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T-Mobile Northeast LLC, a subsidiary of T-Mobile USA, Inc.

Connecticut Market

July 11, 2019

Honorable Robert Stein, Chairman,  
and members of the Council  
Connecticut Siting Council  
10 Franklin Square  
New Britain, CT 06051

Re: T-MOBILE Northeast LLC notice of intent to install a temporary cellular telephone facility located at 281 Route 169 South Woodstock, Connecticut

Dear Chairman Stein and Members of the Council:

TRM is pleased to submit this Notice of Exempt Modification on behalf of T-MOBILE Northeast LLC

T-MOBILE Northeast LLC hereby notifies the Connecticut Siting Council of its intent for the temporary use of telecommunications equipment by placing a Cell On Wheels (COW) on the grounds of the Woodstock Fair located at 281 Route 169 South Woodstock, Connecticut. Please accept this Notice to the Connecticut Siting Council, Pursuant to RSCA Section 16-50j-73, of construction that constitutes an exempt modification under RSCA Section 16-50j-72 (d). In compliance with RSCA Section 16-50j-73, copies of this Notice of Exempt Modification are being sent to the First Selectman and Zoning and Enforcement officer of Woodstock and the Woodstock Fair.

The proposed temporary cell site meets the criteria set forth in RSCA 16-50j-72(d) for temporary cellular service for events of statewide significance. The site is necessary to provide additional system capacity to accommodate the increased communication needs during the Woodstock Fair.

The Woodstock Fair is August 30 to September 2 but T-Mobile will need to do testing beforehand to make sure the site is up and running before the Fair.

### **Proposed Temporary Facility**

The temporary site will be located at 281 Route 169 South Woodstock, Connecticut on the property known as the Woodstock Fair owned by the Woodstock Agricultural Society. (See attached location map) Coordinates for the location are N 41 56 15.44, W -71 57 15.26. A 15 kw diesel generator will be used for power and the proposed temporary cell site will not increase the noise level by six decibels or more. There will be three APX16DWV-16DWVS-E-A20 antennas with a 60 ft rad center.

Equipment installation will start on August 23, 2019 and the site will be on-air until September 3, 2019. The COLT will be removed on September 3, 2019, the morning after the Fair.

T-Mobile's temporary cell site will consist of a "Cell On Wheels" ("COW") (See attached photo) which needs a 30' x 30' footprint and will be located behind a 6' fence.

### **Power Density Calculations**

T-Mobile's temporary cell site will not result in a total radio frequency electromagnetic radiation power density, measured at ground level at the COW location, at or above State or Federal standards. The following table shows the power density at the site from the proposed temporary cellular transmissions from the COW:

T-Mobile Sector	Power Density Value (%)
Sector A:	8.79 %
Sector B:	8.79 %
Sector C:	8.79 %
T-Mobile Per Sector Maximum:	8.79 %
Site Total:	8.79 %
Site Compliance Status:	<b>COMPLIANT</b>

See attached full report

### **Conclusion**

For the reasons above, we respectfully request the Council acknowledge T-Mobile's Notice of Exempt Modification for the temporary cell site to be operated during the Woodstock Fair game pursuant to RCSA Section 16-50j-72(d).

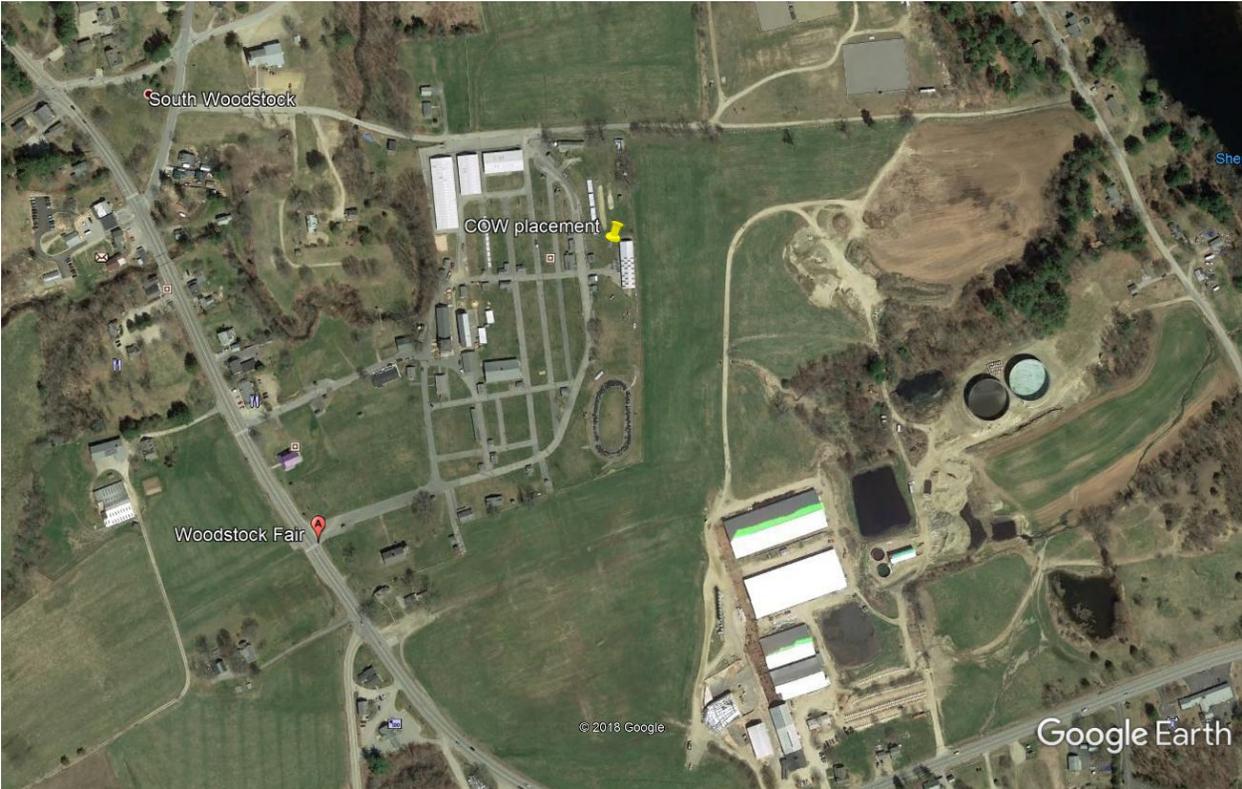
Please call me with any questions concerning this Notice at 203-417-4446. Thank you.

Respectfully,

Thomas White  
Agent of T-Mobile

Cc: Woodstock First Selectman  
Michael D'Amato Zoning and Enforcement Officer  
Woodstock Fair

**COW LOCATION**



**PHOTO OF COW**



May 21, 2019

**Re:** This Letter Agreement ("**Agreement**") by and between the Woodstock Agricultural Society Inc. ("**Landlord**"), and T-Mobile Northeast LLC, as successor-in interest to Omnipoint Communications, Inc. ("**Tenant**") (collectively, the "**Parties**") regarding the Installation of Temporary Cell-On-Wheels ("**COW**"), which is non-transferable

**Site Number:** CTCOW01A

**Site Address:** 281 Route 169 South, Woodstock, CT 06281 ("**Property**")

To Whom It May Concern,

This Letter Agreement between the Parties is to document that Landlord hereby grants Tenant the right to install a temporary COW on the Property for the period of August 23, 2019 to September 5, 2019 (the "**COW Term**"). The site will be 30' x 30'. Tenant shall coordinate the temporary installation of the COW and utilities (as necessary) on the Property (as further described on Exhibit A, attached hereto), at Tenants' sole cost and expense. Tenant will have 24-hour-a-day, 7-days-a-week access to the Property and the COW during the COW Term.

Tenant shall, at its sole cost and expense, repair and restore any damage to the Property and remove any improvements thereon caused by the installation, operation and/or removal of the COW (reasonable wear and tear excepted) by or within five (5) days of the expiration of the COW Term or earlier termination of this Letter Agreement. In return the Tenant agrees to pay Landlord a one-time fee of One Thousand Five Hundred and 00/100 Dollars (\$1,500.00) within fifteen (15) days of full execution of this Letter Agreement.

Tenant will provide Landlord, within 30 days prior to installation, a Certificate of Liability Insurance naming the Woodstock Agricultural Society, Inc. as additional insured, in the minimum amount of One Million Dollars (\$1,000,000.00) coverage single limit.

Tenant agrees to indemnify and hold harmless Landlord, including Landlord's employees, agents, directors, and officers from claims arising from the installation, use, maintenance, repair or removal of the COW, excepting claims arising from the negligence or intentional acts made by the Landlord, Landlord's employees, agents directors, or officers.

This Agreement contains the complete agreement between the Parties and cannot be varied except by the written agreement of the Parties. The Parties agree that there are no oral agreements, understandings, representations or warranties that are not expressly set forth herein. This Agreement may be executed in counterparts and signatures exchanged by facsimile, PDF or other electronic means are effective for all purposes to the same extent as original signatures.

Please signify your approval by signing and dating one (1) original of this Agreement in the space provided below. Kindly return the Agreement to Thomas White via email to [twhite@clinellc.com](mailto:twhite@clinellc.com).

Should you have any questions, please contact Thomas White at 203-417-4446. Thank you in advance for your cooperation in this matter.

Very truly yours,

Thomas White  
Agent for T-Mobile

**Landlord Acknowledged, Accepted and Agreed:**

By: *Pamela Kelly / Susan B. L.*



**COLT LOCATION MAP**

SCALE: 1" = 400'

**NOTES:**

1. ALL EQUIPMENT LOCATIONS ARE APPROXIMATE AND ARE SUBJECT TO APPROVAL BY T-MOBILE NORTHEAST, LLC STRUCTURAL & RF ENGINEERS. LOCATIONS OF POWER & TELEPHONE FACILITIES ARE SUBJECT TO APPROVAL BY UTILITY COMPANIES.

