Soils and Foundations: 2012 IBC

Presented by
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for the
Office of Education and Data Management
Spring 2016 Career Development Series

Soils and Foundations

- Seminar will review the International Building Code requirements for soils and foundations:
  - Geotechnical investigations, foundation and soils
  - excavation, grading and fill,
  - load bearing values of soils,
  - dampproofing and waterproofing
  - design and construction of foundations
    - Shallow Foundations
    - Deep Foundations
Chapter 18 - Soils and Foundations
International Building Code 2012

- 1801 General
- 1802 Definitions
- 1803 Geotechnical Investigations
- 1804 Excavation, Grading, and Fill
- 1805 Dampproofing & Waterproofing
- 1806 Presumptive Load-Bearing Values of Soils
- 1807 Walls, Posts, Poles
- 1808 Foundations
- 1809 Shallow Foundations
- 1810 Deep Foundations
Section 1801
General

• Scope
  – The provisions of IBC Chapter 18 Soils and Foundations applies to building and foundation systems

• Design
  – Allowable bearing pressure, allowable stresses and design formulas provided shall be used with the allowable stress design load combinations specified in Section 1605.3 - Load combinations using allowable stress design.
  – Quality and design of materials used structurally in excavations, footings and foundations shall conform to requirements specified in Chapter 16-Structural Design, 19-Concrete, 21-Masonry, 22-Steel and 23-Wood.
  – Excavations and fills shall also comply with Chapter 33 Safeguards During Construction.
Design Basis

- **1801.2 Design basis**
  - Section 1605.3 design considerations
    - All loads must be considered
      - Dead
      - Live
      - Wind
      - Flood
      - Seismic

Section 1801
General

- Refer to Structural Design Section 1605.3 - Load combinations using allowable stress design.
  - **Section 1605.3.1 Basic load Calculations**
    
    \[
    D + F \\
    D + H + F + L \\
    D + H + F + (L_r \text{ or } S \text{ or } R) \\
    D + H + F + 0.75(L) + 0.75 (L_r \text{ or } S \text{ or } R) \\
    D + H + F + 0.75(0.6W) + 0.75L + 0.75 (L_r \text{ or } S \text{ or } R) \\
    D + H + F + 0.75(0.7E) + 0.75 L + 0.75 S \\
    0.6 D + 0.6W + H \\
    0.6 (D+F) + 0.7 E + H
    \]
Section 1801

General

• Refer to Structural Design Section 1605.3 - Load combinations using allowable stress design.
  
  – Section 1605.3.2 Alternative basic load calculations

  \[ D + L + (L_e \text{ or } S \text{ or } R) \]
  \[ D + L + 0.6 \omega W \]
  \[ D + L + 0.6 \omega W + S/2 \]
  \[ D + L + S + 0.6 \omega W/2 \]
  \[ D + L + S + E/1.4 \]
  \[ 0.9D + E/1.4 \]

Lateral Soils Loads

<table>
<thead>
<tr>
<th>TABLE 1610.1</th>
<th>LATERAL SOIL LOAD</th>
</tr>
</thead>
<tbody>
<tr>
<td>DESCRIPTION OF BACKFILL MATERIAL</td>
<td>UNIFIED SOIL CLASSIFICATION</td>
</tr>
<tr>
<td></td>
<td></td>
</tr>
<tr>
<td>Well-graded, clean gravels, gravel-sand mixes</td>
<td>GW</td>
</tr>
<tr>
<td>Poorly graded clean gravels, gravel-sand mixes</td>
<td>GP</td>
</tr>
<tr>
<td>Silty gravels, poorly graded gravel-sand mixes</td>
<td>GM</td>
</tr>
<tr>
<td>Chucky gravels, poorly graded gravel-clay mixtures</td>
<td>GC</td>
</tr>
<tr>
<td>Well-graded, clean sands, gravel-sand mixes</td>
<td>SW</td>
</tr>
<tr>
<td>Poorly graded clean sands, sand-gravel mixtures</td>
<td>SP</td>
</tr>
<tr>
<td>Silty sands, poorly graded sand-silt mixtures</td>
<td>SM</td>
</tr>
<tr>
<td>Sand-silt clay mix with plastic fines</td>
<td>SM-SC</td>
</tr>
<tr>
<td>Chucky sands, poorly graded sand-clay mixtures</td>
<td>SC</td>
</tr>
<tr>
<td>Inorganic silts and clayey silts</td>
<td>ML</td>
</tr>
<tr>
<td>Mixture of inorganic silt and clay</td>
<td>ML-CL</td>
</tr>
<tr>
<td>Inorganic clays of low to medium plasticity</td>
<td>CL</td>
</tr>
<tr>
<td>Organic silts and silt clays, low plasticity</td>
<td>OL</td>
</tr>
<tr>
<td>Inorganic clayey silts, elastic silts</td>
<td>NH</td>
</tr>
<tr>
<td>Inorganic clays of high plasticity</td>
<td>CH</td>
</tr>
<tr>
<td>Organic clays and silt clays</td>
<td>CH</td>
</tr>
</tbody>
</table>

