COHEN and WOLF P.C.
ATTORNEYS AT LAW

PHILIP C. PIRES

Please Reply To Bridgeport
E-Mail: ppires@cohenandwolf.com

February 23, 2023

Via e-mail and overnight mail

Attorney Melanie Bachman
Executive Director
Connecticut Siting Council
Ten Franklin Square
New Britain, CT 06051

Re: Docket No. 510 – Application of New Cingular Wireless PCS, LLC d/b/a AT&T and Tarpon Towers II, LLC for a Certificate of Environmental Compatibility and Public Need for the construction, maintenance, and operation of a telecommunications facility located at 92 Greens Farms Road, Westport, Connecticut.

Development and Management Plan

Dear Attorney Bachman:

On behalf of New Cingular Wireless PCS, LLC d/b/a AT&T ("AT&T") and Tarpon Towers II, LLC (the "Certificate Holders"), I've enclosed an original and fifteen (15) copies of the Certificate Holders' Development and Management Plan pertaining to the telecommunications facility approved by the Connecticut Siting Council ("Council") in the above-captioned docket (the "D&M Plan"). The Certificate Holders submit this D&M Plan in accordance with the Council's Decision and Order dated January 5, 2023 ("Decision").

Development and Management Plan

Pursuant to Order Number 1, the telecommunications facility to be located at 92 Greens Farms Road, Connecticut ("Facility") includes a monopole at a height of 124 feet above ground level ("AGL"). The monopole will accommodate the antennas of AT&T, Cellco Partnership d/b/a Verizon Wireless ("Verizon"), and other co-locators, both public and private. AT&T's antennas will be located at a centerline height of 120 feet AGL, and Verizon's antennas will be located at a centerline height of 110 feet AGL.

Pursuant to Order Number 2, the Certificate Holders have prepared a D&M Plan in accordance with the Decision and applicable Regulations.

The proposed D&M Plan includes:

a) Pursuant to Condition 2(a) of the Order, the Certificate Holders have provided the enclosed final site plans for the development of the facility that employ the governing standard in the State of Connecticut for tower design in accordance with the currently adopted International Building Code, including specifications for the tower, tower foundation, antennas and equipment compound, including fence design, ground equipment, access road, utility installation, and emergency backup power.

b) Pursuant to Condition 2(b) of the Order, the Certificate Holders have provided the enclosed site plans which include construction plans for site clearing, grading, water drainage and stormwater control, site stabilization measures during construction, and erosion and sedimentation controls consistent with the *2002 Connecticut Guidelines for Soil Erosion and Sediment Control*, as amended. The Certificate Holders have also provided a Geotechnical Study dated January 18, 2023 prepared by Welti Geotechnical, P.C.

c) Pursuant to Condition 2(c) of the Order, the Certificate Holders have reconfigured the vehicle turnaround located just before the access gate to increase the buffer between the limit of disturbance and Wetland 2. This is depicted on Sheets C-2, C-101, and C-102.

d) Pursuant to Condition 2(d) of the Order, the tower has been designed with a yield point to ensure that the tower setback radius remains within the boundaries of the subject property. Specifically, as shown in the Tapp Tower Drawings, the tower is designed for a maximum 34-foot fall radius. As shown on Sheet C-101 of the Certificate Holders' plans, the 34-foot fall radius remains within the boundaries of the subject property.

e) Pursuant to Condition 2(e) of the Order, the Certificate Holders have provided a landscaping plan which is reflected in Sheets C-101, C-102, and C-103, which includes evergreen plantings to screen the tower compound and associated equipment.

f) Pursuant to Condition 2(f), the Certificate Holders propose to use natural gas emergency backup generators for the site, as described and depicted on Sheets T-001

and C-102.

g) Pursuant to Condition 2(g), the Certificate Holders state that they have not provided a Petroleum Materials Storage and Spill Prevention Plan because they now intend to use natural gas emergency backup generators at the site.

h) Pursuant to Condition 2(h) of the Order, the Certificate Holders have provided the enclosed construction schedule including hours and days of the week for construction activities. Construction will occur Mondays through Fridays, 7:30 a.m. to 6:30 p.m. The Certificate Holders will coordinate with the Town of Westport, as necessary.

Pursuant to Order Number 4, prior to the commencement of operation, the Certificate Holders will provide the Council with a rigorous cumulative far-field radio frequency analysis for the facility that accounts for all entities on the tower, a 6-foot tall person at ground level and the actual antenna pattern for antennas on the facility with a cumulative percent maximum permissible exposure at or below 100 percent, consistent with FCC, Office of Engineering and Technology, Bulletin No. 65, August 1997. The Certificate Holders will further ensure a recalculated report of the electromagnetic radio frequency power density be submitted to the Council if and when circumstances in operation cause a change in power density above the levels calculated and provided pursuant to this D&M submittal.

Conclusion

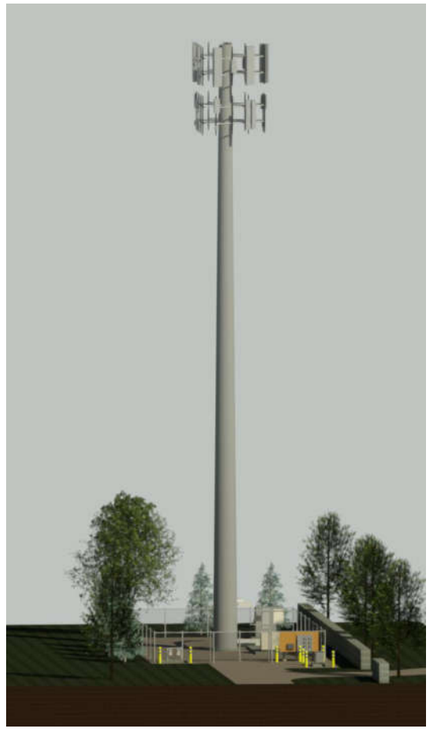
The Certificate Holders respectfully request that this matter be included on the Council's next agenda for review and approval. As indicated below, a copy of this D&M submittal has been provided to the service list. Please contact me if you have any questions.

Very truly yours,

Philip C. Pires

Enclosures

cc: Service List



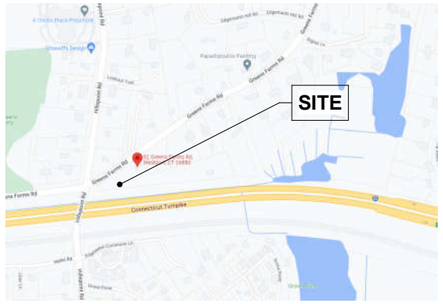
WIRELESS COMMUNICATIONS FACILITY

DOCKET NUMBER 510

CT1024A WESTPORT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT

VICINITY MAP



CBYD.811 .COM
Call Before You Dig, Inc.

CODE REFERENCES

2022 Connecticut State Building Code

2021 International Building Code
2021 International Existing Building Code
2021 International Plumbing Code
2021 International Mechanical Code
2021 International Residential Code
2021 International Energy Conservation Code
2020 National Electrical Code (NFPA 70)
2017 ICC A117.1 Accessible and Usable Buildings & Facilities

PROJECT SUMMARY

PROJECT NAME:	CT1024A WESTPORT
SITE ADDRESS:	92 GREENS FARMS ROAD
PARCEL ID:	E06/ / 074/000 /
TARPON TOWERS II CONTACT:	KEITH COPPINS 110 WASHINGTON AVENUE FOURTH FLOOR NORTH HAVEN, CT 06473 203.623.3287
LEGAL/REGULATORY COUNSEL:	COHEN AND WOLF, P.C. 1115 BROAD STREET BRIDGEPORT, CT 06604 203.337.4134
LAND LORD CONTACT:	PRADIV MAHESH & SHARUNA MOOLA-MAHESH 92 GREENS FARMS ROAD WESTPORT, CT 06880
ARCHITECT:	DOUGLAS J. ROBERTS - ARCHITECT 110 WASHINGTON AVENUE FOURTH FLOOR NORTH HAVEN, CT 06473
SURVEYOR:	NORTHEAST SURVEY CONSULTANTS 3 FERRY STREET STUDIO 1 EAST EAST HAMPTON, MA 01027
LATITUDE:	N 41° - 07' - 25.39"
LONGITUDE:	W 73° - 20' - 41.26"
GRADE (PROPOSED)	19' - 0" +/- AMSL

SHEET INDEX

SHEET NUMBER	SHEET NAME	CURRENT REVISION	CURRENT REVISION DATE
T - 001	TITLE SHEET	4	FEB. 11, 2023
GN - 001	GENERAL NOTES	4	FEB. 11, 2023
C - 1	ABUTTERS PLAN	2	JAN. 24, 2023
C - 2	EXISTING CONDITIONS PLAN	2	JAN. 24, 2023
C - 101	SITE PLAN	4	FEB. 11, 2023
C - 102	COMPOUND PLAN AND COMPOUND ISOMETRIC VIEW	4	FEB. 11, 2023
C - 103	EAST AND SOUTH ELEVATION	4	FEB. 11, 2023
C - 104	SITE DETAILS	4	FEB. 11, 2023
C - 105	EROSION CONTROL NOTES AND DETAILS	4	FEB. 11, 2023
C - 106	SITE DETAILS	4	FEB. 11, 2023
A - 101	VERIZON EQUIPMENT AND DETAILS	4	FEB. 11, 2023
A - 102	AT&T EQUIPMENT AND DETAILS	4	FEB. 11, 2023

SCOPE OF WORK

TARPON TOWERS II IS PROPOSING TO INSTALL THE FOLLOWING IMPROVEMENTS ON PROPOSED TELECOMMUNICATION SITE:

1. 35' x 64' FENCED COMPOUND WITHIN A 35' X 64' LEASE AREA.
2. 124' MONOPOLE AND FOUNDATION DESIGNED FOR FOUR CARRIER PLATFORMS WITH ANTENNAS.
3. POWER AND TELCO SERVICES WILL BE ROUTED UNDERGROUND FROM A PROPOESED UTILITY POLE ON GREEN FARMS ROAD TO PROPOSED TRASFORMER AND ELECTRICAL METER BANK AND TELCO BOX ON PROPOSED H-FRAME.
4. NATURAL GAS SERVICE WILL BE ROUTED UNDERGROUND FROM THE EXISTING GAS LINE WITHIN GREEN FARMS ROAD TO NATURAL GAS METER CENTER AT COMPOUND

VERIZON IS PROPOSING TO INSTALL THE FOLLOWING EQUIPMENT:

1. RADIO EQUIPMENT CABINETS AND 50 KW NATURAL GAS GENERATOR ON A CONCRETE PAD
2. FOUR (4) ANTENNAS PER SECTOR FOR A TOTAL OF TWELVE (12) ANTENNAS, TWO (2) HYBRID CABLES, ONE (1) OVB BOX, NINE (9) REMOTE RADIO HEADS

AT&T IS PROPOSING TO INSTALL THE FOLLOWING EQUIPMENT:

1. WALK IN CABINET (8'-0" x 8'-0") AND 15 KW NATURAL GAS GENERATOR ON A CONCRETE PADS
2. THREE (3) ANTENNAS PER SECTOR FOR A TOTAL OF NINE (9) ANTENNAS, TWO (2) HYBRID CABLES, THREE (3) SQUID SURGE ARESSTOR, FIFTEEN (15) REMOTE RADIO HEADS

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT
06880

Prepared For:

TARPON TOWERS II

8916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

DOUGLAS J. ROBERTS – ARCHITECT
110 Washington Avenue
Fourth Floor
North Haven, CT 06473

Tel: 203.234.6368
Email: droberts - architect@outlook.com

STATE OF CONNECTICUT
DOUGLAS J. ROBERTS
No. 4310
LICENSED ARCHITECT

Key Plan

Do not scale dimensions from drawings
Site verify all dimensions prior to construction
Report all discrepancies to Architect immediately
This drawing is to be read in conjunction with all relevant documents and drawings

REVISION SCHEDULE

REVISION	DESCRIPTION	DATE
1	Revision 1	JAN. 10, 2023
2	Revision 2	JAN. 24, 2023
3	Revision 3	FEB. 10, 2023
4	Revision 4	FEB. 11, 2023

Drawing By: Zachary J. Roberts
Drawing Date: JANUARY 2, 2023
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale: 12" = 1'-0"

Sheet Title:

TITLE SHEET

Original drawing is ANSI - D.
Do not scale contents of this drawing.

Sheet Number: T - 001
Revision: 4

2/13/2023 11:41:44 AM
E:\@ PROJECTS\TARPON TOWERS\CT\CT1024A WESTPORT 92 Green Farms Road Westport\1 - CT1024A WESTPORT CT MACRO\5 - DOCUMENTS\3 - CONNECTICUT SITING COUNCIL\3 - DEVELOPMENT AND MANAGEMENT PLAN\CT01024A WESTPORT CT D&M Rev. 4 02112023.rvt

GENERAL NOTES

1 FOR THE PURPOSE OF THE CONSTRUCTION DRAWINGS, THE FOLLOWING DEFINITION SHALL APPY:

CONTRACTOR - TARPON TOWERS II OR IT'S AFFILIATES

SUB CONTRACTOR - GENERAL CONTRACTOR (CONSTRUCTION)

OWNER - PROPERTY OWNER OF RECORDED / TARPON TOWERS II

2 PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE SITE TO FORMULARIZE THEMSELF WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DOCUMENTS

3 ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN COMPLIANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDNANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS IR ANT PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK.

4 ALL WORK SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL, STATE AND FEDERAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS

5 DRAWINGS PROVIDED HERE ARE NOT RO BE SCALED

6 UNLESS OTHERWISE NOTED, THE WORK SHALL INCLUDE ALL FINISHED MATERIALS, EQUIPMENT, APPURTENANCES AND ALL LABOR NEEDED TO COMPLETE THE INSTALLATIONS AS SHOWN ON THE CONTRACT DOCUMENTS.

7 THE SUB CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN APPROACNED EITIH THE MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS UNLESS OTHERWISE NOTED.

8 IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THE CONTRACT DOCUMENTS, THE SUBCONTRACTOR SHALL PROPOSE AND ALTERNATIVE INSTALLED FOR APPROVAL BY THE CONTRACTOR AND A&E.

9 SUBCONTRACTOR SHALL DETERMINE THE ACTUAL ROUTING OF ALL CONDUITS, TOWER, FIBER, AND GROUNDING CABLE AS SHOWN ON THE POWER, GROUNDING AND TELCO DRAWINGS.

10 THE SUB CONTRACTOR SHALL PROTECT EXISTING SITE IMPROVEMENTS, PAYMENTS, CURB, LANDSCAPING AND STRUCTURES. ANY DAMAGE PORTIONS SHALL BE REPAIRED AT THE SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF THE OWNER

11 THE SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATURES FOR THE SITE.

12 THE SUB CONTRACTOR SHALL LEAVE THE PREMISES IN A CLEANED CONDITION AND RETURN DISTURBED AREAS TO THERE ORIGINAL CONDITIONS.

13 THE SUBCONTRACTOR SHALL SUPERVISED AND THE PROJECT SCOPE AS DESCRIBED WITHIN THESE DOCUMENTS

14 THE SUB CONTRACTOR SHALL COMELY WITH ALL REQUIREMENT OF TARPON TOWERS II STANDARDS, SPECIFICATIONS AND ENVIRONMENTAL REPORTS

15 THE SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR RO COMMENCING AND Y WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS SHALL BE VERIFIED BY THE SUBCONTRACTOR. THE SUB CONTRACTOR SHALL NOTIFY THE CONTRACTOR AND A&E OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH THE CONSTRUCTION.

16 IN THE CASE OF A COLOCATION OR TOWER SWAP PLEASE NOTE THAT THE EQUIPMENT ON SITE MAY BE IN OPERATION AND ANY WORK ON OR NEAR THIS EQUIPMENT MUST BE COORDINATIZED WITH THE CONTRACTOR.

17 IF ANY RADIO EQUIPMENT IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROTECHNICAL RADIATION. THIS EQUIPMENT SHOULD BE SHUT DOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE ANY WORKERS TO DANGER. PERSONNEL rf MONITORS ARE RO BE WORN TO ALERT ONE OF ANY DANGEROUS EXPOSURE LEVELS IF WORKING NEAR ANTENNAS

SITE WORK GENERAL NOTES

1 THE SUBCONTRACTOR SHALL CONTRACT WITH A UTILITY LOCATION SERVICE AND HAVE ANY UTILITY'S MARKED THAT ARE NOT LOCATED BY CBYD PRIOR TO THE START OF CONSTRUCTION

2 ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTIC AND ANY OTHER UTILITY'S WHERE ENCOUNTERED DURING CONSTRUCTION SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPOSED EXECUTION OF THE WORK, SHALL BE RELOCATION AS DIRECTED BY THE A&E. EXTREAM CAUTION SHALL BE USED BY THE SUBCONTRACTOR WHEN EXCAVATING OR DRILLING NEAR UTILITY'S AND ANY SITE IMPROVEMENTS. THE SUBCONTRACTOR SHALL PROVIDED SAFETY TRAINING FOR ALL STAFF, THIS TRAINING WILL INCUDE BUT IS NOT LIMITED TO, FALL PROTECTION, CONFINED SPACE, ELECTRICAL SAFETY, AND TRENCHING AND EXCAVATION

3 ALL SITE WORK IS SHOWN AND INDICATED ON THE CONTRACT DRAWINGS AND SPECIFICATIONS

4 ALL RUBBISH STUMPS, STONE, ORGANIC MATERIALS NOT BEING REUSED ON SITE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY

5 THE SITE SHALL BE GRADED TO PREVENT SURFACE WATER FROM PONDING WITHIN THE COMPOUND AND EQUIPMENT

6 NO MATERIAL SHALL BE PLACED ON FROZEN MATERIALS NORE SNOW OR ICE

7 THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO THE INSTALLATION OF THE FINISHED SURFACE MATERIALS

8 ALL ABANDON OR INACTIVE SEWER, WATER, GAS, ELECTRIC AND MISALANIOUS UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED , PLUGED OR OTHERWISE DISCONTINUED AT POINTS WHITCH WILL NOT INTERFERE WITH THE EXECUTION OF THE PROPOSED WORK, SUBJECT TO THE APPROVALS OR THE A&E, OWNER/LAND LORD AND/OR LOCAL UTILITIES

9 THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE PROPOSED WORK IMPACTED BY THE PROPOSED TOWER, EQUIPMENT OR ACCESS ROAD AND TURNAROUND, SHALL BE GRADED TI A UNIFORM SLOPE AND STABILIZED TO PREVENT EROSION AS SPECIFIED IN THE PROJECT DOCUMENTS

10 THE SUBCONTRACTOR SHALL MINIMIZE THERE DISTURBANCE TO THE SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMATIONS WITH THE GUYED LINES FOR SEDIMENTATION AND EROSION CONTROL.