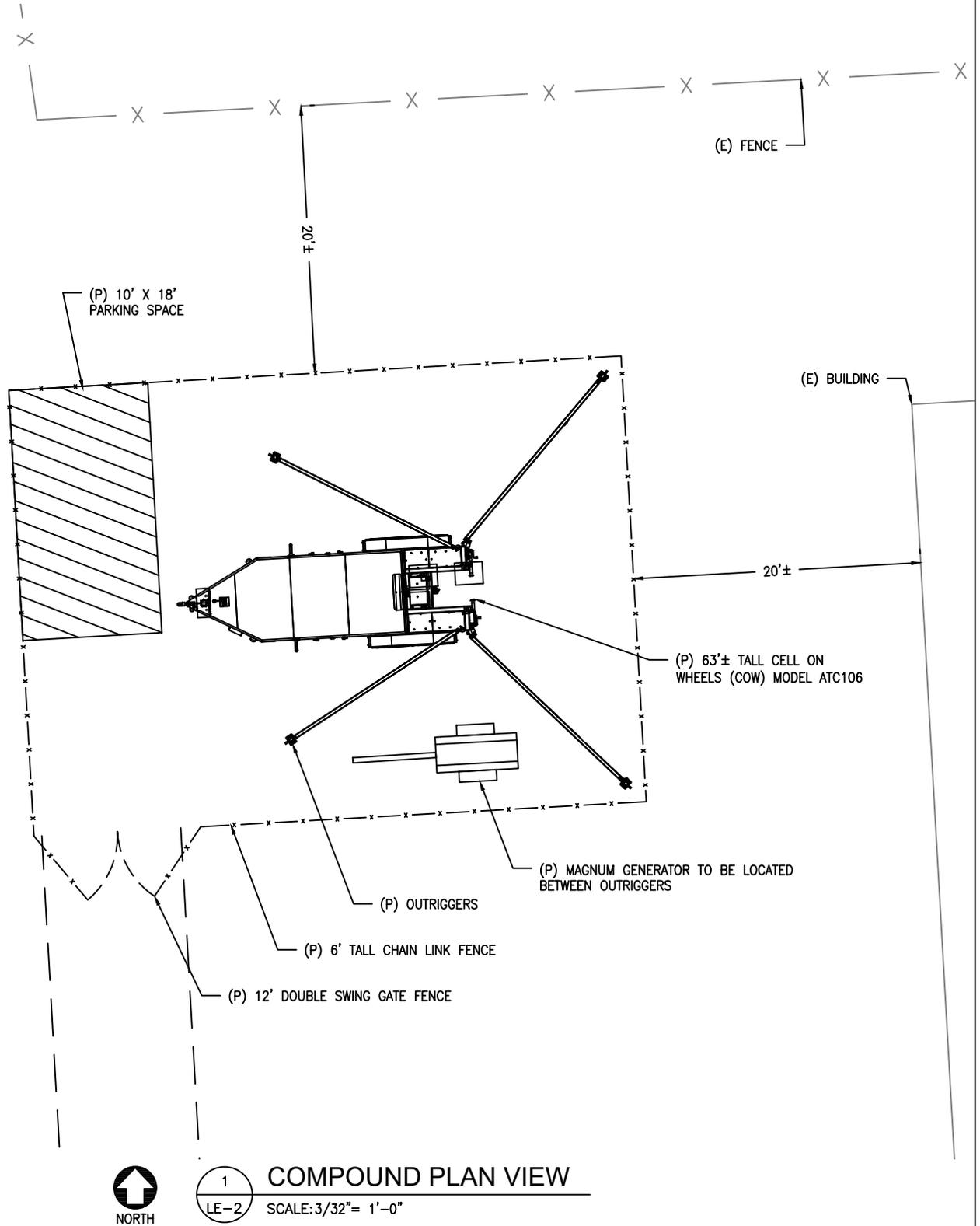
**EG ADVANCED**  
**ENGINEERING GROUP, P.C.**  
 Civil Engineering - Site Development - Surveying - Telecommunications  
 500 NORTH BROADWAY  
 EAST PROVIDENCE, RI 02914  
 TEL: (401) 354-2403  
 FAX: (401) 633-6354

T-MOBILE NORTHEAST LLC  
 35 GRIFFIN ROAD SOUTH  
 BLOOMFIELD CT 06002

TITLE: LEASE EXHIBIT  
 SITE NO: CTCOW01A  
 SITE NAME: WOODSTOCK FAIR COW  
 ADDRESS: 281 ROUTE 169  
 WOODSTOCK, CT 06281

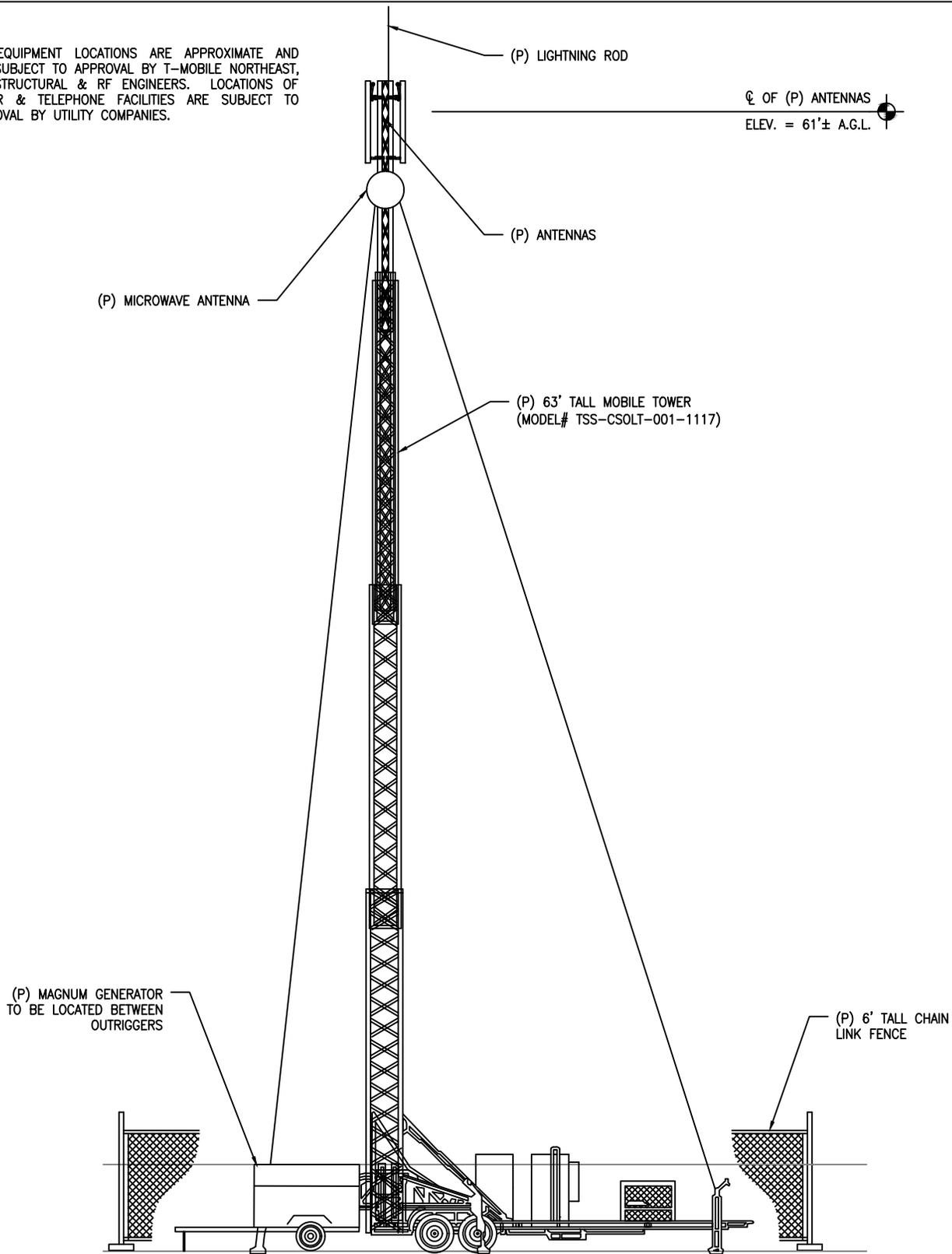
DATE: 07/11/2019  
 DRAWN BY: JWH  
 REVISION: 2  
 SCALE: NOTED  
 SHEET: LE-1

**NOTE:**  
 ALL EQUIPMENT LOCATIONS ARE APPROXIMATE AND  
 ARE SUBJECT TO APPROVAL BY T-MOBILE NORTHEAST,  
 LLC STRUCTURAL & RF ENGINEERS.



**NOTE:**

ALL EQUIPMENT LOCATIONS ARE APPROXIMATE AND ARE SUBJECT TO APPROVAL BY T-MOBILE NORTHEAST, LLC STRUCTURAL & RF ENGINEERS. LOCATIONS OF POWER & TELEPHONE FACILITIES ARE SUBJECT TO APPROVAL BY UTILITY COMPANIES.



1 ELEVATION  
LE-3 SCALE: 1/8"=1'-0"

# 281 RT 169

**Location** 281 RT 169

**Mblu** 5779/ 63/ 08A/ /

**Acct#** W0435600

**Owner** WOODSTOCK AGRICULTURAL SOCIETY

**Assessment** \$491,400

**Appraisal** \$701,900

**PID** 4503

**Building Count** 1

## Current Value

Appraisal			
Valuation Year	Improvements	Land	Total
2016	\$499,200	\$202,700	\$701,900

Assessment			
Valuation Year	Improvements	Land	Total
2016	\$349,500	\$141,900	\$491,400

## Owner of Record

**Owner** WOODSTOCK AGRICULTURAL SOCIETY  
**Co-Owner**  
**Address** PO BOX 1  
SO WOODSTOCK , CT 06267

**Sale Price** \$0  
**Certificate** 1  
**Book & Page** 62/ 343  
**Sale Date**

## Ownership History

Ownership History				
Owner	Sale Price	Certificate	Book & Page	Sale Date
WOODSTOCK AGRICULTURAL SOCIETY	\$0	1	62/ 343	

## Building Information

### Building 1 : Section 1

**Year Built:**  
**Living Area:** 0  
**Replacement Cost:** \$0  
**Building Percent**  
**Good:**  
**Replacement Cost**  
**Less Depreciation:** \$0

Building Attributes	
Field	Description

Style	Commercial
Model	
Grade:	
Stories:	
Living Units	
Exterior Wall 1	
Exterior Wall 2	
Roof Structure:	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Total Bthrms:	
Total Half Baths:	
Total Xtra Fixtrs:	
Total Rooms:	
Bath Style:	
Kitchen Style:	
Whirlpool Tubs	
Bsmt. Garages	

### Building 1 : Section 1

**Year Built:**

**Living Area:** 0

**Replacement Cost:** \$0

**Building Percent**

**Good:**

**Replacement Cost**

**Less Depreciation:** \$0

Building Attributes	
Field	Description
Style	Outbuildings
Model	
Grade:	
Stories:	
Living Units	
Exterior Wall 1	

### Building Photo



(<http://images.vgsi.com/photos/WoodstockCTPhotos//\00\00\29>)

### Building Layout

Building Layout

Building Sub-Areas (sq ft)	Legend
No Data for Building Sub-Areas	

### Building Photo



(<http://images.vgsi.com/photos/WoodstockCTPhotos//default.jpg>)

Exterior Wall 2	
Roof Structure:	
Roof Cover	
Interior Wall 1	
Interior Wall 2	
Interior Flr 1	
Interior Flr 2	
Heat Fuel	
Heat Type:	
AC Type:	
Total Bedrooms:	
Total Bthrms:	
Total Half Baths:	
Total Xtra Fixtrs:	
Total Rooms:	
Bath Style:	
Kitchen Style:	
Whirlpool Tubs	
Bsmt. Garages	

## Building Layout

 Building Layout

Building Sub-Areas (sq ft)	Legend
No Data for Building Sub-Areas	

## Extra Features

Extra Features	Legend
No Data for Extra Features	

## Land

### Land Use

**Use Code** 980  
**Description** Non-Profit Lnd  
**Zone**  
**Neighborhood** 400  
**Alt Land Appr** No  
**Category**

### Land Line Valuation

**Size (Acres)** 5.12  
**Frontage**  
**Depth**  
**Assessed Value** \$141,900  
**Appraised Value** \$202,700

## Outbuildings

Outbuildings						Legend
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
FN1	Fence 4'			2500 L.F.	\$16,600	1
FN3	Fence 6'			7400 L.F.	\$67,700	1
PAV1	Paving Asph.			220000 S.F.	\$319,000	1

LT1	Light 1			19 UNITS	\$18,100	1
LT2	Light 2			19 UNITS	\$52,200	1
LT3	Light 3			4 UNITS	\$11,900	1
LT4	Light 4			2 UNITS	\$6,400	1
SHD1	Shed	FR	Frame	20 S.F.	\$100	1
SHD1	Shed	FR	Frame	470 S.F.	\$2,900	1
SHD1	Shed	FR	Frame	80 S.F.	\$800	1
PMPR	Pump Hse Res	FR	Frame	140 S.F.	\$3,500	1

### Valuation History

Appraisal			
Valuation Year	Improvements	Land	Total
2016	\$499,200	\$202,700	\$701,900
2015	\$98,800	\$202,700	\$301,500
2014	\$98,800	\$202,700	\$301,500

Assessment			
Valuation Year	Improvements	Land	Total
2016	\$349,500	\$141,900	\$491,400
2015	\$69,100	\$141,900	\$211,000
2014	\$69,100	\$141,900	\$211,000

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**Structural Analysis Report**

