MAY CONSIST OF NATIVE MATERIALS IF FOUND ACCEPTABLE BY THE ENGINEER. SUBBASE TO BE COMPACTED TO

SUBBASE IS TO BE FREE FROM DEBRIS AND UNSUITABLE MATERIALS.



JGH OF NAUQ.	BITUMINOUS CONCRETE	Scale: NTS
SOUGH OF MAUG VILL	DRIVEWAY APRON	Drawing No.
and have	BOROUGH OF NAUGATUCK	SD-24
OFFICE AND THE	ENGINEERING DEPARTMENT	
CONVECTION	STANDARD DETAIL 229 Church Street, Naugatuck, CT 06770 www.naugatuck-ct.gov	Date:10/2011
	229 Charch Street, Madgatack, CT 00770 WWW.hadgatack-ct.gov	

PAVED DRIVEWAY APRON SECTION



DRAWN BY: ELZ

09/12/25 CHECKED BY: RCE

470709

8916 77th TERRACE EAST, SUITE 103

'ALL-POINTS

Cellco Partnership d/b/a

*r*erizon

 \mathbf{T} - Mobile

Northeast LLC

15 COMMERCE WAY

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D&M DOCUMENTS

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VZW PSLC:

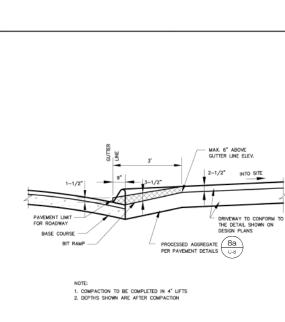
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NO DATE REVISION

0 09/12/25 FOR REVIEW: RCB

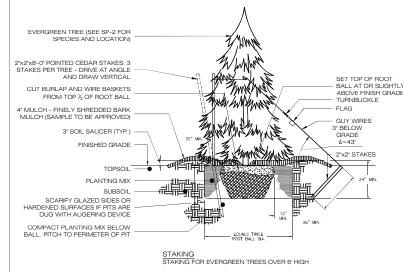
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EROSION CONTROL NOTES

- TRACTOR SHALL CONSTRUCT ALL SEDIMENT AND EROSION CONTROLS IN ACCORDANCE WITH THE 2024 CONNECTICUT GUIDELINES
 EROSION AND SEDIMENT CONTROL LATEST EDITION. IN ACCORDANCE WITH THE CONTRACT DOOR MENTS, AND AS DIRECTED BY THE BOROUGH OF NAUGATUCK AND/OR PERMITTEE. ALL PERMITTET SEDIMENTATION AND EROSION CONTROL MEASURES SHALL BE INSTALLE! PRIOR TO THE START OF CLEARING AND GRUBBING AND DEMOLITION OPERATIONS.
- THESE DRAWINGS ARE ONLY INTENDED TO DESCRIBE THE SEDIMENT AND EROSION CONTROL MEASURES FOR THIS SITE. SEE CONSTRUCTION SEQUENCE FOR ADDITIONAL INFORMATION. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHOWN ON THE EROSION A SEDIMENT CONTROL MEASURES SHOWN ON THE EROSION AS SEDIMENT CONTROL PLAN ARE SHOWN AS RECUIRED BY THE ROSINERE THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT ALL BROSION CONTROL MEASURES ARE CONFIGURED AND CONSTRUCTED IN A MANNER THAT WILL MINIMIZE EROSION OF SOLD AND PREVENT THAT TRANSPORT OF SEDIMENTS AND OTHER POLLUTANTS TO STORM DRANAGE SYSTEMS AND/OR WATERCOURSES. ACTUAL SITE CONDITIONS OR SEASONAL AND CLIMATIC CONDITIONS ON YEARANT ADDITIONAL CONTROLS OF ONE METAL STATE CONDITIONS OR SEASONAL AND CLIMATIC CONDITIONS WARRANT ADDITIONAL CONTROLS OR CONSIGNATIONS, AS RESULTED, AND AS DIRECTED BY THE PERMITTEE AND/OR SWPCP MONITOR. REFER TO SITE PLAN FOR GENERAL INFORMATION AND OTHER CONTRACT PLANS FOR APPROPRIATE WERDMATCH.
- A BOND OR LETTER OF CREDIT MAY BE REQUIRED TO BE POSTED WITH THE GOVERNING AUTHORITY FOR THE EROSION CONTROL INSTALLATION
- THE CONTRACTOR SHALL APPLY THE MINIMUM EROSION & SEDIMENT CONTROL MEASURES SHOWN ON THE PLAN IN CONJUNCTION WITH CONSTRUCTION SEQUENCING, SUCH THAT ALL ACTIVE WORK ZONES ARE PROTECTED. ADDITIONAL ANJOOR ALTERNATIVE SEDIMENT AND EROSION CONTROL MEASURES MAY BE INSTALLED DURING THE CONSTRUCTION PERIOD IF FOUND NECESSARY BY THE CONTRACTOR, GWNER, SITE ENGINEER, MUNICIPAL OFFICIALS, OR ANY GOVERNING AGENCY. THE CONTRACTOR SHALL CONTRACT THE OWNER AND APPROPRIATE GOVERNING AGENCY.
- 5. THE CONTRACTOR SHALL TAKE EXTREME CARE DURING CONSTRUCTION SO AS NOT TO DISTURB UNPROTECTED WETLAND AREAS OR INSTALLED SEDIMENTATION AND EROSION CONTROL MEASURES. THE CONTRACTOR SHALL INSPECT ALL SEDIMENT AND EROSION CONTROLS WEEKLY AND WITHIN 24 HOURS OF A STORM WITH A RAINFALL AMOUNT OF 0.25 INCHES OR GREATER TO VERIFY THAT THE CONTROLS ARE OPERATING PROPERLY AND MAKE REPAIRS AS NECESSARY IN A TIMELY MANOR.
- THE CONTRACTOR SHALL KEEP A SUPPLY OF EROSION CONTROL MATERIAL (SILT FENCE, COMPOST FILTER SOCK, EROSION CONTROL BLANKET, ETC.) ON-SITE FOR PERIODIC MAINTENANCE AND EMERGENCY REPAIRS.
- ALL FILL MATERIAL PLACED ADJACENT TO ANY WETLAND AREA SHALL BE GOOD QUALITY, WITH LESS THAN 5% FINES PASSING THROUGH A #200 SIEVE (BANK RIUN), SHALL BE PLACED IN MAXIMUM ONE FOOT LIFTS, AND SHALL BE COMPACTED TO 95% MAX. DRY DENSITY MODIFIED PROCTOR OR AS SPECIFIED IN THE CONTINACT SPECIFICATIONS.
- CONSTRUCTION ENTRANCES (ANTI-TRACKING PADS) SHALL BE INSTALLED PRIOR TO ANY SITE EXCAVATION OR CONSTRUCTION ACTIVITY AND SHALL BE MAINTAINED THROUGHOUT THE DURATION OF ALL CONSTRUCTION IF REQUIRED. THE LOCATION OF THE TRACKING PADS MAY CHANGE AS VARIOUS PHASES OF CONSTRUCTION ARE COMPLETED. CONTRACTOR SHALL ENSURE THAT ALL VEHICLES EXTING THE SITE ARE PASSING OVER THE ANTI-TRACKING PADS PRIOR TO EXISTING.
- 10. ALL CONSTRUCTION SHALL BE CONTAINED WITHIN THE LIMIT OF DISTURBANCE, WHICH SHALL BE MARKED WITH SILT FENCE, SAFETY FENCE, HAY BALES, RIBBONS, OR OTHER MEANS PRIOR TO CLEARING. CONSTRUCTION ACTIVITY SHALL REMAIN ON THE UPHILL SIDE OF THE SEDIMENT BARRIER UNLESS WORK IS SPECIFICALLY CALLED FOR ON THE DOWNHILL SIDE OF THE BARRIER.
- NO CUT OR FILL SLOPES SHALL EXCEED 2:1 EXCEPT WHERE STABILIZED BY ROCK FACED EMBANKMENTS OR EROSION CONTROL BLANKETS. ALL SLOPES SHALL BE SEEDED AND BANKS WILL BE STABILIZED IMMEDIATELY UPON COMPLETION OF FINAL GRADING UNTIL TURF IS ESTABLISHED.
- DIRECT ALL DEWATERING PUMP DISCHARGE TO A SEDIMENT CONTROL DEVICE CONFORMING TO THE GUIDELINES WITHIN THE APPROVED OF DISTURBANCE IP REQUIRED. DISCHARGE TO STORM DRAINS OR SURFACE WATERS FROM SEDIMENT CONTROLS SHALL BE CLEAR AND APPROVED BY THE FERMITTEE OR MUNICIPALITY.
- THE CONTRACTOR SHALL MAINTAIN A CLEAN CONSTRUCTION SITE AND SHALL NOT ALLOW THE ACCUMULATION OF RUBBISH OR CONSTRUCTION DEBRIS ON THE SITE. PROPER SANITARY DEVICES SHALL BE MAINTAINED ON-SITE AT ALL TIMES AND SECURED AF THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO AVIO THE SPILLAGE OF FUEL OR OTHER POLLUTAINS ON THE CONSTRUCTION SITE AND SHALL ADHERE TO ALL APPLICABLE POLICIES AND REGULATIONS RELATED TO SPILL PREVENTION AND DESCRIPCIONS TO SHALL ADDRESS ON SECONDAL STATEMENT OF THE PROPERTY OF THE STATEMENT OF THE STA
- 14. MINIMIZE LAND DISTURBANCES. SEED AND MULCH DISTURBED AREAS WITH TEMPORARY MIX AS SOON AS PRACTICABLE (2 WEEK MAXIMUM UNSTABILIZED PERIOD) USING PERENNIAL RYEGRASS AT 40 LBS PER AGRE. MULCH ALL CLIT AND FILL SLOPES AND SWALES WITH LOOSE HAY A RATE OF 2 TONS PER AGRE. IF NECESSARY, REPLACE LOOSE HAY ON SLOPES WITH EROSION CONTROL BLANKETS OR JUTE CLOTH. MODERATELY GRADED AREAS, ISLANDS, AND TEMPORARY CONSTRUCTION STAGING AREAS MAY BE HYDROSEEDED WITH TACKFIER.
- 5. SWEEP AFFECTED PORTIONS OF OFF SITE ROADS ONE OR MORE TIMES A DAY (OR LESS FREQUENTLY IF TRACKING IS NOT A PROBLEM) DURING CONSTRUCTION. FOR DUST CONTROL, PERIODICALLY MOISTEN EXPOSED SOIL SURFACES WITH WATER ON UNPAYED TRAVELWAYS TO KEEP THE TRAVELWAYS DAMP. CALCIUM CHLORIDE MAY ALSO BE APPLIED TO ACCESS ROADS. DUMP TRUCK LOADS EXTING THE SITE SHALL BE COVERED.
- VEGETATIVE ESTABLISHMENT SHALL OCCUR ON ALL DISTURBED SOIL, UNLESS THE AREA IS UNDER ACTIVE CONSTRUCTION, IT IS STONE OR SCHEDULED FOR PAVING WITHIN 30 DAYS. TEMPORARY SEEDING OR NON-LIVING SOIL PROTECTION OF ALL EMOSED SLOPES SHALL BE INITIATED WITHIN THE FIRST 7 DAYS OF SUSPENDING WORK IN AREAS TO BE LEFT LONGENT THAN 30 DAYS.
- 7. MAINTAIN ALL PERMANENT AND TEMPORARY SEDIMENT CONTROL DEVICES IN EFFECTIVE CONDITION THROUGHOUT THE CONSTRUCTIC PERIOD. UPON COMPLETION OF WORK SWEEP CONCRETE PADS, CLEAN THE STORMWATER MANAGEMENT SYSTEMS AND REMOVE ALL TEMPORARY SEDIMENT CONTROLS ONCE THE SITE IS FULLY STABILIZED AND APPROVAL HAS BEEN RECEIVED FROM PERMITTEE OR THE MINICIPALITY.
- 18. SEEDING MIXTURES SHALL BE NEW ENGLAND SEMI-SHADE GRASS AND FORBS MIX, OR APPROVED EQUAL BY OWNER



1 EVERGREEN TREE PLANTING

SEDIMENT & EROSION CONTROL NARRATIVE

- THE PROJECT INCLUDES THE INSTALLATION OF A 150'± AGL MONOPOLE WITH ASSOCIATED GROUND MOUNTED EQUIPMENT. ALL DISTURBED AREAS ARE TO BE SEEDED AND STABILIZED PRIOR TO THE INSTALLATION OF THE PROPOSED EQUIPMENT.
- OPOSED PROJECT INVOLVES THE FOLLOWING CONSTRUCTION
- A. CONSTRUCTION OF 150 ± AGL MONOPOLE.

 C. CONSTRUCTION OF A 41x88' (2,788 ± SF) FENCED EQUIPMENT COMPOUND W/ GRAVEL SURFACE TREATMENT AND ASSOCIATED LITTLETE.
- ASSICOLA IEU UIIUTIES. CONSTRUCTION OF A 55±12 WIDE GRAVFIL ACCESS DRIVE. CONSTRUCTION OF CONORETE EQUIPMENT PADS W. EQUIPMENT CABINETS & DIESEL FIRED GENERATORS. THE STABILIZATION OF PERVICUOS DISTURBEDS AREAS WITH PERMANENT GRASS TREATMENTS.
- 2. FOR THIS PROJECT, THERE ARE APPROXIMATELY 8.850± SF OF THE SITE BEING DISTURBED.
- A GEOTECHNICAL ENGINEERING REPORT BY DELTA OAKS GROUP DATED OCTOBER 14, 2024 HAS BEEN COMPLETED FOR THIS
 PROJECT IS AVAILABLE UNDER SEPARATE COVER.
- 4. IT IS ANTICIPATED THAT CONSTRUCTION WILL BE COMPLETED IN APPROXIMATELY 12 WEEKS
- REFER TO THE CONSTRUCTION SEQUENCING AND EROSION AND SEDIMENTATION NOTES FOR INFORMATION REGARDING SEQUENCING OF MAJOR OPERATIONS IN THE ON-SITE CONSTRUCTION PHASES.
- 6. MEASURES ARE BASED UPON ENGINEERING PRACTICE, JUDGEMENT AND THE APPLICABLE SECTIONS OF THE 2024 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL.
- DETAILS FOR THE TYPICAL EROSION AND SEDIMENTATION MEASURES ARE SHOWN ON PLAN SHEET C-1 OR PROVIDED AS SEPARATE SUPPORT DOCUMENTATION FOR REVIEW IN THIS PLAN.
- ERVATION PRACTICES TO BE USED DURING CONSTRUCTION AREA

 - INSERVATION PRACTICES TO BE USED DURING CONSTRUCTION.

 STAGED CONSTRUCTION.