11 THE SUBCONTRACTOR SHALL COMPLY WITH WORK HOURS OF 8:00 AM TO 5:00 PM UNLESS OTHERWISE NOTED.

12 THE SUB CONTRACTOR SHALL PROVIDE AND INSTALL SITE SIGNAGE IN ACCORDANCE WITH THE TARPOMN TOWERS II SPECIFICATIONS FOR SIGNAGE

STRUCTURAL NOTES

1 DESIGN REQUIREMENTS ARE PER STATE OF CONNECTICUT BUILDING CODE AND APPLICABLE SUPPLEMENTS, INTERNATIONAL BUILDING CODE (IBC 2015), ASCE 7-05, EIA/TIA-222-H STRUCTURAL STANDARDS FOR STEEL ANTENNA, TOWERS AND ANTENNA SUPPORTING STRUCTURES.

2 CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE CONSTRUCTION MANAGER AND ENGINEER OF RECORD.

3 DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".

4 STRUCTURAL STEEL SHALL CONFORM TO ASTM A992 (Fy=50 ksi), MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A36 UNLESS OTHERWISE INDICATED.

5 STEEL PIPE SHALL CONFORM TO ASTM A500 "COLD-FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING", GRADE B, OR ASTM A53 PIPE STEEL BLACK AND HOT-DIPPED ZINC-COATED WELDED AND SEAMLESS TYPE E OR S, GRADE B. PIPE SIZES INDICATED ARE NOMINAL. ACTUAL OUTSIDE DIAMETER IS LARGER.

6 STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLTS (BEARING TYPE) AND CONFORM TO ASTM A325 TYPE-X "HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS". ALL BOLTS SHALL BE 3/4" DIA UON.

7 ALL STEEL MATERIALS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS", UNLESS OTHERWISE NOTED.

8 ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.

9 FIELD WELDS, DRILL HOLES, SAW CUTS AND ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED WITH AN ORGANIC ZINC REPAIR PAINT COMPLYING WITH REQUIREMENTS OF ASTM A780. GALVANIZING REPAIR PAINT SHALL HAVE 65 PERCENT ZINC BY WEIGHT, ZIRP BY DUNCAN GALVANIZING, GALVA BRIGHT PREMIUM BY CROWN OR EQUAL. THICKNESS OF APPLIED GALVANIZING REPAIR PAINT SHALL BE NOT NOT LESS THAN 4 COATS (ALLOW TIME TO DRY BETWEEN COATS) WITH A RESULTING COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.

10 CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND FOR METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND DI.I. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION". 14TH EDITION.

11 INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON-CONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE CONSTRUCTION MANAGER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE CONSTRUCTION MANAGER APPROVAL.

12 UNISTRUT SHALL BE FORMED STEEL CHANNEL STRUT FRAMING AS MANUFACTURED BY UNISTRUT CORP., WAYNE, MI OR EQUAL. STRUT MEMBERS SHALL BE 1 5/8"x1 5/8"x12GA, UNLESS OTHERWISE NOTED, AND SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION.

13 EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF STAINLESS STEEL ANCHOR ROD WITH NUTS & WASHERS. AN INTERNALLY THREADED INSERT, A SCREEN TUBE AND A EPOXY ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HILTI-HIT HY-20 AND OR HY-150 SYSTEMS (AS SPECIFIED IN DWG.) OR ENGINEERS APPROVED EQUAL.

14 EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION FF-S-325, GROUP II, TYPE 4, CLASS I, HILTI KWIK BOLT III OR APPROVED EQUAL. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

15 LUMBER SHALL COMPLY WITH THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION AND THE NATIONAL FOREST PRODUCTS ASSOCIATION'S NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. ALL LUMBER SHALL BE PRESSURE TREATED AND SHALL BE STRUCTURAL GRADE NO. 2 OR BETTER.

16 WHERE ROOF PENETRATIONS ARE REQUIRED, THE CONTRACTOR SHALL CONTACT AND COORDINATE RELATED WORK WITH THE BUILDING OWNER AND THE EXISTING ROOF INSTALLER. WORK SHALL BE PERFORMED IN SUCH A MANNER AS TO NOT VOID THE EXISTING ROOF WARRANTY. ROOF SHALL BE WATERTIGHT.

17 ALL FIBERGLASS MEMBERS USED ARE AS MANUFACTURED BY STRONGWELL COMPANY OF BRISTOL, VA 24203. ALL DESIGN CRITERIA FOR THESE MEMBERS IS BASED ON INFORMATION PROVIDED IN THE DESIGN MANUAL. ALL REQUIREMENTS PUBLISHED IN SAID MANUAL MUST BE STRICTLY ADHERED TO.

18 NO MATERIALS TO BE ORDERED AND NO WORK TO BE COMPLETED UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED IN WRITING.

SPECIAL INSPECTIONS (REFERENCE IBC CHAPTER 17):

1. GENERAL: WHERE APPLICATION IS MADE FOR CONSTRUCTION, THE OWNER OR THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE INSPECTION CHECKLIST ABOVE.
2. THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND ENGINEERS OF RECORD INVOLVED IN THE DESIGN OF THE PROJECT ARE PERMITTED TO ACT AS THE APPROVED AGENCY AND THEIR PERSONNEL ARE PERMITTED TO ACT AS THE SPECIAL INSPECTOR FOR THE WORK DESIGNED BY THEM, PROVIDED THOSE PERSONNEL MEET THE QUALIFICATION REQUIREMENTS.
3. STATEMENT OF SPECIAL INSPECTIONS: THE APPLICANT SHALL SUBMIT A STATEMENT OF SPECIAL INSPECTIONS PREPARED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IN ACCORDANCE WITH SECTION 107.1 AS A CONDITION FOR ISSUANCE. THIS STATEMENT SHALL BE IN ACCORDANCE WITH SECTION 1705.
4. REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS SHALL BE SUBMITTED.

SPECIAL INSPECTION CHECK LIST

PRE CONSTRUCTION ACTIVATES	
CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD)	REPORT ITEM
REQUIRED	ENGINEER OF RECORD APPROVED SHOP DRAWINGS
REQUIRED	MATERIAL CERTIFICATION REPORTS
REQUIRED	PACKING SLIPS
REQUIRED	BORROWED FILL CERTIFICATION
REQUIRED	CONCRETE MIX DESIGN
DURING CONSTRUCTION ACTIVATES	
CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD)	REPORT ITEM
REQUIRED	SUB BASE INSPECTIONS
REQUIRED	BASE INSPECTIONS
REQUIRED	FORMWORK INSPECTIONS
REQUIRED	REINFORCEMENT STEEL INSPECTIONS
REQUIRED	ANCHOR BOLT INSPECTIONS
REQUIRED	CONCRETE SLUMP AND AIR ENTRAINMENT INSPECTIONS
REQUIRED	CONCRETE COMPRESSIVE TESTING
REQUIRED	CONCRETE PROTECTION
REQUIRED	BACK FILL COMPACTION TESTING
POST CONSTRUCTION	
CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD)	REPORT ITEM
REQUIRED	CONCRETE COMPRESSIVE TESTS REPOTS
REQUIRED	BACK FILL COMPACTION TEST REPORTS
REQUIRED	AS BUILT DRAWINGS
REQUIRED	PHOTOS OF ALL CONSTRUCTION ACTIVATES

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT
06880

Prepared For:

TARPON TOWERS II

8916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

DOUGLAS J. ROBERTS – ARCHITECT
110 Washington Avenue
Fourth Floor
North Haven, CT 06473

Tel: 203.234.6368
Email: droberts - architect@outlook.com

STATE OF CONNECTICUT · DOUGLAS J. ROBERTS · No. 43310 · LICENSED ARCHITECT

Key Plan

Do not scale dimensions from drawings
Site verify all dimensions prior to construction
Report all discrepancies to Architect immediately
This drawing is to be read in conjunction with all relevant documents and drawings

REVISION SCHEDULE

REVISION	DESCRIPTION	DATE
1	Revision 1	JAN. 10, 2023
2	Revision 2	JAN. 24, 2023
3	Revision 3	FEB. 10, 2023
4	Revision 4	FEB. 11, 2023

Drawing By: Zachary J. Roberts
Drawing Date: JANUARY 2, 2023
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale: 12" = 1'-0"

Sheet Title:

GENERAL NOTES

Original drawing is ANSI - D.
Do not scale contents of this drawing.

Sheet Number: GN - 001

Revision: 4

2/13/2023 11:41:45 AM
E:\@ PROJECTS\TARPON TOWERS\CT\CT1024A WESTPORT 92 Green Farms Road Westport\1 - CT1024A WESTPORT CT MACRO\5 - DOCUMENTS\3 - CONNECTICUT SITING COUNCIL\3 - DEVELOPMENT AND MANAGEMENT PLAN\CT01024A WESTPORT CT D&M Rev. 4 02112023.rvt

SITE SPECIFIC NOTES:

1. FIELD SURVEY DATE: 4–28–2021
2. HORIZONTAL DATUM: NORTH AMERICAN DATUM OF 1983 (NAD83)
3. VERTICAL DATUM: NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88)
4. OWNER: PRADIV MAHESH & SHARUNA MOOLA–MAHESH 92 GREEN FARMS ROAD WESTPORT, CT 06880
5. SITE NAME: CT1024 WESTPORT
6. SITE ADDRESS 92 GREENS FARMS ROAD WESTPORT, CT 06880
7. JURISDICTION: TOWN OF WESTPORT FAIRFIELD COUNTY
8. TAX ID: E06–074A
9. DEED REFERENCE: DEED VOL. 2415 PAGE 2
10. PLAN REFERENCE: PLAN#7333
11. ZONING DISTRICT: AA

12. THE HORIZONTAL DATUM AND VERTICAL DATUM WERE DERIVED FROM AN RTK GPS SURVEY.

13. ALL UNDERGROUND UTILITY INFORMATION PRESENTED HEREON WAS DETERMINED FROM SURFACE EVIDENCE AND PLANS OF RECORD. ALL UNDERGROUND UTILITIES SHOULD BE LOCATED IN THE FIELD PRIOR TO COMMENCEMENT OF ALL SITE WORK. CALL DIGSAFE 1–800–322–4844 A MINIMUM OF 72 HOURS PRIOR TO PLANNED ACTIVITY.

14. ACCORDING TO FEDERAL EMERGENCY MANAGEMENT AGENCY MAPS, THE LOCUS PROPERTY IS LOCATED IN AREAS DESIGNATED AS ZONE AE (BFE=10') AND ZONE X (AREAS OF MINIMAL FLOODING). COMMUNITY PANEL NO. 09001C 0551 G EFFECTIVE DATE: 7/8/2013

15. FIELD SURVEY BY EDM TOTAL STATION & RTK GPS. THE HORIZONTAL DATUM AND VERTICAL DATUM WERE DERIVED FROM AN RTK GPS SURVEY.

16. THIS IS NOT A BOUNDARY SURVEY.

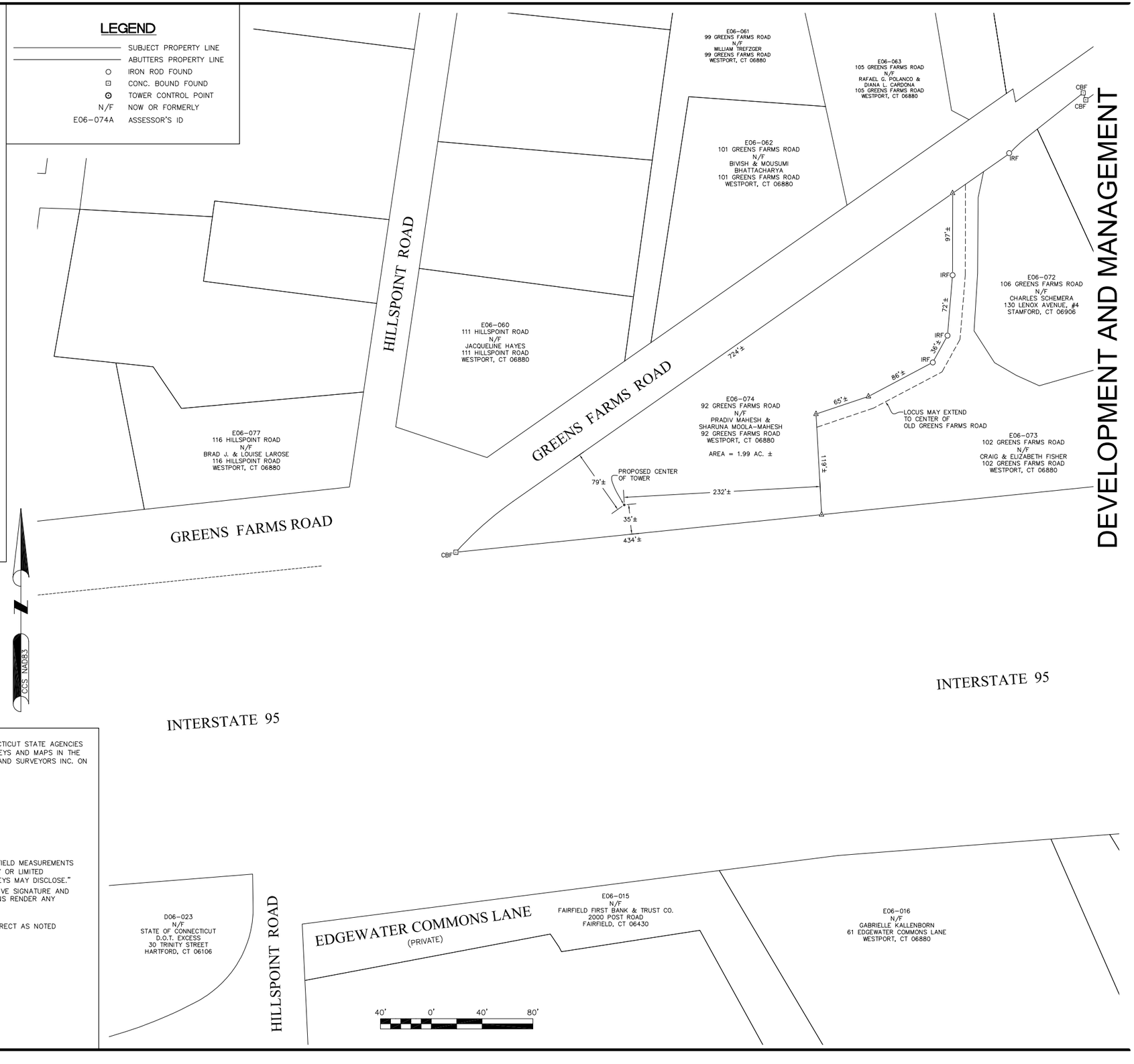
17. ALL PROPERTY LINES SHOWN ARE FROM DEEDS AND PLANS OF RECORD, MONUMENTS FOUND AND THE TOWN OF WESTPORT GIS AND ARE APPROXIMATE ONLY.

18. ABUTTING PROPERTY LINES ARE TAKEN FROM THE REFERENCE PLANS AND THE TOWN OF WESTPORT GIS AND ARE APPROXIMATE ONLY.

19. WETLANDS AND 2 CULVERTS SHOWN ON HEREON WERE DELINEATED AND LOCATED BY ALL–POINTS TECHNOLOGY CORPORATION, PC.

LEGEND

———	SUBJECT PROPERTY LINE
———	ABUTTERS PROPERTY LINE
○	IRON ROD FOUND
⊡	CONC. BOUND FOUND
⊙	TOWER CONTROL POINT
N/F	NOW OR FORMERLY
E06–074A	ASSESSOR'S ID



THIS SURVEY HAS BEEN PREPARED PURSUANT TO THE REGULATIONS OF CONNECTICUT STATE AGENCIES SECTIONS 20–300B–1 THROUGH 20–300B–20 AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS INC. ON SEPTEMBER 26, 1997.

TYPE OF SURVEY: IMPROVEMENT LOCATION SURVEY

BOUNDARY SURVEY CATEGORY: DEPENDENT RESURVEY

CLASS OF ACCURACY: HORIZONTAL CLASS C
VERTICAL CLASS V–2
TOPOGRAPHIC CLASS T–2

PURPOSE OF SURVEY: SITE PLAN FOR CELLULAR MONOPOLE

1. "THIS MAP WAS PREPARED FROM RECORD RESEARCH, OTHER MAPS, LIMITED FIELD MEASUREMENTS AND OTHER SOURCES. IT IS NOT TO BE CONSTRUED AS A PROPERTY/BOUNDARY OR LIMITED PROPERTY/BOUNDARY SURVEY AND IS SUBJECT TO SUCH FACTS AS SAID SURVEYS MAY DISCLOSE."
2. THIS DOCUMENT AND COPIES THEREOF ARE VALID ONLY IF THEY BEAR THE LIVE SIGNATURE AND EMBOSSED SEAL OF THE DESIGNATED PROFESSIONAL. UNAUTHORIZED ALTERATIONS RENDER ANY DECLARATION NULL AND VOID.