*120-ft Portable Tower (extended at 61-ft)*

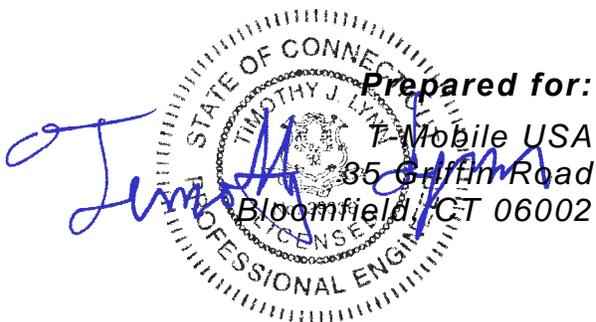
*Proposed Temporary  
Antenna Installation*

*Site Ref: CTCOW01A*

*281 Route 169 Woodstock, CT*

*Centek Project No. 18097.00*

*Date: June 20, 2018*



# **Table of Contents**

## **SECTION 1 - REPORT**

- INTRODUCTION
- ANTENNA AND APPURTENANCE SUMMARY
- PRIMARY ASSUMPTIONS USED IN THE ANALYSIS
- ANALYSIS
- TOWER LOADING
- TOWER CAPACITY
- CONCLUSION

## **SECTION 2 – CONDITIONS & SOFTWARE**

- STANDARD ENGINEERING CONDITIONS
- GENERAL DESCRIPTION OF STRUCTURAL ANALYSIS PROGRAM

## **SECTION 3 – CALCULATIONS**

- tnxTower INPUT/OUTPUT SUMMARY
- tnxTower DETAILED OUTPUT

## **SECTION 4 – REFERENCE MATERIAL**

- RF DATA SHEET
- ANTENNA CUT SHEETS

**CEN TEK** Engineering, Inc.  
Structural Analysis - 120-ft Portable Tower  
T-Mobile Temporary Antenna Installation  
Site Ref – CTCOW01A  
Woodstock, CT  
June 20, 2018

## Introduction

The purpose of this report is to summarize the results of the non-linear, P- $\Delta$  structural analysis of the temporary antenna/tower installation located in Woodstock, CT.

The host tower is a 120-ft tall, six section, telescoping lattice tower. Original design documents were unavailable for use in this report. The tower geometry was obtained from field verification conducted by Centek personnel on June 6, 2018.

Antenna and appurtenance information were obtained from an RF data sheet provided by T-Mobile.

The tower consists of six (6) telescoping vertical sections consisting of steel pipe legs and steel solid round diagonal and horizontal bracing. The width of the tower face is 0.8125-ft at the top and 2.5-ft at the base.

## Antenna and Appurtenance Summary

- **T-MOBILE (Proposed):**  
**Antennas: Three (3) RFS APX16DWV-16DWVS panel antennas leg mounted with a RAD center elevation of 60-ft above grade level.**  
**Coax Cables: Twelve (12) 1/2"  $\varnothing$  coax cables running on a leg/face of the tower as specified in Section 3 of this report.**

## Primary Assumptions Used in the Analysis

- The tower structure's theoretical capacity not including any assessment of the condition of the tower.
- The tower carries the horizontal and vertical loads due to the weight of antennas, ice load and wind.
- Tower is properly installed and maintained.
- Tower is in plumb condition.
- Tower loading for antennas and mounts as listed in this report.
- All bolts are appropriately tightened providing the necessary connection continuity.
- All welds are fabricated with ER-70S-6 electrodes.
- All members are assumed to be as specified in the original tower design documents.
- All members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards.
- All member protective coatings are in good condition.
- All tower members were properly designed, detailed, fabricated, installed and have been properly maintained since erection.
- Any deviation from the analyzed antenna loading will require a new analysis for verification of structural adequacy.

## A n a l y s i s

The existing tower was analyzed using a comprehensive computer program entitled tnxTower. The program analyzes the tower, considering the worst case loading condition. The tower is considered as loaded by concentric forces along the tower, and the model assumes that the tower members are subjected to bending, axial, and shear forces.

The existing tower was analyzed for the controlling basic wind speed (3-second gust) with no ice and the applicable wind and ice combination to determine stresses in members as per guidelines of TIA-222-G-2005 entitled “Structural Standard for Antenna Support Structures and Antennas”, the American Institute of Steel Construction (AISC) and the Manual of Steel Construction; Load and Resistance Factor Design (LRFD).

The controlling wind speed is determined by evaluating the local available wind speed data as provided in Appendix N of the CSBC<sup>1</sup> and the wind speed data available in the TIA-222-G-2005 Standard.

## T o w e r L o a d i n g

Tower loading was determined by the basic wind speed as applied to projected surface areas with modification factors per TIA-222-G-2005, gravity loads of the tower structure and its components, and the application of 0.75” radial ice on the tower structure and its components.

Basic Wind Speed:	Woodstock; $v = 101$ mph (3 second gust)	<i>[Appendix N of the 2016 CT Building Code]</i>
Load Cases:	<u>Load Case 1</u> ; 101 mph wind speed w/ no ice plus gravity load – used in calculation of tower stresses and rotation.	<i>[Appendix N of the 2016 CT Building Code]</i>
	<u>Load Case 2</u> ; 50 mph wind speed w/ 0.75” radial ice plus gravity load – used in calculation of tower stresses.	<i>[Annex B of TIA-222-G-2005]</i>

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<sup>1</sup> The 2012 International Building Code as amended by the 2016 Connecticut State Building Code (CSBC).

## Tower Capacity

- Calculated stresses **were found** to be within allowable limits. This tower was found to be at **87.6%** of its total capacity.

Tower Section	Elevation	Stress Ratio (percentage of capacity)	Result
Leg (T2)	23'-0"-43'-0"	63.6%	<b>PASS</b>
Diagonal (T3)	2'-0"-23'-0"	68.6%	<b>PASS</b>
Guy C (T1)	61'	87.6%	<b>PASS</b>

- Guy wires shall be 5/16" diameter EHS at 25' radius from the tower. Provide 8,000lbs of ballast block at each guy point.**

## Conclusion

This analysis shows that the subject tower **is adequate** to support the proposed antenna configuration.

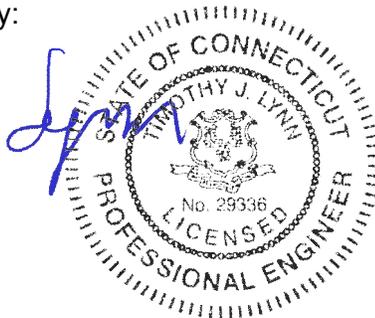
The analysis is based, in part, on the information provided to this office by T-Mobile. If the existing conditions are different than the information in this report, Centek Engineering, Inc. must be contacted for resolution of any potential issues.

Please feel free to call with any questions or comments.

Respectfully Submitted by:



Timothy J. Lynn, PE  
Structural Engineer



*CENTEK Engineering, Inc.*  
*Structural Analysis - 120-ft Portable Tower*  
*T-Mobile Temporary Antenna Installation*  
*Site Ref – CTCOW01A*  
*Woodstock, CT*  
*June 20, 2018*

*Standard Conditions for Furnishing of  
Professional Engineering Services on  
Existing Structures*

All engineering services are performed on the basis that the information used is current and correct. This information may consist of, but is not necessarily limited to:

- Information supplied by the client regarding the structure itself, its foundations, the soil conditions, the antenna and feed line loading on the structure and its components, or other relevant information.
- Information from the field and/or drawings in the possession of CENTEK engineering, Inc. or generated by field inspections or measurements of the structure.
- It is the responsibility of the client to ensure that the information provide to CENTEK engineering, Inc. and used in the performance of our engineering services is correct and complete. In the absence of information to the contrary, we assume that all structures were constructed in accordance with the drawings and specifications and are in an un-corroded condition and have not deteriorated. It is therefore assumed that its capacity has not significantly changed from the “as new” condition.
- All services will be performed to the codes specified by the client, and we do not imply to meet any other codes or requirements unless explicitly agreed in writing. If wind and ice loads or other relevant parameters are to be different from the minimum values recommended by the codes, the client shall specify the exact requirement. In the absence of information to the contrary, all work will be performed in accordance with the latest revision of ANSI/ASCE10 & ANSI/EIA-222
- All services performed, results obtained, and recommendations made are in accordance with generally accepted engineering principles and practices. CENTEK engineering, Inc. is not responsible for the conclusions, opinions and recommendations made by others based on the information we supply.

*CENTEK Engineering, Inc.*  
*Structural Analysis - 120-ft Portable Tower*  
*T-Mobile Temporary Antenna Installation*  
*Site Ref – CTCOW01A*  
*Woodstock, CT*  
*June 20, 2018*

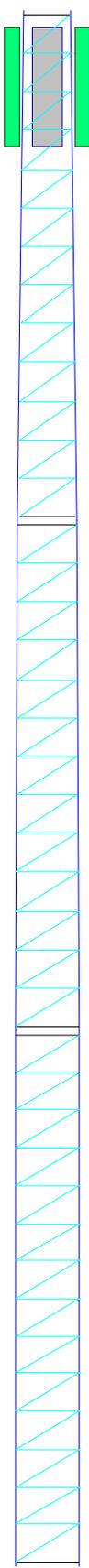
## *General Description of Structural Analysis Program*

tnxTower, is an integrated structural analysis and design software package for Designed specifically for the telecommunications industry, tnxTower, formerly ERITower, automates much of the tower analysis and design required by the TIA/EIA 222 Standard.

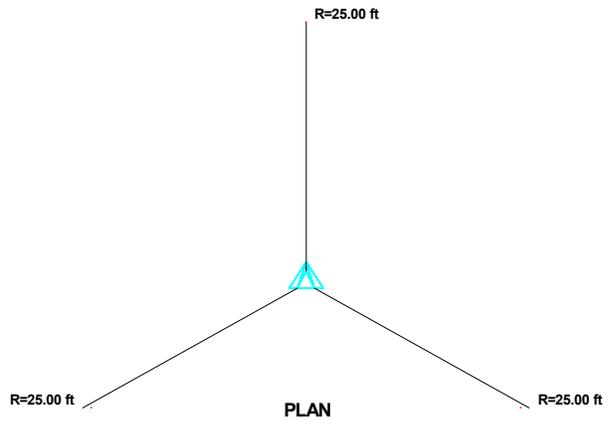
### tnxTower Features:

- tnxTower can analyze and design 3- and 4-sided guyed towers, 3- and 4-sided self-supporting towers and either round or tapered ground mounted poles with or without guys.
- The program analyzes towers using the TIA-222-G (2005) standard or any of the previous TIA/EIA standards back to RS-222 (1959). Steel design is checked using the AISC ASD 9th Edition or the AISC LRFD specifications.
- Linear and non-linear (P-delta) analyses can be used in determining displacements and forces in the structure. Wind pressures and forces are automatically calculated.
- Extensive graphics plots include material take-off, shear-moment, leg compression, displacement, twist, feed line, guy anchor and stress plots.
- tnxTower contains unique features such as True Cable behavior, hog rod take-up, foundation stiffness and much more.