For SFL 1 pound per square foot per foot of depth = 0.137 kPa, 1 foot = 0.3048 m

a. Design lateral soil loads are given for in situ conditions for the specified soils at their optimum densities. Actual field conditions shall govern. Submerged or saturated soil pressures shall include the weight of the buoyant soil plus the hydrostatic loads.

b. Unavailable as backfill material.

c. The definition and classification of soil materials shall be in accordance with ASTM D 2487.
Section 1803
Geotechnical Investigations

• 1803.1 General (Geotechnical Investigations)
  – 1803.2 Investigations Required
    • Exception: Building Official permitted to waived requirement where satisfactory
  – 1803.3 Basis
    • Bore samples
    • Test pits
    • Other sub-surface exploration
      – Trained gopher?

Section 1803.3
Basis Investigations

• Soil classification shall be based on
  – observation
  – Any other necessary tests of the materials disclosed by
    • Borings
    • Test pits
    • Other subsurface explorations

• Additional Studies as necessary
Section 1803.3
Basis Investigations

• Additional Studies as necessary to evaluate
  – Slope stability
  – Soil strength
  – Position
  – Adequacy of load bearing soils
  – Effects of moisture variation on soil bearing capacity, compressibility, liquefaction and expansiveness

Scope of Investigation

• 1803.3.1 Scope of Investigation
  Including
  – Number and types of boring or soundings
  – The Equipment used to drill or sample
  – The in-situ testing equipment
  – Laboratory testing program
  – Shall be determined by a registered design professional.
Section 1803
Geotechnical Investigations

1803.5 Investigated conditions.
This section provides parameters for classification of soils in accordance with the code.

• 1803.5.1 Classification.
  – Soil Materials shall be classified in accordance with ASTM D 2487.
Section 1803
Geotechnical Investigations

1803.5.2 Questionable soil.
The building official shall be permitted to require that a Geotechnical Investigation be conducted
• Where the classification, strength or compressibility of the soil is in doubt, or
• Where a load-bearing value superior to that specified in this code is claimed,

Section 1803
Geotechnical Investigations

1803.5.3 Expansive soils.
• In areas likely to have expansive soil, the building official shall require soil tests to determine where such soils do exist.
• Soils Meeting all four of the following provisions
  — Plasticity index (PI) of 15 or greater, (ASTM D 4318)
  — More than 10 % of the soil particles pass a No. 200 sieve (D442)
  — More than 10 % of the soil particles are less than 5 micrometers in size (ASTM D 422)
  — Expansion index greater than 20 (ASTM D 4829)
Section 1803
Geotechnical Investigations

1803.5.4 Ground-water table.
• A subsurface soil investigation shall be performed to determine whether the existing ground-water table is above or within 5 feet (1524 mm) below the elevation of the lowest floor level where such floor is located below the finished ground level adjacent to the foundation.

Exception: A subsurface soil investigation shall not be required where waterproofing is provided in accordance with Section 1805.

Section 1803
Geotechnical Investigations

1803.5.5 Deep foundations
• Where used
  – Recommend deep foundation types & installed capacities
  – Center to center spacing of elements
  – Driving criteria
  – Installation procedures
  – Field inspections and Reporting procedures
  – Load Testing requirements
  – Designation of bearing stratum or strata
  – Reductions for group action where necessary
Section 1803
Geotechnical Investigations

1803.5.6 Rock strata.
Where subsurface explorations at the project site indicate variations or doubtful characteristics in the structure of the rock upon which foundations are to be constructed, a sufficient number of borings shall be made to a depth of not less than 10 feet (3048 mm) below the level of the foundations to provide assurance of the soundness of the foundation bed and its load-bearing capacity.