TO THE BEST OF MY KNOWLEDGE AND BELIEF, THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON.

STATE OF CONNECTICUT CHARLES G. GIDMAN 970105 LICENSED LAND SURVEYOR

CHARLES G. GIDMAN, P.L.S. #70103

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT

92 GREENS FARMS ROAD
WESTPORT, CT

Prepared For:

TARPON TOWERS II

8916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

DOUGLAS J. ROBERTS– ARCHITECT
110 Washington Avenue
Fourth Floor
North Haven, CT 06473

Tel: 203.234.6368
Email: droberts - architect@outlook.com

Key Plan

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REVISION SCHEDULE

REVISION	DESCRIPTION	DATE
0	REVIEW	03/09/2022
1	ADD WETFLAG #S	08/19/2022
2	REV. PROP. DRIVE	01/24/2023

Drawing By:	Brian Franetovich, PLS
Drawing Date:	January 24, 2023
Reviewed By:	Charles Gidman, PLS
Project No:	2021.12
Scale:	AS SHOWN

Sheet Title:

ABUTTERS PLAN

Original drawing is ANSI – D.
Do not scale contents of this drawing.

Sheet Number: C-1

Revision: 2

LEGEND

- SUBJECT PROPERTY LINE
- ABUTTERS PROPERTY LINE
- WIRE FENCE
- CHAIN LINK FENCE
- STOCKADE FENCE
- CONTOUR LINE
- OHW — OVERHEAD WIRES
- WETLAND DELINEATION
- FEMA FLOOD ZONE LINE
- N/F — NOW OR FORMERLY
- E06–074 — ASSESSORS ID
- TREE
- STAKE
- UTILITY POLE
- GUY WIRE ANCHOR
- TOWER CONTROL POINT
- WETLAND FLAG

E06–060
111 HILLSPOINT ROAD
N/F
JACQUELINE HAYES
111 HILLSPOINT ROAD
WESTPORT, CT 06880

GREENS FARMS ROAD

NGS NAD83

UP #23K11–91T

CONTROL POINT A
–IRON ROD SET–
N:606476.90
E:836139.78

STONE RET. WALL

HOUSE

8" MAPLE

10" MAPLE

14" DEAD PINE

12" CEDAR

DECORATIVE WELL

PROPOSED OVERHEAD WIRES

PROPOSED POLE

BIT. ROAD

GRAVEL

PROPOSED DRIVE

10" ASH

12" ASH

16" OAK

10" CHERRY

6" CHERRY

14" CHERRY

8" CHERRY

6" ELM

PROPOSED UNDERGROUND ELEC. LINE

E06–074
92 GREENS FARMS ROAD
N/F
PRADIV MAHESH &
SHARUNA MOOLA–MAHESH
92 GREENS FARMS ROAD
WESTPORT, CT 06880

AREA = 1.99 AC. ±

14" ASH

15" OAK

13" ASH

18" MAPLE

24" MAPLE

WALL

SHED

16" MAPLE

8" ASH

13" ASH

10" ASH

14" ASH

RETAINING WALL

8" CHERRY

PROPOSED ACCESS & UTILITY EASEMENT

20" ASH

16" MAPLE

TBM
SPIKE IN ROOT
EL=28.30'

6" CHERRY

8" OAK

6" ASH

STONE

PROPOSED COMPOUND FENCE

PROPOSED EQUIPMENT

PROPOSED DRIVE

20" CHERRY

24" ASH

24" MAPLE

CONTROL POINT B
–IRON ROD SET–
N:606365.28
E:835979.05

UP#11509
(GUY POLE)

14" TWIN ASH

6" MAPLE

6" OAK

20" ASH

6" ASH

PROPOSED CENTER OF TOWER
LAT: 41°07'25.22"
LON: 73°20'41.45"
N: 606351.60
E: 836109.67
GROUND EL: 19'±

PROPOSED EQUIPMENT

14" MAPLE

13" OAK

6" ASH

6" CHERRY

UP#26138

STONE RETAINING WALL

14" ASH

12" ASH

16" ASH

8" ASH

7" CHERRY

10" MAPLE

14" MAPLE

12" MAPLE

11" LOCUST

10" CEDAR

6" LOCUST

6" LOCUST

6" BIRCH

7" CHERRY

7" CHERRY

20" OAK

16" MAPLE

22" OAK

2–11

2–10

2–09

2–08

2–07

2–06

FEMA ZONE X

FEMA ZONE AE
(BFE=10')

WETLAND

72" RCP

2–04

2–05

2–03

2–02

2–01

WETLAND

1–06

1–05

1–04

1–03

1–02

1–01

WETLAND

INTERSTATE 95

HIGHWAY FENCE

15' 0' 15' 30'

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT

92 GREENS FARMS ROAD
WESTPORT, CT

Prepared For:

TARPON TOWERS II

8916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

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STATE OF CONNECTICUT · CHARLES S. GIDMAN · 20105 · LICENSED LAND SURVEYOR

Key Plan

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Drawing By:	Brian Franetovich, PLS
Drawing Date:	January 24, 2023
Reviewed By:	Charles Gidman, PLS
Project No:	2021.12
Scale:	AS SHOWN

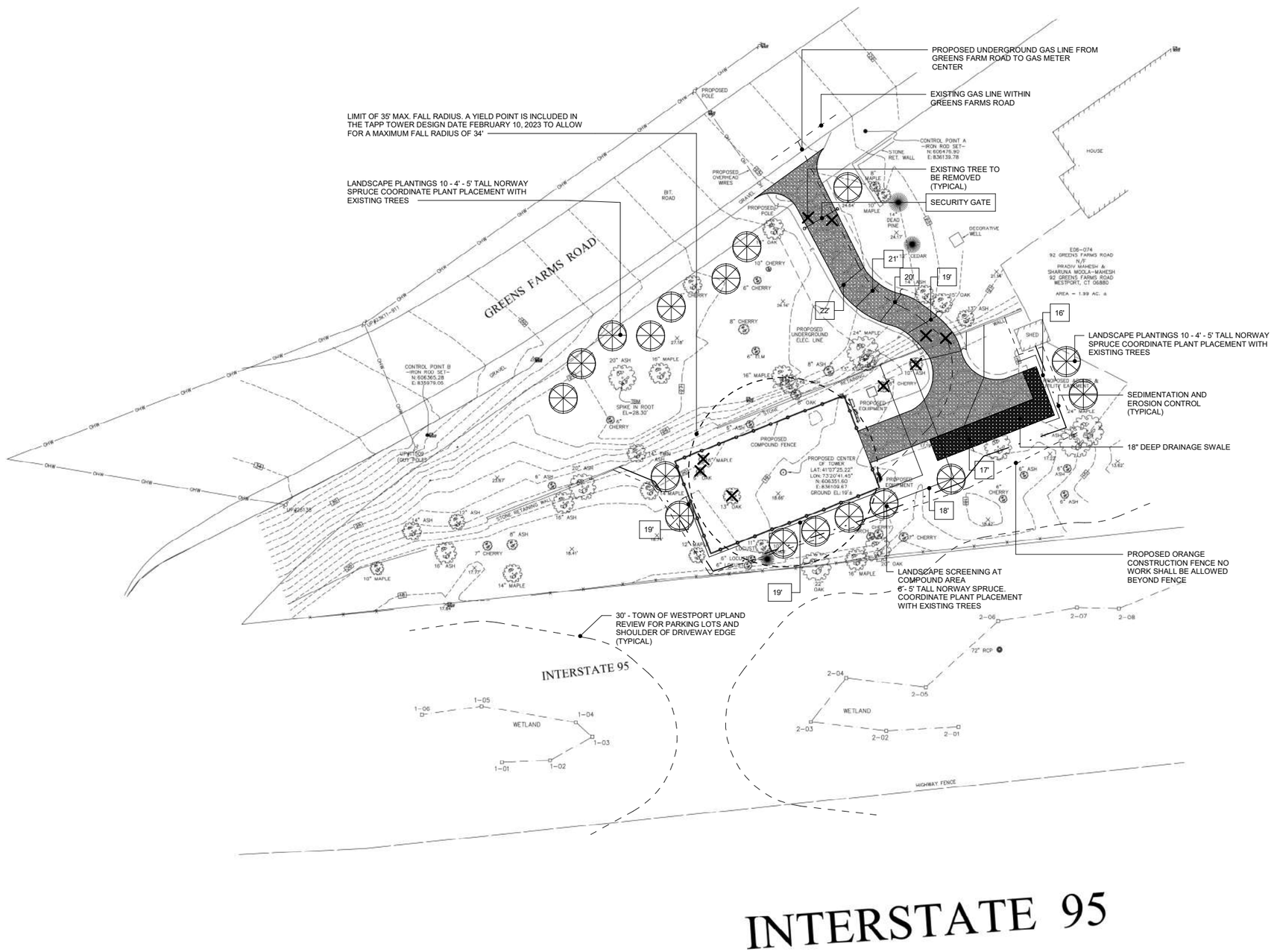
Sheet Title:

EXISTING CONDITIONS PLAN

Original drawing is ANSI – D.
Do not scale contents of this drawing.

Sheet Number: C-2

Revision: 2



1 C - 101 SITE PLAN
1" = 20'-0"

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT
06880

Prepared For:

TARPON TOWERS II

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STATE OF CONNECTICUT · DOUGLAS J. ROBERTS · No. 4310 · LICENSED ARCHITECT

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REVISION	DESCRIPTION	DATE
1	Revision 1	JAN. 10, 2023
2	Revision 2	JAN. 24, 2023
3	Revision 3	FEB. 10, 2023
4	Revision 4	FEB. 11, 2023

Drawing By: Zachary J. Roberts
Drawing Date: JANUARY 2, 2023
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale: 1" = 20'-0"

Sheet Title:

SITE PLAN

Original drawing is ANSI - D.
Do not scale contents of this drawing.

Sheet Number: C - 101
Revision: 4

2/13/2023 11:41:53 AM
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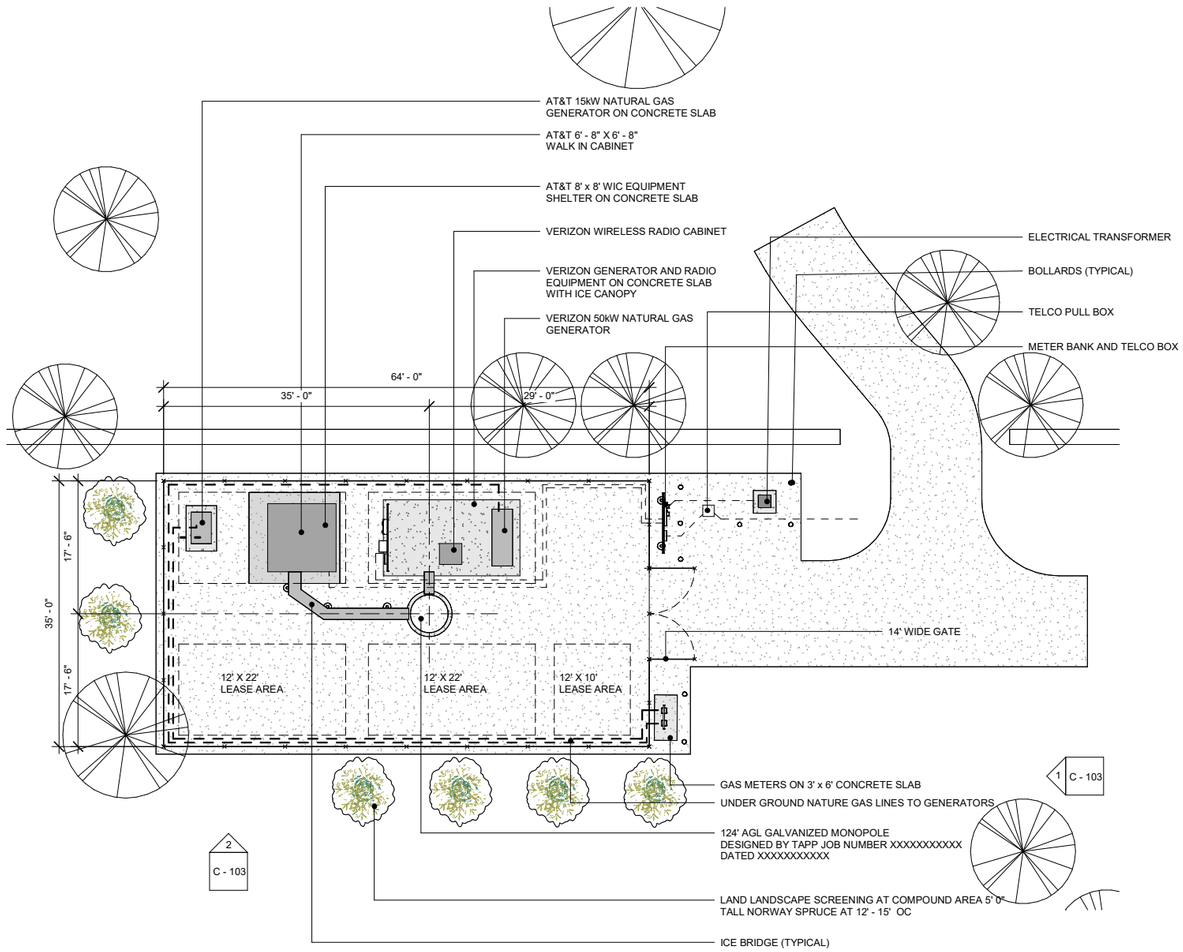

1 C - 102 COMPOUND PLAN
1" = 10'-0"

2 C - 102 COMPOUND ISOMETRIC VIEW

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT
06880

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STATE OF CONNECTICUT
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No. 4310
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Scale: 1" = 10'-0"

Sheet Title:

COMPOUND PLAN AND COMPOUND ISOMETRIC VIEW

Original drawing is ANSI - D
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Sheet Number: C - 102
Revision: 4