Section	T1	T2	T3
Legs	P1.25x.191	P1.5x.145	P2x.218
Leg Grade		A53-B-35	
Diagonals		SR 9/16	
Diagonal Grade		A36	
Top Girts	4x3/8		C8x11.5
Bottom Girts	4x3/8		C8x11.5
Horizontalis		SR 9/16	
Face Width (ft)	1.78	2.25	2.5
# Panels @ (ft)	26 @ 1.51282	0.4	14 @ 1.47619
Weight (K)	1.2	0.7	2.2



2.9 K (Vx B)  
 23.6 K (Vy B)  
 1.1 K (Vz B)  
 3.4 K (Vx C)  
 23.5 K (Vy C)  
 1.2 K (Vz C)



**DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION
APX16DWV-16DWVS-E-A20 (T-Mobile - Proposed)	60	APX16DWV-16DWVS-E-A20 (T-Mobile - Proposed)	60
APX16DWV-16DWVS-E-A20 (T-Mobile - Proposed)	60		

**MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
A53-B-35	35 ksi	63 ksi	A36	36 ksi	58 ksi

**TOWER DESIGN NOTES**

1. Tower designed for Exposure C to the TIA-222-G Standard.
2. Tower designed for a 101 mph basic wind in accordance with the TIA-222-G Standard.
3. Tower is also designed for a 50 mph basic wind with 0.75 in. ice. Ice is considered to increase in thickness with height.
4. Deflections are based upon a 60 mph wind.
5. Tower Structure Class II.
6. Topographic Category 1 with Crest Height of 0.00 ft
7. TOWER RATING: 87.6%

<b>Centek Engineering Inc.</b>		
63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587		
Job: <b>18097.00 - CTCOW01A</b>	Project: <b>281 Route 169 Woodstock, CT</b>	
Client: T-Mobile	Drawn by: T.JL	App'd:
Code: TIA-222-G	Date: 06/20/18	Scale: NTS
Path: J:\Jobs\1809700_W104_Structural\Backup Documentation\TrxTower\Woodstock Fairgrounds Cow.er		Dwg No. E-1

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 1 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

## Tower Input Data

The main tower is a 3x guyed tower with an overall height of 63.00 ft above the ground line.

The base of the tower is set at an elevation of 2.00 ft above the ground line.

The face width of the tower is 1.78 ft at the top and 2.50 ft at the base.

This tower is designed using the TIA-222-G standard.

The following design criteria apply:

Basic wind speed of 101 mph.

Structure Class II.

Exposure Category C.

Topographic Category 1.

Crest Height 0.00 ft.

Nominal ice thickness of 0.7500 in.

Ice thickness is considered to increase with height.

Ice density of 56 pcf.

A wind speed of 50 mph is used in combination with ice.

Temperature drop of 50 °F.

Deflections calculated using a wind speed of 60 mph.

Pressures are calculated at each section.

Safety factor used in guy design is 1.

Stress ratio used in tower member design is 1.

Special horizontal and vertical support at 10.50 on leg B,C.

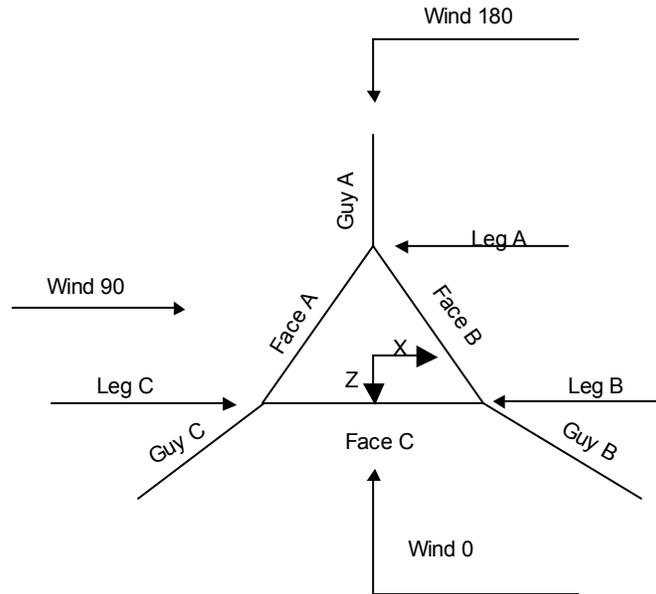
The support at the base was eliminated.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## Options

Consider Moments - Legs	Distribute Leg Loads As Uniform	Use ASCE 10 X-Brace Ly Rules
Consider Moments - Horizontals	Assume Legs Pinned	Calculate Redundant Bracing Forces
Consider Moments - Diagonals	√ Assume Rigid Index Plate	Ignore Redundant Members in FEA
Use Moment Magnification	√ Use Clear Spans For Wind Area	SR Leg Bolts Resist Compression
√ Use Code Stress Ratios	√ Use Clear Spans For KL/r	√ All Leg Panels Have Same Allowable
√ Use Code Safety Factors - Guys	√ Retension Guys To Initial Tension	Offset Girt At Foundation
Escalate Ice	Bypass Mast Stability Checks	√ Consider Feed Line Torque
Always Use Max Kz	Use Azimuth Dish Coefficients	Include Angle Block Shear Check
Use Special Wind Profile	√ Project Wind Area of Appurt.	Use TIA-222-G Bracing Resist. Exemption
√ Include Bolts In Member Capacity	√ Autocalc Torque Arm Areas	Use TIA-222-G Tension Splice Exemption
Leg Bolts Are At Top Of Section	Add IBC .6D+W Combination	Poles
Secondary Horizontal Braces Leg	Sort Capacity Reports By Component	Include Shear-Torsion Interaction
Use Diamond Inner Bracing (4 Sided)	Triangulate Diamond Inner Bracing	Always Use Sub-Critical Flow
SR Members Have Cut Ends	√ Treat Feed Line Bundles As Cylinder	Use Top Mounted Sockets
SR Members Are Concentric		

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 2 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL



**Corner & Starmount Guyed Tower**

**Tower Section Geometry**

Tower Section	Tower Elevation	Assembly Database	Description	Section Width	Number of Sections	Section Length
	ft			ft		ft
T1	63.00-43.00			1.78	1	20.00
T2	43.00-23.00			2.25	1	20.00
T3	23.00-2.00			2.50	1	21.00

**Tower Section Geometry (cont'd)**

Tower Section	Tower Elevation	Diagonal Spacing	Bracing Type	Has K Brace End Panels	Has Horizontals	Top Girt Offset	Bottom Girt Offset
	ft	ft				in	in
T1	63.00-43.00	1.51	Z Brace	No	Yes	2.0000	2.0000
T2	43.00-23.00	1.51	Z Brace	No	Yes	2.0000	2.0000
T3	23.00-2.00	1.48	Z Brace	No	Yes	2.0000	2.0000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 3 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> T.J.L.

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Leg Type	Leg Size	Leg Grade	Diagonal Type	Diagonal Size	Diagonal Grade
T1 63.00-43.00	Pipe	P1.25x.191	A53-B-35 (35 ksi)	Solid Round	9/16	A36 (36 ksi)
T2 43.00-23.00	Pipe	P1.5x.145	A53-B-35 (35 ksi)	Solid Round	9/16	A36 (36 ksi)
T3 23.00-2.00	Pipe	P2x.218	A53-B-35 (35 ksi)	Solid Round	9/16	A36 (36 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Top Girt Type	Top Girt Size	Top Girt Grade	Bottom Girt Type	Bottom Girt Size	Bottom Girt Grade
T1 63.00-43.00	Flat Bar	4x3/8	A36 (36 ksi)	Flat Bar	4x3/8	A36 (36 ksi)
T2 43.00-23.00	Flat Bar	4x3/8	A36 (36 ksi)	Flat Bar	4x3/8	A36 (36 ksi)
T3 23.00-2.00	Channel	C8x11.5	A36 (36 ksi)	Channel	C8x11.5	A36 (36 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	No. of Mid Girts	Mid Girt Type	Mid Girt Size	Mid Girt Grade	Horizontal Type	Horizontal Size	Horizontal Grade
T1 63.00-43.00	None	Flat Bar		A36 (36 ksi)	Solid Round	9/16	A36 (36 ksi)
T2 43.00-23.00	None	Flat Bar		A36 (36 ksi)	Solid Round	9/16	A36 (36 ksi)
T3 23.00-2.00	None	Flat Bar		A36 (36 ksi)	Solid Round	9/16	A36 (36 ksi)

**Tower Section Geometry (cont'd)**

Tower Elevation ft	Gusset Area (per face) ft <sup>2</sup>	Gusset Thickness in	Gusset Grade	Adjust. Factor A <sub>f</sub>	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals in	Double Angle Stitch Bolt Spacing Horizontals in	Double Angle Stitch Bolt Spacing Redundants in
T1 63.00-43.00	0.00	0.0000	A36 (36 ksi)	1	1	3	36.0000	36.0000	36.0000
T2 43.00-23.00	0.00	0.0000	A36 (36 ksi)	1	1	1	36.0000	36.0000	36.0000
T3 23.00-2.00	0.00	0.0000	A36	1	1	1	36.0000	36.0000	36.0000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	4 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor $A_r$	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	ft <sup>2</sup>	in					in	in	in
(36 ksi)									

### Tower Section Geometry (cont'd)

Tower Elevation	Calc K Single Angles	Calc K Solid Rounds	Legs	K Factors <sup>1</sup>							
				X Brace Diags	K Brace Diags	Single Diags	Girts	Horiz.	Sec. Horiz.	Inner Brace	
				X Y	X Y	X Y	X Y	X Y	X Y	X Y	
T1 63.00-43.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T2 43.00-23.00	Yes	Yes	1	1	1	1	1	1	1	1	1
T3 23.00-2.00	Yes	Yes	1	1	1	1	1	1	1	1	1

<sup>1</sup>Note: K factors are applied to member segment lengths. K-braces without inner supporting members will have the K factor in the out-of-plane direction applied to the overall length.

### Tower Section Geometry (cont'd)

Tower Elevation ft	Leg		Diagonal		Top Girt		Bottom Girt		Mid Girt		Long Horizontal		Short Horizontal	
	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U	Net Width Deduct in	U
T1 63.00-43.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T2 43.00-23.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75
T3 23.00-2.00	0.0000	1	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75	0.0000	0.75

### Guy Data

Guy Elevation	Guy Grade	Guy Size	Initial Tension	%	Guy Modulus	Guy Weight	$L_u$	Anchor Radius	Anchor Azimuth Adj. °	Anchor Elevation	End Fitting Efficiency %	
ft			K		ksi	plf	ft	ft		ft		
61.3205	EHS	A	5/16	1.12	10%	21000	0.205	65.77	25.00	0.0000	0.00	100%
		B	5/16	1.12	10%	21000	0.205	65.77	25.00	0.0000	0.00	100%
		C	5/16	1.12	10%	21000	0.205	65.77	25.00	0.0000	0.00	100%

### Guy Data(cont'd)

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 5 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Guy Elevation ft	Mount Type	Torque-Arm Spread ft	Torque-Arm Leg Angle °	Torque-Arm Style	Torque-Arm Grade	Torque-Arm Type	Torque-Arm Size
61.3205	Corner						

### Guy Data (cont'd)

Guy Elevation ft	Diagonal Grade	Diagonal Type	Upper Diagonal Size	Lower Diagonal Size	Is Strap.	Pull-Off Grade	Pull-Off Type	Pull-Off Size
61.32	A572-50 (50 ksi)	Solid Round				A572-50 (50 ksi)	Solid Round	

### Guy Data (cont'd)

Guy Elevation ft	Cable Weight A K	Cable Weight B K	Cable Weight C K	Cable Weight D K	Tower Intercept A ft	Tower Intercept B ft	Tower Intercept C ft	Tower Intercept D ft
61.3205	0.01	0.01	0.01		0.39	0.39	0.39	
					1.1 sec/pulse	1.1 sec/pulse	1.1 sec/pulse	

### Guy Data (cont'd)

Guy Elevation ft	Calc K Single Angles	Calc K Solid Rounds	Torque Arm		Pull Off		Diagonal	
			K <sub>x</sub>	K <sub>y</sub>	K <sub>x</sub>	K <sub>y</sub>	K <sub>x</sub>	K <sub>y</sub>
61.3205	No	No			1	1	1	1

### Guy Data (cont'd)

Guy Elevation ft	Torque-Arm				Pull Off				Diagonal			
	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U	Bolt Size in	Number	Net Width Deduct in	U
61.3205	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75	0.6250 A325N	0	0.0000	0.75