 MININIZE THE DISTURBED AREAS DURING CONSTRUCTION;

 STABILIZE DISTURBED AREAS AS SOON AS POSSIBLE WITH TEMPORARY OR PERMANENT MEASURES;

 MININIZE IMPERIVIOUS AREAS.

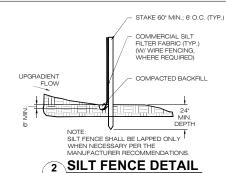
 UTILIZE APPORIALTE CONSTRUCTION EROSION AND SEDIMENTATION MEASURES.

SUGGESTED CONSTRUCTION SEQUENCE

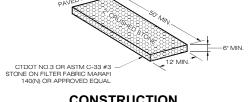
THE FOLLOWING SUGGESTED SEQUENCE OF CONSTRUCTION ACTIVITIES IS PROJECTED BASED UPON THE POLLUVING SUGGESTED SEQUENCE OF COURS INDICITION ACTIVITIES IS PROJECTED BRISED OFFOR ENABLEBRING JUDGEMENT AND BEST MANAGEMENT PRACTICES. THE CONTRACTOR MAY ELECT TO ALTER THE SEQUENCING TO BEST MEET THE CONSTRUCTION SCHEDULE, THE EXISTING SITE ACTIVITIES AND WEATHER CONDITIONS. THE CONTRACTOR SHALL SUBMIT THE FINAL CONSTRUCTION SCHEDULE TO THE PROJECT ENGINEER FOR REVIEW AND APPROVAL PRIOR TO MOBILIZING TO THE SITE AND INITIATING CONSTRUCTION ACTIVITIES. THE CONSTRUCTION SEQUENCE IS ALSO SUBJECT TO REQUIREMENTS AS NOTED IN THE RESOURCE PROTECTION MEASURES.

- CONTACT THE OWNER TO SCHEDULE A PRE-CONSTRUCTION MEETING. PHYSICALLY FLAG THE TREES TO BE REMOVED IN THE FIELD AS NECESSARY TO FACILITATE THE PRE-CONSTRUCTION MEETING.
- CONDUCT A PRE-CONSTRUCTION MEETING TO DISCUSS THE PROPOSED WORK AND EROSION AND SEDIMENTATION CONTROL MEASURES. THE MEETING SHOULD BE ATTENDED BY THE OWNER, THE OWNER REPRESENTATIVE(S), THE GENERAL CONTRACTOR, DESIGNATED SUB-CONTRACTORS AND THE PERSON, OR PERSONS, RESPONSIBLE FOR THE IMPLEMENTATION, OPERATION, MONITORING AND MAINTENANCE OF THE EROSION AND SEDIMENTATION MEASURES. THE CONSTRUCTION PROCEDURES FOR THE ENTIRE PROJECT SHALL BE REVIEWED AT THIS DESIGNATION.
- NOTIFY THE OWNER AT LEAST FORTY-EIGHT (48) HOURS PRIOR TO COMMENCEMENT OF ANY DEMOLITION, CONSTRUCTION OR REGULATED ACTIVITY ON THIS PROJECT. NOTIFY CALL BEFORE YOU DIG CONNECTICUT AT (800) 922-4455.
- CLEAR AND GRUB AS REQUIRED, TO INSTALL THE PERIMETER EROSION AND SEDIMENTATION CONTROL MEASURES AND, IF APPLICABLE, TREE
- 5. INSTALL CONSTRUCTION ENTRANCE.
- PERFORM THE REMAINING CLEARING AND GRUBBING AS NECESSARY. REMOVE CUT WOOD AND STUMPS. CHIP BRUSH AND STOCKPILE FOR FUTURE USE OR REMOVE OFF-SITE. REMOVE AND DISPOSE OF DEMOLITION DEBRIS OFF-SITE.
- 7. TEMPORARILY SEED DISTURBED AREAS NOT UNDER CONSTRUCTION FOR THIRTY (30) DAYS OR MORE
- 8. EXCAVATE AND ROUGH GRADE NEW ACCESS DRIVE
- 9. INSTALL ADDITIONAL EROSION AND SEDIMENTATION CONTROL MEASURES AS DICTATED BY SITE CONDITIONS IN ORDER TO PROPERLY CONTROL AND TREAT RUNOFF.
- 11. EXCAVATE AND ROUGH GRADE EQUIPMENT COMPOUND.
- 12. EXCAVATE FOR TOWER FOUNDATION & EQUIPMENT PADS.
- 13. FINALIZE ACCESS ROAD GRADES
- 14. INSTALL PAVED DRIVEWAY APRON & GRAVEL SURFACE ON THE ACCESS DRIVEWAY
- 15. PREPARE SUBGRADE AND INSTALL FORMS, STEEL REINFORCING, & CONCRETE FOR TOWER FOUNDATION & EQUIPMENT PADS
- 16. INSTALL BURIED GROUND RINGS, GROUND RODS, GROUND LEADS, & UTILITY EQUIPMENT.
- 17 BACKELL TOWER FOUNDATION
- 19. INSTALL TELECOMMUNICATIONS EQUIPMENT ON TOWER & COMPOUND.
- 20. INSTALL COMPOUND GRAVEL SURFACES.
- 22. CONNECT GROUNDING LEADS & LIGHTNING PROTECTION.
- 23. FINAL GRADE AROUND COMPOUND.
- 25. LOAM & SEED DISTURBED AREAS OUTSIDE COMPOUND, AS REQUIRED
- 26. TEST ALL NEW EQUIPMENT.
- 27. AFTER THE SITE IS STABILIZED AND WITH THE APPROVAL OF THE OWNER, REMOVE PERIMETER EROSION AND SEDIMENTATION CONTROLS
- 28. PERFORM FINAL PROJECT CLEANUP.

NOTE: CONSTRUCTION OF THE FACILITY WILL ONLY TAKE PLACE BETWEEN THE HOURS OF 8:00 AM AND 5:30 PM, MONDAY THROUGH SATURDAY



EC-1 SCALE : N.T.S.



CONSTRUCTION **3 ENTRANCE DETAIL**

CONSTRUCTION OPERATION AND MAINTENANCE PLAN - BY CONTRACTOR

INSPECTION SCHEDULE

DAILY & WITHIN 24 HOURS OF RAINFALL > 0.2"

CONSTRUCTION ENTRANCE	DAILT	THE STONE. CLEAN PAVED SURFACES OF TRACKED SEDIMENT.
HAY BALES	WEEKLY & WITHIN 24 HOURS OF RAINFALL > 0.2'	REPAIR/REPLACE WHEN FAILURE, OR OBSERVED DETERIORATION, IS OBSERREMOVE SILT WHEN IT REACHES 1/2 THE HEIGHT OF THE BALE.
SILT FENCE/FILTER SOCKS	WEEKLY & WITHIN 24 HOURS OF RAINFALL > 0.2'	REPAIR/REPLACE WHEN FAILURE, OR OBSERVED DETERIORATION, IS OBSERREMOVE SILT WHEN IT REACHES 1/2 THE HEIGHT OF THE FENCE.
SILT SACKS	WEEKLY & WITHIN 24 HOURS OF RAINFALL > 0.2'	REPAIR/REPLACE WHEN FAILURE, OR OBSERVED DETERIORATION, IS OBSERREMOVE SILT WHEN IT REACHES 1/2 THE HEIGHT OF THE SACK.
TOPSOIL/BORROW STOCKPILES	DAILY	REPAIR/REPLACE SEDIMENT BARRIERS AS NECESSARY.
WATER BARS	DAILY	REPAIR/RESHAPE AS NECESSARY. REMOVE SILT WHEN IT REACHES 1/2 THE HEIGHT OF THE WATER BAR.

TEMPORARY SEDIMENT TRAPS/BASINS WEEKLY & WITHIN 24 HOURS OF RAINFALL > 0.2 REMOVE SEDIMENT WHEN IT REACHES 1/2 OF THE MINIMUM REQUIRED WET

MAINTENANCE REQUIRED

REPAIR/RESHAPE AS NECESSARY. REVIEW CONDITIONS IF REPETITIVE FAILURE

TEMPORARY SOIL PROTECTION WEEKLY & WITHIN 24 HOURS OF RAINFALL > 0.2" REPAIR ERODED OR BARE AREAS IMMEDIATELY. RESEED AND MULCH

ENVIRONMENTAL NOTES RESOURCES PROTECTION MEASURES

TEMPORARY DIVERSION DITCHES

E&S MEASURE

THE PROPOSED FACILITY IS LOCATED WITHIN SENSITIVE HABITAT POTENTIALLY LISED BY TRICOLORED BAT ("TCR": PERIMYOTIS SUBELAVUS). A FEDERALLY PROPOSED ENDANGERED AND STATE ENDANGERED SPECIES. IN ORDER TO PROTECT THIS BAT SPECIES AND PREVENT INCIDENTAL TAKE, PROTECTION MEASURES ARE PROPOSED DURING CONSTRUCTION OF THE FACILITY.

IT IS OF THE UTMOST IMPORTANCE THAT THE CONTRACTOR COMPLIES WITH THE REQUIREMENT FOR IMPLEMENTATION OF THESE PROTECTIVE MEASURES AND THE EDUCATION OF ITS EMPLOYEES AND SUBCONTRACTORS PERFORMING WORK ON THE PROJECT SITE.

ALL-POINTS TECHNOLOGY CORPORATION, P.C. ("APT") WILL SERVE AS THE ENVIRONMENTAL MONITOR FOR THIS PROJECT TO ENSURE THAT THESE PROTECTION AND CONSERVATION MEASURES ARE IMPLEMENTED PROPERLY. APT WILL PROVIDE AN EDUCATION SESSION FOR THE CONTRACTOR PRIOR TO THE STATE OF CONSTRUCTION ACTIVITIES ON IN THE POTENTIAL PRESENCE OF TOE. THE CONTRACTOR SHALL CONTRACTOR SHA

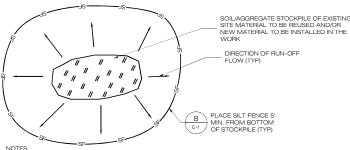
THIS PROTECTION PROGRAM CONSISTS OF SEVERAL COMPONENTS: EDUCATION OF ALL CONTRACTORS AND SUB-CONTRACTORS PRIOR TO INITIATION OF WORK ON THE SITE; PROTECTIVE MEASURES; PERIODIC INSPECTION OF THE CONSTRUCTION PROJECT; AND, REPORTING, DETAILS OF THE TOB PROTECTION MEASURES TO BE IMPLEMENTED IN ASSOCIATION WITH CONSTRUCTION AND OPERATION OF THE FACILITY ARE PROVIDED BELOW.

- a. PRIOR TO WORK ON SITE, THE CONTRACTOR SHALL ATTEND AN EDUCATIONAL SESSION AT THE PRE-CONSTRUCTION MEETING WITH APT. THIS ORIENTATION AND EDUCATIONAL SESSION WILL CONSIST OF AN INTRODUCTORY MEETING WITH APT TO EMPHASIZE THE ENVIRONMENTALLY SENSITIVE NATURE OF THE PROJECT, THE RARE SPECIES RESOURCES, AND THE REQUIREMENT TO DILIGENTLY FOLLOW THE PROTECTIVE MEASURES AS DESCRIBED IN SECTIONS BELOW.
- . THE CONTRACTOR WILL BE PROVIDED WITH CELL PHONE AND EMAIL CONTACTS FOR APT PERSONNEL TO IMMEDIATELY REPORT ANY ENCOUNTERS WITH ANY RARE SPECIES. EDUCATIONAL POSTER MATERIALS WILL BE PROVIDED BY APT AND DISPLAYED ON THE JOB SITE TO MAINTAIN WORKER AWARENESS AS THE PROJECT PROGRESSES.
- c. IF ANY RARE SPECIES ARE ENCOUNTERED, THE CONTRACTOR SHALL IMMEDIATELY CEASE ALL WORK, AVOID ANY DISTURBANCE TO

2. BAT HABITAT - TREE CLEARING RESTRICTION

a. A TIME OF YEAR RESTRICTION ("TOYR") FOR TREE CLEARING RESTRICTS TREE REMOVAL TO OCCUR ONLY BETWEEN AUGUST 16TH THROUGH MAY 31ST, DURING THE TOBS INACTIVE SEASON, WHEN TOB WOULD LIKELY NOT BE PRESENT IN FORESTED HABITAT ON THE SUBJECT PROPERTY. DO NOT REMOVE TREES BETWEEN JUNE 1ST THROUGH AUGUST 15TH.

- a. A COMPLIANCE MONITORING REPORT (BRIEF NARRATIVE AND APPLICABLE PHOTOS) DOCUMENTING APT INSPECTION VERIFYING TOYR FOR TREE REMOVAL WAS ADHERED TO WILL BE SUBMITTED BY APT TO THE PERMITTEE FOR COMPLIANCE VERIFICATION. ANY OBSERVATIONS OF BATS WILL BE INCLUDED IN THE REPORTS.
- b. FOLLOWING COMPLETION OF THE CONSTRUCTION PROJECT, APT WILL PROVIDE A FINAL COMPLIANCE MONITORING REPORT TO THE PERMITTEE DOCUMENTING IMPLEMENTATION OF THIS TOB PROTECTION PROGRAM AND ANY SPECIES OBSERVATIONS. THE PERMITTEE SHALL PROVIDE A COPY OF THE FINAL COMPLIANCE MONITORING REPORT TO THE CONNECTICUT SITING COUNCIL FOR COMPLIANCE
- c. ANY OBSERVATIONS OF RARE SPECIES WILL BE REPORTED TO DEEP BY APT ON THE APPROPRIATE SPECIAL ANIMAL REPORTING FORM, WITH PHOTO-DOCUMENTATION (IF POSSIBLE) AND SPECIFIC INFORMATION ON THE LOCATION AND DISPOSITION OF THE ANIMA



ALL EXISTING EXCAVATED MATERIAL THAT IS NOT TO BE REUSED IN THE WORK IS TO BE IMMEDIATELY VED FROM THE SITE AND PROPERLY DISPOSED O

- 2 SOIL/AGGREGATE STOCKPILE SITES TO BE WHERE SHOWN ON THE DRAWINGS
- 3. RESTORE STOCKPILE SITES TO PRE-EXISTING PROJECT CONDITION AND RESEED AS REQUIRED.
- 4. STOCKPILE HEIGHTS MUST NOT EXCEED 35'. STOCKPILE SLOPES MUST BE 2:1 OR FLATTER.

EC-1 SCALE : N.T.S.