1803.5.7 Excavation near Foundation
Where excavation will remove lateral support from any foundation, an investigation shall be conducted to assess potential consequences.

Section 1803
Geotechnical Investigations

1803.5.8 Compact fill materials.
- Where shallow foundations will bear of compacted fill more than 12 inches in depth a geotechnical investigation shall be conducted.

1803.5.9 Controlled low strength materials
- Where shallow foundations will bear of controlled low-strength materials (CLSM) a geotechnical investigation shall be conducted.

1803.5.10 Alternate Setback and Clearance.
- Where setbacks or clearances other than those required in Section 1808.7 are desired, The building official shall be permitted to require a geotechnical investigation by a registered design professional to demonstrate that the intent of Section 1808.7 would be satisfied.
Section 1803
Geotechnical Investigations

1803.5.11 Seismic Design Category C through F.
- For Seismic Design Category C, D, E or F an investigation shall be conducted, and evaluate earthquake motions potential hazards resulting from: slope instability, liquefaction and surface rupture due to faulting or seismically induced lateral spreading or Lateral flow.

1803.5.12 Seismic Design Category D through F. The Seismic Design Category D, E or F includes soils investigation requirements for Seismic Design Category C through F, **Plus**
- lateral earth pressures on basement and retaining walls
- Potential for liquefaction and soil strength loss etc.
- An assessment of potential consequences
- Mitigation measures

Reporting

- **1803.6 Reporting of Geotechnical Investigations**
  - Submitted to the BO
  - Includes:
    - 1. Plot
    - 2. Record of samples
    - 3. Soil profile
    - 4. Water table, if encountered
    - 5. Foundation recommendation
    - 6. Settlement expectation
    - 7. Deep foundation consideration
    - 8. Expansive soil foundations
    - 9. Compacted fill requirements
    - 10. Shallow foundation bearing
Section 1804
Excavation, Grading and Fill

1804.1 - Excavations near foundations

Excavations for any purpose shall not remove lateral support from any foundation without first underpinning or protecting foundation against settlement or lateral translation.
Excavation, Grading, Filling

- 1804 Excavation, Grading, Filling
  - Adjacent structures protected
    - Underpinned
    - Lateral support
  - ‘Clean’ backfill
    - No organics
    - Junk
    - Boulders
    - Installed in compacted ‘lifts’
    - No damage to moisture protection

Section 1804 - Excavation, Grading and Fill
1804.2 - Placement of backfill.

The excavation outside the foundations
- Shall be backfilled with soil that is free of organic material, construction debris, cobbles and boulders or a controlled low strength material (CLSM)
- Backfill shall be placed in lifts and compacted in a manner that does not damage the foundation or the waterproofing or dampproofing material.
Excavation, Grading, Filling

- **1804.3 Site Grading**
  - 1:20 (5% slope) for distance of 10’
  - Drainage required if 10’ not possible
  - 2% slope for swales and pavement

**1804.3 - Site Grading.**

- 20 units horizontal
- 1 unit vertical
- 5% slope
Excavation, Grading, Filling

• 1804 Excavation, Grading, Filling
  – 1804.4 Grading and fill in flood hazard Areas
    • Placed & sloped to minimize movement
    • Material will not exacerbate flooding
    • Will not divert waves toward structures
    • Will not become a dam to drainage

Section 1804 - Excavation, Grading and Fill

1804.4 – (2) Grading and fill in floodways.
• Requires hydrologic and hydraulic analyses performed by a registered design professional
• In accordance with standard engineering practice
• Proposed grading or fill or both will not result in any increase in flood levels during the occurrence of the design flood.
Section 1804 - Excavation, Grading and Fill
1804.5 – Compacted Fill Material.

Where Shallow Foundations will bear on compacted fill materials, compacted fill shall comply with the provisions of an approved geotechnical report which contains:

1. Specifications for site preparations
2. Specifications for materials to be used as compacted fill.
3. Test methods to determine max dry density and optimum moisture content
4. Maximum allowable thickness of each lift
5. Field test methods for determining in-place dry density
6. Minimum acceptable in-place dry density (accordance with 3)
7. Number and frequency of field tests

1804 Excavation, Grading, Filling

– 1804.5 Compacted Fill
  • Engineered design
    – Exception for shallow fill
  • Requires 1705.6 Special Inspection

– 1804. 6 Controlled low-strength material (CLSM)
  • CLSM comply with provisions of approved geotechnical report
Section 1804 - Excavation, Grading and Fill
1804.6 – Controlled Low-Strength Material.