### Guy Pressures

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	6 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Guy Elevation ft	Guy Location	z ft	q <sub>z</sub> psf	q <sub>z</sub> Ice psf	Ice Thickness in
61.3205	A	30.66	22	5	1.4890
	B	30.66	22	5	1.4890
	C	30.66	22	5	1.4890

### Guy-Tensioning Information

Temperature At Time Of Tensioning																	
Guy Elevation ft	H ft	V ft	0 F		20 F		40 F		60 F		80 F		100 F		120 F		
			Initial Tension K	Intercept ft													
61.3205	A	23.95	61.32	1.184	0.37	1.163	0.38	1.141	0.39	1.120	0.39	1.099	0.40	1.077	0.41	1.056	0.42
	B	23.95	61.32	1.184	0.37	1.163	0.38	1.141	0.39	1.120	0.39	1.099	0.40	1.077	0.41	1.056	0.42
	C	23.95	61.32	1.184	0.37	1.163	0.38	1.141	0.39	1.120	0.39	1.099	0.40	1.077	0.41	1.056	0.42

### Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description	Face or Leg	Allow Shield	Component Type	Placement ft	Total Number	Number Per Row	Clear Spacing in	Width or Diameter in	Perimeter in	Weight plf
1/2	A	Yes	Ar (CaAa)	57.00 - 10.00	12	6	0.5800	0.5800		0.25

### Feed Line/Linear Appurtenances Section Areas

Tower Section	Tower Elevation ft	Face	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	63.00-43.00	A	0.000	0.000	9.744	0.000	0.04
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
T2	43.00-23.00	A	0.000	0.000	13.920	0.000	0.06
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00
T3	23.00-2.00	A	0.000	0.000	9.048	0.000	0.04
		B	0.000	0.000	0.000	0.000	0.00
		C	0.000	0.000	0.000	0.000	0.00

### Feed Line/Linear Appurtenances Section Areas - With Ice

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	A <sub>R</sub> ft <sup>2</sup>	A <sub>F</sub> ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> In Face ft <sup>2</sup>	C <sub>A</sub> A <sub>A</sub> Out Face ft <sup>2</sup>	Weight K
T1	63.00-43.00	A	1.573	0.000	0.000	18.734	0.000	0.26
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
T2	43.00-23.00	A	1.500	0.000	0.000	26.274	0.000	0.36
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00
T3	23.00-2.00	A	1.361	0.000	0.000	16.473	0.000	0.22

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 7 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Tower Section	Tower Elevation ft	Face or Leg	Ice Thickness in	$A_R$ $ft^2$	$A_F$ $ft^2$	$C_{AA}$ In Face $ft^2$	$C_{AA}$ Out Face $ft^2$	Weight K
		B		0.000	0.000	0.000	0.000	0.00
		C		0.000	0.000	0.000	0.000	0.00

### Feed Line Shielding

Section	Elevation ft	Face	$A_R$ $ft^2$	$A_R$ Ice $ft^2$	$A_F$ $ft^2$	$A_F$ Ice $ft^2$
T1	63.00-43.00	A	0.269	5.149	0.135	0.370
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T2	43.00-23.00	A	0.373	6.754	0.193	0.521
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000
T3	23.00-2.00	A	0.246	3.974	0.239	0.626
		B	0.000	0.000	0.000	0.000
		C	0.000	0.000	0.000	0.000

### Feed Line Center of Pressure

Section	Elevation ft	$CP_x$ in	$CP_z$ in	$CP_x$ Ice in	$CP_z$ Ice in
T1	63.00-43.00	-0.9862	-0.5694	-0.2042	-0.1179
T2	43.00-23.00	-1.2080	-0.6974	-0.2981	-0.1721
T3	23.00-2.00	-0.8909	-0.5144	-0.2344	-0.1353

### Shielding Factor Ka

Tower Section	Feed Line Record No.	Description	Feed Line Segment Elev.	$K_a$ No Ice	$K_a$ Ice
T1	1		1/2 43.00 - 57.00	1.0000	1.0000
T2	1		1/2 23.00 - 43.00	1.0000	1.0000
T3	1		1/2 10.00 - 23.00	1.0000	1.0000

### Discrete Tower Loads

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	8 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Description	Face or Leg	Offset Type	Offsets:		Azimuth Adjustment	Placement	C <sub>AA</sub> Front	C <sub>AA</sub> Side	Weight	
			Horz	Lateral						
			ft	ft	°	ft	ft <sup>2</sup>	ft <sup>2</sup>	K	
APX16DWV-16DWVS-E-A 20 (T-Mobile - Proposed)	A	From Leg	0.50	0.00	0.0000	60.00	No Ice 1/2" Ice 1" Ice	6.46 6.83 7.21	2.15 2.49 2.84	0.04 0.07 0.11
APX16DWV-16DWVS-E-A 20 (T-Mobile - Proposed)	B	From Leg	0.50	0.00	0.0000	60.00	No Ice 1/2" Ice 1" Ice	6.46 6.83 7.21	2.15 2.49 2.84	0.04 0.07 0.11
APX16DWV-16DWVS-E-A 20 (T-Mobile - Proposed)	C	From Leg	0.50	0.00	0.0000	60.00	No Ice 1/2" Ice 1" Ice	6.46 6.83 7.21	2.15 2.49 2.84	0.04 0.07 0.11

### Tower Pressures - No Ice

$$G_H = 0.850$$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face
ft	ft		psf	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 63.00-43.00	53.00	1.107	31	43.067	A	1.116	7.751	5.534	62.41	9.744	0.000
					B	1.251	8.020		59.69	0.000	0.000
					C	1.251	8.020		59.69	0.000	0.000
T2 43.00-23.00	33.00	1.002	22	50.667	A	1.284	8.809	6.333	62.75	13.920	0.000
					B	1.478	9.182		59.41	0.000	0.000
					C	1.478	9.182		59.41	0.000	0.000
T3 23.00-2.00	12.50	0.85	19	56.656	A	2.830	11.224	8.313	59.15	9.048	0.000
					B	3.069	11.470		57.17	0.000	0.000
					C	3.069	11.470		57.17	0.000	0.000

### Tower Pressure - With Ice

$$G_H = 0.850$$

Section Elevation	z	K <sub>Z</sub>	q <sub>z</sub>	t <sub>z</sub>	A <sub>G</sub>	F <sub>a</sub>	A <sub>F</sub>	A <sub>R</sub>	A <sub>leg</sub>	Leg %	C <sub>AA</sub> In Face	C <sub>AA</sub> Out Face
ft	ft		psf	in	ft <sup>2</sup>	c	ft <sup>2</sup>	ft <sup>2</sup>	ft <sup>2</sup>		ft <sup>2</sup>	ft <sup>2</sup>
T1 63.00-43.00	53.00	1.107	8	1.5728	48.310	A	0.881	28.245	16.020	55.00	18.734	0.000
						B	1.251	33.393		46.24	0.000	0.000
						C	1.251	33.393		46.24	0.000	0.000
T2 43.00-23.00	33.00	1.002	5	1.5000	55.667	A	0.957	28.729	16.334	55.02	26.274	0.000
						B	1.478	35.483		44.19	0.000	0.000
						C	1.478	35.483		44.19	0.000	0.000
T3 23.00-2.00	12.50	0.85	5	1.3612	61.421	A	2.443	33.350	17.841	49.84	16.473	0.000
						B	3.069	37.324		44.17	0.000	0.000
						C	3.069	37.324		44.17	0.000	0.000

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 9 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

**Tower Pressure - Service**

$G_H = 0.850$

Section Elevation ft	z ft	K <sub>Z</sub>	q <sub>z</sub> psf	A <sub>G</sub> ft <sup>2</sup>	F a c e	A <sub>F</sub> ft <sup>2</sup>	A <sub>R</sub> ft <sup>2</sup>	A <sub>leg</sub> ft <sup>2</sup>	Leg %	C <sub>AA</sub> In Face ft <sup>2</sup>	C <sub>AA</sub> Out Face ft <sup>2</sup>
T1 63.00-43.00	53.00	1.107	11	43.067	A	1.116	7.751	5.534	62.41	9.744	0.000
					B	1.251	8.020			0.000	0.000
					C	1.251	8.020			0.000	0.000
T2 43.00-23.00	33.00	1.002	8	50.667	A	1.284	8.809	6.333	62.75	13.920	0.000
					B	1.478	9.182			0.000	0.000
					C	1.478	9.182			0.000	0.000
T3 23.00-2.00	12.50	0.85	7	56.656	A	2.830	11.224	8.313	59.15	9.048	0.000
					B	3.069	11.470			0.000	0.000
					C	3.069	11.470			0.000	0.000

**Tower Forces - No Ice - Wind Normal To Face**

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 63.00-43.00	0.04	1.16	A	0.206	2.576	31	1	1	5.732	0.65	32.27	C
			B	0.215	2.546				5.881			
			C	0.215	2.546				5.881			
T2 43.00-23.00	0.06	0.40	A	0.199	2.599	22	1	1	6.558	0.59	29.56	C
			B	0.21	2.562				6.770			
			C	0.21	2.562				6.770			
T3 23.00-2.00	0.04	0.67	A	0.248	2.443	19	1	1	9.534	0.53	25.00	C
			B	0.257	2.418				9.798			
			C	0.257	2.418				9.798			
Sum Weight:	0.14	2.23								1.76		

**Tower Forces - No Ice - Wind 60 To Face**

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 63.00-43.00	0.04	1.16	A	0.206	2.576	31	0.8	1	5.508	0.63	31.44	C
			B	0.215	2.546				5.631			
			C	0.215	2.546				5.631			
T2 43.00-23.00	0.06	0.40	A	0.199	2.599	22	0.8	1	6.301	0.58	28.84	C
			B	0.21	2.562				6.475			
			C	0.21	2.562				6.475			
T3 23.00-2.00	0.04	0.67	A	0.248	2.443	19	0.8	1	8.968	0.50	23.87	C
			B	0.257	2.418				9.184			
			C	0.257	2.418				9.184			
Sum Weight:	0.14	2.23								1.71		

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 10 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

**Tower Forces - No Ice - Wind 90 To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K				psf			ft <sup>2</sup>	K	plf	
T1 63.00-43.00	0.04	1.16	A	0.206	2.576	31	0.85	1	5.564	0.63	31.65	C
			B	0.215	2.546		0.85	1	5.694			
			C	0.215	2.546		0.85	1	5.694			
T2 43.00-23.00	0.06	0.40	A	0.199	2.599	22	0.85	1	6.365	0.58	29.02	C
			B	0.21	2.562		0.85	1	6.548			
			C	0.21	2.562		0.85	1	6.548			
T3 23.00-2.00	0.04	0.67	A	0.248	2.443	19	0.85	1	9.110	0.51	24.15	C
			B	0.257	2.418		0.85	1	9.338			
			C	0.257	2.418		0.85	1	9.338			
Sum Weight:	0.14	2.23								1.72		

**Tower Forces - With Ice - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K				psf			ft <sup>2</sup>	K	plf	
T1 63.00-43.00	0.26	2.36	A	0.603	1.802	8	1	1	25.778	0.45	22.35	C
			B	0.717	1.778		1	1	28.746			
			C	0.717	1.778		1	1	28.746			
T2 43.00-23.00	0.36	1.66	A	0.533	1.861	5	1	1	25.919	0.36	18.19	C
			B	0.664	1.778		1	1	29.365			
			C	0.664	1.778		1	1	29.365			
T3 23.00-2.00	0.22	2.01	A	0.583	1.816	5	1	1	29.801	0.29	13.82	C
			B	0.658	1.78		1	1	32.243			
			C	0.658	1.78		1	1	32.243			
Sum Weight:	0.84	6.03								1.10		