8916 77th TERRACE EAST, SUITE 103



Cellco Partnership d/b/a



T··Mobile

15 COMMERCE WAY SUITE B NORTON, MA 02766



550 COCHITUATE ROAD FRAMINGHAM, MA 01701

D&M DOCUMENTS NO DATE REVISION 09/12/25 FOR REVIEW: RCB 10/06/25 CARRIER REVISIONS: RCB 2 10/29/25 ADD TOWER INFO: RCB 3 11/20/25 FINAL: RCB 4 12/02/25 AT&T EQUIP REVISIONS: RCB

DESIGN PROFESSIONALS OF RECORD

PROF: ROBERT C. BURNS. P.E. COMP: ALL-POINTS TECHNOLOGY
CORPORATION, P.C.
ADD: 567 VAUXHALL STREET EXT. SUITE 311 WATERFORD, CT 06385

OWNER: BOROUGH OF NAUGATUCK ADDRESS: 229 CHURCH STREET

CT1239 CONRAD ST

SITE 161 CONRAD STREET ADDRESS: NAUGATUCK, CT 06770

APT FILING NUMBER: CT752130 DRAWN BY: ELZ

09/12/25 CHECKED BY: RCI VZW MDG LOC. CODE: 616755202 VZW PSLC: 470709

VZW FUZE ID: 17453771

EROSION CONTROL. PLANTING & **ENVIRONMENTAL NOTES & DETAILS**



5. ANY SOIL IN STOCKPILES IN EXCESS OF SEVEN (7) DAYS SHALL BE SEEDED AND MULCHED OR COVERED. **4 TEMPORARY STOCKPILE DETAIL**

DESIGN BASIS:

GOVERNING CODES/DESIGN STANDARDS: 2021 INTERNATIONAL BUILDING CODE (BC) AS AMENDED BY THE 2022 CONNECTICUT STATE BUILDING CODE

DESIGN CRITERIA: SNOW LOAD: 30 PSF (2022 CSBC APPENDIX P) 30 PSF (MIN. PER 2022 CSBC ADD 1608.1.1) (ASCE 7-16 SEC. 7.3, EQ. 7.3-1) GROUND, Pg: ROOF, Pf: EXPOSURE FACTOR, C₈: 0.9 (ASCE 7-16 TABLE 7.3-1) THERMAL FACTOR, C₁: 1.2 (ASCE 7-16 TABLE 7.3-2) SNOW IMPORTANCE FACTOR, I_k: 1.0 (ASCE 7-16 TABLE 1.5-2) WIND LOADS: 120 MPH (2022 CSBC APPENDIX P) EXPOSURE CATEGORY B (2021 IBC SEC. 1609.4.3) SEISMIC LOAD: SITE CLASS: D (2021 IBC SEC. 1613.2) MCE GROUND MOTION (PERIOD = 0.2S), S₁: 0.187 (2021 IBC FIG. 1613.3.1(1)) MCE GROUND MOTION

(PERIOD = 1.0S) So: 0.054 (2021 IBC FIG. 1613.3.1(2)) SEISMIC DESIGN
CATEGORY: B (2021 IBC SEC. 1613.3.5)

01 GENERAL:

- ASSENDATIONS USED IN THESE SPECIFICATIONS INCLUDE THE FOLLOWING:
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- REFERENCE HEREIN TO AN OR EQUAL ITEM, THAT EQUAL ITEM SHALL RE-APPROVED BY THE CONSTRUCTION MANAGER BEFORE
- RE-APPROVED BY THE CURRENT HOUSE.

 TRADES SHALL COORDINATE THEIR DAYS WITH ALL OTHER TRADES OTHER WORK AND CONDITIONS AS APPROPRIATE OR REQUIRED TO LEASE OF THE PROPERTY OF THE CONTROL OF THE PROPERTY OF TH MALL BE WITH THE OWNER, OR OWNERS SECCEED REPRESENTATIVE, FOR
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- OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

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- COULTI'S WALL BE BROUGHT TO THE ATTENTION OF THE DWINED OR THE MINE PRIOR TO NOT WOOK SHOULD BE ATTENDED AND THE STATE OF THE STATE OF

- SUPPLUS MATERIAL SHALL BE REMOVED FROM THE SITE PROMPTLY
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 EVERY CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF HIS

 WORK AND NEWLY INSTALLED OR DESTING WORK, INCLUDING PROTECTION

 OF THE SITE ALL EXPLICATION OF THE PROTECTION OF THE SITE ALL EXPLICATION OF THE SITE ALL EXPLICA
- ALL CONTRACTORS SHALL PROVIDE ALL NECESSARY TOOLS, FIXTURES, SERVICES, MATERIALS, JOB AIDS, AND PERSONNEL REQUIRED FOR THE
- EACLUTION OF THEIR WORK.

 ACH CONTRACTOR SHALL GUARANTEE ALL MATERIALS AND

 MORMANISHED BY THEM TO BE FREE OF DEFECTS AND MAINTAINED FOR A

 FERIOD OF ONE YEAR AFTER ACCEPTANCE OF THE INSTALLATION BY THE

 WINELA ROLD ENISTIES.
- ALL WORK SHALL BE PERFORMED BY LICENSED CONTRACTORS IN THE
- ANY DEVIATION, MODIFICATION, ADDITION, OR CHANGE IN DESIGN SHALL NOT BE MADE WITHOUT WRITTEN APPROVAL OF THE OWNER OR ENGINEER.

- ALL CONTRACTORS SHALL SUBJECT SHOP DRAWINGS OF ALL ENJAMENT AND MATERIALS TO THE BROWNERS FOR PROFUND, I PROPORT TO FARRISATION AND SHALL MOTOR PROCEDURAL PROPORT TO FARRISATION AND SHALL MOTOR OF THE CONTRACTOR SHALL MANTARIO AT DEAD THE CONTRACTOR SHALL BROWNERS APPROVAL IN WERTHAN OF THE CONTRACTOR SHALL BROWNERS AND EQUIPMENT SHALL BE NEW, WITHOUT BLOWNERS OF CONTRACTOR SHALL BROWN WITHOUT BLOWNERS OF CONTRACTOR SHALL BROWN WITHOUT BLOWNERS OF CONTRACTOR AND SHALL BROWN WITHOUT BLOWNERS OF CONTRACTOR AND THE CONTRACTOR AND SHALL BROWNERS OF THE REPORT AND AND SHALL BROWNERS OF THE REPORT AND AND SHALL BROWNERS OF THE SHALL BROWNERS OF THE REPORT AND AND AND SHALL BROWNERS OF THE SHALL BROWNERS OF THE REPORT OF THE SHALL BROWNERS OF THE SHALL BROWNERS OF THE SHALL BROWNERS OF THE SHALL BROWNERS OF SHALL BROWNERS OF THE BROWNERS OF THE SHALL BROWNERS OF SHALL BROWNERS SHALL BROWNERS OF SHALL BROWNERS AND APPLY OF THE NOWER.

- SHALL BE COMPLETELY REMOVED AFTER ITS PLAPPOSES HAVE BEEN SERVED.

 ANY ENSTINA UTILITY SERVICE, STRUCTURE, COUPMENT, OR PIKTURE OBSTRUCTURE WORK SHALL BE REMOVED ANDOID RELOCATED AS GREAT CONTROL OF THE PROPERTY OF THE CONSTRUCTION AND ANY OF THE PROPERTY OF THE CONSTRUCTION AND ANY OF THE PROPERTY OF THE CONSTRUCTION AND ANY OF THE PROPERTY OF THE CONSTRUCTION AND AND ANY OF THE PROPERTY OF THE CONSTRUCTION AND AND AND THE PROPERTY OF THE CONSTRUCTION AND AND AND ANY OF THE PROPERTY OF TH
- 05 ANCHORS: THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS
- EXPANSION ANCHORS SHALL BE USED WHERE ATTACHING TO CONCRETE.
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- 05 STEEL:
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- THESE SECTIONS SHALL INCLIDE THE GENERAL SPECIFICATIONS HEREIS.

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- IS NOT FERMITTED EXCEPT WITH THE PROR APPROVAL OF THE ENGINEER CONTRACTOR TO REMOVE AND PERSONAL ALL PIER PROCINICA AS REQUIRED DURING CONSTRUCTION.

 THE SIELL SITUATURE SHALL BE DESIGNED TO BE SELF-SUPPORTING AND THE SIELL SITUATURE SHALL BE DESIGNED TO BE SELF-SUPPORTING AND SECURITY OF THE PROPERTY OF THE PERSON PROCEDURE AND SECURIOR RESPONSIBILITY TO DETERMINE FRECTION PROCEDURE AND SECURIOR AND LONG-PLANT PARTIES DURING RECTION.

 JUNIOR SECTION.

 ALL STEEL ELEMENTS SHALL BE INSTALLED PLUMB AND LEVEL.
- ALL STEEL, ELEMENTS SHALL BE NISTALLED PLUMB AND LEVEL.
 TOWER MANAPACTURERS DESIGNED SHILL PREVAIL FOR TOWER
 CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR AND
 CONSTRUCTION AND ACCURANCE WITH THE ALISE! SHILLING IT HE ASSET
 MANAKAL OF STEEL CONSTRUCTION. CONNECTIONS SHALL BE PROVIDED
 STRUCTURAL CONNECTIONS SHALL BE PROVIDED
 STRUCTURAL CONNECTION BOTS SHALL CONNECTION SHALL
 HAVE NAMBLAM TWO BOLTS. LOCK WASHERS ARE NOT SHATIN AGES. ALL
 BOLTS SHALL BE MINIMAM SHE DIAMETER AND EACH CONNECTION SHALL
 HAVE NAMBLAM ALL BE DESIGNED ON SUP ORTIOL BOLTS ARE USED.
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 BOLTS SHALL BE DESIGNED ON SUP ORTIOL BOLT ALL COMBILE
 DESIGN CONNECTIONS AT BEAM BY THE SHAPE WITH TO SUBJECT WITH
- ALL U-BOLTED CONNECTIONS SHALL BE COMPLETED WITH DOUBLE NUTS OR A LOCK WASHER.
- GR A LOCK WASHER.

 OOTHER/OTO SHALL COMEN, WITH AWAS CODE FOR PROCEDURES. APPEARANCE AND CUMUNT OF WELLDS, AND WELLDING PROCESSES SHALL BE CUALIFIED IN ACCUPATION OF WELLDS, AND WELLDING PROCESSES SHALL BE CHARLED AND ACCUPATION.

 PROCEDURES. ALL WELLDING SHALL BE PERFORMED USING ETOXIC PROCESSES SHALL BE CHARLED AND ACCUPATION.

 WELLD SEZES ARE NOT SHAVIN, PROVIDED THE LARGER OF 14" FILLET OR MINIMAN SEZ PER TABLE 2.2" IN THE ASS. WANDAL OF STEEL CONSTRUCTOR. A 11" HE CUMPANION OF WELLDING ALL DAMAGE IN CONSTRUCTOR. A 11" HE CUMPANION OF WELLDING ALL DAMAGE IN CONSTRUCTOR. A 11" HE CUMPANION OF WELLDING ALL DAMAGE IN CONSTRUCTOR. A 11" HE CUMPANION OF WELLDING ALL DAMAGE IN CONSTRUCTOR. A 11" HE CUMPANION OF WELLDING ALL DAMAGE IN CONSTRUCTOR.
- ALL ARC AND GAS WELDING SHALL BE DONE BY A LICENSED AND CERTIFIE WELDER IN ACCORDANCE WITH AWS. SEAL ALL PENETRATIONS AND SEAMS BETWEEN MASONRY AND STEEL WITH DOW CORNING 790 SILICONE BUILDING SEALANT OR EQUAL.
- WITH DUW CONSISTENCY OF THE SELECTRICAL:
 THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS
- THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL PREVAILABLE PREPAIR AND ASSESSED AS A SHALL BE ASSESSED AS A SHALL

- BLEGTRICAL METALLIC TUBRIG (BMI)
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 FRANCIONES TO MEDITATION OF ADJUSTABLE
 HILLIES
 HACCURST TRANSPORMERS MOTORS, FOLOR WHERE EQUIPMENT IS
 PLACED LIFON SLAB ON-GRADE

 ALL ETTIMAS, CONNECTORS, AND COLIFILINGS SHALL BE THERADED
 MADE UP WIREHOLD TIGHT.

- FINISH MATERIAL.

 ALL FEEDER AND BRANCH CIRCUITS SHALL HAVE A SEPARATE PROPERLY SIZED AND MARKED GROUNDING CONDUCTOR, PER APPLICABLE CODES,
- SZED AND MARKED GROUNDING CONDUCTOR, FERFAPILCABLE CODES, HILL ISSUED AS A GROUNDING OF BINDING CONDUCTOR, FERFAPILCABLE CODES, HILL SEDUL ISSUED AS A GROUNDING OF BINDING CONDUCTOR. HE DESTRING LEICHIOL SERVICE BIS PORMAN, CONTINCTOR PAUL BE FORTING LEICHIOL SERVICE BIS PORMAN, CONTINCTOR PAUL BE CONTINCTOR SHALL BE SHADED OR REPLACED AS A PART OF THIS WORK. OCCURRANCED SHALL DESCRIPTION COORDINATE WITH AND CAN THIS WORK. OF THE CONTINCTOR SHALL BE AS SPECIFED AND AS APPROVED BY THE LOCAL UTILITY WHERE APPLICABLE.
- APPLICABLE.

 ALL EQUIPMENT, ENCLOSURES, ETC. SHALL BE SUITABLE FOR THE INSTALLED ENVIRONMENT, MINIMUM NEMA 3R FOR ALL EXTERIOR INSTALLATIONS.
- MANUFACTURER.
 ALL FIRE-RATED PENETRATIONS SHALL BE SEALED USING A SUITABLE AND
 LISTED FIRE SEALING DEVICE OR GROUT THAT WILL MAINTAIN THE FIRE
 RATING OF THE STRUCTURE PENETRATED.
- TO MAN OF THE STREET HE TENTERATED. PROVIDE PERMANENT LY FIRSE OF DEPAREY EN MANERALES FOR ALL CODE REQUIRED LABELING AND ON ALL PINGLES, METERNIQ, DESCONDENCES, AND SOURCE WITH ORCHIT CHARTEN AND OUT TO SERVE WITHOUT TO SERVE SERVE WITHOUT TO ALL EDITIONS OF THE METERS OF THE SERVE WITHOUT TO ALL EDITIONS TO ALL EDITIONS OF THE METERS OF THE ME
- TO ALL EQUIPMENT.