Where shallow foundations will bear on controlled low-strength materials (CLSM), CLSM shall comply with the provisions of an approved geotechnical report (1803) which contain:

1. Specifications for site preparations
2. Specifications for the CLSM
3. Laboratory or Field Test method(s) used to determine the compressive strength or bearing capacity of the CLSM
4. Test methods for determining the acceptance of the CLSM in the field
5. Number and frequency of field test required to determine compliance with item 4.
Examples of Compactors
1. Manually Operated Walk-Behind Vibratory Plate Compactor
2. Single Drum Vibratory Soil Compactor
Dampproofing and Waterproofing

• 1805.2 Dampproofing
  – Water vapor impediment
  – Compound or coating
  – Footing top to above ground
  – Sub-grade occupied areas
  – Water table is 5” below floor

• 1803.5.4 Ground-water table
  – Investigation may eliminate damp proofing as option
    • Must – waterproof
Dampproofing and Waterproofing

• 1805.2.1 Dampproofing Floors
  – 6 mil polyethylene
  – Proper laps
  – Other approved materials
    • 4 mil plastic above floor
      – With finish floor
    • Mopped bitumen

• 1805.3 Waterproofing
  – As 1803.5.4 investigation indicates
    • Hydrostatic pressure found at floor level
  – Higher degree of protection
  – Generally a membrane material
  – Hydrostatic pressure resistant
  – Bottom of wall to 12” over water table
Dampproofing and Waterproofing

• 1805.3.1 Waterproofing Floors
  – Must be concrete
    • To counter hydraulic pressure
  – Membrane materials
    • Simple placement of 4 or 6 mil polyethylene would not meet compliance
    • 6 mil PVC lapped and sealed will work

• 1805.3.1 Waterproofing Walls
  – Must be concrete or masonry
    • Designed to handle the hydrostatic pressure
  – 12” above water table
    • Area above this need only be damp proofed
  – Prescriptive materials
  – Other approved methods
    • 104.11
Dampproofing and Waterproofing

• 1805.3.3 Joint treatment
  – Waterproofing requires sealing at floor / wall joint
    • Water tight


Dampproofing and Waterproofing

• 1805.3.3 Penetration Waterproofing
  – “Approved methods”
• **1805.4 Subsoil Drainage System**
  - Used in conjunction with dampproofing
  - Under floor - 4” thick gravel of specific size
  - Exterior perimeter drain in gravel
  - Gravity or mechanical disposal

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• **1805.1.3 Engineered system**
  - (Big scale drain application)
  - Hydraulic considerations
    - Soil type(s)
    - Hydraulic pressure
  - Water table modification
    - Drainage
    - Retention pond control dams
  - Possibly very large scale (acres)
    - Slurry wall barriers
    - Continuous pumping
    - Relief wells
Section 1806
Presumptive Load Bearing Values of Soils

• Presumptive Load-Bearing Values
  – See Table 1806.2 Allowable Foundation and Lateral Pressure
  – Shall be used with the allowable stress design load combination

Presumptive Load Bearing Values of Soils

• Table 1806.2 Presumptive Load Bearing Values
  – Lateral bearing pressure – soils close to surface
    • Resistance to pressure exerted horizontally
    • Increases with depth
      – 15X limitation per 1806.3.3

  – Lateral sliding resistance
    • Coefficient of friction
      – Difficulty factor related to making soil move
    • Cohesion
      – Ability of ‘muck’ to prevent movement
      – 130 psf
Section 1806
Allowable Load Bearing Values of Soils

<table>
<thead>
<tr>
<th>CLASS OF MATERIALS</th>
<th>ALLOWABLE FOUNDATION PRESSURE (psf)</th>
<th>LATERAL BEARING (psf)</th>
<th>LATERAL SLIDING</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Crystalline bedrock</td>
<td>12,000</td>
<td>1,200</td>
<td>0.79</td>
</tr>
<tr>
<td>2. Sedimentary and foliated rock</td>
<td>4,000</td>
<td>400</td>
<td>0.35</td>
</tr>
<tr>
<td>3. Sandy gravel and/or gravel (GW and GP)</td>
<td>3,000</td>
<td>200</td>
<td>0.35</td>
</tr>
<tr>
<td>4. Sand, silty sand, clayey sand, silty gravel and clayey gravel (SW, SM, SC, GM and GC)</td>
<td>2,500</td>
<td>150</td>
<td>0.25</td>
</tr>
<tr>
<td>5. Clay, sandy clay, silty clay, clayey silt, silt and sandy silt (CL, ML, MH and CH)</td>
<td>1,500</td>
<td>100</td>
<td>—</td>
</tr>
</tbody>
</table>