**Tower Forces - With Ice - Wind 60 To Face**

Section Elevation	Add Weight	Self Weight	F a c e	e	C <sub>F</sub>	q <sub>z</sub>	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub>	F	w	Ctrl. Face
ft	K	K				psf			ft <sup>2</sup>	K	plf	
T1 63.00-43.00	0.26	2.36	A	0.603	1.802	8	0.8	1	25.602	0.44	22.21	C
			B	0.717	1.778		0.8	1	28.496			
			C	0.717	1.778		0.8	1	28.496			
T2 43.00-23.00	0.36	1.66	A	0.533	1.861	5	0.8	1	25.728	0.36	18.06	C
			B	0.664	1.778		0.8	1	29.069			
			C	0.664	1.778		0.8	1	29.069			
T3 23.00-2.00	0.22	2.01	A	0.583	1.816	5	0.8	1	29.312	0.29	13.62	C

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 11 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
			B	0.658	1.78		0.8	1	31.629			
			C	0.658	1.78		0.8	1	31.629			
Sum Weight:	0.84	6.03								1.09		

### Tower Forces - With Ice - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 63.00-43.00	0.26	2.36	A	0.603	1.802	8	0.85	1	25.646	0.44	22.24	C
			B	0.717	1.778		0.85	1	28.558			
			C	0.717	1.778		0.85	1	28.558			
T2 43.00-23.00	0.36	1.66	A	0.533	1.861	5	0.85	1	25.776	0.36	18.10	C
			B	0.664	1.778		0.85	1	29.143			
			C	0.664	1.778		0.85	1	29.143			
T3 23.00-2.00	0.22	2.01	A	0.583	1.816	5	0.85	1	29.434	0.29	13.67	C
			B	0.658	1.78		0.85	1	31.782			
			C	0.658	1.78		0.85	1	31.782			
Sum Weight:	0.84	6.03								1.09		

### Tower Forces - Service - Wind Normal To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 63.00-43.00	0.04	1.16	A	0.206	2.576	11	1	1	5.732	0.23	11.39	C
			B	0.215	2.546		1	1	5.881			
			C	0.215	2.546		1	1	5.881			
T2 43.00-23.00	0.06	0.40	A	0.199	2.599	8	1	1	6.558	0.21	10.43	C
			B	0.21	2.562		1	1	6.770			
			C	0.21	2.562		1	1	6.770			
T3 23.00-2.00	0.04	0.67	A	0.248	2.443	7	1	1	9.534	0.19	8.82	C
			B	0.257	2.418		1	1	9.798			
			C	0.257	2.418		1	1	9.798			
Sum Weight:	0.14	2.23								0.62		

### Tower Forces - Service - Wind 60 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
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<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 12 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 63.00-43.00	0.04	1.16	A	0.206	2.576	11	0.8	1	5.508	0.22	11.10	C
			B	0.215	2.546		0.8	1	5.631			
			C	0.215	2.546		0.8	1	5.631			
T2 43.00-23.00	0.06	0.40	A	0.199	2.599	8	0.8	1	6.301	0.20	10.18	C
			B	0.21	2.562		0.8	1	6.475			
			C	0.21	2.562		0.8	1	6.475			
T3 23.00-2.00	0.04	0.67	A	0.248	2.443	7	0.8	1	8.968	0.18	8.42	C
			B	0.257	2.418		0.8	1	9.184			
			C	0.257	2.418		0.8	1	9.184			
Sum Weight:	0.14	2.23								0.60		

### Tower Forces - Service - Wind 90 To Face

Section Elevation ft	Add Weight K	Self Weight K	F a c e	e	C <sub>F</sub>	q <sub>z</sub> psf	D <sub>F</sub>	D <sub>R</sub>	A <sub>E</sub> ft <sup>2</sup>	F K	w plf	Ctrl. Face
T1 63.00-43.00	0.04	1.16	A	0.206	2.576	11	0.85	1	5.564	0.22	11.17	C
			B	0.215	2.546		0.85	1	5.694			
			C	0.215	2.546		0.85	1	5.694			
T2 43.00-23.00	0.06	0.40	A	0.199	2.599	8	0.85	1	6.365	0.20	10.24	C
			B	0.21	2.562		0.85	1	6.548			
			C	0.21	2.562		0.85	1	6.548			
T3 23.00-2.00	0.04	0.67	A	0.248	2.443	7	0.85	1	9.110	0.18	8.52	C
			B	0.257	2.418		0.85	1	9.338			
			C	0.257	2.418		0.85	1	9.338			
Sum Weight:	0.14	2.23								0.61		

### Force Totals (Does not include forces on guys)

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Torques kip-ft
Leg Weight	1.02			
Bracing Weight	1.21			
Total Member Self-Weight	2.23			
Guy Weight	0.04			
Total Weight	2.54			
Wind 0 deg - No Ice		0.00	-2.11	-0.15
Wind 30 deg - No Ice		1.03	-1.78	-0.17
Wind 60 deg - No Ice		1.78	-1.03	-0.15
Wind 90 deg - No Ice		2.07	0.00	-0.09
Wind 120 deg - No Ice		1.81	1.05	0.00
Wind 150 deg - No Ice		1.03	1.79	0.09
Wind 180 deg - No Ice		0.00	2.04	0.15
Wind 210 deg - No Ice		-1.03	1.78	0.17
Wind 240 deg - No Ice		-1.83	1.05	0.15
Wind 270 deg - No Ice		-2.07	0.00	0.09

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 13 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJJ

Load Case	Vertical Forces K	Sum of Forces X K	Sum of Forces Z K	Sum of Torques kip-ft
Wind 300 deg - No Ice		-1.78	-1.03	0.00
Wind 330 deg - No Ice		-1.03	-1.79	-0.09
Member Ice	3.79			
Guy Ice	0.65			
Total Weight Ice	8.05			
Wind 0 deg - Ice		0.00	-1.21	-0.02
Wind 30 deg - Ice		0.57	-0.99	-0.02
Wind 60 deg - Ice		1.04	-0.60	-0.02
Wind 90 deg - Ice		1.20	0.00	-0.01
Wind 120 deg - Ice		0.99	0.57	0.00
Wind 150 deg - Ice		0.60	1.04	0.01
Wind 180 deg - Ice		0.00	1.14	0.02
Wind 210 deg - Ice		-0.57	0.99	0.02
Wind 240 deg - Ice		-1.05	0.60	0.02
Wind 270 deg - Ice		-1.20	0.00	0.01
Wind 300 deg - Ice		-1.04	-0.60	0.00
Wind 330 deg - Ice		-0.60	-1.04	-0.01
Total Weight	2.54			
Wind 0 deg - Service		0.00	-0.74	-0.05
Wind 30 deg - Service		0.36	-0.63	-0.06
Wind 60 deg - Service		0.63	-0.36	-0.05
Wind 90 deg - Service		0.73	0.00	-0.03
Wind 120 deg - Service		0.64	0.37	0.00
Wind 150 deg - Service		0.36	0.63	0.03
Wind 180 deg - Service		0.00	0.72	0.05
Wind 210 deg - Service		-0.36	0.63	0.06
Wind 240 deg - Service		-0.64	0.37	0.05
Wind 270 deg - Service		-0.73	0.00	0.03
Wind 300 deg - Service		-0.63	-0.36	0.00
Wind 330 deg - Service		-0.36	-0.63	-0.03

## Load Combinations

Comb. No.	Description
1	Dead Only
2	1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy
3	1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy
4	1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy
5	1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy
6	1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy
7	1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy
8	1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy
9	1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy
10	1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy
11	1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy
12	1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy
13	1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy
14	1.2 Dead+1.0 Ice+1.0 Temp+Guy
15	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy
16	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy
17	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy
18	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy
19	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 14 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJJ

Comb. No.	Description
20	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy
21	1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy
22	1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy
23	1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy
24	1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy
25	1.2 Dead+1.0 Wind 300 deg+1.0 Ice+1.0 Temp+1.0 Guy
26	1.2 Dead+1.0 Wind 330 deg+1.0 Ice+1.0 Temp+1.0 Guy
27	Dead+Wind 0 deg - Service+Guy
28	Dead+Wind 30 deg - Service+Guy
29	Dead+Wind 60 deg - Service+Guy
30	Dead+Wind 90 deg - Service+Guy
31	Dead+Wind 120 deg - Service+Guy
32	Dead+Wind 150 deg - Service+Guy
33	Dead+Wind 180 deg - Service+Guy
34	Dead+Wind 210 deg - Service+Guy
35	Dead+Wind 240 deg - Service+Guy
36	Dead+Wind 270 deg - Service+Guy
37	Dead+Wind 300 deg - Service+Guy
38	Dead+Wind 330 deg - Service+Guy

### Maximum Member Forces

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
T1	63 - 43	Leg	Max Tension	2	7.07	-0.02	0.00
			Max. Compression	2	-9.90	0.01	-0.00
			Max. Mx	6	-3.22	-0.04	0.01
			Max. My	10	-5.36	0.02	0.06
			Max. Vy	6	-0.27	0.00	-0.00
			Max. Vx	10	0.26	-0.00	0.02
		Diagonal	Max Tension	2	1.22	0.00	0.00
			Max. Compression	2	-1.27	0.00	0.00
			Max. Mx	15	-0.13	0.01	0.00
			Max. My	2	-0.05	0.00	-0.00
			Max. Vy	15	0.01	0.00	0.00
			Max. Vx	2	0.00	0.00	0.00
		Horizontal	Max Tension	2	1.25	0.00	0.00
			Max. Compression	2	-0.82	0.00	0.00
			Max. Mx	19	0.00	0.00	0.00
			Max. My	2	0.77	0.00	0.00
			Max. Vy	19	-0.01	0.00	0.00
			Max. Vx	2	-0.00	0.00	0.00
		Top Girt	Max Tension	6	0.01	0.00	0.00
			Max. Compression	10	-0.01	0.00	0.00
			Max. Mx	19	0.00	0.01	0.00
			Max. My	15	-0.00	0.00	-0.00
			Max. Vy	19	-0.03	0.00	0.00
			Max. Vx	15	0.00	0.00	0.00
		Bottom Girt	Max Tension	5	0.31	0.00	0.00
			Max. Compression	6	-0.27	0.00	0.00
			Max. Mx	18	0.14	0.02	0.00
			Max. My	15	-0.06	0.00	-0.00
			Max. Vy	18	-0.03	0.00	0.00
			Max. Vx	15	0.00	0.00	0.00
Guy A	Bottom Tension	8	4.98				
	Top Tension	8	4.99				
	Top Cable Vert	8	4.64				
	Top Cable Norm	8	1.82				