 ALL ELECTROLA APPLRTENANCES THAT ARE DISCONNECTED SHALL BE COMPLETELY REMOVED WITH EXISTING STRUCTURES TO REMAIN, REPAIR FINSHED, FILLED, PAINTED, ETC. ALL PANEL SCHEDULES, EQUIPMENT LABELING, AND CODE-REQUIRED LABELING, SHALL BE VERFIED AND PROPERLY COMPLETED TO MATCH THE INSTALLATION.
- 26 GROUNDING:
 THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS
 HEREIN. HERBIN.
 GROUND ALL SYSTEMS AND EQUIPMENT IN ACCORDANCE WITH BEST INDUSTRY PRACTICE, THE REQUIREMENTS OF THE NEPA 70 NATIONAL ELECTRICAL CODE (NEO, AND ALL OTHER APPLICABLE CODES AND REGULATIONS).
- ALL GROUNDING ELECTRODES PRESENT AT EACH SERVICE LOCATION SHALL BE BONDED TOGETHER TO FORM THE GROUNDING ELECTRODE
- SYNCLES AND CONTROL OF THE STATE OF THE STAT
- PASSED THROUGH, CÓNDUT SHALL NOT BE USED AS A GROUNDING OR
 BOND ALL METALLE CONDUITS TORSHER THAT ARE CONNECTED TO
 NOTWARFAILLE SHALLD CONDUITS TORSHER THAT ARE CONNECTED TO
 NOTWARFAILLE SHALLD SHALLD SHALLD BOXES, AND TO AN ENCLOSIVE
 BURLU WHIT GRADULOUS BUS SECREDED OR SUPPLIED. ACCOMPLISH THIS
 BURLU WHIT GRADULOUS BUS SECREDED OR SUPPLIED. ACCOMPLISH THE
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- CONDUCTOR SINCL, IS SOLD AND INSTALLED THE ITEM INFOUMM OF ALL
 SOLDEN THIN PROTECTION.

 THESE SPRODECTIONS SHALL INCLUDE THE GRIFFAL SPECIFICATIONS AND
 THE GROUNDING SPECIFICATIONS FALL INCLUDE THE GRIFFAL SPECIFICATION SHALL INCLUDE THE GRIFFAL SPECIFICATION SHALL INCLUDE THE GRIFFAL SPECIFICATION SPECIFIC

- BONGED TO PROVIDE A COMMON ELECTRICAL EQUIPOTENTIAL SYSTEM FOR ALL CONDUCTIVE ELEMENTS AND STRUCTURES.

 CONDUCTORS.

 CONDUCTORS.

 LAM 192 AND COPPER GET INVED COPPER (SETC) FOR ALL IN GROUND CONDUCTORS.

 LAM 192 AND COPPER GETEN STRANGED FOR BONDING STRUCTURES.

 AND FOR INTER-SYSTEM DONDING OF INVENDUAL ELEMENTS SUCH AS THE ADDRESS OF THE ADDRESS OF
- HORDOWTH, OR COMMINACY TOWARDS BASTH. IL BRIEVAND WILL BE HORDOWTH A DOWN AND A STATE OF THE MAN TO THE MAN TH

- SMILLAR.

 INSTALL ALL, IN-GROUND RINGS, RADIALS, BONDS CONNECTING THEM, AND ALL BRILLAR GROUNDING.

 AND 30 NOISES BELOW GRADE, OR 6 INO-ES BELOW THE FROST LINE, WHILE HE REPORT LINE, WHILE HE REPORT LINE, WHILE HE REPORT LINE, AND ALL BRILLARS FROM FOLKHATIONS, FOOTINGS, OTHER GROUNDING SYSTEMS, AND SMILLARS THAT LINESS, SLOGET WHEM MAKING A BOND TO ANY OF THESE STEUTURES. DO NOT BOND TO FOUNDATION INTERNAL REPORT CHARGE.
- INTERNAL BEINFORCEMENT.

 ALL EQUIPMENT ENQUEED IN A COMMON AREA, COMPOUND, STRUCTURE, OH SMILLAR SHALL BE BEINGED IT OF A SINSELF-UNIT GENOUND, FINE-HAMBLY AN ISOLATED REQUIPMED ARE AND THE REQUIPMENT OF THE SYSTEM-WITH MINIMUM SINGLE BONDING CONDUCTOR. IF BONDING TO AN IN-GROUND RAY, INSTALL BE SONDING CONDUCTOR, MINIMUM WITH EACH CONDUCTOR INSTALLED IN
- AWALE, TO WER GROUNDING.

 EACH TOWER LEG SHALL BE BONDED TO ITS RING. SINGLE-LEGGED LIGHES, OR MONOPHICES, SIMIL HAVE 2 BUNDS ON OH-DOSITE SIDES.

 BOND TO TO TOWER BASE, NOT TO VERTICAL TOWER STRUCTURE, AWAY FROM TOWER NOUNTING HARDWARE.

 EACH BOND SHALL HAVE A CORPESPONDING GROUND ROD ON THE
- RING.
 EACH BOND SHALL CONSIST OF 2 CONDUCTORS FROM THE TOWER TO ITS RING WITH EACH CONDUCTOR DIRECTED IN OPPOSITE DIRECTIONS WITH A PARALLEL CONNECTION ON THE RING ON OPPOSITE SIDES OF THE GROUND ROD.
- WITH A PARALLEL CONNECTION ON THE RINK ON O POPOSITE SIDES OF THE GROUND ROUGH.

 DURINGHT AREA GROUNDING:

 DURINGHT AREA GROUND RINK:

 DURINGHT AREA GROUND RINK:

 BOND ALL EQUIPMENT ON A SMALE FROM TORAUM DORAUM PARIS

 BOND THE EXPRINENT TO A SMALE FROM TORAUM DORAUM PARIS

 BOND THE EXPRINENT TO A SMALE FROM TORAUM TO TO THE DURINGHT ROUGH.

 BOND THE GOMENT SINKS. FROM TORAUM TO TO THE DURINGHT SINKS.

 FINE SHELTER IS CONSIDERED TO BE DOWN THE RINK.

 FINE SHELTER IS CONSIDERED TO BE DOWN THE RINK.

 FINE SHELTER IS CONSIDERED TO BE DOWN THINKS PROTECTION SYSTEM PER PAUGALE VERSION OF NEAT AND

 BOND ALL PRED CONDUCTIVE BULLDING COMPONENTS TOGETHER AND TO THE BULLDING RINK GROUND THE BULLDING THE DURING THE STANKS.

 BOND ALL FREED CONDUCTIVE BULLDING COMPONENTS TO THE HALD

 CALLED THE HALD GROUND. DO NOT BOND BULPHENT TO THE HALD

 SHOND ALL BURDWENT TOGETHER TO A SINKE-PORT ON THE HEAD
- GROUND.

 BOND ALL EQUIPMENT TOGETHER TO A SINGLE-POINT OR INTERIOR EQUIPMENT RING GROUND (IEGR). BOND THE SINGLE-POINT OR IEGR TO THE EXTERNAL EQUIPMENT RING GROUND.

 PLACE GROUND ROOS AT THE EQUIPMENT GROUND RING CORNERS.

- GROUND NOTES

 CHARACTER STATE OF THE STATE OF CHARACTER STATE OF CHARA
- ELEMENT (TOWNER SELEMENT), ELLJ.

 ADALS (TYP. NEW DEDICATED OXMANICATION SITES):

 ***MINIMUM CF 4, MAXIMAM 10 PING PAGALA**, ABLE. INSTALL A

 ***EACH RADIAL S LEMENT I SHALL BE MIN 20 FT, MAX 20 FT.

 **EXTEND PAGALAS PERPENDICULAR FROM RINGS IN AS STRAIGHT LINE

 **EXTEND PAGALAS PERPENDICULAR FROM RINGS IN AS STRAIGHT LINE

 **EXTEND PAGALAS PERPENDICULAR FROM RINGS IN AS STRAIGHT LINE

 **EXTEND PAGALAS PERMENT PRING GROUNDS, RADIALS, BONDS,

 **EXTEND PAGALAS PERMENT PAGALAS PE
- AND SMILLAN.

 A COMMON PRACTICE IS TO PLACE 4 RADIALS FROM THE TOWER RING.

 TO THE 4 CORNERS OF THE AVAILABLE AREA.

 AT A MINIMUM, BOOD ALL COMPOUND CONDUCTIVE FENCE CORNER POSTS.

 AND GATE POSTS TO THE PROCE LINE, BROWING ALL POSTS TO THE RING.

 THAT FOLLOWS THE FENCE LINE, BROWING ALL POSTS TO THE RING. 27 ANTENNAS & CABLES: THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS
- HEERIN THE CONTRACTION SHALL FLIRNINGH AND INSTALL ALL TRANSMISSION CARLES, JUMPERS, CONNECTORS, GROUNDING STRAPS, ANTENNAS, MOURIN AND HEROWARS, ALL MORHALS SHALL BE INSPECILED BY IT HE RESPECTIVE OF THE PROPERTY OF THE PROVIDED WITH OWNER PRIOR TO SUBMITTING BY AND TENDER OF THE PROVIDED WITH OWNER PRIOR TO SUBMITTING BY AND TENDER OF THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTING BY AND CREEKE THE PROPURED WITH OWNER PRIOR TO SUBMITTEND THE PROPURED THE PRO
- BID AND ORDERING MATERIALS.
 AFTER INSTALLATION, THE TRANSMISSION LINE SYSTEM SHALL BE RIM /
 SWEET TESTED FOR PROPER INSTALLATION AND DAMAGE WITH ANTENNAS
 CONNECTED. CONTRACTOR SHALL OBTAIN AND USE LATEST TESTING
 PROCEDURES FROM OWNER OR MANUFACTURE PRIOR TO BIDDING. ANTENNA CABLES SHALL BE UNIQUELY COLOR-CODED AT THE ANTENNAS, BOTH SIDES OF EQUIPMENT SHELTER WALL, AND JUMPER CABLES AT THE
- EQUIPMENT. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL CONNECTORS, ASSOCIATED CABLE MOUNTING AND GROUNDING HARDWARE, WALL MOUNTS, STANDOFFS, AND ALL ASSOCIATED HARDWARE TO INS CABLES AND ANTENNAS TO THE MANUFACTURER'S AND OWNER
- CABLES AND ANTENNAST OT THE MANUFACTURERS AND OWNERS
 SECROPLATION.
 ANTENNA CABLES SAVILL BE FORM DELECTRIC COAVAL CABLES AS
 TO ANTENNAS.

 7. 76 DAMETER FOR CABLE LENGTHS UP TO 100 FT.

 1. 56 DAMETER FOR CABLE LENGTHS UP TO 100 FT.

 1. 57 DAMETER FOR CABLE LENGTHS UP TO 100 FT.

 1. 76 DAMETER FOR CABLE LENGTHS UP TO 200 FT.

 1. 76 DAMETER FOR CABLE LENGTHS UP TO 200 FT.

 MINIMUM BEDING PROBLES LICKTHS UP TO 200 FT.

 CABLE SHALL BE RATIFALLED WITH A MINIMUM NUMBER OF BENDS WHEF
- CABLE SHALL BE INSTALLED WITH A MINIMUM NUMBER OF BENDS WHERE POSSIBLE. CABLE SHALL NOT BE LEFT UNTERMINATED AND SHALL BE SEALED IMMEDIATELY AFTER BEING INSTALLED. ALL EXTERIOR CABLE CONNECTIONS SHALL BE COVERED WITH A WATER-PROOF SPLICING RIT.
- CONTRACTOR SHALL VERIFY EXACT LENGTH AND DIRECTION OF TRAVEL IN FIELD PRIOR TO CONSTRUCTION. CABLE SHALL BE FURNISHED AND INSTALLED WITHOUT SPLICES AND WITH CONNECTORS AT EACH END. CONNECTORS AT EXPANSION.

 27 CABLE TRAY:

 THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS

 THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS
- HERBIN.
 CABLE TRAY SHALL BE MADE OF EITHER CORPOSION RESISTANT METAL OR
 WITH A CORPOSION RESISTANT FINISH.
 CABLE TRAY SHALL BE OF JALOBER TRAY TYPE WITH FLAT COVER CLAMPED
 TO SIDE FRALS.
 SHALL BE OF JALOBER TRAY TYPE WITH FLAT COVER CLAMPED
 TO SIDE FRALS.
 SHALL BE SIZED TO FIT ALL CABLES IN ACCORDANCE WITH
 NEXT AND THANK 11-15-BE.
 CABLE LADGER TRAYS SHALL BE NEMA CLASS 12A BY PW INDUSTRIES, INC.
 OR ECUAL.
- OH EQUAL.

 CABLE LADDER TRAY SHALL BE SUPPORTED IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS. ALL WORKMANSHIP SHALL CONFORM TO THESE REQUIREMENTS AND ALL LOCAL CODES AND STANDARDS TO ENSURE SAFE AND ADEQUATE GROUNDING SYSTEM.

- 31 EXCAVATION & FILL:
 THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS
 HERDELS. CONTRACTOR SHALL GRADE ONLY AREAS SHOWN TO BE MODIFIED AS A PART OF THIS WORK AND ONLY TO THE EXTENT REQUIRED TO SHED
- ORGANIC MATERIAL AND DEBRIS SHALL BE STRIPPED AND STOCKPILED BEFORE ADDING FILL MATERIAL. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT BE PLACED IN ANY FILL OR EMBANAMENT.
- ANY FILL OR BUBLANAMENT.

 ALT FILL SHALL BE PLACED IN ONE FOOT LET'S AND COMPACTED IN PLACE. STRUCTURAL FILL SHALL BE COMPACTED TO 95% OF TIS MAXIMUM DRY UNITY WEIGHT TESTED IN ACCORDANCE WITH ASTEN DISP.

 DICHANTONS FOR FOOTINGS SHALL BE CUT LEVEL TO THE REQUIRED DEPTH AND TO UNDSTREED DIC. REPORT UNSUTTABLE SOIL CONDITIONS TO THE CONSTRUCTION MANAGER.

 TERCHO ENCONATIONS SHALL BE ADMITTED. TOWER FOUNDATION EXCAVATION, BACKFILL AND COMPACTION SHALL BE IN ACCORDANCE WITH TOWER MANUFACTURER'S DESIGNS AND
- DAMETER
 BANK OR CRUSHED GRAVEL SHALL CONSIST OF TOLIGH, DURABLE
 PARTICLES OF CRUSHED OR HUNCHLISHED GRAVEL FREE OF SOFT, THIN,
 ELOWARTED OR LAMINATED PIECES AND MEET THE SPOCHED GRADATION
 PROCESSED AGGREGATE BASE SHALL CONSIST OF COURSE AND FINE
 AGGREGATES COMMEND AND MEMBES OF THAT THE RESULTING MATERIAL
 CONFORMS TO THE GRADATION. COURSE AGGREGATE SHALL BE EITHER
 GRAVEL OR BROWNER STOME AND THE AGGREGATES SHALL CONSIST OF
- GRAVEL OR BECKEN STONE AND PINE ACCRECATE SHALL CONSIST OF SHALL PASSE WITH THE FOLLOWING SIZE SQUARE MESH SERVES.

 SHALL PASSE SHALL PASSE WITH THE FOLLOWING SIZE SQUARE MESH SERVES.

 SHALL PASSE SHALL PASSE SHALL PASSE SHALL PASSE SHALL PASSE SHALL PASSE WITH FOLLOWING SIZE SQUARE MESH SERVES.