For SI: 1 pound per square foot = 0.0479 kPa. 1 pound per square foot per foot = 0.1574 kN/m.
a. Coefficient to be multiplied by the dead load.
b. Lateral sliding resistance value to be multiplied by the contact area, as limited by Section 1804.3.
c. Where the building official determines that in place soils with an allowable bearing capacity of less than 1,500 psf are likely to be present at the site, the allowable bearing capacity shall be determined by soils investigation.
d. An increase of one-third is permitted when using the ultimate load combinations in Section 1605.3.2 that include wind or earthquake loads.

Foundation Walls, Retaining Walls and Embedded Post and Poles

- Section 1807.1 Foundation Walls
  - Foundation walls
  - Lateral soil loads
  - Unbalanced backfill
  - Rubble stone
  - Wood foundations
  - Concrete & Masonry Foundations
  - Prescriptive Concrete & Masonry Foundations
Foundation Walls, Retaining Walls and Embedded Post and Poles

- Section 1807.2  Retaining Walls
  - Designed
    - Ensure stability against
      - Overturning
      - Sliding
      - Excessive Foundation Pressure
      - Water uplift
      - lateral resistance must be countered at the wall's base?
    - Lateral Soil Loads
    - Safety Factor

Retaining Walls

- Design to ensure Stability against:
  - Overturning
  - Sliding
  - Excessive Foundation Pressure
  - Water Uplift
- Design for a Safety Factor of 1.5 against Lateral Sliding and overturning
Embedded Posts & Poles

- Section 1807.3 Embedded Posts & Poles
  - Axial & lateral loads must be considered
  - Type of soil is important to resist:
    - Lateral movement due to soil movement
    - Lateral movement due to ‘slippery’ soil
  - Depth of embedment is important
  - Type of embedment is important
    - Backfill is prescriptive
      - 2000 psi concrete
      - Sand
      - CLSM

Embedded Posts & Poles

- 1807.3.2 Design Criteria

1807.3.2.1 Non-constrained Installation
1807.3.2.2 Constrained Installation
Embedded Posts & Poles

- **1807.3.2.1 Non constrained Embedment**
  - **Equation 18-1**
    - $d = 0.5A[1 + [1 + (4.36h/A)]^{1/2}]$
    - Where: $A = \frac{2.34P}{(S_1b)}$
  - $b$ = diameter of round post or footing / diagonal of other
  - $d$ = depth of embedment to maximum of 12’
  - $h$ = height to application of lateral force
  - $P$ = applied lateral force in pounds
  - $S_1$ = Section 1806.2 lateral pressure limit at 1/3 embed depth

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**Example Calculation**

- **Step 1**
  - $A = \frac{2.34 \cdot 200}{450 \cdot 1}$
  - $A = 1.04$

- **Step 2**
  - $d = 0.5A[1 + [1 + (4.36h/A)]^{1/2}]$
  - $d = 0.5 \cdot 1.04[1 + [1 + (4.36 \cdot 40/1.04)]^{1/2}]$
  - $d = 0.52[1 + [1 + (174.4/1.04)]^{1/2}]$
  - $d = 0.52[1 + [1 + (174.4/1.04)]^{1/2}]$
  - $d = 7.3’ minimum$

12” diameter utility pole, 9’ embedment in sand
Embedded Posts & Poles

• 1807.3.2.2 Constrained Embedment
  – Equation 18.2
  – Soil resistance based on bury depth

Embedded Posts & Poles

• 1807.3.2 Design Criteria
  – Or by other methods “approved by BO”
Foundations

• 1808 Foundations
  – General foundation requirements
• 1809 Shallow Foundations (202)
  – Grade foundations
  – Mat foundations on fill
  – “slab” foundations
• 1810 Deep foundations (202)
  – 1801.2
    • Design based on Section 1803 geotechnical investigation

Foundations

• 1808.2 Design for capacity & settlement
  – Based on soil conditions
• 1808.3 Design loads
  – Most severe load conditions
  – Seismic overturn resistant
• 1808.4 Vibratory loads
  – Based on machines within structure
• 1808.5 Shifting or moving soils
  – Sufficient depth to avoid movement
Foundations