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	15 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft					
T2	43 - 23	Guy B	Top Cable Tan	8	0.00							
			Bot Cable Vert	8	-4.61							
			Bot Cable Norm	8	1.87							
			Bot Cable Tan	8	0.00							
			Bottom Tension	2	5.84							
			Top Tension	2	5.85							
			Top Cable Vert	2	5.43							
			Top Cable Norm	2	2.17							
			Top Cable Tan	2	0.09							
			Bot Cable Vert	2	-5.41							
			Bot Cable Norm	2	2.20							
			Bot Cable Tan	2	0.14							
			Bottom Tension	2	5.88							
			Top Tension	2	5.89							
			Top Cable Vert	2	5.47							
			Top Cable Norm	2	2.19							
			Top Cable Tan	2	0.09							
			Bot Cable Vert	2	-5.44							
		Bot Cable Norm	2	2.21								
		Bot Cable Tan	2	0.14								
		Leg	43 - 23		Max Tension	6	9.90	0.02	0.04			
					Max. Compression	11	-15.18	0.06	0.02			
					Max. Mx	5	8.85	-0.07	-0.01			
					Max. My	4	1.84	-0.01	-0.06			
					Max. Vy	5	0.63	-0.07	-0.01			
					Max. Vx	11	0.46	-0.04	-0.02			
					Diagonal			Max Tension	5	1.31	0.00	0.00
								Max. Compression	11	-1.33	0.00	0.00
								Max. Mx	15	0.14	0.00	0.00
								Max. My	15	-0.01	0.00	-0.00
								Max. Vy	15	-0.01	0.00	0.00
								Max. Vx	15	0.00	0.00	0.00
					Horizontal			Max Tension	11	1.01	0.00	0.00
								Max. Compression	5	-1.01	0.00	0.00
								Max. Mx	15	0.01	0.00	0.00
								Max. My	26	0.04	0.00	0.00
								Max. Vy	15	-0.01	0.00	0.00
								Max. Vx	26	-0.00	0.00	0.00
		Top Girt			Max Tension	11	0.31	0.00	0.00			
					Max. Compression	6	-0.35	0.00	0.00			
					Max. Mx	18	0.09	0.01	0.00			
					Max. My	15	-0.04	0.00	-0.00			
Max. Vy	18				-0.02	0.00	0.00					
Max. Vx	15				0.00	0.00	0.00					
Bottom Girt			Max Tension	6	0.54	0.00	0.00					
			Max. Compression	5	-0.46	0.00	0.00					
			Max. Mx	15	-0.02	0.01	0.00					
			Max. My	26	0.06	0.00	-0.00					
			Max. Vy	15	0.02	0.00	0.00					
			Max. Vx	26	0.00	0.00	0.00					
T3	23 - 2	Leg	Max Tension	11	16.21	0.10	-0.04					
			Max. Compression	6	-23.71	-0.12	-0.08					
			Max. Mx	18	0.60	-0.21	-0.05					
			Max. My	9	-17.01	0.11	-0.14					
			Max. Vy	11	-0.74	0.18	-0.02					
			Max. Vx	9	0.44	0.06	-0.12					
			Diagonal			Max Tension	5	1.54	0.00	0.00		
						Max. Compression	11	-1.51	0.00	0.00		
						Max. Mx	15	0.23	0.00	0.00		
						Max. My	15	0.00	0.00	-0.00		
						Max. Vy	15	-0.01	0.00	0.00		
						Max. Vx	15	-0.01	0.00	0.00		

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 16 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Component Type	Condition	Gov. Load Comb.	Axial K	Major Axis Moment kip-ft	Minor Axis Moment kip-ft
		Horizontal	Max. Vx	15	0.00	0.00	0.00
			Max Tension	14	2.34	0.00	0.00
			Max. Compression	5	-1.33	0.00	0.00
			Max. Mx	19	0.43	0.00	0.00
			Max. Vy	19	-0.01	0.00	0.00
		Top Girt	Max Tension	11	0.78	0.00	0.00
			Max. Compression	6	-0.88	0.00	0.00
			Max. Mx	15	0.09	0.02	0.00
			Max. My	9	-0.68	0.00	0.00
			Max. Vy	15	-0.04	0.00	0.00
			Max. Vx	9	-0.00	0.00	0.00
		Bottom Girt	Max Tension	8	0.02	0.00	0.00
			Max. Compression	2	-0.02	0.00	0.00
			Max. Mx	15	0.00	0.02	0.00
			Max. My	2	-0.02	0.00	0.00
			Max. Vy	15	-0.04	0.00	0.00
			Max. Vx	2	-0.00	0.00	0.00

### Maximum Reactions

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
Leg B Elev 12.5 ft	Max. Vert	6	23.65	-0.72	-1.01
	Max. H <sub>x</sub>	25	0.34	2.95	0.31
	Max. H <sub>z</sub>	12	-10.33	0.64	1.12
	Min. Vert	11	-15.76	0.57	0.74
	Min. H <sub>x</sub>	6	23.65	-0.72	-1.01
	Min. H <sub>z</sub>	6	23.65	-0.72	-1.01
Leg C Elev 12.5 ft	Max. Vert	10	23.54	2.02	-1.10
	Max. H <sub>x</sub>	10	23.54	2.02	-1.10
	Max. H <sub>z</sub>	4	-10.04	-1.73	1.21
	Min. Vert	5	-15.70	-1.94	0.75
	Min. H <sub>x</sub>	18	-0.27	-3.42	0.20
	Min. H <sub>z</sub>	9	16.87	1.42	-1.10
Guy C @ 25 ft Elev 0 ft Azimuth 240 deg Elev 0.0 ft	Max. Vert	10	-0.22	-0.05	0.03
	Max. H <sub>x</sub>	10	-0.22	-0.05	0.03
	Max. H <sub>z</sub>	2	-5.44	-1.85	1.23
	Min. Vert	2	-5.44	-1.85	1.23
	Min. H <sub>x</sub>	3	-5.37	-1.85	1.18
	Min. H <sub>z</sub>	9	-0.24	-0.07	0.02
Guy B @ 25 ft Elev 0 ft Azimuth 120 deg Elev 0.0 ft	Max. Vert	6	-0.22	0.05	0.03
	Max. H <sub>x</sub>	13	-5.35	1.84	1.18
	Max. H <sub>z</sub>	2	-5.41	1.83	1.22
	Min. Vert	2	-5.41	1.83	1.22
	Min. H <sub>x</sub>	6	-0.22	0.05	0.03
	Min. H <sub>z</sub>	7	-0.24	0.07	0.02
Guy A @ 25 ft Elev 0 ft Azimuth 0 deg Elev 0.0 ft	Max. Vert	15	-0.00	0.00	0.00

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 17 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJJ

Location	Condition	Gov. Load Comb.	Vertical K	Horizontal, X K	Horizontal, Z K
	Max. H <sub>x</sub>	24	-0.97	0.05	-0.42
	Max. H <sub>z</sub>	15	-0.00	0.00	0.00
	Min. Vert	8	-4.61	0.00	-1.87
	Min. H <sub>x</sub>	18	-0.97	-0.05	-0.42
	Min. H <sub>z</sub>	8	-4.61	0.00	-1.87

## Tower Mast Reaction Summary

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
Dead Only	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 0 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 30 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 60 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 90 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 120 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 150 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 180 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 210 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 240 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 270 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 300 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.6 Wind 330 deg - No Ice+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Ice+1.0 Temp+Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 180 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 210 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 240 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00
1.2 Dead+1.0 Wind 270 deg+1.0 Ice+1.0 Temp+1.0 Guy	0.00	0.00	0.00	0.00	0.00	0.00

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 18 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJJ

Load Combination	Vertical K	Shear <sub>x</sub> K	Shear <sub>z</sub> K	Overturning Moment, M <sub>x</sub> kip-ft	Overturning Moment, M <sub>z</sub> kip-ft	Torque kip-ft
deg+1.0 Ice+1.0 Temp+1.0 Guy						
1.2 Dead+1.0 Wind 300	0.00	0.00	0.00	0.00	0.00	0.00
deg+1.0 Ice+1.0 Temp+1.0 Guy						
1.2 Dead+1.0 Wind 330	0.00	0.00	0.00	0.00	0.00	0.00
deg+1.0 Ice+1.0 Temp+1.0 Guy						
Dead+Wind 0 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 30 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 60 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 90 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 120 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 150 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 180 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 210 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 240 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 270 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 300 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00
Dead+Wind 330 deg - Service+Guy	0.00	0.00	0.00	0.00	0.00	0.00

## Solution Summary

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
1	0.00	-2.54	0.00	0.00	2.54	-0.00	0.004%
2	0.00	-3.04	-3.54	0.00	3.04	3.54	0.016%
3	1.74	-3.03	-3.01	-1.74	3.03	3.01	0.008%
4	2.99	-3.03	-1.73	-2.99	3.03	1.73	0.006%
5	3.47	-3.03	0.00	-3.47	3.03	0.00	0.003%
6	3.06	-3.04	1.77	-3.06	3.04	-1.77	0.003%
7	1.74	-3.03	3.01	-1.74	3.03	-3.01	0.003%
8	0.00	-3.03	3.45	-0.00	3.03	-3.45	0.005%
9	-1.74	-3.03	3.01	1.74	3.03	-3.01	0.001%
10	-3.06	-3.04	1.77	3.06	3.04	-1.77	0.002%
11	-3.47	-3.03	0.00	3.47	3.03	0.00	0.003%
12	-2.99	-3.03	-1.73	2.99	3.03	1.73	0.008%
13	-1.74	-3.03	-3.01	1.74	3.03	3.01	0.011%
14	0.00	-8.55	0.00	0.00	8.55	-0.00	0.003%
15	0.00	-8.55	-1.48	0.00	8.55	1.48	0.006%
16	0.73	-8.55	-1.27	-0.73	8.55	1.27	0.005%
17	1.27	-8.55	-0.73	-1.27	8.55	0.73	0.003%
18	1.47	-8.55	0.00	-1.47	8.55	-0.00	0.002%
19	1.28	-8.55	0.74	-1.28	8.55	-0.74	0.002%
20	0.73	-8.55	1.27	-0.73	8.55	-1.27	0.001%
21	0.00	-8.55	1.47	-0.00	8.55	-1.47	0.002%
22	-0.73	-8.55	1.27	0.73	8.55	-1.27	0.001%
23	-1.28	-8.55	0.74	1.28	8.55	-0.74	0.002%

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	19 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Load Comb.	Sum of Applied Forces			Sum of Reactions			% Error
	PX K	PY K	PZ K	PX K	PY K	PZ K	
24	-1.47	-8.55	0.00	1.47	8.55	-0.00	0.002%
25	-1.27	-8.55	-0.73	1.27	8.55	0.73	0.005%
26	-0.73	-8.55	-1.27	0.73	8.55	1.27	0.007%
27	0.00	-2.54	-0.78	0.00	2.54	0.78	0.005%
28	0.38	-2.54	-0.66	-0.38	2.54	0.66	0.005%
29	0.66	-2.54	-0.38	-0.66	2.54	0.38	0.004%
30	0.77	-2.54	0.00	-0.77	2.54	-0.00	0.003%
31	0.68	-2.54	0.39	-0.68	2.54	-0.39	0.004%
32	0.38	-2.54	0.66	-0.38	2.54	-0.66	0.004%
33	0.00	-2.54	0.76	-0.00	2.54	-0.76	0.003%
34	-0.38	-2.54	0.66	0.38	2.54	-0.66	0.004%
35	-0.68	-2.54	0.39	0.68	2.54	-0.39	0.004%
36	-0.77	-2.54	0.00	0.77	2.54	-0.00	0.005%
37	-0.66	-2.54	-0.38	0.66	2.54	0.38	0.004%
38	-0.38	-2.54	-0.66	0.38	2.54	0.66	0.005%

## Non-Linear Convergence Results

Load Combination	Converged?	Number of Cycles	Displacement Tolerance	Force Tolerance
1	Yes	13	0.00000001	0.00011977
2	Yes	35	0.00009460	0.00013739
3	Yes	36	0.00000001	0.00012856
4	Yes	32	0.00000001	0.00013250
5	Yes	20	0.00000001	0.00010129
6	Yes	20	0.00000001	0.00010663
7	Yes	20	0.00000001	0.00009401
8	Yes	18	0.00000001	0.00014664
9	Yes	21	0.00000001	0.00011022
10	Yes	21	0.00000001	0.00010062
11	Yes	20	0.00000001	0.00013991
12	Yes	31	0.00000001	0.00013510
13	Yes	35	0.00000001	0.00012222
14	Yes	17	0.00000001	0.00012850
15	Yes	33	0.00000001	0.00011070
16	Yes	30	0.00000001	0.00012852
17	Yes	25	0.00000001	0.00012761
18	Yes	19	0.00000001	0.00012870
19	Yes	11	0.00000001	0.00013607
20	Yes	17	0.00000001	0.00008547
21	Yes	17	0.00000001	0.00010778
22	Yes	17	0.00000001	0.00009416
23	Yes	12	0.00000001	0.00008790
24	Yes	19	0.00000001	0.00010630
25	Yes	24	0.00000001	0.00013955
26	Yes	29	0.00000001	0.00013469
27	Yes	16	0.00000001	0.00011039
28	Yes	15	0.00000001	0.00013711
29	Yes	14	0.00000001	0.00011806
30	Yes	12	0.00000001	0.00009325
31	Yes	12	0.00000001	0.00010065
32	Yes	13	0.00000001	0.00013566
33	Yes	14	0.00000001	0.00009351
34	Yes	13	0.00000001	0.00014653
35	Yes	12	0.00000001	0.00011699
36	Yes	11	0.00000001	0.00013461

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 20 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> T.J.L.