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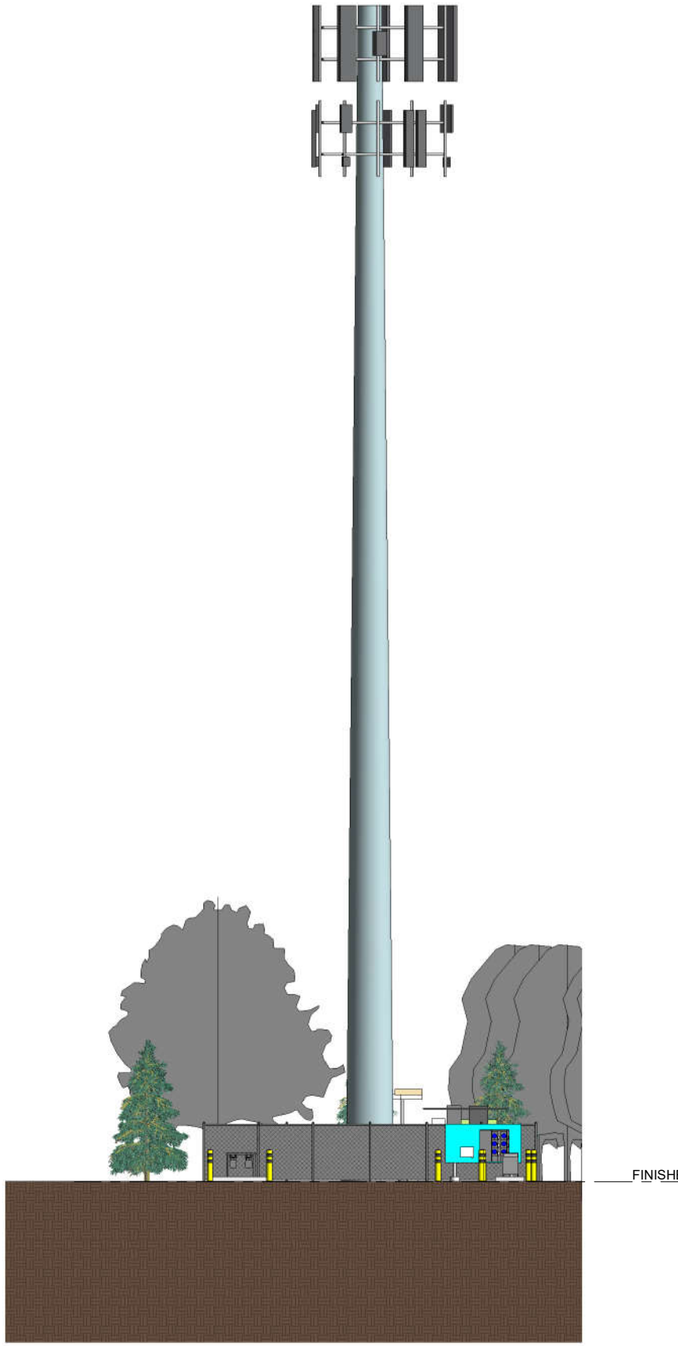
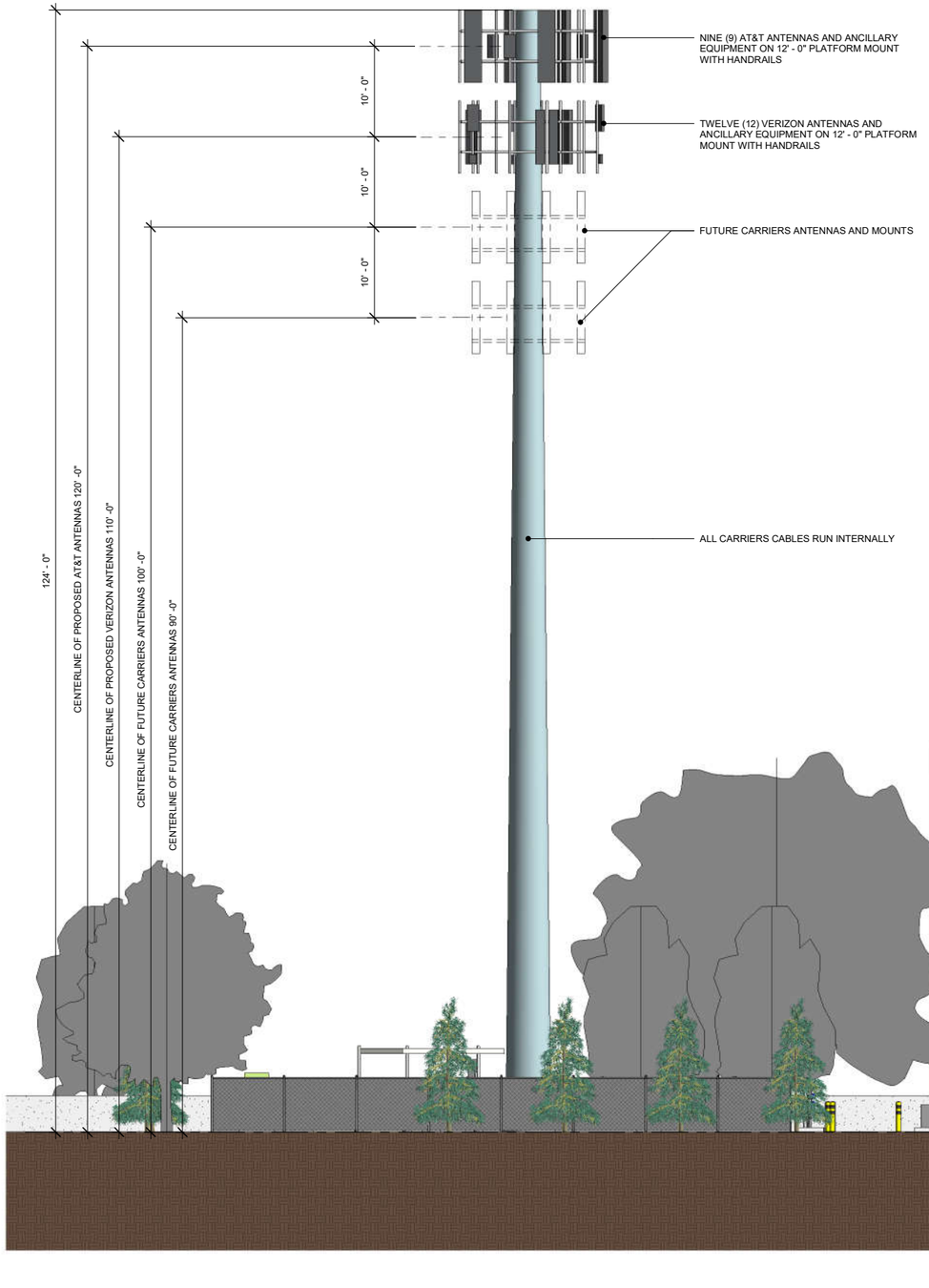


1 EAST ELEVATION
C - 103 1/8" = 1'-0"



2 SOUTH ELEVATION
C - 103 1/8" = 1'-0"

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT
06880

Prepared For:

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STATE OF CONNECTICUT
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Drawing Date: JANUARY 2, 2023
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale: 1/8" = 1'-0"

Sheet Title:

EAST AND SOUTH ELEVATION

Original drawing is ANSI - D.
Do not scale contents of this drawing.

Sheet Number: C - 103

Revision: 4

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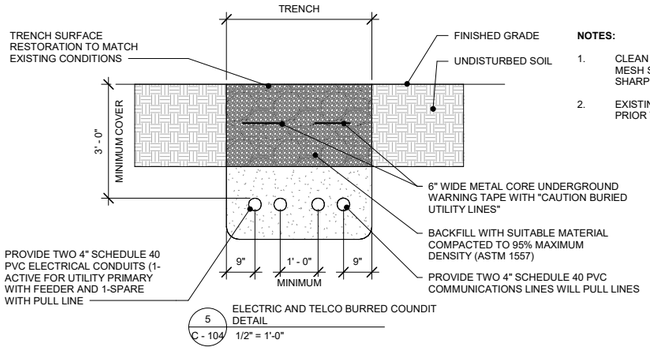
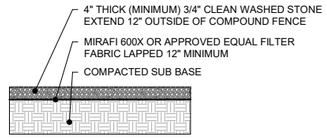


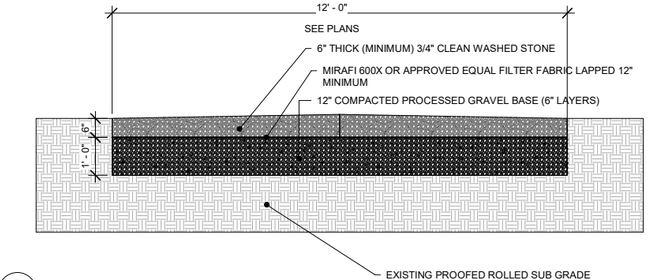
5 C - 104 ELECTRIC AND TELCO BURRED COUNDIT DETAIL
1/2" = 1'-0"

NOTES:

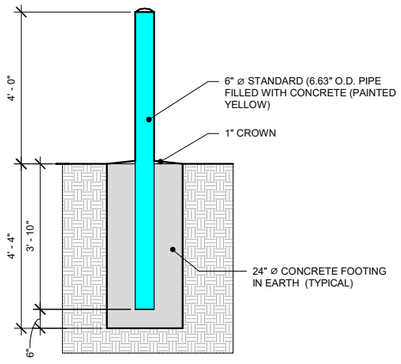
1. CLEAN FILL SHALL PASS THROUGH A 3/8" MESH SCREEN AND SHALL NOT CONTAIN SHARP STONES
2. EXISTING PAVEMENT SHALL BE SAW-CUT PRIOR TO TRENCH EXCAVATION



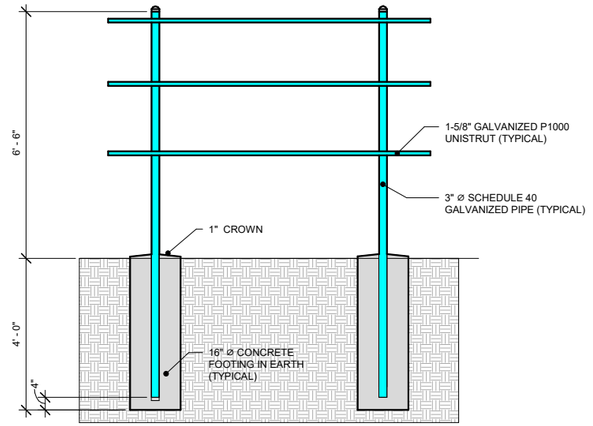
6 C - 104 COMPOUND SURFACE DETAIL
1/2" = 1'-0"



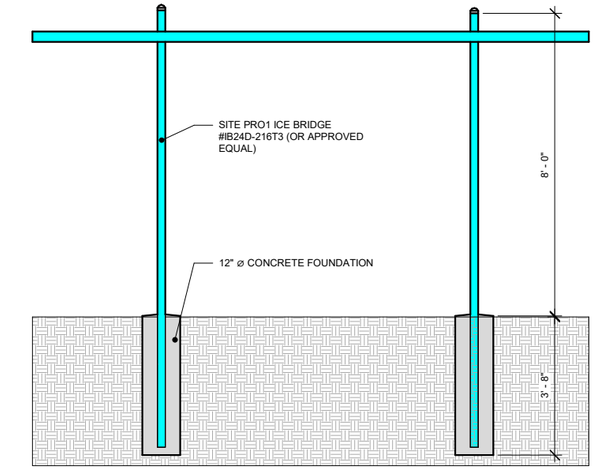
7 C - 104 GRAVEL ACCESS DRIVE
1/2" = 1'-0"



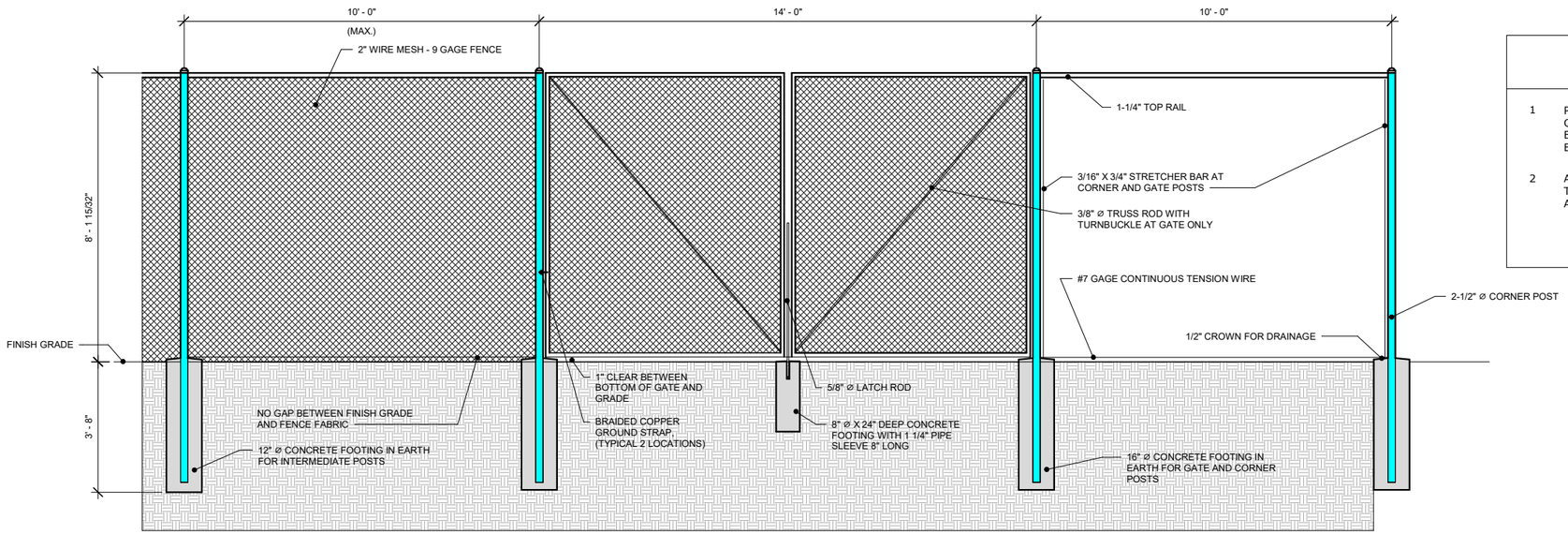
2 C - 104 CONCRETE FILLED BOLLARD
1/2" = 1'-0"



3 C - 104 TYPICAL H - FRAME DETAIL
1/2" = 1'-0"



4 C - 104 ICE BRIDGE DETAIL
1/2" = 1'-0"



1 C - 104 CHAINLINK FENCE DETAIL STANDARD
1/2" = 1'-0"

FENCE NOTES

1 POST IN THE HOLE AND FILL WITH CONCRETE OR GROUT. IF LEDGE IS BELOW FINISH GRADE, COAT BACKFILLED SECTION OF POST WITH COAL TAR, AND BACKFILL WITH WELL-DRAINING GRAVEL.

2 ATTACH EACH GATE WITH 1-1/2 PAIR OF NON-LIFT-OFF TYPE, MALLEABLE IRON OR FORGING, PIN-TYPE HINGES. ASSEMBLIES SHALL ALLOW FOR 180° OF GATE TRAVEL.

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

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WESTPORT, CONNECTICUT
06880

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TARPON TOWERS II

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STATE OF CONNECTICUT DOUGLAS J. ROBERTS No. 4310 LICENSED ARCHITECT

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Drawing By: Zachary J. Roberts
Drawing Date: JANUARY 2, 2023
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale: As indicated

Sheet Title:

SITE DETAILS

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Sheet Number: C - 104

Revision: 4

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EROSION CONTROL MEASURE NOTES

1 DISTURBED AREAS SHALL BE KEPT TO THE MINIMUM AREA NECESSARY TO CONSTRUCT THE ROADWAYS AND ASSOCIATED DRAINAGE FACILITIES.

2 HAY BALE BARRIERS AND SEDIMENT TRAPS SHALL BE INSTALLED AS REQUIRED. BARRIERS AND TRAPS ARE TO BE MAINTAINED AND CLEANED UNTIL ALL SLOPES HAVE A HEALTHY STAND OF GRASS.

3 BALED HAY AND MULCH SHALL BE MOWING'S OF ACCEPTABLE HERBACEOUS GROWTH, FREE FROM NOXIOUS WEEDS OR WOODY STEMS, AND SHALL BE DRY. NO SALT HAY SHALL BE USED.

4 FILL MATERIAL SHALL BE FREE FROM STUMPS, WOOD, ROOTS, ETC.

5 STOCKPILED MATERIALS SHALL BE PLACED ONLY IN AREAS SHOWN ON THE PLANS. STOCKPILES SHALL BE PROTECTED BY SILTATION FENCE AND SEEDED TO PREVENT EROSION. THESE MEASURES SHALL REMAIN UNTIL ALL MATERIAL HAS BEEN PLACED OR DISPOSED OFF SITE.

6 ALL DISTURBED AREAS SHALL BE LOAMED AND SEEDED. A MINIMUM OF 4 INCHES OF LOAM SHALL BE INSTALLED WITH NOT LESS THAN ONE POUND OF SEED PER 50 SQUARE YARDS OF AREAS.

7 APPLICATION OF GRASS SEED, FERTILIZERS AND MULCH SHALL BE ACCOMPLISHED BY BROADCAST SEEDING OR HYDROSEEDING AT THE RATES OUTLINED:

8 AFTER ALL DISTURBED AREAS HAVE BEEN STABILIZED THE TEMPORARY EROSION CONTROL MEASURES ARE TO BE REMOVED.

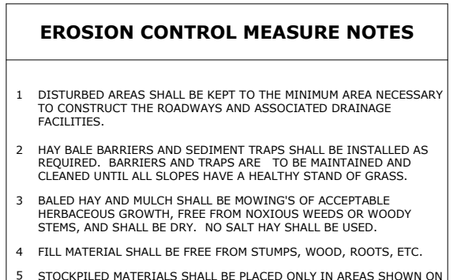
9 PAVED ROADWAYS MUST BE KEPT CLEAN AT ALL TIMES.

10 ALL CATCH BASIN INLETS WILL BE PROTECTED WITH LOW POINT SEDIMENTATION BARRIER.

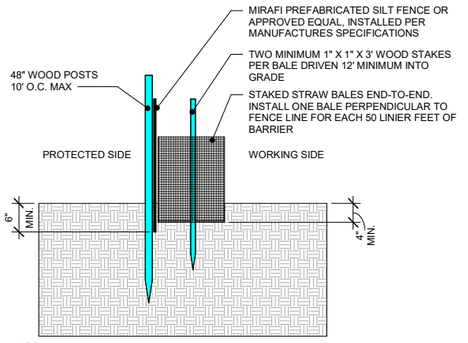
11 ALL STORM DRAINAGE OUTLETS WILL BE STABILIZE AND CLEANED AS REQUIRED, BEFORE THE DISCHARGE POINTS BECOME OPERATIONAL.

12 ALL DEWATERING OPERATIONS MUST DISCHARGE DIRECTLY INTO A SEDIMENT FILTER AREA.

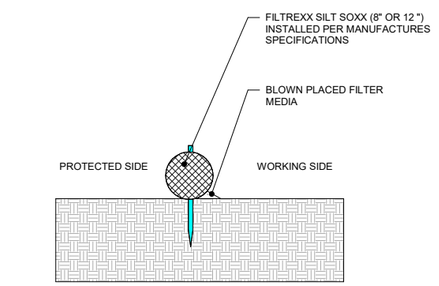
13 NO DISCHARGE SHALL BE DIRECTED TOWARDS ANY PROPOSED DITCHES, SWALES, OR PONDS UNTIL THEY HAVE BEEN PROPERLY STABILIZED.