- 1808.6 Design Expansive Soils
  - As identified by 1803.5.3
- 1808.6.1 Foundations
  - Designed to deal with expansive soil conditions
- 1808.6.2 Slab foundations
  - Reinforcement
- 1808.6.3 Removal of expansive soil
  - Removed to water table
- 1808.6.4 Stabilization
  - Dewatering, pre-saturation

Foundations

- 1808.7 Foundations at slopes
  - Slope drainage concern
  - Earth movement concern
    - Landslide towards structure
    - Landslide involving the foundation
- 1008.7.5 Alternate setback & clearance
  - Approval by BO based on geotechnical report
Foundations

- **1808.7.3 Pools**
  - ½ set back requirement of buildings
  - Pools < 7’ from slope top
    - Greater strength required

Foundations

- **1808.7.4 Drainage**
  - Foundation wall top >12” street drain elevation
    - Exception for alternate drainage plan
Foundations

- 1808.8 Concrete Foundations
  - Materials, design, construction details
    - Exception for 'light frame' structures
- 1808.8.1 Concrete strength
  - Tabular required compressive strength

<table>
<thead>
<tr>
<th>TABLE 1808.8.1</th>
<th>MINIMUM SPECIFIED COMPRESSIVE STRENGTH f', OF CONCRETE OR GROUT</th>
</tr>
</thead>
<tbody>
<tr>
<td>FOUNDATION ELEMENT OR CONDITION</td>
<td>SPECIFIED COMPRESSIVE STRENGTH f'</td>
</tr>
<tr>
<td>1. Foundations for structures assigned to Seismic Design Category A, B or C</td>
<td>2,500 psi</td>
</tr>
<tr>
<td>2a. Foundations for Group II or III occupancies of light-frame construction, two stories or less in height, assigned to Seismic Design Category E or F</td>
<td>2,500 psi</td>
</tr>
<tr>
<td>2b. Foundations for other structures assigned to Seismic Design Category D, E or F</td>
<td>3,000 psi</td>
</tr>
<tr>
<td>3. Precast non Prestressed driven piles</td>
<td>4,000 psi</td>
</tr>
<tr>
<td>4. Secant drilled shafts</td>
<td>4,000 psi</td>
</tr>
<tr>
<td>5. Micropiles</td>
<td>4,000 psi</td>
</tr>
<tr>
<td>6. Prestressed driven piles</td>
<td>5,000 psi</td>
</tr>
</tbody>
</table>

Foundations

- 1808.8.2 Concrete cover
  - Minimum thickness of cover over reinforcing steel

<table>
<thead>
<tr>
<th>TABLE 1808.8.2</th>
<th>MINIMUM COVER</th>
</tr>
</thead>
<tbody>
<tr>
<td>FOUNDATION ELEMENT OR CONDITION</td>
<td>MINIMUM COVER</td>
</tr>
<tr>
<td>1. Shallow foundation</td>
<td>In accordance with Section 7.11 of ACI 318</td>
</tr>
<tr>
<td>2. Prestressed or nonprestressed concrete elements</td>
<td>2 inches</td>
</tr>
<tr>
<td>3. Prestressed or nonprestressed concrete elements, manufactured under dry (nominal) conditions</td>
<td>3 inches</td>
</tr>
<tr>
<td>4. Concrete slab foundation elements constructed by a method with a compressive strength</td>
<td>3 inches</td>
</tr>
<tr>
<td>5. Concrete slab foundation elements constructed by a method with a compressive strength</td>
<td>4 inches</td>
</tr>
<tr>
<td>6. Concrete slab foundation elements constructed by a method with a compressive strength</td>
<td>5 inches</td>
</tr>
<tr>
<td>7. Concrete slab foundation elements constructed by a method with a compressive strength</td>
<td>6 inches</td>
</tr>
<tr>
<td>8. Concrete slab foundation elements constructed by a method with a compressive strength</td>
<td>7 inches</td>
</tr>
</tbody>
</table>
Foundations

• 1808.8.3 Concrete placement
  – Foreign matter excluded
  – Fully fill form
  – Avoid standing water
    • Tremie
    • Proper mix

Foundations

• 1808.8.4 Concrete protection
  – Protect from:
    • Freezing – 5 days
    • Running water
Foundations

• 1808.8.5 Concrete forms
  – Not required if BO ‘approves’
    • *Based on soil conditions*
  – If required:
    • ACI 318

Foundations

• 1808.8.6 Seismic requirements
  References
  – Section 1908
  – ACI 318
Foundations

- 1808.9 Vertical masonry foundation elements
  - If it’s not a ‘pier’ then it is a wall
    - Foundation pier: Isolated foundation member where the horizontal dimension to depth dimension is a maximum of 3:1 ratio and the height is no more than 4 times the thickness.
    - In other words; a short, stubby pile of concrete.