- TH PASS #200 GG BASE SHALL PASS WITH THE FOLLOWING SIZE SQUARE /ES: WITH PASS 3-1/2*
- 55-95. WITH PASS 1-1/2°
 50-758. WITH PASS 1-4°
 25-459. WITH PASS 1-8'
 5-20%. WITH PASS 1-8'
 5-20%. WITH PASS 1-8'
 FOR PASS 1-8'
- FILL MATERIAL REQUIREMENTS.
 31 SEDIMENTATION & EROSION CONTROL: DISTURBANCE. DISCHARGE TO STORM DRAINS OR SUFFACE WATERS FROM SEDIMENT CONTROLS SHALL BE CLEAR AND APPROVED BY THE ENGINEER.
- THESE SPECIFICATIONS SHALL INCLUDE THE GENERAL SPECIFICATIONS HEREIN HEREIN.
 CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXIST. SITE DURING CONSTRUCTION, SEGONO CONTROL MEASURES, IF REQUESTED LURING CONSTRUCTION, SEGONO CONTROL MEASURES, SEGONO CONTROL C

SEMENTATION AND ERDSON CONTINU. SECS MEASURES SHOWN SHALL BE INSTALLED PROTE TO LAND CERRADE, SECANATION OR SPRANDS OPERATIONS. REQUIREMENTS OF LOCAL WETLAND AGENCY SHALL BE MET PRIOR TO SHATHMORK OPERATIONS. IT IS THE CONTINUED OF SEASONS IT IS THE CONTRACTIONS RESPONSIBILITY TO MAINTAIN SEC MEASURES THE OWN TRACTION OF SEASONS BUT TO THE TRACTION OF THE PROPERTY OF THE PRO

- FRONGOGILT VEGETATED.

 FAILURE OF THE SEC SYSTEMS SHALL BE CORRECTED IMMEDIATELY AND SUPPLEMENTED WITH ADDITIONAL MEASURES AS NEEDED. TOPSOIL SHALL BE SPREAD TO FINISH GRADES AND SEEDED AS SOON AS FINISHED GRADES ARE ESTABLISHED. STRAW MULCH, JUTE NETTING OR MATS SHALL BE USED WHERE THE NEW SEED IS PLACED.
- VEGETATIVE SEEDING.

 AREA TO BE SEEDED SHALL BE LOOSE AND FRIABLE TO A DEPTH OF 9°.
 TORSOIL SHALL BE LOOSENED BY RINKING OR DISRING BEFORE
 SEEDING. APPLY 90 Lbs. OF DOLOMITIC LIMISTONS AND 25 Lbs. OF
 10-10-10 FETHILIZER PER 1000 SF. HARPOV LIMIS AND FETHILIZER NITO.
- OSE SOIL.
 PLY COMMON BERMUDA AND RYE GRASS AT 50 LBS PER ACRE. USE
 PLY COMMON BERMUDA AND RYE GRASS AT 50 LBS PER ACRE. USE



8916 77th TERRACE EAST, SUITE 103



Cellco Partnership d/b/a



 \mathbf{T} -- Mobile Northeast LLC 15 COMMERCE WAY



550 COCHITUATE ROAD FRAMINGHAM, MA 01701

D&M DOCUMENTS NO DATE REVISION 0 09/12/25 FOR REVIEW: RCB 1 10/06/25 CARRIER REVISIONS: RCB 2 10/29/25 ADD TOWER INFO: RCB 4 12/02/25 AT&T EQUIP REVISIONS: RCB

DESIGN PROFESSIONALS OF RECORD

PROF: ROBERT C. BURNS. P.E. COMP: ALL-POINTS TECHNOLOGY CORPORATION, P.C. ADD: 567 VAUXHALL STREET EXT. SUITE 311 WATERFORD, CT 06385

OWNER: BOROUGH OF NAUGATUCK ADDRESS: 229 CHURCH STREET

CT1239 CONRAD ST

SITE 161 CONRAD STREET ADDRESS: NAUGATUCK, CT 06770 APT FILING NUMBER: CT752130

DATE: 09/12/25 CHECKED BY: RCB

VZW PSLC: 470709 VZW FUZE ID: 17453771

VZW MDG LOC. CODE: 616755202

NOTES & **SPECIFICATIONS**



SHEET NUMBER:



Michael F. Plahovinsak, P.E.

18301 State Route 161, Plain City, Ohio 43064
(614) 398-6250 - mike@mfpeng.com

November 25, 2025

Tarpon Towers

Re: Proposed 150-ft Monopole

Located in New Haven Co., CT: CT1239 Conrad St.

MFP Project #: 23525-716 / TAPP Project Number: TP-25179

I understand that there may be some concern on the part of local building officials regarding the potential for failure of the proposed communication monopole. Communication structures are designed in accordance with the Telecommunications Industry Association TIA-222-H, "Structural Standards for Steel Antenna Towers and Antenna Supporting Structures". This Structure is to be fabricated by TransAmerican Power Products

I have designed this monopole to withstand a 3-sec. gusted wind speed of 130 mph as recommended by TIA-222-H for New Haven Co., CT. The design also conforms to the requirements of the 2022 Connecticut Building Code.

This monopole has been designed to accommodate a theoretical fall radius. The upper 75' of the pole has been designed to meet the wind loads of the design, however, the lower portion of the pole has been designed with a minimum 10% extra capacity. Assuming the pole has been fabricated according to my design, and well maintained, in the event of a failure due to extreme wind and comparable appurtenance antenna load (winds in excess of the design wind load), it would yield/buckle at the 75' elevation. The yielded section is designed to swing down and rest on the ground, resulting in an approximate 0-ft fall radius. The pole shafts are designed to buckle and bend, not rupture and disconnect. The designed 0-ft fall radius is less than the required 58-ft fall radius.

The structure has been designed with all of the applicable factors as required by the code. A properly designed, constructed and maintained pole has never collapsed; monopoles are safe structures with a long history of reliable operation.

I hope this review of the monopole design has given you a greater degree of comfort regarding the design capacity inherent in pole structures. If you have any additional questions please call me at 614-398-6250 or email mike@mfpeng.com.

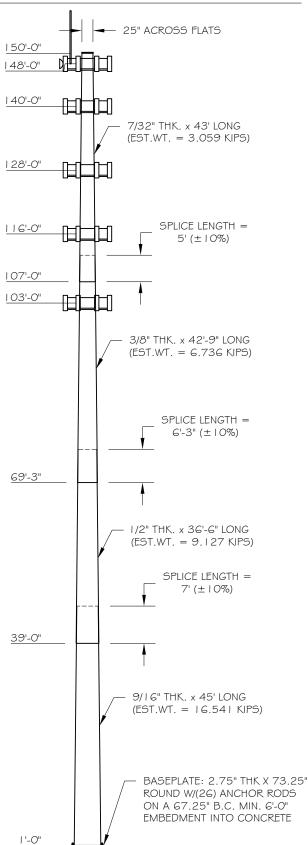
Sincerely,

Michael F. Plahovinsak, P.E.









- 59.75" ACROSS FLATS

Page 1 of 3	3	Job Number:	23525-716	
Eng:		Customer Ref:	TP-25179	
MITT		Date:	10/27/2025	
Structure:	150-	FT MONOPOLE		
Site:	CT123	CT 239 CONRAD ST.		
Location:	NEW HAVEN CO.,	CT / 41°29'53",	-73°4'12"	
Owner:	Owner: TARPON TOWERS			
Revision No	.: Revision Date:			

Revision No.: R	(evision Date:		
	DES	IGN	
Building Code: 2	022 CONNECTICUT	BUILDING CODE	
Design Standard:	TIA-222-H		
Wind Speed Load	Cases: AS	CE-7-16 WIND SPE	ED
Load Case #1: 1	30 MPH Design Wind	d Speed	
Load Case #2: 5	O MPH Wind with	I" Ice Accumu	lation
Load Case #3 6	O MPH Service Win	d Speed	
Structure Class Risk Category	Exposure Cat.	Topography Cat.	Crest Height
Ш	С	1	

STRUCTURE MEETS THE MINIMUM REQUIREMENTS OF TIA-222-I

	EQUIPMENT LIST			
Elev.	Description			
153	(2) CC807 + (1) DS7CO9PU-N			
148	(I) TTA + (I) 3-FT DISH + I2-FT PLATFORM WITH HANDRAIL			
140	ANTENNAS + MOUNTING (EPA 42,000 IN2)			
140	GENERIC ANTENNA MOUNT			
128	ANTENNAS + MOUNTING (EPA 30,000 IN2)			
128	GENERIC ANTENNA MOUNT			
116	ANTENNAS + MOUNTING (EPA 30,000 IN2)			
116	GENERIC ANTENNA MOUNT			
103	(3) FFVV-65B-R3 + (6) RRU + (1) RAYCAP			
103	8-FT PLATFORM WITH HANDRAIL			

ANTENNA FEED LINES ROUTED ON THE INSIDE OF THE POLE POLE DESIGNED FOR A MAX O-FT FALL RADIUS

	STRUCTURE PROPERTIES											
Cross-S	ection: 18-5	ided	Taper:	0.2479) ın/ft							
Shaft St	eel: ASTM A5	72 GR 65	Baseplate	Steel: ASTM	A572 GR 50							
Anchor R	Rods: 2.25 ır	ı. AG15 GR. 75	5 X 7'-0"									
Sect.	Length (ft)	Thickness (in)	Splice (ft)	Top Dia. (in)	Bot Dia. (in)							
1	43.00	0.2188	5.00	25.00	35.66							
2	42.75	0.3750	6.25	33.98	44.58							
3	36.50	0.5000	7.00	42.28	51.33							
4	45.00	0.5625	0	48.59	59.75							

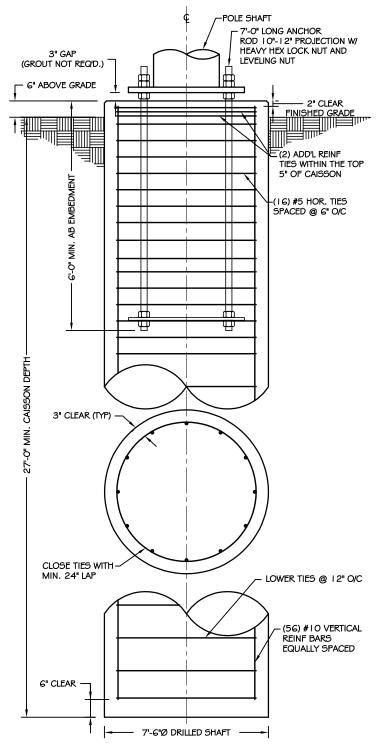


MICHAEL F. PLAHOVINSAK, P.E. #25849 Sole Proprietor - Independent Englineer 18301 S.R. 161, Plain City, OH 43064 614-398-6250 / mike@mfpeng.com

BASE REACTIONS FOR FOUNDATION DESIGN

Moment: 8869 ft-kip

Shear: 75 kip Axial: 77 kip



Page 2 of 3		Job Number:	23525-716			
Eng: MEP		Customer Ref:	TP-25179			
IVII F		Date:	10/27/2025			
Structure:	I 50-FT MONOPOLE					
Site:	CT I 23	39 CONRAD ST.				
Location:	NEW HAVEN CO.,	CT / 41°29'53", -	73°4'12"			
Owner:	TAR	PON TOWERS				
Revision No.:	Revision Date:					

FOUNDATION NOTES:

- I. ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45. IN AREAS OF POTENTIAL FREEZING, CONCRETE SHALL BE AIR ENTRAINED 6% (±1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318, "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.
- 2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM AG I 5 VERTICAL BARS SHALL BE GRADE 60, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 3 I 5, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.
- 3. CAISSON FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 33G, "STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF DRILLED PIERS", LATEST EDITION.
- 4. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.
- 5. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY:
 ENGINEER: WELTI GEOTECHNICAL
 REPORT NO.: N/A (DATED 10/16/25)
- 6. ESTIMATED CONCRETE VOLUME = 45 CUBIC YARDS.
- 7. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED

LOADS:

MOMENT: 8869 FT*KIPS SHEAR: 75 KIPS AXIAL: 77 KIPS

8. GEOTECHNICAL REPORT INDICATES GROUNDWATER MAY BE ENCOUNTERED AT I 3'-6" BELOW GRADE.



MICHAEL F. PLAHOVINSAK, P.E. #25849 Sole Proprietor - Independent Engineer 18301 S.R. 161, Plain City, OH 43064 614-398-6250 / mike@mfpeng.com

CAISSON FOUNDATION

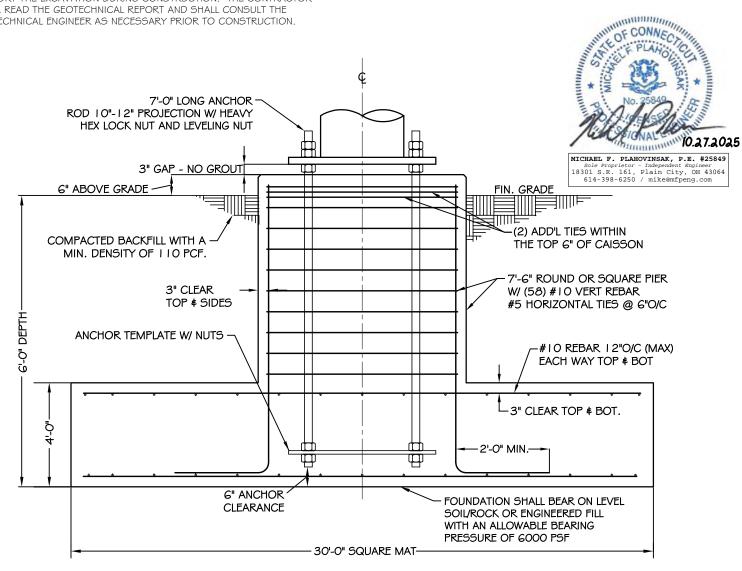
Page 3 of 3		Job Number:	23525-716
Eng: MFP		Customer Ref:	TP-25179
		Date:	10/27/2025
Structure:	150-	FT MONOPOLE	
Site:	CT123	39 CONRAD ST.	
Location:	NEW HAVEN CO.,	CT / 41°29'53",	-73°4'12"
Owner:	TAR	PON TOWERS	
Revision No.:	Revision Date:		

FOUNDATION NOTES:

- I ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45 AND SHALL BE AIR ENTRAINED 6% (± 1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318, "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.
- 2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM AG I 5 VERTICAL BARS SHALL BE GRADE 60, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.
- 3. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.

- 4. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY: ENGINEER: WELTI GEOTECHNICAL
 - REPORT NO .: N/A (DATED 10/16/25)
- 5. ESTIMATED CONCRETE VOLUME = 139 CUBIC YARDS.
- 6. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED LOADS:

MOMENT: 8869 FT*KIPS SHEAR: 75 KIPS AXIAL: 77 KIPS



SPREAD FOOTING

tro w	014074
<i>IIIX</i> I	'ower

Michael Plahovinsak, P.E.