RIPRAP SWALE



1 C - 105 EROSION CONTROL BARRIER HAY BALE 3/4" = 1'-0"



2 C - 105 EROSION CONTROL BARRIER SILT SOXX 3/4" = 1'-0"

SILT SOXX NOTES

1 USE SILT SOXX WHERE CONDITIONS DO NOT ALLOW STAKES TO BE DRIVEN.

2 STRAW BALES TO BE TIED WITH BIODEGRADABLE TWINE

3 SILT SOXX FILL TO MEET FILTER SPECIFICATIONS AND APPLICATION REQUIREMENTS

4 SILT SOXX COMPOSE MATERIAL TO BE DISPERSED ON SITE OR AS DETERMINED BY THE ARCHITECT

GENERAL CONSTRUCTION SEQUENCE

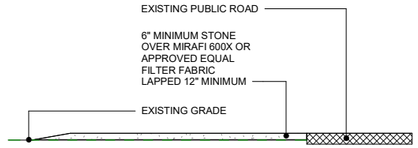
THIS IS A GENERAL CONSTRUCTION SEQUENCE OUTLINING SOME ITEMS OF WHICH MAY NOT APPLY TO THIS SITE

1 CLEAR AND GRUB AREAS OF PROPOSED CONSTRUCTION

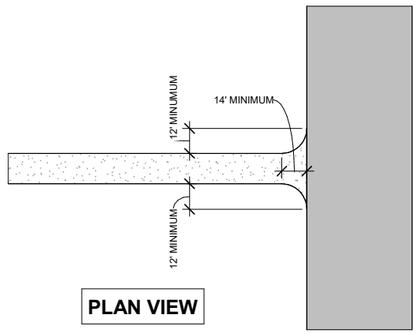
2 INSTALL TEMPORARY SEDIMENTATION AND EROSION CONTROL MEASURES AS REQUIRED

3 REMOVE AND STOCKPILE TOPSOIL TO LOCATION AS SHOWN ON THE DRAWINGS. STOCKPILE SHALL BE SEEDED TO PREVENT EROSION

4 CONSTRUCT CLOSED DRAINAGE SYSTEM. PROTECT CULVERT INLETS AND CATCH BASINS WITH SEDIMENTATION BARRIERS



PROFILE



PLAN VIEW

CONSTRUCTION ENTRANCE SPECIFICATIONS

1 STONE SIZE - 1.5-2.5 INCH STONE SHALL BE USED, OR RECYCLED CONCRETE EQUIVALENT.

2 LENGTH - THE CONSTRCUTION ENTERANCE SHALL BE A MINIMUM OF 30-FT.

3 THICKNESS - THE STONE LAYER SHALL NE AT LEAST 6-INCH THICK.

4 WIDTH - THE ENTERANCE SHALL NE AT LEAST 14-FEET WIDE.

5 GEOTEXTILE - a GEOTEXTILE FABRIC SHALL BE PLAVED OVER THE ENTIRE AREA PRIOR TO PLACING STONE.

6 SCHEDULE - THE CONSTRUCTION ENTERANCE SHALL BE INSTALLED AS SOON AS PRACTICL AND BE FOR MAJOR GRADING.

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
WESTPORT, CONNECTICUT
06880

Prepared For:

TARPON TOWERS II

8916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

DOUGLAS J. ROBERTS – ARCHITECT
110 Washington Avenue
Fourth Floor
North Haven, CT 06473

Tel: 203.234.6368
Email: droberts - architect@outlook.com

STATE OF CONNECTICUT · DOUGLAS J. ROBERTS · No. 4310 · LICENSED ARCHITECT

Key Plan

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Drawing By: Zachary J. Roberts
Drawing Date: JANUARY 2, 2023
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale: As indicated

Sheet Title:

EROSION CONTROL NOTES AND DETAILS

Original drawing is ANSI - D.
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Sheet Number: C - 105

Revision: 4

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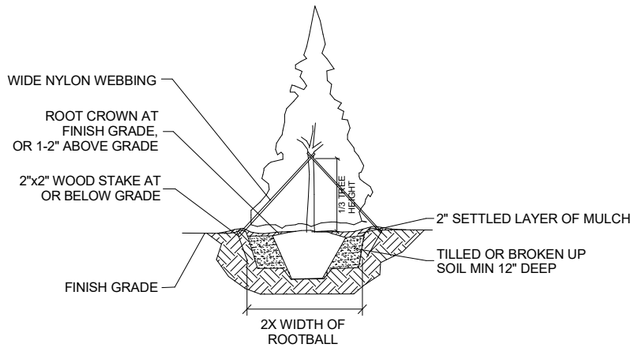


EVERGREEN TREE PLANTING SPECIFICATIONS

1 ALL PLANT MATERIALS SHALL BE IN ACCORDANCE WIALL PLANT MATERIALS SHALL BE IN ACCORDANCE WITH THE AMERICAN STANDARDS FOR NURSERY STOCK (ANSI Z60.1-2004). PLANT ACCORDING TO ANSI A300 PART 6.

2 DIG THE PLANTING HOLE A MINIMUM OF 2x WIDTH OF ROOTBALL FOR AT LEAST THEDIG THE PLANTING HOLE A MINIMUM OF 2x WIDTH OF ROOTBALL FOR AT LEAST THE FIRST 12 INCHES OF DEPTH. BELOW 12 INCHES, DIG HOLE WIDE ENOUGH TO PERMIT ADJUSTING. DO NOT DIG THE HOLE DEEPER THAN ROOT BALL DEPTH.

3 SCARIFY THE SUBGRADE AND SIDES OF THE PLANTING HOLE WHEN PLANTING IN CLAY SOILS (MORE THAN 15% CLAY).

4 LIFT AND SET THE TREE BY ROOT BALL ONLY. DO NOT LIFT USING THE TREE TRUNK AND DO NOT USE TREE TRUNK ASLIFT AND SET THE TREE BY ROOT BALL ONLY. DO NOT LIFT USING THE TREE TRUNK AND DO NOT USE TREE TRUNK AS A LEVER.

5 SET THE TOP OF THE ROOT BALL LEVEL WITH THE SOIL SURFACE OR SLIGHTLY HIGHER IF THE SOIL IS PRONE TO SETTLING.SET THE TOP OF THE ROOT BALL LEVEL WITH THE SOIL SURFACE OR SLIGHTLY HIGHER IF THE SOIL IS PRONE TO SETTLING.

6 AFTER THE TREE IS SET IN PLACE, REMOVE BURLAP, WIRE AND STRAPS FROM AT LEAST THE UPPER 1/3 OF THE ROOTBALL.

7 BACKFILL WITH EXISTING SOIL THAT HAS BEEN WELL-TILLED OR BROKEN UP. DO NOT ADD AMENDMENTS TO THE BACKFILL SOIL. AMEND THE SURFACE WITH MULCH.

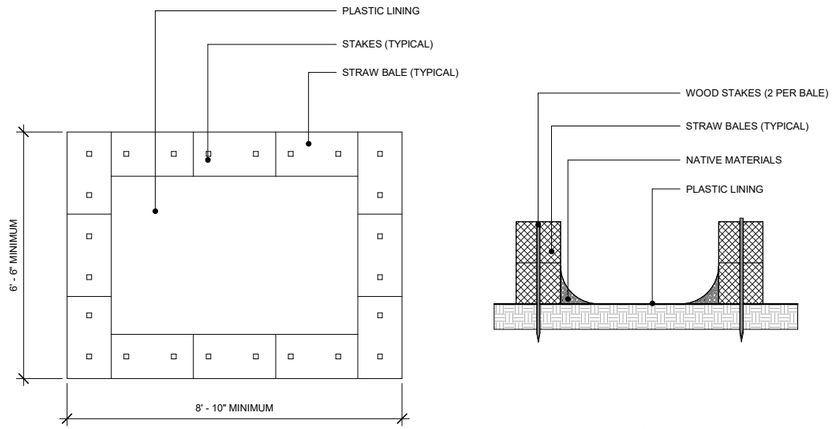
8 USE THREE 2" X 2" WOOD STAKES DRIVEN INTO UNDISTURBED SOIL A MINIMUM OF 16 INCHES. SPACE STAKES EQUALLY AROUND THE TREE.

9 ATTACH 3/4" NYLON WEBBING TO CONNECT THE TREE TO STAKES. ATTACH WEBBING AT 1/3 THE TREE HEIGHT.

10 APPLY A 2-3" (SETTLED) DEPTH OF PINE STRAW OR BARK MULCH TO THE PLANTING SURFACE. LEAVE A 2" SPACE AROUND THE TRUNK FOR AIR CIRCULATION.

11 PRUNING SHALL BE LIMITED TO DEAD, DISEASED, OR BROKEN LIMBS ONLY AND SHALL BE IN ACCORDANCE WITH ANSI A300 SPECIFICATIONS.PRUNING SHALL BE LIMITED TO DEAD, DISEASED, OR BROKEN LIMBS ONLY AND SHALL BE IN ACCORDANCE WITH ANSI A300 SPECIFICATIONS.

12 REMOVE ANY TRUNK WRAP REMAINING AT TIME OF PLANTING. NO WRAPS SHALL BE PLACED ON TRUNK.

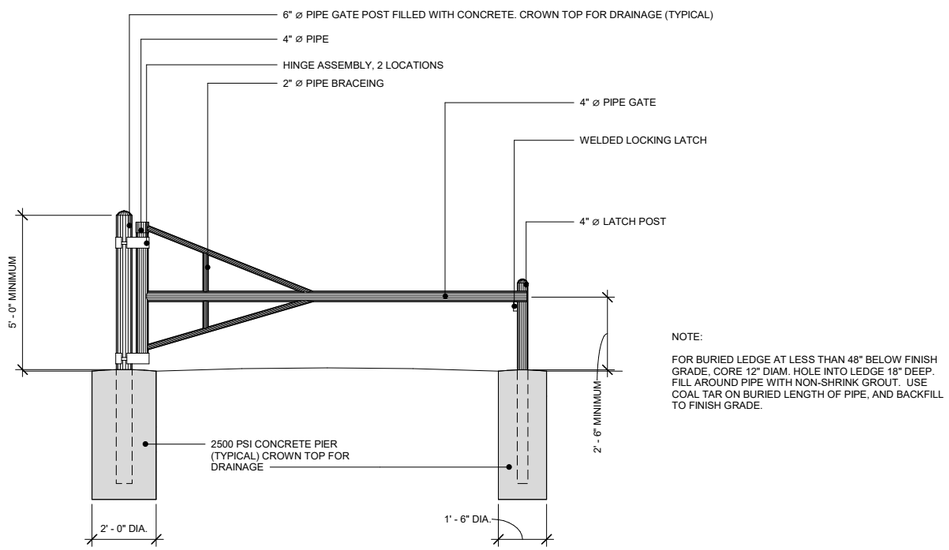


CONCRETE WASHOUT SPECIFICATIONS

1 PERFORM WASHOUT OF CONCRETE TRUCK OFF SITE OR IN DESIGNATED WASHOUT AREA ONLY.

2 DO NOT ALLOW EXCESS CONCRETE TO BE PLACED ONSITE, EXCEPT ON DESIGNATED CONCRETE WASHOUT AREA.

3 STAPLE PLASTID LINING TO TOP STRAW BALES WITH A MINIMUM OF TWO STAPLES



NOTE:

FOR BURIED LEDGE AT LESS THAN 48" BELOW FINISH GRADE, CORE 12" DIAM. HOLE INTO LEDGE 18" DEEP. FILL AROUND PIPE WITH NON-SHRINK GROUT. USE COAL TAR ON BURIED LENGTH OF PIPE, AND BACKFILL TO FINISH GRADE.

3 / C - 106 GATE
3/8" = 1'-0"

DEVELOPMENT AND MANAGEMENT

TARPON TOWERS

Project:

WESTPORT CT

92 GREENS FARMS ROAD
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06880

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STATE OF CONNECTICUT · DOUGLAS J. ROBERTS · No. 43101 · LICENSED ARCHITECT

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Project No: 2021.12
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SITE DETAILS

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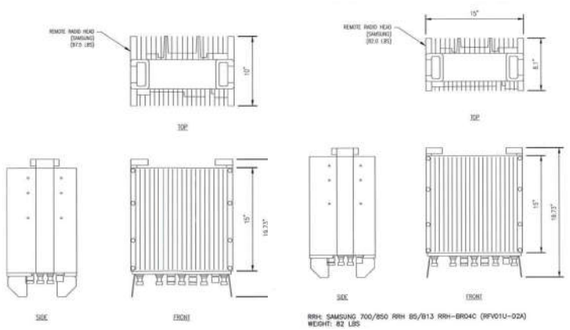
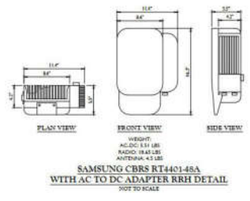
VERIZON - DIPLEXER						
MANUFACTURER	**MODEL NUMBER**	**QUANITY**	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
COMSCOPE	CBC78T-DS-43-2X	3	6.3"	9.6"	6.9"	24.8 LBS.





SAMSUNG CBRS RT4401-48A
WITH AC TO DC ADAPTER RRH DETAIL
NOT TO SCALE

XXDWMM-12.5-65-8T-CBRS

JAHH-45B-R3B

JAHH-65B-R3B

MT6407-77A

VERIZON - RRU						
MANUFACTURER	**MODEL NUMBER**	**QUANITY**	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
SAMSUNG	B2/B66A RRH ORAN (RF4439d-25A)	3	15"	10"	15"	98 LBS.
SAMSUNG	B5/B13 RRH ORAN (RF4440d-13A)	3	15"	9"	16.2"	82 LBS.
SAMSUNG	CBRS RRH - RT4401-48A	3	8.5"	4.1"	12.1"	28.66 LBS.
SAMSUNG	MT6407-77A	3	16"	4"	35"	81 LBS.

VERIZON - ANTENNAS						
MANUFACTURER	**MODEL NUMBER**	**QUANITY**	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
ACE TECHNOLOGY	XXDWMM-12.5-65-8T-CBRS	3	8.6"	4.2"	13.9"	18.6 LBS.
ANDREW	JAHH-45B-R3B	2	17.9"	7"	72"	91 LBS.
ANDREW	JAHH-65B-R3B	4	13.8"	8.2"	72"	64 LBS.
SAMSUNG	MT6407-77A	3	16"	4"	35"	81 LBS.

VERIZON GENERATOR SPECIFICATIONS				
MODEL NUMBER	LENGTH	WIDTH	HEIGHT	WEIGHT
QT050A - 50kW	77"	34"	46"	1,414 LBS.

VERIZON RADIO CABINET SPECIFICATIONS				
MODEL NUMBER	WIDTH	DEPTH	HEIGHT	WEIGHT
RBA84 EQUIPMENT CABINET	32"	35.4"	85.2"	1,955 LBS

VERIZON - OVP BOX RCMDC-6627-PF-48				
MODEL NUMBER	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
OVP BOX RCMDC-6627-PF-48	**21"**	**18"**	**35"**	**45.15 LBS.**

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STATE OF CONNECTICUT
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LICENSED ARCHITECT

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Sheet Title:

VERIZON EQUIPMENT AND DETAILS

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Sheet Number: A - 101

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TPA65R-BU8DA-K

AIR 6449 B77D

DMP65R-BU8DA-K

AT&T - ANTENNAS						
MANUFACTURER	**MODEL NUMBER**	**QUANITY**	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
CCI ANTENNAS	TPA65R-BU8DA-K	3	20.7"	7.7"	96"	119 LBS.
ERICSSON	AIR 6449 B77D	3	16"	8"	31"	87.1 LBS.
CCI ANTENNAS	DMP65R-BU8DA-K	3	20.7"	7.7"	96"	87.1 LBS.

AT&T - SQUID						
MANUFACTURER	**MODEL NUMBER**	**QUANITY**	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
RAYCAP	DC9-48-60-0-8C-EV	1	10.24"	24"	33"	33 LBS.
RAYCAP	DC9-48-60-24-8C-EV	1	10.24"	24"	33"	33 LBS.

AT&T GENERATOR SPECIFICATIONS				
MODEL NUMBER	LENGTH	WIDTH	HEIGHT	WEIGHT
15 kW -48 VDC 834063-NG-15	50"	32"	72"	765 LBS.

AT&T WALK-IN-CABINET SPECIFICATIONS				
MODEL NUMBER	WIDTH	DEPTH	HEIGHT	WEIGHT
XTE 801 SERIES CABINET	79.2"	79.2"	114"	7,500 LBS.

RRU 4478 B14

RRU 4449 B5/B12

RRU 4415 B30

RRU 8843 B2 B66A

AT&T - RRU						
MANUFACTURER	**MODEL NUMBER**	**QUANITY**	**WIDTH**	**DEPTH**	**HEIGHT**	**WEIGHT**
ERICSSON	4478 B14	3	13.3"	7.4"	15"	60 LBS.
ERICSSON	4449 B5/B12	3	15"	9.3"	15"	70 LBS.
ERICSSON	4415 B30	3	13.4"	5.9"	16.5"	46 LBS.
ERICSSON	8843 B2/B66A	3	15"	10"	28"	85 LBS.