Shallow Foundations

- 1809 Shallow Foundations
  - 1809.1 General
    - Proper design and constructed per this section
  - 1809.2 Supporting soils
    - Undisturbed earth
    - controlled fill
Shallow Foundations

1809 Shallow Foundations
- 1809.3 Stepped Footings
  • 10% slope maximum

Bottom of footing

Diagram of a Step Footing

A = Horizontal Step
B = Vertical Step
T = Footing Thickness
P = Projection of Footing
W = Width of Footing

- All footings and steps should be level
- Step (B) should not exceed 3/4 of step (A)

Level
Shallow Foundations

- **1809.4 Footing Width**
  - 12” minimum width
- **1809.5 Frost Protection**
  - Footings
    - 42” below grade
    - ASCE 32
      - Frost protected shallow footings
    - On solid rock
  - Several exceptions to frost protection
    - Risk category I
      - 1604.5
    - 600 ft² light frame
    - 400 ft² other frame
    - 10’ height

Shallow Foundations

- **1809.6 Location of footings**
  - (Isolated Footings)
  - 30° slope maintained
    - Controls lateral soil pressure
  - Or other means
    - Engineered solution
Shallow Foundations

• 1809 Prescriptive Footings
  – 1809.7 covers Light frame (Wood or steel stud)
    • Table 1809.7
  – 1809.8 covers other than light frame
    • 8” minimum thickness
      – Reduction for R-3

Shallow Foundations

• 1809.8 Plain concrete footings
  – Shear strength is a concern
    • Thickness regulated by Table 1809.7
  – Modified by 1809.8 for heavier structures
    • 8” with specific exception
Shallow Foundations

- 1809.9 Masonry unit footings
  - 1809.9.1 Maximum overall projection limitation
  - 1809.9.2 Maximum projection per course

Shallow Foundations

- 1809.10 Pier & Curtain Wall Foundations
  - Permitted for 2 story light frame
    - Continuous footing required
    - Single brick min. thickness
    - 6’ maximum pier intervals
    - Pier height / area ratio of 10:1
    - Hollow piers restricted
      - Some exceptions
    - Maximum height restrictions
    - Unbalanced fill restriction
Shallow Foundations

- 1809.10 Timber Footings
  - Type V Structures
  - PT Treated timbers
    - Exception for below water level
  - .7 compressive resistance reduction
    - Timbers on piles

Shallow Foundations

- 1809.11 Steel Grillage Foundations
  - Spacers required
  - Completely filled with grout or concrete
Shallow Foundations

• 1809.12 Timber Foundations
  – OK for type V construction
  – OK if [A]104.11

Deep Foundations

• Section 1810 Deep Foundations
  – 1810.1.1 Geotechnical investigation
    • Required for all deep foundations
  – 1810.1.2 Use of Existing
    • Based on testing
  – 1801.3 Deep Foundation Elements classified as columns
    • Non-braced or poorly supported – treat as a column
  – 1810.1.4 Special types of Deep Foundations (other)
    • Engineered components
    • With BO approval
    • Approval based on:
      – [A] 104.11
      – [A] 104.11.1
      – [A] 104.11.2
Deep Foundations

- 1810.2 Analysis of deep Foundations
  - 1810.2.1 Lateral support of elements
    • Are unsupported portions of piles properly sized per 1810.1.3?
  - 1810.2.2 Stability
    • Are components structurally connected?
    • Will components move as a group?
  - 1810.2.3 Settlement
    • Was settlement considered in design?
    • Will settlement damage superstructure?
  - 1810.2.4 Lateral load
    • Will design handle anticipated Seismic earth shear?
    • Was soil diversity considered in lateral load resistance?
  - 1810.2.5 Grouping effect
    • Will closely grouped elements impact lateral behavior?