18301 State Route 161 Plain City, OH 43064 Phone: 614-398-6250 FAX: mike@mfpeng.com

Job		Page
	150-ft Monopole - MFP #23525-716 r1	1 of 8
Project		Date
	CT1239 Conrad St.	18:09:51 10/27/25
Client	TP-25179	Designed by JC

Tower Input Data

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower base elevation above sea level: 515.00 ft.

Basic wind speed of 130 mph.

Risk Category III.

Exposure Category C.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1.

Crest Height: 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Tapered Pole Section Geometry

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L1	150.00-107.00	43.00	5.00	18	25.0000	35.6598	0.2188	0.8750	A572-65 (65 ksi)
L2	107.00-69.25	42.75	6.25	18	33.9828	44.5806	0.3750	1.5000	A572-65 (65 ksi)
L3	69.25-39.00	36.50	7.00	18	42.2812	51.3297	0.5000	2.0000	A572-65 (65 ksi)
L4	39.00-1.00	45.00		18	48.5944	59.7500	0.5625	2.2500	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia.	Area	I	r	C	I/C	J	It/Q	w	w/t
	in	in^2	in^4	in	in	in^3	in^4	in^2	in	
L1	25.3519	17.2059	1334.9410	8.7973	12.7000	105.1135	2671.6385	8.6046	4.0150	18.354
	36.1762	24.6072	3904.9253	12.5816	18.1152	215.5609	7814.9887	12.3059	5.8911	26.931
L2	35.7078	40.0017	5708.1578	11.9308	17.2633	330.6535	11423.8265	20.0046	5.3210	14.189
	45.2105	52.6158	12989.9879	15.6930	22.6470	573.5862	25997.0684	26.3129	7.1862	19.163
L3	44.4297	66.3068	14623.7470	14.8323	21.4789	680.8432	29266.7362	33.1597	6.5615	13.123
	52.0444	80.6667	26330.9765	18.0445	26.0755	1009.7981	52696.5999	40.3410	8.1540	16.308
L4	51.0193	85.7549	24995.1432	17.0513	24.6859	1012.5253	50023.1756	42.8856	7.5626	13.445
	60.5850	105.6719	46768.8712	21.0116	30.3530	1540.8319	93599.2822	52.8460	9.5260	16.935

tnxTo	W	er
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Client	TP-25179	Designed by JC

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A _r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft^2	in					in	in	in
L1				1	1	1			
150.00-107.00									
L2				1	1	1			
107.00-69.25									
L3 69.25-39.00				1	1	1			
L4 39.00-1.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg		Torque Calculation		ft			ft²/ft	plf
Safety Climb & Step	С	No	Yes	CaAa (Out	150.00 - 1.00	1	No Ice	0.06	0.09
Bolts Exposed				Of Face)			1/2" Ice	0.14	0.63
•							1" Ice	0.24	1.77
**									
1/2"	C	No	Yes	Inside Pole	148.00 - 1.00	1	No Ice	0.00	0.15
							1/2" Ice	0.00	0.15
							1" Ice	0.00	0.15
1 5/8"	C	No	Yes	Inside Pole	148.00 - 1.00	2	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
1.3"	C	No	Yes	Inside Pole	148.00 - 1.00	1	No Ice	0.00	0.66
							1/2" Ice	0.00	0.66
**							1" Ice	0.00	0.66
1 5/8"	C	No	Yes	Inside Pole	140.00 - 1.00	18	No Ice	0.00	0.92
1 3/0	C	NO	168	niside Foie	140.00 - 1.00	10	1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
1 5/8"	С	No	Yes	Inside Pole	128.00 - 1.00	18	No Ice	0.00	0.92
1 3/0	C	140	103	mside i oic	120.00 - 1.00	10	1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
1 5/8"	С	No	Yes	Inside Pole	116.00 - 1.00	18	No Ice	0.00	0.92
1 3/0	C	110	103	maide i ole	110.00 - 1.00	10	1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
1 5/8"	C	No	Yes	Inside Pole	103.00 - 1.00	18	No Ice	0.00	0.92
1 3/0	C	110	103	maide i ole	103.00 - 1.00	10	1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92

Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	A_R	A_F	$C_A A_A$	C_AA_A	Weight
Section	Elevation ft		ft^2	ft ²	In Face ft ²	Out Face ft²	K
L1	150.00-107.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	2.365	1.15
L2	107.00-69.25	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	2.076	2.53
L3	69.25-39.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	1.664	2.08

tnx _T	ower

Michael Plahovinsak, P.E. 18301 State Route 161

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Tower Section	Tower Elevation	Face	A_R	A_F	C _A A _A In Face	C _A A _A Out Face	Weight
	ft		ft^2	ft ²	ft^2	ft^2	K
L4	39.00-1.00	A	0.000	0.000	0.000	0.000	0.00
		В	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	2.090	2.61

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	A_R	A_F	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	ft ²	ft ²	ft^2	ft^2	K
L1	150.00-107.00	A	1.316	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	12.933	1.28
L2	107.00-69.25	A	1.268	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	11.354	2.64
L3	69.25-39.00	A	1.208	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	8.805	2.16
L4	39.00-1.00	A	1.094	0.000	0.000	0.000	0.000	0.00
		В		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	10.605	2.71

Discrete Tower Loads

Description	Face	Offset	Offsets:	Azimuth	Placement		$C_A A_A$	$C_A A_A$	Weight
	or Leg	Type	Horz Lateral	Adjustment			Front	Side	
	Leg		Vert						
			ft	0	ft		ft^2	ft^2	K
			ft		Ji		Ji	Ji	n
			ft						
3-Wire De-Tuning Equipment	С	None	, , , , , , , , , , , , , , , , , , ,	0.0000	150.00 - 1.00	No Ice	20.00	20.00	0.20
0 1 1						1/2" Ice	25.00	25.00	0.25
						1" Ice	30.00	30.00	0.30
**									
RFI CC807 10' Whip	Α	From Face	3.00	0.0000	153.00	No Ice	3.10	3.10	3.00
•			0.00			1/2" Ice	4.17	4.17	3.02
			0.00			1" Ice	5.25	5.25	3.05
RFI CC807 10' Whip	В	From Face	3.00	0.0000	153.00	No Ice	3.10	3.10	3.00
_			0.00			1/2" Ice	4.17	4.17	3.02
			0.00			1" Ice	5.25	5.25	3.05
DBSpectra DS7C09PU-N	C	From Face	3.00	0.0000	153.00	No Ice	4.34	4.34	0.04
Omni			0.00			1/2" Ice	5.82	5.82	0.07
			0.00			1" Ice	7.31	7.31	0.11
TTA	В	From Face	3.00	0.0000	148.00	No Ice	1.50	1.50	0.05
			0.00			1/2" Ice	2.00	2.00	0.07
			0.00			1" Ice	3.00	3.00	0.07
12' Platform w/ Handrail	C	None		0.0000	148.00	No Ice	30.00	30.00	1.80
						1/2" Ice	35.00	35.00	2.60
						1" Ice	40.00	40.00	3.40

EPA 42,000 in2	C	None		0.0000	140.00	No Ice	292.00	292.00	4.00
						1/2" Ice	300.00	300.00	6.00
						1" Ice	308.00	308.00	8.00
EPA 30,000 in2	C	None		0.0000	128.00	No Ice	208.33	208.33	4.00
						1/2" Ice	225.00	225.00	5.00
						1" Ice	241.67	241.67	6.00

Michael Plahovinsak, P.E.

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Client	TP-25179	Designed by JC

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		C _A A _A Front	C _A A _A Side	Weight
			ft ft ft	0	ft		ft²	ft ²	K
EPA 30,000 in2	С	None		0.0000	116.00	No Ice 1/2" Ice 1" Ice	208.33 225.00 241.67	208.33 225.00 241.67	4.00 5.00 6.00
Andrew FFVV-65B-R3 w/ mount pipe	A	From Face	3.00 0.00 0.00	0.0000	103.00	No Ice 1/2" Ice 1" Ice	15.31 15.84 16.37	8.02 8.98 9.82	0.12 0.23 0.35
Andrew FFVV-65B-R3 w/ mount pipe	В	From Face	3.00 0.00 0.00	0.0000	103.00	No Ice 1/2" Ice 1" Ice	15.31 15.84 16.37	8.02 8.98 9.82	0.12 0.23 0.35
Andrew FFVV-65B-R3 w/ mount pipe	С	From Face	3.00 0.00 0.00	0.0000	103.00	No Ice 1/2" Ice 1" Ice	15.31 15.84 16.37	8.02 8.98 9.82	0.12 0.23 0.35
Raycap RDIDC-9181-PF-48	A	From Face	2.00 0.00 0.00	0.0000	103.00	No Ice 1/2" Ice 1" Ice	1.87 2.04 2.21	1.07 1.20 1.35	0.10 0.12 0.14
(3) Samsung RT4450	В	From Face	2.00 0.00 0.00	0.0000	103.00	No Ice 1/2" Ice 1" Ice	0.30 0.37 0.45	0.30 0.37 0.45	0.02 0.02 0.03
(3) Samsung RT4451	С	From Face	2.00 0.00 0.00	0.0000	103.00	No Ice 1/2" Ice 1" Ice	0.30 0.37 0.45	0.30 0.37 0.45	0.02 0.02 0.03
8' Platform w/ Handrail	С	None		0.0000	103.00	No Ice 1/2" Ice 1" Ice	30.00 35.00 40.00	30.00 35.00 40.00	1.80 2.60 3.40

Dishes											
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				ft	0	٥	ft	ft		ft ²	K
3 ft standard	A	Paraboloid w/o	From	1.00	0.0000		148.00	3.00	No Ice	7.06	0.10
		Radome	Face	0.00					1/2" Ice	7.47	0.18
				0.00					1" Ice	7.88	0.25

Load Combinations

Comb.	Description
No.	
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 90 deg - No Ice
5	0.9 Dead+1.0 Wind 90 deg - No Ice
6	1.2 Dead+1.0 Wind 180 deg - No Ice
7	0.9 Dead+1.0 Wind 180 deg - No Ice
8	1.2 Dead+1.0 Ice+1.0 Temp
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp

Michael Plahovinsak, P.E.

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Comb.	Description
No.	
12	Dead+Wind 0 deg - Service
13	Dead+Wind 90 deg - Service
14	Dead+Wind 180 deg - Service

Maximum Member Forces

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Type		Load		Moment	Moment
		• •		Comb.	K	kip-ft	kip-ft
L1	150 - 107	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-43.70	0.25	15.67
			Max. Mx	4	-22.94	-1015.35	2.12
			Max. My	2	-22.99	2.74	1019.06
			Max. Vy	4	52.23	-1015.35	2.12
			Max. Vx	6	52.09	-21.33	-995.48
			Max. Torque	4			-1.38
L2	107 - 69.25	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-62.42	0.45	16.45
			Max. Mx	4	-37.38	-3105.22	-10.88
			Max. My	2	-37.41	5.70	3097.17
			Max. Vy	4	60.21	-3105.22	-10.88
			Max. Vx	6	60.04	-43.09	-3079.59
			Max. Torque	4			1.91
L3	69.25 - 39	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-77.56	0.46	16.71
			Max. Mx	4	-51.40	-4933.88	-21.39
			Max. My	2	-51.43	7.93	4915.94
			Max. Vy	4	63.58	-4933.88	-21.39
			Max. Vx	6	63.42	-60.55	-4903.56
			Max. Torque	4			1.90
L4	39 - 1	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-105.29	0.46	16.62
			Max. Mx	4	-77.40	-7893.31	-37.24
			Max. My	2	-77.40	11.25	7860.46
			Max. Vy	4	67.49	-7893.31	-37.24
			Max. Vx	6	67.34	-86.48	-7856.17
			Max. Torque	4			1.89

Maximum Tower Deflections - Service Wind

Elevation	Horz.	Gov.	Tilt	Twist
	Deflection	Load		
ft	in	Comb.	0	٥
150 - 107	23.994	12	1.4878	0.0002
112 - 69.25	13.011	12	1.1756	0.0001
75.5 - 39	5.598	12	0.7200	0.0000
46 - 1	2.028	12	0.4116	0.0000
	ft 150 - 107 112 - 69.25 75.5 - 39	ft in 150 - 107 23.994 112 - 69.25 13.011 75.5 - 39 5.598	ft in Load Comb. 150 - 107 23.994 12 112 - 69.25 13.011 12 75.5 - 39 5.598 12	ft in Comb. 150 - 107 23.994 12 1.4878 112 - 69.25 13.011 12 1.1756 75.5 - 39 5.598 12 0.7200

Critical Deflections and Radius of Curvature - Service Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	٥	0	ft
153.00	RFI CC807 10' Whip	12	23.994	1.4878	0.0008	31656
150.00	3-Wire De-Tuning Equipment	12	23.994	1.4878	0.0008	31656

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Elevation	Appurtenance	Gov. Load	Deflection	Tilt	Twist	Radius of Curvature
ft		Comb.	in	0	0	ft
148.00	3 ft standard	12	23.380	1.4740	0.0008	31656
144.86	3-Wire De-Tuning Equipment	12	22.417	1.4522	0.0007	30806
140.00	EPA 42,000 in2	12	20.934	1.4178	0.0006	15828
139.72	3-Wire De-Tuning Equipment	12	20.850	1.4158	0.0006	15403
134.59	3-Wire De-Tuning Equipment	12	19.304	1.3780	0.0006	10268
129.45	3-Wire De-Tuning Equipment	12	17.789	1.3380	0.0005	7701
128.00	EPA 30,000 in2	12	17.368	1.3262	0.0005	7194
124.31	3-Wire De-Tuning Equipment	12	16.314	1.2950	0.0005	6160
119.17	3-Wire De-Tuning Equipment	12	14.891	1.2483	0.0005	5133
116.00	EPA 30,000 in2	12	14.043	1.2173	0.0005	4655
114.03	3-Wire De-Tuning Equipment	12	13.530	1.1972	0.0005	4425
108.90	3-Wire De-Tuning Equipment	12	12.241	1.1410	0.0005	4236
103.76	3-Wire De-Tuning Equipment	12	11.026	1.0801	0.0005	4343
103.00	Andrew FFVV-65B-R3 w/ mount	12	10.853	1.0708	0.0005	4361
	pipe					
98.62	3-Wire De-Tuning Equipment	12	9.885	1.0159	0.0005	4463
93.48	3-Wire De-Tuning Equipment	12	8.814	0.9496	0.0004	4590
88.34	3-Wire De-Tuning Equipment	12	7.812	0.8825	0.0004	4724
83.21	3-Wire De-Tuning Equipment	12	6.878	0.8160	0.0003	4867
78.07	3-Wire De-Tuning Equipment	12	6.008	0.7513	0.0003	5013
72.93	3-Wire De-Tuning Equipment	12	5.202	0.6897	0.0003	5033
67.79	3-Wire De-Tuning Equipment	12	4.459	0.6316	0.0002	4962
62.66	3-Wire De-Tuning Equipment	12	3.779	0.5765	0.0002	4883
57.52	3-Wire De-Tuning Equipment	12	3.164	0.5238	0.0002	4802
52.38	3-Wire De-Tuning Equipment	12	2.616	0.4729	0.0002	4724
47.24	3-Wire De-Tuning Equipment	12	2.134	0.4234	0.0001	4690
42.10	3-Wire De-Tuning Equipment	12	1.720	0.3747	0.0001	5070
36.97	3-Wire De-Tuning Equipment	12	1.368	0.3266	0.0001	5793
31.83	3-Wire De-Tuning Equipment	12	1.071	0.2790	0.0001	6758
26.69	3-Wire De-Tuning Equipment	12	0.821	0.2318	0.0001	8110
21.55	3-Wire De-Tuning Equipment	12	0.610	0.1850	0.0001	10137
16.41	3-Wire De-Tuning Equipment	12	0.430	0.1385	0.0000	13516
11.28	3-Wire De-Tuning Equipment	12	0.274	0.0922	0.0000	20274
6.14	3-Wire De-Tuning Equipment	12	0.133	0.0461	0.0000	40548
1.00	3-Wire De-Tuning Equipment	0	0.000	0.0000	0.0000	41667