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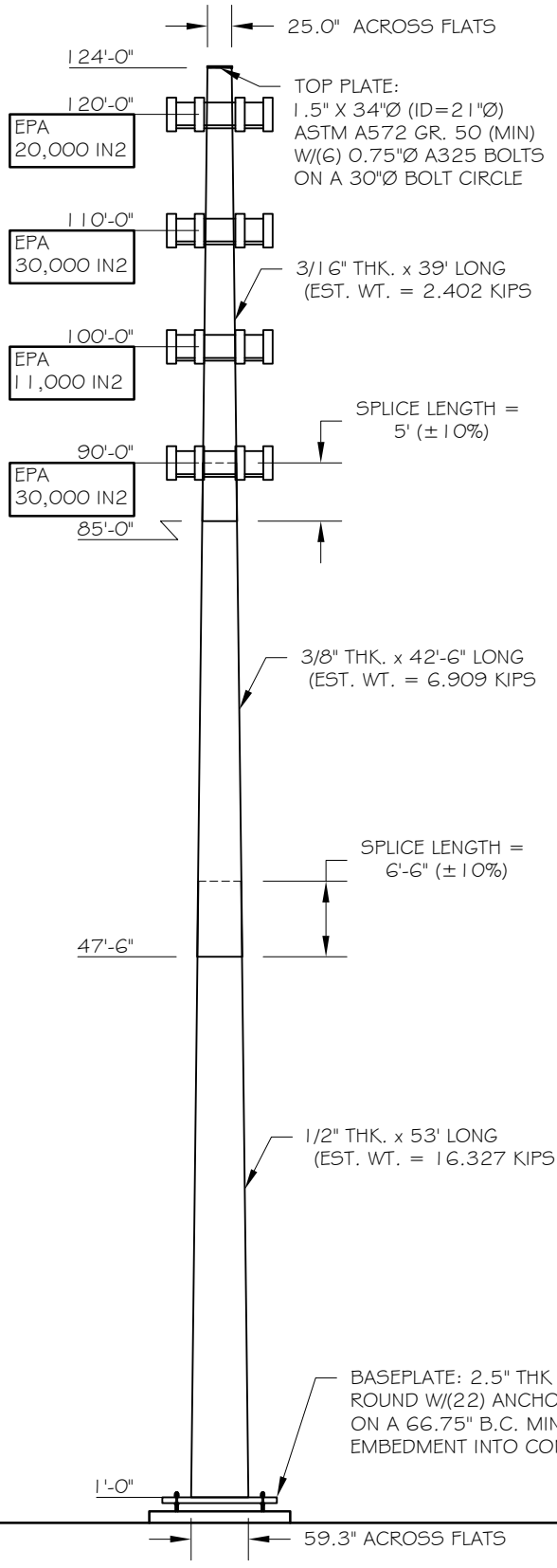
TARPON TOWERS II, LLC
8916 77th Terrace East, Ste. 103
Lakewood Ranch, FL 34202
941-757-5010



Page 1 of 3		Job Number:	94122-132
Eng: MFP		Customer Ref:	
		Date:	2/10/2023
Structure:	124-FT MONOPOLE		
Site:	CT1024 WESTPORT		
Location:	FAIRFIELD CO., CT / 41°7'25.39", -73°20'41.26"		
Owner:	TARPON TOWERS II LLC		
Revision No.:	Revision Date:		

DESIGN

Building Code:	2022 CONNECTICUT BUILDING CODE		
Design Standard:	TIA-222-H		
Wind Speed Load Cases:	ASCE-7-16 WIND SPEED		
Load Case #1:	120 MPH Design Wind Speed		
Load Case #2:	50 MPH Wind with 1.5" Ice Accumulation		
Load Case #3	60 MPH Service Wind Speed		
Structure Class Risk Category	Exposure Cat.	Topography Cat.	Crest Height
II	C	1	

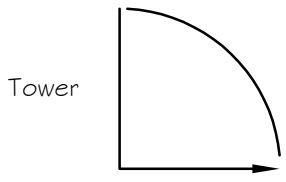
EQUIPMENT LIST

Elev.	Description
120	(3) TPA-65R-BU8DA-K + (3) HPA65R-BU8A + (3) MP65R-BU8D +
120	(21) RRU + (3) RAYCAP + 12-FT PLATFORM WITH HANDRAIL
110	(3) XXDWMM-12.5-65-8TCBRS 3550 + (2) JAHH-45B-R38 +
110	(4) JAHH-65B-R38 + (3) MT6407-77A + (3) CBC78T-DS-43-2X +
110	(9) RRH + (1) OVP + 12-FT PLATFORM WITH HANDRAIL
100	(9) FFW-65B-R2 + (18) RRH + (1) OVP
100	MC-PK8-DSH MOUNT
90	(3) XXDWMM-12.5-65-8TCBRS 3550 + (2) JAHH-45B-R38 +
90	(4) JAHH-65B-R38 + (3) MT6407-77A + (3) CBC78T-DS-43-2X +
90	(9) RRH + (1) OVP + 12-FT PLATFORM WITH HANDRAIL

ANTENNA FEED LINES ROUTED ON THE INSIDE OF THE POLE
POLE DESIGNED FOR A MAX 34-FT FALL RADIUS

STRUCTURE PROPERTIES

Cross-Section: 18-Sided			Taper: 0.28760 in/ft		
Shaft Steel: ASTM A572 GR 65			Baseplate Steel: ASTM A572 GR 50		
Anchor Rods: 2.25 in. A615 GR. 75 X 7'-0"					
Sect.	Length (ft)	Thickness (in)	Splice (ft)	Top Dia. (in)	Bot Dia. (in)
1	39.00	0.1875	5.00	25.00	36.22
2	42.50	0.3750	6.50	34.40	46.63
3	53.00	0.5000	0.00	44.01	59.25



MICHAEL F. PLAHOVINSAK, P.E. #25849
Sole Proprietor - Independent Engineer
18301 S.R. 161, Plain City, OH 43064
614-398-6250 / mike@mfpeng.com

BASE REACTIONS FOR FOUNDATION DESIGN

Moment:	7200 ft-kip
Shear:	77 kip
Axial:	57 kip

TARPON TOWERS II, LLC
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Lakewood Ranch, FL 34202
941-757-5010

Page 2 of 3		Job Number:	94122-132
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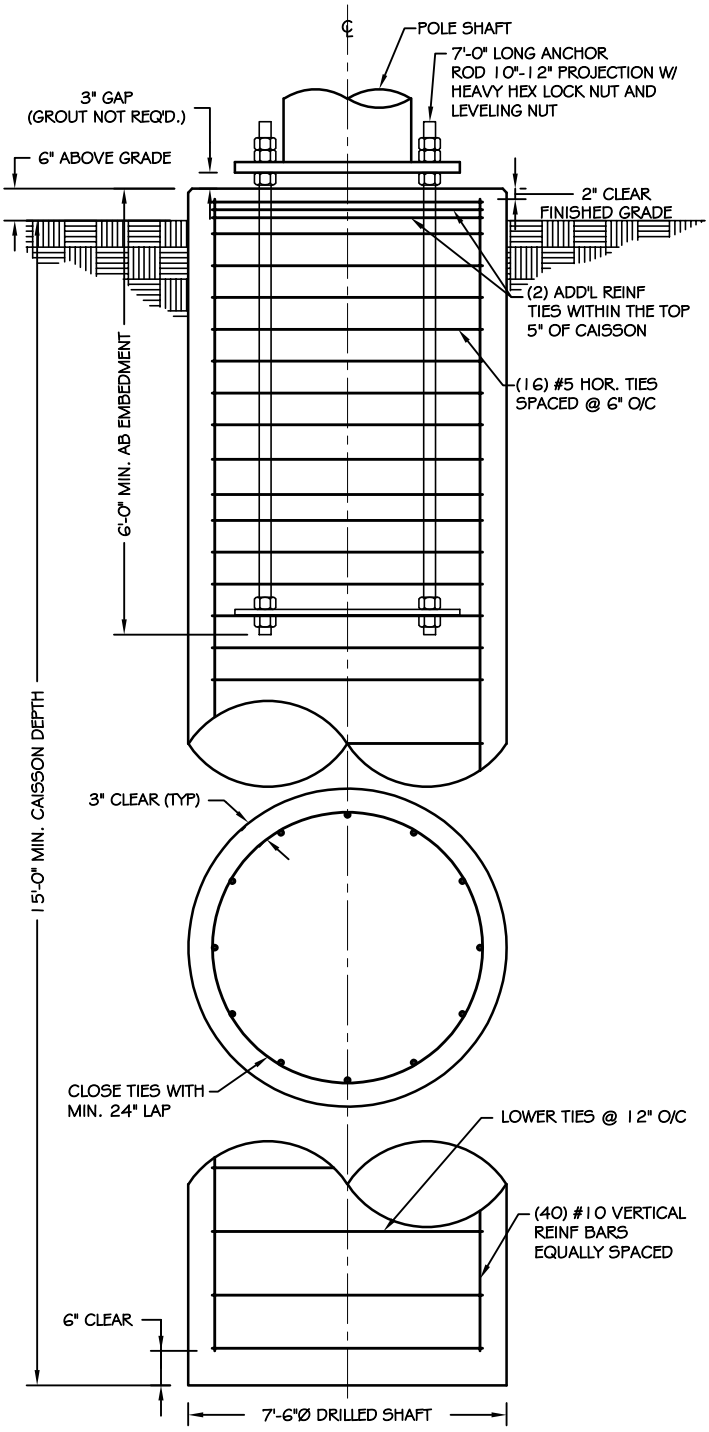

CAISSON FOUNDATION

NOT TO SCALE

FOUNDATION NOTES:

1. ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45. IN AREAS OF POTENTIAL FREEZING, CONCRETE SHALL BE AIR ENTRAINED 6% (±1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318, "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.

2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 VERTICAL BARS SHALL BE GRADE 60, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.

3. CAISSON FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 336, "STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF DRILLED PIERS", LATEST EDITION.

4. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.

5. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY:
ENGINEER: WELTI GEOTECHNICAL
REPORT NO.: N/A (DATED 1/18/23)

6. ESTIMATED CONCRETE VOLUME = 25 CUBIC YARDS.

7. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED LOADS:
MOMENT: 7200 FT*KIPS
SHEAR: 77 KIPS
AXIAL: 57 KIPS

MICHAEL F. PLAHOVINSAK, P.E. #25849
Sole Proprietor - Independent Engineer
18301 S.R. 161, Plain City, OH 43064
614-398-6250 / mike@mfpeng.com

TARPON TOWERS II, LLC
8916 77th Terrace East, Ste. 103
Lakewood Ranch, FL 34202
941-757-5010

Page 3 of 3		Job Number:	94122-132
Eng: MFP		Customer Ref:	
		Date:	2/10/2023
Structure:	124-FT MONOPOLE		
Site:	CT1024 WESTPORT		
Location:	FAIRFIELD CO., CT / 41°7'25.39", -73°20'41.26"		
Owner:	TARPON TOWERS II LLC		
Revision No.:	Revision Date:		

FOUNDATION NOTES:

1. ALL FOUNDATION CONCRETE SHALL USE TYPE II CEMENT AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45 AND SHALL BE AIR ENTRAINED 6% (±1.5%). ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318, "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION.

2. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 VERTICAL BARS SHALL BE GRADE 60, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION.

3. THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.

4. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY:
ENGINEER: WELTI GEOTECHNICAL
REPORT NO.: N/A (DATED 1/18/23)

5. ESTIMATED CONCRETE VOLUME = 129.8 CUBIC YARDS.

6. THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED LOADS:
MOMENT: 7200 FT*KIPS
SHEAR: 77 KIPS
AXIAL: 57 KIPS

STATE OF CONNECTICUT
MICHAEL F. PLAHOVINSAK
No. 25849
PROFESSIONAL ENGINEER
2.10.2023

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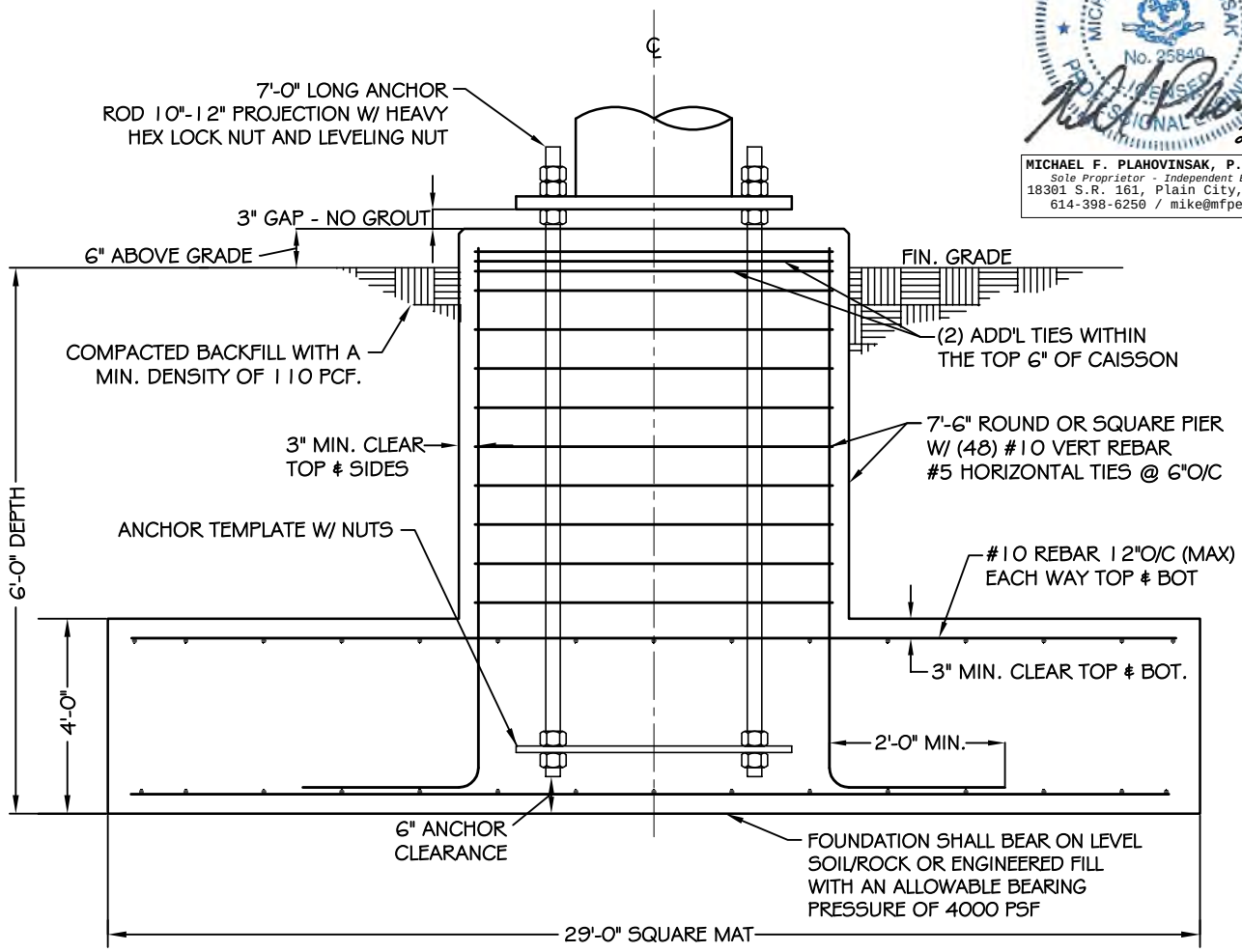

SPREAD FOOTING

NOT TO SCALE

Tower Input Data

The tower is a monopole.
This tower is designed using the TIA-222-H standard.
The following design criteria apply:

Tower base elevation above sea level: 25.00 ft.
Basic wind speed of 120 mph.
Risk Category II.
Exposure Category C.
Simplified Topographic Factor Procedure for wind speed-up calculations is used.
Topographic Category: 1.
Crest Height: 0.00 ft.
Nominal ice thickness of 1.5000 in.
Ice thickness is considered to increase with height.
Ice density of 56 pcf.
A wind speed of 50 mph is used in combination with ice.
Temperature drop of 50 °F.
Deflections calculated using a wind speed of 60 mph.
A non-linear (P-delta) analysis was used.
Pressures are calculated at each section.
Stress ratio used in pole design is 1.
Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

Tapered Pole Section Geometry

Section	Elevation ft	Section Length ft	Splice Length ft	Number of Sides	Top Diameter in	Bottom Diameter in	Wall Thickness in	Bend Radius in	Pole Grade
L1	124.00-85.00	39.00	5.00	18	25.0000	36.2165	0.1875	0.7500	A572-65 (65 ksi)
L2	85.00-47.50	42.50	6.50	18	34.4035	46.6265	0.3750	1.5000	A572-65 (65 ksi)
L3	47.50-1.00	53.00		18	44.0071	59.2500	0.5000	2.0000	A572-65 (65 ksi)

Tapered Pole Properties

Section	Tip Dia. in	Area in^2	I in^4	r in	C in	I/C in^3	J in^4	It/Q in^2	w in	w/t
L1	25.3567	14.7665	1148.5693	8.8084	12.7000	90.4385	2298.6500	7.3847	4.0700	21.707
	36.7462	21.4417	3516.4215	12.7903	18.3980	191.1310	7037.4701	10.7229	6.0441	32.235
L2	36.3365	40.5024	5925.1907	12.0801	17.4770	339.0288	11858.1780	20.2550	5.3950	14.387
	47.2880	55.0509	14878.3161	16.4193	23.6863	628.1408	29776.2096	27.5307	7.5463	20.123
L3	46.5071	69.0458	16511.8298	15.4450	22.3556	738.5988	33045.3864	34.5295	6.8653	13.731
	60.0869	93.2362	40657.2490	20.8562	30.0990	1350.7840	81367.9959	46.6270	9.5480	19.096

Tower Elevation ft	Gusset Area (per face) ft^2	Gusset Thickness in	Gusset Grade	Adjust. Factor A_f	Adjust. Factor A_r	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
L1 124.00-85.00				1	1	1			
L2 85.00-47.50				1	1	1			
L3 47.50-1.00				1	1	1			

Feed Line/Linear Appurtenances - Entered As Area

Description	Face or Leg	Allow Shield	Exclude From Torque Calculation	Component Type	Placement ft	Total Number		C_AA_A ft^2/ft	Weight plf
1 5/8"	C	No	Yes	Inside Pole	120.00 - 1.00	18	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
							2" Ice	0.00	0.92
1 5/8"	C	No	Yes	Inside Pole	110.00 - 1.00	18	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
							2" Ice	0.00	0.92
1 5/8"	C	No	Yes	Inside Pole	100.00 - 1.00	18	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
							2" Ice	0.00	0.92
1 5/8"	C	No	Yes	Inside Pole	90.00 - 1.00	18	No Ice	0.00	0.92
							1/2" Ice	0.00	0.92
							1" Ice	0.00	0.92
							2" Ice	0.00	0.92

Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A_R ft^2	A_F ft^2	C_AA_A In Face ft^2	C_AA_A Out Face ft^2	Weight K
L1	124.00-85.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	1.32
L2	85.00-47.50	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	2.48
L3	47.50-1.00	A	0.000	0.000	0.000	0.000	0.00
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	3.07

Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A_R ft^2	A_F ft^2	C_AA_A In Face ft^2	C_AA_A Out Face ft^2	Weight K
L1	124.00-85.00	A	1.682	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	1.32
L2	85.00-47.50	A	1.607	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	2.48
L3	47.50-1.00	A	1.455	0.000	0.000	0.000	0.000	0.00
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	3.07

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Discrete Tower Loads

Description	*Face or Leg*	*Offset Type*	*Offsets: Horz Lateral Vert* ft ft ft	*Azimuth Adjustment* °	*Placement* ft		C_AA_A *Front* ft^2	C_AA_A *Side* ft^2	*Weight* K
(3) CCI TPA65R-BU8DA-K w/ mount pipe	A	From Face	3.00 0.00	0.0000	120.00	No Ice 1/2" Ice	17.87 18.50	10.02 11.44	0.12 0.23
(ATT)			0.00			1" Ice 2" Ice	19.14 20.44	12.72 14.94	0.36 0.66
(3) CCI HPA65R-BU8A w/ mount pipe	B	From Face	3.00 0.00	0.0000	120.00	No Ice 1/2" Ice	11.23 11.85	10.02 11.44	0.08 0.17
(ATT)			0.00			1" Ice 2" Ice	12.47 13.72	12.72 14.94	0.27 0.50
(3) CCI DMP65R-BU8D w/ mount pipe	C	From Face	3.00 0.00	0.0000	120.00	No Ice 1/2" Ice	17.87 18.50	10.02 11.44	0.15 0.27
(ATT)			0.00			1" Ice 2" Ice	19.14 20.44	12.72 14.94	0.40 0.69
(3) Ericsson RRUS-4478	A	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	1.65 1.81	0.91 1.04	0.06 0.07
(ATT)			0.00			1" Ice 2" Ice	1.98 2.34	1.18 1.47	0.09 0.13
(3) Ericsson E2/B29	B	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	3.15 3.36	1.29 1.44	0.05 0.08
(ATT)			0.00			1" Ice 2" Ice	3.59 4.07	1.60 1.95	0.10 0.17
(3) Ericsson RRUS-4415/B25	C	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	1.63 1.78	0.64 0.75	0.05 0.06
(ATT)			0.00			1" Ice 2" Ice	1.95 2.31	0.86 1.13	0.08 0.11
(3) Ericsson 4449	A	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	1.97 2.15	1.41 1.57	0.07 0.09
(ATT)			0.00			1" Ice 2" Ice	2.33 2.72	1.73 2.08	0.11 0.16
(3) Ericsson RRUS-8843	B	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	1.65 1.81	1.16 1.30	0.07 0.09
(ATT)			0.00			1" Ice 2" Ice	1.98 2.34	1.45 1.76	0.10 0.15
(6) Samsung BR049 B2/B66A RRH	C	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	1.88 2.05	1.25 1.39	0.08 0.10
(ATT)			0.00			1" Ice 2" Ice	2.22 2.60	1.54 1.86	0.12 0.18
(3) Raycap DC6-48-60-0-8C-EV	A	From Face	2.00 0.00	0.0000	120.00	No Ice 1/2" Ice	4.78 5.06	2.74 2.96	0.03 0.06
(ATT)			0.00			1" Ice 2" Ice	5.35 5.95	3.20 3.68	0.10 0.20
12' Platform w/ Handrail	C	None		0.0000	120.00	No Ice 1/2" Ice	30.00 35.00	30.00 35.00	1.80 2.60
(ATT)						1" Ice 2" Ice	40.00 50.00	40.00 50.00	3.40 5.00
**									
EPA 30,000 in2 (Verizon Reserved)	C	None		0.0000	110.00	No Ice 1/2" Ice	208.33 225.00	208.33 225.00	4.00 5.00
						1" Ice 2" Ice	241.67 275.01	241.67 275.01	6.00 8.00
**									
EPA 11,000 in2 (Dish Reserved)	C	None		0.0000	100.00	No Ice 1/2" Ice	76.39 85.00	76.39 85.00	4.00 5.00
						1" Ice 2" Ice	93.61 110.83	93.61 110.83	6.00 8.00

Description	Face or Leg	Offset Type	Offsets: Horz Lateral ft / Vert ft / ft	Azimuth Adjustment °	Placement ft		C_AA_A Front ft^2	C_AA_A Side ft^2	Weight K
**									
EPA 30,000 in2	C	None		0.0000	90.00	No Ice	208.33	208.33	4.00
						1/2" Ice	225.00	225.00	5.00
						1" Ice	241.67	241.67	6.00
						2" Ice	275.01	275.01	8.00
**									

Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 90 deg - No Ice
5	0.9 Dead+1.0 Wind 90 deg - No Ice
6	1.2 Dead+1.0 Wind 180 deg - No Ice
7	0.9 Dead+1.0 Wind 180 deg - No Ice
8	1.2 Dead+1.0 Ice+1.0 Temp
9	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
10	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
11	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp
12	Dead+Wind 0 deg - Service
13	Dead+Wind 90 deg - Service
14	Dead+Wind 180 deg - Service

Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
L1	124 - 85	Pole	Max Tension	6	0.00	-0.00	0.00
			Max. Compression	8	-36.10	2.99	-2.14
			Max. Mx	4	-17.38	-540.39	-13.91
			Max. My	6	-17.34	-11.48	-552.95
			Max. Vy	4	25.44	-540.39	-13.91
			Max. Vx	6	25.78	-11.48	-552.95
			Max. Torque	7			-0.85
L2	85 - 47.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-57.56	2.98	-2.13
			Max. Mx	4	-32.31	-1879.78	-28.92
			Max. My	6	-32.29	-26.43	-1904.66
			Max. Vy	4	38.81	-1879.78	-28.92
			Max. Vx	6	39.15	-26.43	-1904.66
			Max. Torque	7			-0.85
L3	47.5 - 1	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	8	-86.44	2.96	-2.12
			Max. Mx	4	-56.53	-4061.26	-50.60
			Max. My	6	-56.53	-48.08	-4103.93
			Max. Vy	4	43.27	-4061.26	-50.60
			Max. Vx	6	43.60	-48.08	-4103.93
			Max. Torque	7			-0.85

Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	124 - 85	11.213	14	0.8121	0.0014
L2	90 - 47.5	5.806	14	0.6339	0.0004
L3	54 - 1	1.995	14	0.3494	0.0001

Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	(3) CCI TPA65R-BU8DA-K w/ mount pipe	14	10.537	0.7941	0.0013	45924
110.00	EPA 30,000 in2	14	8.869	0.7475	0.0009	16401
100.00	EPA 11,000 in2	14	7.275	0.6954	0.0006	9567
90.00	EPA 30,000 in2	14	5.806	0.6339	0.0004	6907

Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
L1	124 - 85	50.161	6	3.6219	0.0071
L2	90 - 47.5	26.012	6	2.8393	0.0019
L3	54 - 1	8.938	6	1.5658	0.0006

Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
120.00	(3) CCI TPA65R-BU8DA-K w/ mount pipe	6	47.144	3.5435	0.0063	10425
110.00	EPA 30,000 in2	6	39.699	3.3400	0.0046	3722
100.00	EPA 11,000 in2	6	32.578	3.1112	0.0031	2170
90.00	EPA 30,000 in2	6	26.012	2.8393	0.0019	1565

Pole Design Data

Section No.	Elevation ft	Size	L ft	L_u ft	Kl/r	A in^2	P_u K	ϕP_n K	Ratio $\frac{P_u}{\phi P_n}$
L1	124 - 85 (1)	TP36.2165x25x0.1875	39.00	0.00	0.0	20.5859	-17.34	1186.48	0.015
L2	85 - 47.5 (2)	TP46.6265x34.4035x0.375	42.50	0.00	0.0	52.8258	-32.29	3090.31	0.010
L3	47.5 - 1 (3)	TP59.25x44.0071x0.5	53.00	0.00	0.0	93.2363	-56.53	5454.32	0.010

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Pole Bending Design Data

Section No.	Elevation ft	Size	M_{ux} kip-ft	ϕM_{nx} kip-ft	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	M_{uy} kip-ft	ϕM_{ny} kip-ft	Ratio $\frac{M_{uy}}{\phi M_{ny}}$
L1	124 - 85 (1)	TP36.2165x25x0.1875	553.07	845.99	0.654	0.00	845.99	0.000
L2	85 - 47.5 (2)	TP46.6265x34.4035x0.375	1904.84	3370.68	0.565	0.00	3370.68	0.000
L3	47.5 - 1 (3)	TP59.25x44.0071x0.5	4104.21	7892.49	0.520	0.00	7892.49	0.000

Pole Shear Design Data

Section No.	Elevation ft	Size	Actual V_u K	ϕV_n K	Ratio $\frac{V_u}{\phi V_n}$	Actual T_u kip-ft	ϕT_n kip-ft	Ratio $\frac{T_u}{\phi T_n}$
L1	124 - 85 (1)	TP36.2165x25x0.1875	25.78	361.28	0.071	0.85	1094.43	0.001
L2	85 - 47.5 (2)	TP46.6265x34.4035x0.375	39.16	927.09	0.042	0.84	3603.39	0.000
L3	47.5 - 1 (3)	TP59.25x44.0071x0.5	43.60	1636.30	0.027	0.84	8418.83	0.000

Pole Interaction Design Data

Section No.	Elevation ft	Ratio $\frac{P_u}{\phi P_n}$	Ratio $\frac{M_{ux}}{\phi M_{nx}}$	Ratio $\frac{M_{uy}}{\phi M_{ny}}$	Ratio $\frac{V_u}{\phi V_n}$	Ratio $\frac{T_u}{\phi T_n}$	Comb. Stress Ratio	Allow. Stress Ratio	Criteria
L1	124 - 85 (1)	0.015	0.654	0.000	0.071	0.001	0.674 ✔	1.000	4.8.2 ✔
L2	85 - 47.5 (2)	0.010	0.565	0.000	0.042	0.000	0.577 ✔	1.000	4.8.2 ✔
L3	47.5 - 1 (3)	0.010	0.520	0.000	0.027	0.000	0.531 ✔	1.000	4.8.2 ✔

Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	ϕP_{allow} K	% Capacity	Pass Fail
L1	124 - 85	Pole	TP36.2165x25x0.1875	1	-17.34	1186.48	67.4	Pass
L2	85 - 47.5	Pole	TP46.6265x34.4035x0.375	2	-32.29	3090.31	57.7	Pass
L3	47.5 - 1	Pole	TP59.25x44.0071x0.5	3	-56.53	5454.32	53.1	Pass
							Summary	
						Pole (L1)	67.4	Pass
						RATING =	**67.4**	**Pass**

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Job	124-ft monopole - MFP #94122-132
Page	BP & AB Calc
Project	CT1024 Westport
Date	2/10/2023
Client	TARPON TOWERS II LLC
Designed by	Mike

Anchor Rod and Base Plate Calculation

TIA-222-H

Factored Base Reactions:		***Pole Shape:***	***Anchor Rods:***	***Base Plate:***
Moment:	4104 ft-kips	18-Sided	(22) 2.25 in. A615 GR. 75	2.5 in. x 72.75 in. Round
Shear:	44 kips	***Pole Dia. (D_f):***	Anchor Rods Evenly Spaced	fy = 50 ksi
Axial:	57 kips	59.25 in	On a 66.75 in Bolt Circle	

Anchor Rod Calculation According to TIA-222-H section 4.9.9

ϕ_t , ϕ_v = 0.75 TIA 4.9.6
I_{bolts} = 12252.80 in^2 Momet of Inertia
P_u = 137 kips Compr Force
V_u = 2.0 kips Shear Force
Rnt = 325.00 kips Nominal Tensile Strength
Rnv = 198.80 kips (0.5 x fu x ag)
Stress Rating = **57.2%** Satisfies TIA-H 4.9.9

Base Plate Calculation According to TIA-222-H

ϕ = 0.90 TIA 4.7
M_{PL} = 314.5 in-kip Plate Moment
L = 8.5 in Section Length
Z = 13.2 Plastic Section Modulus
M_P = 661.0 in-kip Plastic Moment
ϕM_n = 594.9 in-kip Factored Resistance

Calculated Moment vs Factored Resistance
314.45 in-kip $\leq$ 595 in-kip

Stress Rating = **52.9%**

Anchor Rods Are Adequate	**57.2%**	☑
Base Plate is Adequate	**52.9%**	☑

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Job	Page
124-ft monopole - MFP #94122-132	FND
Project	**Date**
CT1024 Westport	2/10/2023
Client	**Designed by**
TARPON TOWERS II LLC	Mike

Caisson Calculation

According to TIA-222-H

1. Foundation overturning resistance calculated with PLS Caisson, for Brom's method for rigid piles. Soil layers modeled after recommendations from the geotechnical report.
2. Cohesion strength for the upper 22.5 ft has been reduced by 50%
3. An additional load factor of 1.3 has been applied to the reinforcement design
4. Foundation has been designed with applied loads per TIA-222-H
5. No groundwater within the depth of the foundation.

```
*** PIER PROPERTIES      CONCRETE STRENGTH (ksi) =   4.50                      STEEL STRENGTH (ksi) =  60.00

                         DIAMETER (ft) =  7.500        DISTANCE FROM TOP OF PIER TO GROUND LEVEL (ft) =   0.50

*** SOIL PROPERTIES      LAYER  TYPE  THICKNESS  DEPTH AT TOP OF LAYER    DENSITY        CU       KP       PHI
                                          (ft)                  (ft)      (pcf)     (psf)          (degrees)
                             1    S       4.00                  0.00      100.0             1.000     -0.00
                             2    C      30.00                  4.00      100.0    6000.0

*** DESIGN (FACTORED) LOADS AT TOP OF PIER  MOMENT (ft-k) =  7200.0   VERTICAL (k) =  57.0   SHEAR (k) =  77.0
                                           ADDITIONAL SAFETY FACTOR AGAINST SOIL FAILURE =  1.33

*** CALCULATED PIER LENGTH  (ft)  =  15.500

*** CHECK OF SOILS PROPERTIES AND ULTIMATE RESISTING FORCES ALONG PIER

    TYPE  TOP OF LAYER BELOW TOP OF PIER   THICKNESS    DENSITY          CU         KP       FORCE         ARM
                                    (ft)        (ft)      (pcf)       (psf)                   (k)        (ft)
      S                             0.50        4.00      100.0                1.000       18.00        3.17
      C                             4.50        5.62      100.0      6000.0                2022.80        7.31
      C                            10.12        5.38      100.0      6000.0               -1937.19       12.81

*** SHEAR AND MOMENTS ALONG PIER
                                  WITH THE ADDITIONAL SAFETY FACTOR     WITHOUT ADDITIONAL SAFETY FACTOR
    DISTANCE BELOW TOP OF PIER (ft)      SHEAR (k)    MOMENT (ft-k)          SHEAR (k)    MOMENT (ft-k)
                              0.00             103.6           9971.8               77.7           7479.1
                              1.55             102.4          10132.0               76.8           7599.2
                              3.10              96.0          10286.4               72.0           7715.0
                              4.65              31.6          10422.9               23.7           7817.3
                              6.20            -526.4          10039.4             -394.8           7529.7
                              7.75           -1084.4           8791.0             -813.3           6593.4
                              9.30           -1642.4           6677.8            -1231.8           5008.5
                             10.85           -1674.0           3892.0            -1255.5           2919.1
                             12.40           -1116.0           1729.8             -837.0           1297.4
                             13.95            -558.0            432.4             -418.5            324.3
                             15.50               0.0              0.0                0.0              0.0

*** TOTAL REINFORCEMENT PCT =  0.76    REINFORCEMENT AREA (in^2) =   48.35
*** USABLE AXIAL CAP.   (k) =  57.0    USABLE MOMENT CAP. (ft-k) =  7852.2
```

For Design:

```
7.5-ft Diameter caisson x 15.5-ft long (15-ft Embeded with 0.5-ft above grade)
Concrete strength =4500 PSI @ 28 days. Estimated Concrete Volume = 25 CY3.
(40) #10 Vertical Rebar. Steel Cross-Section = 50.8 in2
```

Monopole Spread Footing Calculation

TIA-222-H

Factored Base Reactions:		**Footing Dimensions:**		**Concrete:**
Moment:	7200 ft-kips	29 ft x 29 ft	7.5 ft Square Pier	f'c = 4500 psi
Shear:	77 kips	x 4 ft thick	w/6 in Reveal	Steel fy = 60 ksi
Axial:	57 kips	Bearing 6 ft B.G.	129.8 Yd3 Concrete	f = 0.75
Soil Backfill	100 pcf	Ultimate Bearing:	8000 psf	Water Table n/a

Foundation Weight

Weight of Pole	57.0	kips
Weight of Concrete	525.69375	kips
Weight of Soil	156.95	kips
Bouyancy of Water	0.0	kips
Total	739.6	kips

Overturning Resistance:

Overturning Moment (M_u)	7700.5	ft-kips	7200 ft-kips + (77 kips x 6.5 ft)
Resisting Moment (R_s)	10724.834	ft-kips	739.64375 kips x 29 ft / 2
ϕ x R_s > M_u	$M_{overturning}$ / f M_{resist}		**95.7%** **OK**

Soil Bearing Pressure:

Eccentricity (e)	10.41	ft	7700.5 ft-kips / 739.64375 kips
6(e)	62.5 ft	>	29.0 ft 6e > 29
Maximum Soil Bearing	3884.5144	psf	Calculated across corners
Soil Overburden	-600	psf	
Net Soil Bearing	3284.5144	psf	
Resisting Soil Bearing (R_s)	8000	psf	
Net Soil Bearing < ϕ x R_s	Net Bearing / f R_s		**54.7%** **OK**

Bending Moment in Pier:

Bending Moment	7392.5	ft-kips	7200 ft-kips + (77 kips x 2.5 ft)
Min. Pier Steel	40.5	in^2	1/2% (Based on Square Pier)

Bending Moment in Footing:

Max Bending Moment	4932.5528	ft-kips	Σ Moments about pier face
Footing Steel Req'd (Loads)	1.26	in^2/ft	
Min. Footing Steel	1.04	in^2/ft	0.18%

WELTI GEOTECHNICAL, P.C.