Deep Foundations

- 1810.3 Design & detailing
  - 1810.3.1.4 Driven Piles
    • Designed for driving
Deep Foundations

• 1810.3 Design & detailing
  – 1810.3.1.5 Helical Piles
    • Must resist installation & service loads

Deep Foundations

• 1810.3 Design & detailing
  – 1810.3.1.6 Casings
    • May be temporary or permanent
    • Generally used as forms for concrete
Deep Foundations

• 1810.3 Design & detailing
  Materials
  – 1810.3.2.1 Concrete
  – 1810.3.2.3 Structural steel & pipe
  – 1810.3.2.4 Timber
  – 1810.3.1.2 Composites

Deep Foundations

• 1810.3 Design & detailing
  – 1810.3.1.3 Mislocation
  Determination of AllowableLoads
  – 1810.3.1.1 Driving criteria
  – 1810.3.1.4 Frictional resistance
  – 1810.3.1.5 Uplift resistance
  – 1810.3.1.7 Load bearing capacity
  – 1810.3.1.8 Bent foundation element adjustment
  – 1810.3.2 Lateral load resistance
Deep Foundations

- 1810.4.10 Micro piles
  - 1810.3.5.2.3 12” diameter or less

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Energy Code Related Footing and Foundation Inspection

<table>
<thead>
<tr>
<th>Item Number</th>
<th>Footing / Foundation Inspection</th>
<th>Verified Value</th>
<th>Complies</th>
<th>Comments/Notes/Findings</th>
</tr>
</thead>
</table>
| FO1 [B.8.1.7]
| FO2 [B.8.1.7.31] | Exterior insulation protected against damage, sunlight, moisture, wind, landscaping and equipment maintenance activities. | ☐ ☐ ☐ |          |                         |
| FO3 [B.3.2, 6.4.4.1, 6.4.4.21] | Piping, ducts and plenum are insulated and sealed when installed in or under a slab. | ☐ ☐ ☐ |          |                         |
| FO4 [B.5.8.2, 7.4.3] | Any SIYH piping in or under slab is insulated. | ☐ ☐ ☐ |          |                         |
| FO5 [B.5.3.3, 5.8.1.2] | Below-grade wall insulation R-value. Installed per manufacturer’s instructions. | ☐ ☐ ☐ |          |                         |
| FO6 [B.5.3.5, 5.8.1.2] | Slab edge insulation R-value, depth/length, installed per manufacturer’s instructions. | ☐ ☐ ☐ |          |                         |
| FO7 [B.4.3.8] | Freeze protection and snow/ice melting system sensors for future connection to controls. | ☐ ☐ ☐ |          |                         |

From the BUILDING ENERGY CODES UNIVERSITY
Energy Code Related Footing and Foundation Inspection

Below-grade wall insulation R-value. Installed per manufacturer’s instructions.
Energy Code Related Footing and Foundation Inspection

Pressure Treated Mudsill
Flash Protection
Board or Coating
Install Required R-Value

Sill Seal and Capillary Break
Seal all Cracks, Penetrations and Joints with Approved Sealer

Concrete Slab

From the BUILDING ENERGY CODES UNIVERSITY

Energy Code Related Footing and Foundation Inspection

Piping, ducts and plenum are insulated and sealed when installed in or under a slab.

From the BUILDING ENERGY CODES UNIVERSITY
Conclusion and Questions

- Seminar reviewed requirements for soils and foundations under the IBC:
  - foundation and soils investigations,
  - excavation, grading and fill,
  - load bearing values of soils,
  - Damp proofing and waterproofing
  - design and construction of foundations
  - Shallow Foundations
  - Deep Foundations
Thank-You!

State of Connecticut
Division of Construction Services

• Office of the State Building Inspector
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  – (860) 713 - 5750
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  – (860) 713- 5522

Photographs and Discussions

• Following slides are photographs of construction processes related to this seminar on soils and foundations
Examples of Footings

Tremie

Example of a Tremie Method of Placing Concrete Underwater

Superchute Tremie & Funnel Support (shown over a manhole)

3 Foot Tremie Section
Soils and Foundations

January 2016
Construction Pile Driving
See: IBC Section 1809 – Driven Pile Foundations
Example of Pre-Concrete Piles Being Driven

Cast-In-Place Concrete Pile Foundations
Pier Foundation Construction
Figure 10  Pier or Foundation Wall Options by Footing Type

Note: See additional footing detail for Permanent Wood Foundation (PWF) option, Figure 16.