Maximum Tower Deflections - Design Wind

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	0
L1	150 - 107	124.226	4	7.5039	0.0011
L2	112 - 69.25	68.043	4	6.1193	0.0004
L3	75.5 - 39	29.374	4	3.7740	0.0002
L4	46 - 1	10.654	4	2.1619	0.0001

Critical Deflections and Radius of Curvature - Design Wind

Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load				Curvature
ft		Comb.	in	0	٥	ft
153.00	RFI CC807 10' Whip	4	124.226	7.5039	0.0023	6686
150.00	3-Wire De-Tuning Equipment	4	124.226	7.5039	0.0023	6686
148.00	3 ft standard	4	121.095	7.4465	0.0024	6686
144.86	3-Wire De-Tuning Equipment	4	116.188	7.3558	0.0026	6506
140.00	EPA 42,000 in2	4	108.627	7.2115	0.0029	3341

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Elevation	Appurtenance	Gov.	Deflection	Tilt	Twist	Radius of
		Load		0		Curvature
ft		Comb.	in		0	ft
139.72	3-Wire De-Tuning Equipment	4	108.200	7.2031	0.0030	3252
134.59	3-Wire De-Tuning Equipment	4	100.310	7.0411	0.0033	2166
129.45	3-Wire De-Tuning Equipment	4	92.568	6.8654	0.0035	1623
128.00	EPA 30,000 in2	4	90.419	6.8127	0.0036	1515
124.31	3-Wire De-Tuning Equipment	4	85.023	6.6753	0.0037	1297
119.17	3-Wire De-Tuning Equipment	4	77.725	6.4639	0.0038	1079
116.00	EPA 30,000 in2	4	73.363	6.3192	0.0038	977
114.03	3-Wire De-Tuning Equipment	4	70.723	6.2235	0.0038	928
108.90	3-Wire De-Tuning Equipment	4	64.064	5.9498	0.0037	882
103.76	3-Wire De-Tuning Equipment	4	57.767	5.6454	0.0035	894
103.00	Andrew FFVV-65B-R3 w/ mount	4	56.867	5.5983	0.0035	896
	pipe					
98.62	3-Wire De-Tuning Equipment	4	51.826	5.3180	0.0033	907
93.48	3-Wire De-Tuning Equipment	4	46.238	4.9757	0.0030	921
88.34	3-Wire De-Tuning Equipment	4	40.995	4.6262	0.0026	936
83.21	3-Wire De-Tuning Equipment	4	36.095	4.2777	0.0022	950
78.07	3-Wire De-Tuning Equipment	4	31.531	3.9380	0.0019	962
72.93	3-Wire De-Tuning Equipment	4	27.299	3.6149	0.0016	961
67.79	3-Wire De-Tuning Equipment	4	23.398	3.3107	0.0014	947
62.66	3-Wire De-Tuning Equipment	4	19.834	3.0226	0.0012	932
57.52	3-Wire De-Tuning Equipment	4	16.612	2.7475	0.0011	918
52.38	3-Wire De-Tuning Equipment	4	13.736	2.4822	0.0010	904
47.24	3-Wire De-Tuning Equipment	4	11.211	2.2237	0.0009	898
42.10	3-Wire De-Tuning Equipment	4	9.040	1.9691	0.0007	971
36.97	3-Wire De-Tuning Equipment	4	7.195	1.7171	0.0007	1109
31.83	3-Wire De-Tuning Equipment	4	5.636	1.4675	0.0006	1294
26.69	3-Wire De-Tuning Equipment	4	4.322	1.2199	0.0005	1552
21.55	3-Wire De-Tuning Equipment	4	3.213	0.9739	0.0004	1940
16.41	3-Wire De-Tuning Equipment	4	2.267	0.7293	0.0003	2586
11.28	3-Wire De-Tuning Equipment	4	1.443	0.4856	0.0002	3879
6.14	3-Wire De-Tuning Equipment	4	0.701	0.2426	0.0001	7758
1.00	3-Wire De-Tuning Equipment	0	0.000	0.0000	0.0000	7972

	Pol	le [Design	Data
--	-----	------	--------	------

Section No.	Elevation	Size	L	L_u	Kl/r	A	P_u	ϕP_n	Ratio
NO.	ft		ft	ft		in^2	K	K	$\frac{P_u}{\phi P_n}$
L1	150 - 107 (1)	TP35.6598x25x0.2188	43.00	0.00	0.0	23.7466	-22.99	1389.17	0.017
L2	107 - 69.25 (2)	TP44.5806x33.9828x0.375	42.75	0.00	0.0	50.7716	-37.38	2970.14	0.013
L3	69.25 - 39 (3)	TP51.3297x42.2813x0.5	36.50	0.00	0.0	77.9128	-51.40	4557.90	0.011
L4	39 - 1 (4)	TP59.75x48.5944x0.5625	45.00	0.00	0.0	105.672 0	-77.40	6181.80	0.013

Pole Bending Design Data

Section	Elevation	Size	M_{ux}	ϕM_{nx}	Ratio	M_{uy}	ϕM_{ny}	Ratio
No.					M_{ux}			M_{uy}
	ft		kip-ft	kip-ft	ϕM_{nx}	kip-ft	kip-ft	ϕM_{ny}
L1	150 - 107 (1)	TP35.6598x25x0.2188	1019.07	1067.21	0.955	0.00	1067.21	0.000
L2	107 - 69.25 (2)	TP44.5806x33.9828x0.375	3105.23	3192.17	0.973	0.00	3192.17	0.000
L3	69.25 - 39 (3)	TP51.3297x42.2813x0.5	4933.93	5830.32	0.846	0.00	5830.32	0.000
L4	39 - 1 (4)	TP59.75x48.5944x0.5625	7893.40	9416.25	0.838	0.00	9416.25	0.000
	(-)	TP59.75x48.5944x0.5625						

tnx	T_{Δ}		014
unx	1 U	w	er

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Pole Shear Design Data

Section	Elevation	Size	Actual	ϕV_n	Ratio	Actual	ϕT_n	Ratio
No.			V_u		V_u	T_u		T_u
	ft		K	K	ϕV_n	kip-ft	kip-ft	ϕT_n
L1	150 - 107 (1)	TP35.6598x25x0.2188	51.93	416.75	0.125	0.06	1248.26	0.000
L2	107 - 69.25 (2)	TP44.5806x33.9828x0.375	60.21	891.04	0.068	1.90	3328.59	0.001
L3	69.25 - 39 (3)	TP51.3297x42.2813x0.5	63.58	1367.37	0.046	1.89	5878.92	0.000
L4	39 - 1 (4)	TP59.75x48.5944x0.5625	67.49	1854.54	0.036	1.89	9612.75	0.000

Pole Interaction Design Data

Section No.	Elevation	Ratio P_u	Ratio M_{ux}	Ratio M_{uy}	$Ratio$ V_u	Ratio T_u	Comb. Stress	Allow. Stress	Criteria
	ft	ϕP_n	ϕM_{nx}	ϕM_{ny}	ϕV_n	ϕT_n	Ratio	Ratio	
L1	150 - 107 (1)	0.017	0.955	0.000	0.125	0.000	0.987	1.000	/
L2	107 - 69.25 (2)	0.013	0.973	0.000	0.068	0.001	0.990	1.000	~
L3	69.25 - 39 (3)	0.011	0.846	0.000	0.046	0.000	0.860	1.000	~
L4	39 - 1 (4)	0.013	0.838	0.000	0.036	0.000	0.852	1.000	~

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow} \ K$	% Capacity	Pass Fail
L1	150 - 107	Pole	TP35.6598x25x0.2188	1	-22.99	1389.17	98.7	Pass
L2	107 - 69.25	Pole	TP44.5806x33.9828x0.375	2	-37.38	2970.14	99.0	Pass
L3	69.25 - 39	Pole	TP51.3297x42.2813x0.5	3	-51.40	4557.90	86.0	Pass
L4	39 - 1	Pole	TP59.75x48.5944x0.5625	4	-77.40	6181.80	85.2	Pass
							Summary	
						Pole (L2)	99.0	Pass
						RATING =	99.0	Pass

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Anchor Rod and Base Plate Calculation

TIA-222-H

Factored Base Reactions: Pole Shape: Anchor Rods: Base Plate:

Moment: 7893 ft-kips 18-Sided (26) 2.25 in. A615 GR. 75 2.75 in. x 73.25 in. Rour Shear: 67 kips **Pole Dia.** (D_f): Anchor Rods Evenly Spaced fy = 50 ksi

Axial: 77 kips 59.75 in On a 67.25 in Bolt Circle

Anchor Rod Calculation According to TIA-222-H

 ϕ_t , $\phi_v = 0.75 \text{ TIA } 4.9.6$

 $I_{bolts} = 14698.33 \text{ in}^2 \text{ Momet of Inertia}$

P_u = 214 kips Tension Force

 $V_u = 2.6 \text{ kips Shear Force}$

Rnt = 325.00 kips Nominal Tensile Strength

Rnv = 198.80 kips (0.5 x fu x ag)

Stress Rating = 87.7% Satisfies TIA-H 4.9.9

Base Plate Calculation According to TIA-222-H

 $\phi = 0.90 \text{ TIA } 4.7$

 $M_{PL} = 508.1 \text{ in-kip Plate Moment}$

L = 7.2 in Section Length

Z = 13.6 Plastic Section Modulus 508.07 in-kip ≤ 614 in-kip

 $M_P = 682.5$ in-kip Plastic Moment

 ϕ M_n= 614.2 in-kip Factored Resistance

Stress Rating = 82.7%

Anchor Rods Are Adequate87.7%✓Base Plate is Adequate82.7%✓

Calculated Moment vs Factored Resistance

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	TAPP TP-25179	Mike

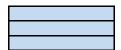
Caisson Calculation

According to TIA-222-H

- 1. Foundation overturning resistance calculated with PLS Caisson, for Brom's method for rigid piles. Soil layers modeled after recommendations from the geotechnical report.
- 2. Cohesion strength for the upper 22.5 ft has been reduced by 50%
- 3. An additional load factor of 1.3 has been applied to the reinforcement design
- 4. Foundation Loads Factored in Accordance with TIA-222-H
- 5. Design water table = 13.5 ft below grade

*** PIER PROPERTIES	CONCRETE STRENGTH	(ksi) = 4	1.50		STEEL ST	RENGTH (ksi) = 60.00
	DIAMETER (ft) =	7.500	DISTANCE F	ROM TOP OF P	IER TO GROUN	D LEVEL (ft) = 0.50
*** SOIL PROPERTIES	LAYER TYPE THIC	KNESS DEPTI	AT TOP OF	LAYER DEN	SITY	CU KP	PHI
		(ft)		(ft) (pcf) (ps	f)	(degrees)
		4.00		0 00 1	00 0	1.000	-0.0
		9.50		4.00 1	30.0	3.255	
	3 S	20.00	:	13.50	67.6	3.255	32.00
*** DESIGN (FACTORED)	LOADS AT TOP OF PIER		L-k) = 8869 L SAFETY FAC				c) = 75.0
*** CALCULATED PIER LE	NGTH (ft) = 27.50	0					
*** CHECK OF SOILS PRO	PERTIES AND ULTIMATE	RESISTING E	FORCES ALONG	PIER			
TYPE TOP OF LAYER	BELOW TOP OF PIER			CU			ARI
	(ft)	(ft)	(pcf)	(psf)		(k)	(ft
s	0.50	4.00	100.0		1.000	18.00	3.1
s	4.50	9.50	130.0		3.255	707.93	
s s			67.6 67.6		3.255	721.22 -1346.47	16.8 23.6
5	19.42	8.08	67.6		3.255	-1346.47	23.0.
*** SHEAR AND MOMENTS							
						DITIONAL SAFE	
DISTANCE BELOW TOP		SHEAR (r (ft-k)	SHEAR	(K) MOMI 75.5	ENT (ft-k) 9301.
	0.00 2.75	95		12402.2 12674.8		75.5	9506.4
	5.50			12897.7		36.5	9673.
	8.25	-94		12851.7	-		9639.
	11.00		2 0	12214 1		231.7	9235.
	13.75	_505	- 6	11086.9		46.7	8315.4
	16.50	-940	0.1	8984.1		05.1	6738.
	19.25	-1322	2.1	5882.2		91.6	4411.
	22.00			2684.7		112 5	2013.6
	24.75	-494		688.3	-3	370.8	516.3
	27.50	(0.0	0.0		0.0	0.0
*** TOTAL REINFORCEMEN	m DCm - 0.06 DET	NEOD CENTRAL	NDD3 (-42)	61.07			
*** TOTAL REINFORCEMEN *** USABLE AXIAL CAP.							
OBWDTE WYTHT CAL.		DUE MUMBRIT (

Pier and Pad Foundation



TIA-222 Revision: Tower Type: H Monopole

Top & Bot. Pad Rein. Different?:	
Block Foundation?:	
Rectangular Pad?:	

Superstructure Analysis Reactions					
Compression, P _{comp} :	77	kips			
Base Shear, Vu_comp:	75	kips			
Moment, M _u :	8869	ft-kips			
Tower Height, H :		ft			
BP Dist. Above Fdn, bp _{dist} :	6	in			