227 Williams Street · P.O. Box 397
Glastonbury, CT 06033-0397

(860) 633-4623 / FAX (860) 657-2514

January 18, 2023

Mr. Keith Coppins, Managing Director
ARX Wireless
110 Washington Avenue
North Haven, CT 06473

Re: Geotechnical Study for Proposed Cell Tower (Site No. CT1024A), 92 Greens Farms Road, Westport, CT

Dear Mr. Coppins:

1.0 Herewith are the data from the test boring taken at the above referenced site. One boring was taken at the proposed tower location. The boring was drilled to a depth 9.5 feet below the existing grade. Hard bedrock was encountered at 4.5 feet below the existing grade. The boring was cored 5 feet into the bedrock. The tower center was staked in the field by others and is shown on the attached plan. *The boring was drilled by Clarence Welti Associates, Inc. and sampling was conducted by this firm solely to obtain indications of subsurface conditions as part of a geotechnical exploration program. No services were performed to evaluate subsurface environmental conditions.*

2.0 The **Subject Project** will include the construction of a monopole tower with a height of about 124 feet. The existing grades in the area of the proposed tower compound range from about Elev.18 to Elev.20. The proposed grade at the base of the tower will be at about Elev.19.5.

3.0 The **Soil/Rock Cross Section** from the boring is generally as follows:

Topsoil to 3.5"

Subsoil; fine SAND and SILT to 1.5 feet, medium compact

Fine to coarse SAND, little to some Silt and Gravel to the top of bedrock at 4.5 feet, dense

Bedrock; Gneiss and Schist
Note: Cored bedrock from 4.5 to 9.5 feet below grade with recovery of 65% and RQD value of 0% .

3.1 The **Ground Water Table** was not evident above the bedrock.

4.0 In general the criteria for tower support is that the foundation capacity would exceed the loads, which might collapse the tower. **Movements from strains in the soils should be limited to differential settlement (or lateral movements of less than ½").**

5.0 The **foundation for the tower** can be with a mat type foundation. The mat would provide the required weight for resistance to over turning. The mat could be placed on the rock or on prepared blasted rock surface. The bottom of the mat should be at least 3.5 feet below finished grades for frost protection or be placed entirely on the hard bedrock. Where the mat is atop weathered rock or a blasted rock surface there should be a minimum 6" layer of 3/8" crushed stone beneath the foundation. The allowable loading on the hard bedrock, on crushed stone atop the weathered rock or on crushed stone atop a blasted rock surface can be 3 Tons/sf. If bedrock removal is required, the pad area could be over blasted by 1 to 2 feet and the area could be leveled with a minimum 6" layer of 3/8" crushed stone, after removal of any large and loose pieces of rock. The foundation sub grades should be observed by someone from our office to confirm the sub grade conditions are preparation are acceptable.

5.0.1 If **rock anchors** are used to provide resistance to uplift the design of such anchorages must address (1) the bond between the grout and the rock and (2) the potential uplift of a truncated pyramid from the applied group loading on the tie downs (global stability). The recommended allowable bond strength between the rock and the hard intact rock is 50 psi. The bond would start 2 feet below top of bedrock. The pull out volume would be a truncated cone with a base of about 5 feet in diameter (at each tie down rod) and a 60°slope (30° off the vertical axis) to ground surface.

5.1 Summary of design parameters:

Parameter	Value
Allowable Bearing Pressure	3 Tons/sf
Soil Unit Weight (natural soils above water table)	125 pcf
Soil Unit Weight (natural soils below water table)	63 pcf
Soil Unit Weight (backfill)	125 pcf
Angle of Internal Friction	34°
At rest coefficient	0.45
Active coefficient	0.28
Allowable Cement Grout to Rock Bond Value	50 psi

Frost Protection Depth	3.5 feet

6.0 This report has been prepared for specific a application to the subject project in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. In the event that any changes in the nature, design and location of structures are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing.

The analyses and recommendations submitted in this report are based in part upon data obtained from referenced explorations. The extent of variations between explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

Welti Geotechnical, P.C., should perform a general review of the final design and specifications in order that geotechnical design recommendations may be properly interpreted and implemented as they were intended.

If you have any questions please call me.

Very truly yours,

Max Welti

Max Welti, P. E.
President, Welti Geotechnical, P.C.

APPENDIX

BORING LOGS

BORING LOCATION PLAN

CONNECTICUT SITING COUNCIL DOCKET

LEGEND

- SUBJECT PROPERTY LINE
- ABUTTERS PROPERTY LINE
- WIRE FENCE
- CHAIN LINK FENCE
- STOCKADE FENCE
- CONTOUR LINE
- OHW — OVERHEAD WIRES
- WETLAND DELINEATION
- FEMA FLOOD ZONE LINE
- N/F — NOW OR FORMERLY
- E06–074 — ASSESSORS ID
- TREE
- STAKE
- UTILITY POLE
- GUY WIRE ANCHOR
- TOWER CONTROL POINT
- WETLAND FLAG

GCS NAD83

E06–060
111 HILLSPOINT ROAD
N/F
JACQUELINE HAYES
111 HILLSPOINT ROAD
WESTPORT, CT 06880

GREENS FARMS ROAD

BIT. ROAD

UP#23K11–91T

CONTROL POINT B
–IRON ROD SET–
N: 606365.28
E: 835979.05

UP#11509
(GUY POLE)

UP#26138

CONTROL POINT A
–IRON ROD SET–
N: 606476.90
E: 836139.78

STONE RET. WALL

HOUSE

DECORATIVE WELL

PROPOSED DRIVE

E06–074
92 GREENS FARMS ROAD
N/F
PRADIV MAHESH &
SHARUNA MOOLA–MAHESH
92 GREENS FARMS ROAD
WESTPORT, CT 06880

AREA = 1.99 AC. ±

SHED

WALL

RETAINING

TBM
SPIKE IN ROOT
EL=28.30'

STONE

PROPOSED COMPOUND FENCE

PROPOSED CENTER OF TOWER
LAT: 41°07'25.22"
LON: 73°20'41.45"
N: 606351.60
E: 836109.67
GROUND EL: 19'±

B-1

STONE RETAINING WALL

INTERSTATE 95

FEMA ZONE X
FEMA ZONE AE
(BFE=10')

CULVERT

WETLAND

TEST BORING LOCATION
CLARENCE WELTI ASSOCIATES, INC.
1/6/23

15' 0' 15' 30'

TARPON TOWERS

Project:

WESTPORT

92 GREENS FARMS ROAD
WESTPORT, CT

Prepared For:

TARPON TOWERS II

8916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

Douglas J. Roberts – Architect
110 Washington Avenue
Fourth Floor
North Haven, CT 06473

Tel: 203.234.6368
Email: droberts - architect@outlook.com

STATE OF CONNECTICUT · DOUGLAS J. ROBERTS · No. 4310 · LICENSED ARCHITECT

Key Plan

Do not scale dimensions from drawings
Site verify all dimensions prior to construction
Report all discrepancies to Architect immediately
This drawing is to be read in conjunction with all relevant documents and drawings

REVISION SCHEDULE

REVISION	DESCRIPTION	DATE
0	REVIEW	03/09/2022

Drawing By:	Brian Franetovich
Drawing Date:	February 9, 2022
Reviewed By:	Charles Gidman
Project No:	2021.12
Scale:	AS SHOWN

Sheet Title:

EXITING CONDITIONS PLAN

Original drawing is ANSI - D.
Do not scale contents of this drawing.

Sheet Number: C-2

Revision: 0

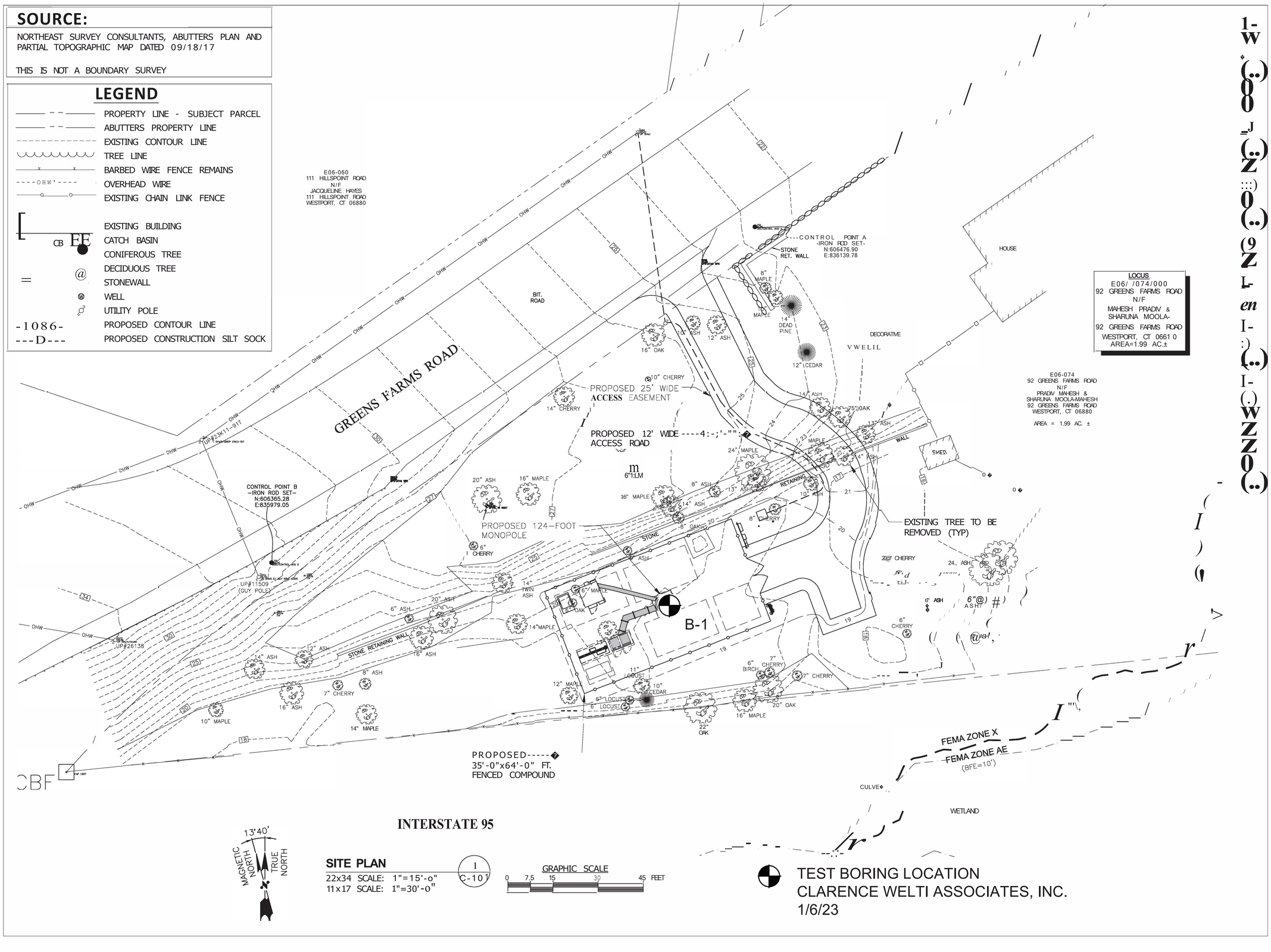
SOURCE:

NORTHEAST SURVEY CONSULTANTS, ABUTTERS PLAN AND PARTIAL TOPOGRAPHIC MAP DATED 09/18/17

THIS IS NOT A BOUNDARY SURVEY

LEGEND

- PROPERTY LINE - SUBJECT PARCEL
- ABUTTERS PROPERTY LINE
- EXISTING CONTOUR LINE
- TREE LINE
- BARBED WIRE FENCE REMAINS
- OVERHEAD WIRE
- EXISTING CHAIN LINK FENCE
- EXISTING BUILDING
- CB CATCH BASIN
- CONIFEROUS TREE
- DECIDUOUS TREE
- STONEWALL
- WELL
- UTILITY POLE
- -1086- PROPOSED CONTOUR LINE
- ---D--- PROPOSED CONSTRUCTION SILT SOCK



12/25/2016 6:10:08 PM

File Path

TARPON TOWERS

Project:

WESTPORT

92 GREENS FARMS ROAD
WESTPORT, CT

Prepared For:

TARPON TOWERS II

3916 77TH TERRACE EAST
SUITE 103
LAKEWOOD RANCH, FL 34202

Project No: 2021.12

DOUGLAS J. R081RTS- ARCHIIICI'
110 Washington Avenue
Fourth Floor
North Haven, CT 06473

Tel: 203.234.6368
Email: droberts - architect@oullook.com

Key Plan

Do not scale dimensions from drawings
Plse verify all dimensions prior to canstnJctlon
Report all discrepancies ta Architect immediately
This drawing is ta be read in conjunction with all relevant documents and drawings

REVISION SCHEDULE

REVISION	DESCRIPTION	DATE
0	REVIEW	03/09/2022
1	FINAL	05/03/2022

Drawing By: Kimberly A. Molitiemo
Drawing. Date.: February 25, 2022
Reviewed By: Niddrie Rowe
Project No: 2021.12
Scale.: AS SHOWN

Sheet Title:

SITE PLAN

Original drawing is ANSI -D.
Do not scale contents of this drawing.

Sheet Number: **8-101**

Revision: **1**

CLARENCE WELTI ASSOC., INC.
P.O. BOX 397
GLASTONBURY, CONN 06033

CLIENT
ARX

PROJECT NAME
PROPOSED CELL TOWER - CT1024A
LOCATION
92 GREENS FARMS ROAD, WESTPORT, CT

	AUGER	CASING	SAMPLER	CORE BAR.
TYPE	HSA		SS	NQ
SIZE I.D.	3.75"		1.375"	2.0"
HAMMER WT.			140lbs	
HAMMER FALL			30"	

OFFSET
LINE & STA.
N. COORDINATE
E. COORDINATE

SURFACE ELEV.
HOLE NO. B-1

GROUND WATER OBSERVATIONS
AT none FT. AFTER 0 HOURS
AT FT. AFTER HOURS

START DATE 1/6/23
FINISH DATE 1/6/23

DEPTH	SAMPLE NO.	SAMPLE BLOWS/6"	SAMPLE DEPTH	A	STRATUM DESCRIPTION + REMARKS	ELEV.
0	1	6-7-7-10	0.0'-2.0'		TOPSOIL 0.30	
					BR.FINE SAND AND SILT	
					GREY/BR.FINE-CRS.SAND, LITTLE TO SOME SILT & GRAVEL 1.5	
	2	21-47-45-19	2.0'-4.0'			
5					CORED BEDROCK - GNEISS AND SCHIST 4.5	
					RUN #1 4.5' - 9.5' RECOVERED 39" RQD=0%	
10					BOTTOM OF BORING @ 9.5' 9.5	
15						
20						
25						
30						
35						

LEGEND: COL. A:
SAMPLE TYPE: D=DRY A=AUGER C=CORE U=UNDISTURBED PISTON S=SPLIT SPOON
PROPORTIONS USED: TRACE=0-10% LITTLE=10-20% SOME=20-35% AND=35-50%

DRILLER: J. BREWER
INSPECTOR:

SHEET 1 OF 1 | HOLE NO. **B-1**

Site Name: **Westport**

Site Number: **CT 1024**

Site Address: **92 Greens Farms Rd**

Keith Coppins

203-623-3287

Week of:	4/17/2023							4/24/2023							5/1/2023							5/7/2023						
ACTIVITY	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S
Mobilization	x																											
Easement		x	x	x	x																							
Primary Utilities									x	x	x	x			x	x	x	x	x									
Compound Clearing, Cut and Fill																						x	x	x				
Foundation - Set Hole																									x	x		
Foundation - Rebar and Formwork																												
Foundation - Pour																												
Foundation - Strip and backfill																												
Ground Ring																												
Compund Stone/Finish																												
Tower Delivery & Erection																												
Fencing																												
Clean up and restoration																												

Week of:	5/14/2023							5/22/2023							5/29/2023							6/5/2023						
ACTIVITY	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S
Mobilization																												
Easement																												
Primary Utilities																												
Compound Clearing, Cut and Fill																												
Foundation - Set Hole																												
Foundation - Rebar and Formwork	x	x	x	x																								
Foundation - Pour					x																							
Foundation - Strip and backfill								x	x																			
Ground Ring								x																				
										x	x	x																
															x	x	x	x	x									
Compund Stone/Finish																									x	x		
Tower Delivery & Erection																												
Fencing																						x	x	x				
Clean up and restoration																												

Week of:	6/12/2023							6/19/2023							6/26/2023																				
ACTIVITY	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S	M	T	W	T	F	S	S
Mobilization																																			
Easement																																			
Primary Utilities																																			
Compound Clearing, Cut and Fill																																			
Foundation - Set Hole																																			
Foundation - Rebar and Formwork																																			
Foundation - Pour																																			
Foundation - Strip and backfill																																			
Ground Ring																																			
					x			x	x	x	x	x			x	x	x	x	x																
Compund Stone/Finish																																			
Tower Delivery & Erection				x																															
Fencing																																			
Clean up and restoration								x	x	x	x	x																							

Notes:

Hours of Operation will be from 7:30 am until 6:30 pm Monday through Friday