Pier Properties				
Pier Shape:	Square			
Pier Diameter, dpier:	7.5	ft		
Ext. Above Grade, E:	0.5	ft		
Pier Rebar Size, Sc :	10			
Pier Rebar Quantity, mc:	58			
Pier Tie/Spiral Size, St :	5			
Pier Tie/Spiral Quantity, mt:	10			
Pier Reinforcement Type:	Tie			
Pier Clear Cover, cc _{pier} :	3	in		

Pad Properties				
Depth, D:	6	ft		
Pad Width, W ₁ :	30	ft		
Pad Thickness, T :	4	ft		
Pad Rebar Size (Bottom dir. 2), Sp ₂ :	10			
Pad Rebar Quantity (Bottom dir. 2), mp ₂ :	30			
Pad Clear Cover, cc _{pad} :	3	in		

Material Properties					
Rebar Grade, Fy:	60	ksi			
Concrete Compressive Strength, F'c:	4.5	ksi			
Dry Concrete Density, δ c :	150	pcf			

Soil Properties					
Total Soil Unit Weight, γ :	110	pcf			
Ultimate Net Bearing, Qnet:	12.000	ksf			
Cohesion, Cu :		ksf			
Friction Angle, $oldsymbol{arphi}$:	20	degrees			
SPT Blow Count, N _{blows} :					
Base Friction, μ :					
Neglected Depth, N:		ft			
Foundation Bearing on Rock?					
Groundwater Depth, gw:	N/A	ft			

Foundation Analysis Checks									
	Capacity	Demand	Rating	Check					
Lateral (Sliding) (kips)	213.02	75.00	35.2%	Pass					
Bearing Pressure (ksf)	9.50	7.42	78.1%	Pass					
Overturning (kip*ft)	9801.76	9394.00	95.8%	Pass					
Pier Flexure (Comp.) (kip*ft)	12217.33	9056.50	74.1%	Pass					
Pier Compression (kip)	40277.25	102.31	0.3%	Pass					
Pad Flexure (kip*ft)	7246.32	5256.21	72.5%	Pass					
Pad Shear - 1-way (kips)	1561.09	560.23	35.9%	Pass					
Pad Shear - 2-way (Comp) (ksi)	0.201	0.000	0.0%	Pass					
Flexural 2-way (Comp) (kip*ft)	9296.35	5433.90	58.5%	Pass					

Soil Rating:	95.8%
Structural Rating:	74.1%

<--Toggle between Gross and Net

WELTI GEOTECHNICAL, P.C.

227 Williams Street · P.O. Box 397 Glastonbury, CT 06033-0397

(860) 633-4623 / FAX (860) 657-2514

October 16, 2025

Mr. Brett Buggeln Tarpon Towers, LLC 8916 77th Terrace East, Ste 103 Lakewood Ranch, FL 34202

Ref: Geotechnical Study for Proposed Cell Tower (CT1239), 161 Conrad Street, Naugatuck, CT

Dear Brett:

- 1.0 Herewith are the data from the test boring taken at the above referenced site. One boring was drilled at the proposed tower location to a depth of 30.4 feet below the existing grade. A tower/boring location plan is included with boring logs. The boring was drilled by Clarence Welti Associates, Inc. and sampling was conducted by this firm solely to obtain indications of subsurface conditions as part of a geotechnical exploration program. No services were performed to evaluate subsurface environmental conditions.
- 2.0 The **Subject Project** will include the construction of a 150-foot monopole tower.
- 3.0 The **Soils Cross Section** from the boring is generally as follows:

Topsoil to 3"

Fine to coarse SAND, little to some Gravel, little Silt to 13.5 feet, dense to very dense

Fine to coarse SAND, some Gravel, Cobbles and Boulders, little Silt to 30+ feet, dense to very dense

Note: Auger refusal was encountered at 13.5 feet below the existing grade. The boring was cored thru dense soil with cobbles and boulders from 13.5 to 30 feet.

Groundwater was in the borehole was below at 13.5 feet below the existing grade at the completion of the boring. The boring was drilled with water below 13.5 feet. An accurate reading could not be taken before the borehole collapsed.

- 4.0 In general the criteria for tower support is that the foundation capacity would exceed the loads, which might collapse the tower. Movements from strains in the soils should be limited to differential settlement (or lateral movements of less than ½").
- 5.0 The **foundation for the tower** can be with a large mat designed to prevent overturning by gravity resistance of the weight of the mat and soil cover. The mat foundation can be placed on the natural inorganic soils at least 4 feet below the existing grade. There should be a minimum 6" layer of 3/8" crushed stone beneath foundations on the natural soils. The **Allowable Bearing Pressure** on the crushed stone atop the natural soils can be 3.0 Tons/sf.
- 5.1 In **summary** the following soil properties and design values would apply to alternate 1.

Soil Property/Parameter	Value			
Soil Unit Weight (Backfill)	125 pcf			
Soil Unit Weight (Natural)	125 pcf			
Soil Unit Weight Submerged (Natural)	63 pcf			
Angle of Internal Friction (φ)	34°			
Cohesion	0			
Pull Out Angle from Vertical	30°			
Sliding Coefficient	0.6			
Frost Protection Depth (by code)	3.5 feet			
Allowable Soil Bearing Pressure on the natural soil inorganic at 4+ feet below the existing grade	3.0 Tons/sf			

6.0 Regarding **backfill of foundations**, the material should conform to the following or be 3/8" crushed stone.

Percent Passing	Sieve Size
100	3.5"
50 - 100	3/4"
25 - 85	No.4

The fraction, passing the No.4 sieve should have less than 15% passing the No. 200 sieve.

All backfill and fill must be compacted to at least 95% of modified optimum density in accordance with ASTM D-1557.

7.0 The soils at the subject site are generally in OSHA class C which would require excavations that are in excess of 5 feet to have slopes which are less than 34° (i.e., 1.5H to 1.0V).

8.0 This report has been prepared for specific application to the subject project in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. In the event that any changes in the nature, design and location of structures are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing.

The analyses and recommendations submitted in this report are based in part upon data obtained from referenced explorations. The extent of variations between explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

Welti Geotechnical, P.C., should perform a general review of the final design and specifications in order that geotechnical design recommendations may be properly interpreted and implemented as they were intended.

If you have any questions please call me.

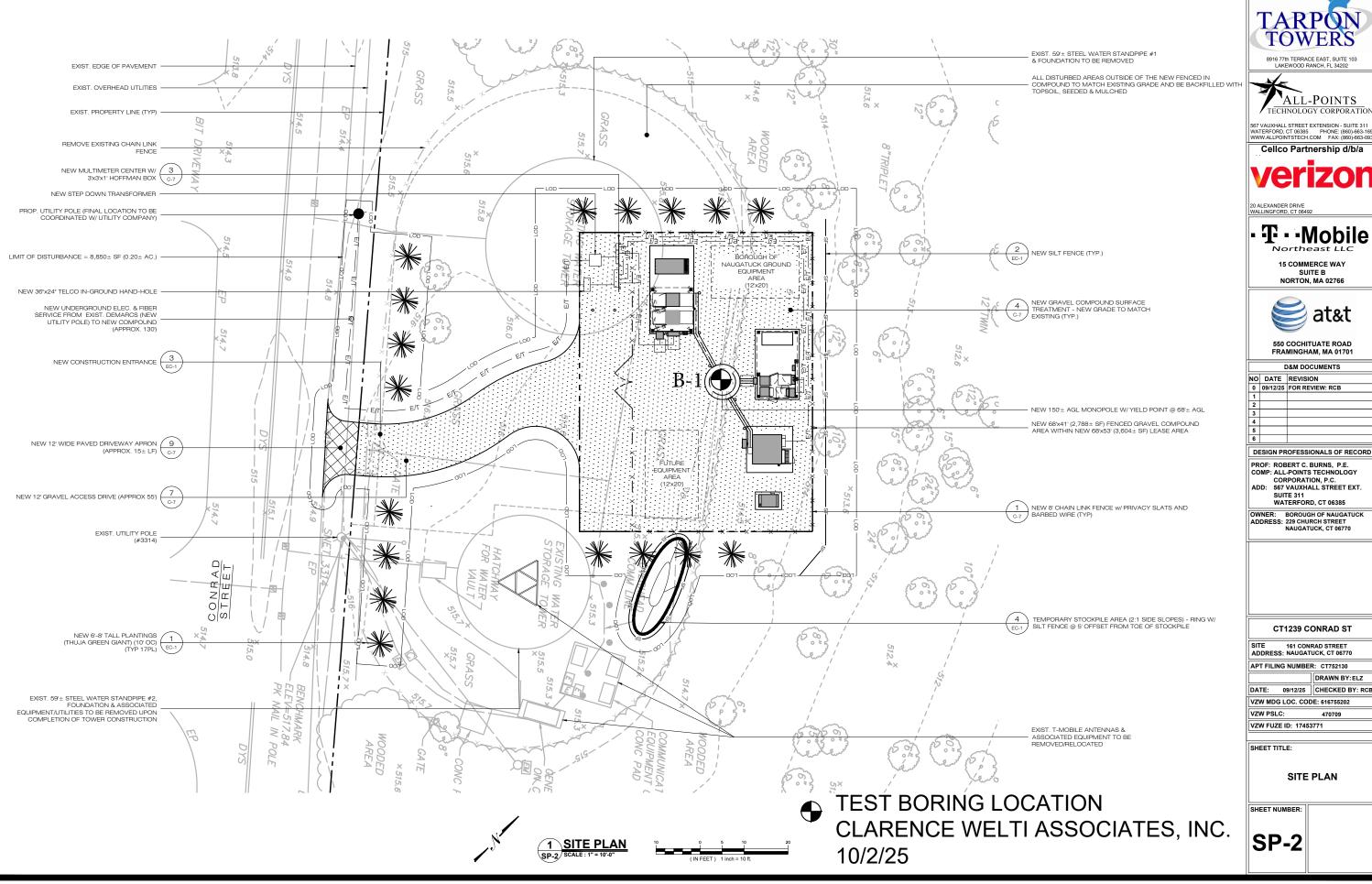
Very truly yours,

Max Welti, P. E.

Max Welti

President, Welti Geotechnical, P.C.

APPENDIX TEST BORING LOCATION + TEST BORING DATA







Cellco Partnership d/b/a



 $-\mathbf{T}$ --Mobile

SUITE B NORTON, MA 02766



550 COCHITUATE ROAD FRAMINGHAM, MA 01701

D&M DOCUMENTS									
NO	DATE	REVISION							
0	09/12/25	FOR REVIEW: RCB							
1									
2									
3									
4									
5									
6									

DESIGN PROFESSIONALS OF RECORD

PROF: ROBERT C. BURNS. P.E. COMP: ALL-POINTS TECHNOLOGY
CORPORATION, P.C.
ADD: 567 VAUXHALL STREET EXT. SUITE 311 WATERFORD, CT 06385

OWNER: BOROUGH OF NAUGATUCK ADDRESS: 229 CHURCH STREET NAUGATUCK, CT 06770

CT1239 CONRAD ST

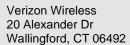
APT FILING NUMBER: CT752130

DRAWN BY: ELZ

VZW MDG LOC. CODE: 616755202

SITE PLAN

CLIENT CLIENT					PROJE	PROJECT NAME								
P.O. BOX 397			INC.					PROF LOCA	POSED CELL TION	TOWER (CT1239,	CON	IRAD ST)	
GLASTONBURY, CONN 06033				TARPON TOWERS, LLC			161 CONRAD STREET, NAUGATUCK, CT							
AUGER CASING SAMP		SAMPLI	PLER CORE BAR.		BAR.	OFFSET	SURFAC	URFACE ELEV. HOLE NO.			B-1			
TYPE	TYPE HSA SS		SS	S NQ			LINE & STA.	GROU	GROUND WATER OBSERVATIONS STAR'		START			
SIZE I.D		3.75"		1.375'	75" 2.0"		'	LONGITUDE	AT	DATI		DATE	10/2/25	
HAMMI	HAMMER WT.		140lbs	lbs			LATITUDE	AT	AT FT. AFTER HOURS FINIS		FINISH DATE	10/2/25		
HAMMI	IAMMER FALL 30		30"		•		LATITUDE				DATE			
DEPTH	NO.	SAM BLOWS/6'		PTH	A			STRATUM	RATUM DESCRIPTION + REMARKS					
0	1	11-60		-0.7'				DPSOIL GHT BR.FINE-CRS.SAND, LIT LT	TLE TO	SOME GRAV	EL, LITTL		.30	
	2	4-60	3.0'-	-3.8'		-								
5 –	3	50-53-40-6	0 5.0'-	-7.0'		- -								
						-								
10 –	4	41-60	10.0'-	-10.8'										
						-						1	3.5	
15 –						_	LIT	R.FINE-CRS.SAND, SOME GF				>,	3.5	
						-	: TH	OTE: ENCOUNTERED AUGEI IRU DENSE SOILS WITH CO .5 TO 30 FEET						
20						-	:	JN #1 13.5'- 18.5' RECOVERE JN #2 18.5'- 23.5' RECOVERE						
20 –							RU	JN #3 23.5'- 30.0' RECOVERE	ED 14"					
25 –						-								
						_								
30 -	5	60	30.0%	-30.4'		-	50	OTTOM OF DODING 2 22 "				з	0.4	
	-		33.0			1	BC	OTTOM OF BORING @ 30.4						
						1	BE	OTE: THERE WAS NO WATE LOW GRADE. THE BORING LOW THAT LEVEL.						
35_														
	ND: COL LE TYPE		=AUGER C=0	CORE U=U	UNDIS	TURBEI	D PIST		DRILLER: J. BREWER INSPECTOR:					
PROP	PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOME=20-35% AND=35-50%						% AND=35-50%	SHEET	1 OF 1	HOLE NO) .	B-	1	





November 24, 2025

Melanie A. Bachman, Esq. Executive Director/Staff Attorney Connecticut Siting Council 10 Franklin Square New Britain, CT 06051

Re: Docket No. 533 – Application of Tarpon Towers III, LLC and Cellco Partnership d/b/a Verizon Wireless for a Certificate of Environmental Compatibility and Public Need for the Construction, Maintenance and Operation of a Wireless Telecommunications Facility at 161 Conrad Street, Naugatuck, Connecticut

Dear Attorney Bachman:

In accordance with condition 2(a) of the Docket No. 533 Decision and Order ("D&O"), this letter serves as notice of Verizon Wireless' commitment to install and operate on the approved Naugatuck telecommunications facility upon completion of construction by Tarpon Towers III, LLC. Verizon Wireless anticipates that its equipment will be operational within the eighteen-month timeframe included in the Council's D&O.

Thank you for your consideration of this information.

Sincerely,

Andrew Candiello (Nov 24, 2025 10:29:24 EST)

Andrew Candiello

Verizon Wireless

Associate Director – Real Estate/Regulatory New England – Network Real Estate Cellco Partnership d/b/a Verizon Wireless