37	Yes	14	0.00000001	0.00009980
38	Yes	15	0.00000001	0.00011860

### Maximum Tower Deflections - Service Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	63 - 43	1.549	27	0.0675	0.1860
T2	43 - 23	1.237	27	0.1203	0.1193
T3	23 - 2	0.514	27	0.2196	0.0322

### Critical Deflections and Radius of Curvature - Service Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
61.32	Guy	27	1.534	0.0688	0.1812	25791
60.00	APX16DWV-16DWVS-E-A20	27	1.521	0.0698	0.1773	25791

### Maximum Tower Deflections - Design Wind

Section No.	Elevation ft	Horz. Deflection in	Gov. Load Comb.	Tilt °	Twist °
T1	63 - 43	18.350	2	1.3385	0.4071
T2	43 - 23	12.290	2	1.6379	0.2788
T3	23 - 2	4.553	2	2.0190	0.0916

### Critical Deflections and Radius of Curvature - Design Wind

Elevation ft	Appurtenance	Gov. Load Comb.	Deflection in	Tilt °	Twist °	Radius of Curvature ft
61.32	Guy	2	17.897	1.3493	0.3982	4799
60.00	APX16DWV-16DWVS-E-A20	2	17.539	1.3583	0.3911	4799

### Guy Design Data

Section No.	Elevation ft	Size	Initial Tension K	Breaking Load K	Actual $T_u$ K	Allowable $\phi T_n$ K	Required S.F.	Actual S.F.
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<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	21 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	Initial Tension K	Breaking Load K	Actual $T_u$ K	Allowable $\phi T_n$ K	Required S.F.	Actual S.F.
T1	61.32 (A) (261)	5/16 EHS	1.12	11.20	4.99	6.72	1.000	1.347 ✓
	61.32 (B) (260)	5/16 EHS	1.12	11.20	5.85	6.72	1.000	1.148 ✓
	61.32 (C) (259)	5/16 EHS	1.12	11.20	5.89	6.72	1.000	1.141 ✓

### Compression Checks

#### Leg Design Data (Compression)

Section No.	Elevation ft	Size	L ft	$L_u$ ft	$Kl/r$	A $in^2$	Mast Stability Index	$P_u$ K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	P1.25x.191	20.00	1.51	34.7 K=1.00	0.8815	0.99	-9.90	25.89	0.382 <sup>1</sup> ✓
T2	43 - 23	P1.5x.145	20.00	1.51	29.2 K=1.00	0.7995	0.99	-15.18	23.86	0.636 <sup>1</sup> ✓
T3	23 - 2	P2x.218	21.00	1.48	23.1 K=1.00	1.4773	0.99	-23.71	44.84	0.529 <sup>1</sup> ✓

<sup>1</sup>  $P_u / \phi P_n$  controls

#### Diagonal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	$L_u$ ft	$Kl/r$	A $in^2$	$P_u$ K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	9/16	2.38	2.21	132.2 K=0.70	0.2485	-1.27	3.21	0.396 <sup>1</sup> ✓
T2	43 - 23	9/16	2.91	2.73	162.9 K=0.70	0.2485	-1.33	2.12	0.630 <sup>1</sup> ✓
T3	23 - 2	9/16	2.90	2.67	159.7 K=0.70	0.2485	-1.51	2.20	0.686 <sup>1</sup> ✓

<sup>1</sup>  $P_u / \phi P_n$  controls

#### Horizontal Design Data (Compression)

Section No.	Elevation ft	Size	L ft	$L_u$ ft	$Kl/r$	A $in^2$	$P_u$ K	$\phi P_n$ K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	9/16	1.86	1.72	102.5	0.2485	-0.82	4.63	0.177 <sup>1</sup>

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 22 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T2	43 - 23	9/16	2.48	2.32	K=0.70 138.6	0.2485	-1.01	2.92	0.347 <sup>1</sup> ✓
T3	23 - 2	9/16	2.50	2.30	K=0.70 137.5	0.2485	-1.33	2.97	0.447 <sup>1</sup> ✓
					K=0.70				✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	4x3/8	1.78	1.65	K=1.00 182.4	1.5000	-0.01	10.18	0.001 <sup>1</sup> ✓
T2	43 - 23	4x3/8	2.25	2.09	K=1.00 232.1	1.5000	-0.35	6.29	0.055 <sup>1</sup> ✓
T3	23 - 2	KL/R > 200 (C) - 88 C8x11.5	2.50	2.30	K=1.00 44.2	3.3800	-0.88	98.81	0.009 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Bottom Girt Design Data (Compression)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	4x3/8	2.25	2.11	K=1.00 233.6	1.5000	-0.27	6.21	0.044 <sup>1</sup> ✓
T2	43 - 23	KL/R > 200 (C) - 7 4x3/8	2.50	2.34	K=1.00 259.3	1.5000	-0.46	5.04	0.091 <sup>1</sup> ✓
T3	23 - 2	KL/R > 200 (C) - 91 C8x11.5	2.50	2.30	K=1.00 44.2	3.3800	-0.02	98.81	0.000 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Tension Checks

### Leg Design Data (Tension)

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	23 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	P1.25x.191	20.00	1.51	34.7	0.8815	7.07	27.77	0.255 <sup>1</sup> ✓
T2	43 - 23	P1.5x.145	20.00	1.51	29.2	0.7995	9.90	25.18	0.393 <sup>1</sup> ✓
T3	23 - 2	P2x.218	21.00	1.48	23.1	1.4773	16.21	46.53	0.348 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Diagonal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	9/16	2.38	2.21	188.8	0.2485	1.22	8.05	0.152 <sup>1</sup> ✓
T2	43 - 23	9/16	2.91	2.73	232.7	0.2485	1.31	8.05	0.162 <sup>1</sup> ✓
T3	23 - 2	9/16	2.90	2.67	228.1	0.2485	1.54	8.05	0.191 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Horizontal Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	9/16	1.82	1.68	143.5	0.2485	1.25	8.05	0.155 <sup>1</sup> ✓
T2	43 - 23	9/16	2.48	2.32	198.0	0.2485	1.01	8.05	0.126 <sup>1</sup> ✓
T3	23 - 2	9/16	2.50	2.30	196.4	0.2485	2.34	8.05	0.291 <sup>*1</sup> ✓

\* DL controls

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Top Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
-------------	-----------------	------	---------	----------------------	------	----------------------	---------------------	----------------------	---------------------------------

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b>	18097.00 - CTCOW01A	<b>Page</b>	24 of 25
	<b>Project</b>	281 Route 169 Woodstock, CT	<b>Date</b>	16:10:24 06/20/18
	<b>Client</b>	T-Mobile	<b>Designed by</b>	TJL

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	4x3/8	1.78	1.65	182.4	1.5000	0.01	48.60	0.000 <sup>1</sup>
T2	43 - 23	4x3/8	2.25	2.09	232.1	1.5000	0.31	48.60	0.006 <sup>1</sup> ✓
T3	23 - 2	C8x11.5	2.50	2.30	44.2	3.3800	0.78	109.51	0.007 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Bottom Girt Design Data (Tension)

Section No.	Elevation ft	Size	L ft	L <sub>u</sub> ft	Kl/r	A in <sup>2</sup>	P <sub>u</sub> K	φP <sub>n</sub> K	Ratio $\frac{P_u}{\phi P_n}$
T1	63 - 43	4x3/8	2.25	2.11	233.6	1.5000	0.31	48.60	0.006 <sup>1</sup>
T2	43 - 23	4x3/8	2.50	2.34	259.3	1.5000	0.54	48.60	0.011 <sup>1</sup> ✓
T3	23 - 2	C8x11.5	2.50	2.30	44.2	3.3800	0.02	109.51	0.000 <sup>1</sup> ✓

<sup>1</sup> P<sub>u</sub> / φP<sub>n</sub> controls

### Section Capacity Table

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	φP <sub>allow</sub> K	% Capacity	Pass Fail	
T1	63 - 43	Leg	P1.25x.191	1	-9.90	25.89	38.2	Pass	
		Diagonal	9/16	77	-1.27	3.21	39.6	Pass	
		Horizontal	9/16	75	-0.82	4.63	17.7	Pass	
		Top Girt	4x3/8	6	-0.01	10.18	0.1	Pass	
		Bottom Girt	4x3/8	7	-0.27	6.21	4.4	Pass	
		Guy A@61.3205	5/16	261	4.99	6.72	74.2	Pass	
		Guy B@61.3205	5/16	260	5.85	6.72	87.1	Pass	
T2	43 - 23	Guy C@61.3205	5/16	259	5.89	6.72	87.6	Pass	
		Leg	P1.5x.145	85	-15.18	23.86	63.6	Pass	
		Diagonal	9/16	94	-1.33	2.12	63.0	Pass	
		Horizontal	9/16	97	-1.01	2.92	34.7	Pass	
		Top Girt	4x3/8	88	-0.35	6.29	5.5	Pass	
		Bottom Girt	4x3/8	91	-0.46	5.04	9.1	Pass	
		T3	23 - 2	Leg	P2x.218	170	-23.71	44.84	52.9
Diagonal	9/16			220	-1.51	2.20	68.6	Pass	
Horizontal	9/16			229	-1.33	2.97	44.7	Pass	
Top Girt	C8x11.5			172	-0.88	98.81	0.9	Pass	
Bottom Girt	C8x11.5			175	-0.01	98.81	0.1	Pass	
Summary									
								Leg (T2)	63.6
						Diagonal (T3)	68.6	Pass	

<b>tnxTower</b>  <b>Centek Engineering Inc.</b> 63-2 North Branford Rd. Branford, CT 06405 Phone: (203) 488-0580 FAX: (203) 488-8587	<b>Job</b> 18097.00 - CTCOW01A	<b>Page</b> 25 of 25
	<b>Project</b> 281 Route 169 Woodstock, CT	<b>Date</b> 16:10:24 06/20/18
	<b>Client</b> T-Mobile	<b>Designed by</b> TJL

Section No.	Elevation ft	Component Type	Size	Critical Element	P K	$\phi P_{allow}$ K	% Capacity	Pass Fail
						Horizontal (T3)	44.7	Pass
						Top Girt (T2)	5.5	Pass
						Bottom Girt (T2)	9.1	Pass
						Guy A (T1)	74.2	Pass
						Guy B (T1)	87.1	Pass
						Guy C (T1)	87.6	Pass
						<b>RATING =</b>	<b>87.6</b>	<b>Pass</b>

<b>RAN Template:</b> 4E-U19	<b>A&amp;L Template:</b> 4E-U19_2DP	<b>Power System Template:</b> Custom
--------------------------------	--	---

CTCOW01A\_Modernization\_0.1\_draft

## Section 1 - Site Information

**Site ID:** CTCOW01A  
**Status:** Draft  
**Version:** R0.1  
**Project Type:** Modernization  
**Approved:** Not Approved  
**Approved By:** Not Approved  
**Last Modified:** 3/15/2018 3:35:08 PM  
**Last Modified By:** GSM1900MLow1

**Site Name:** CTCOW01A  
**Site Class:** <undefined>  
**Site Type:** <undefined>  
**Solution Type:**  
**Plan Year:**  
**Market:** CONNECTICUT  
**Vendor:** Ericsson  
**Landlord:** Not Specified

**Latitude:** 41.9377390000  
**Longitude:** -71.9542410000  
**Address:** 39 N Gate Rd  
**City, State:** Woodstock, CT  
**Region:** NORTHEAST

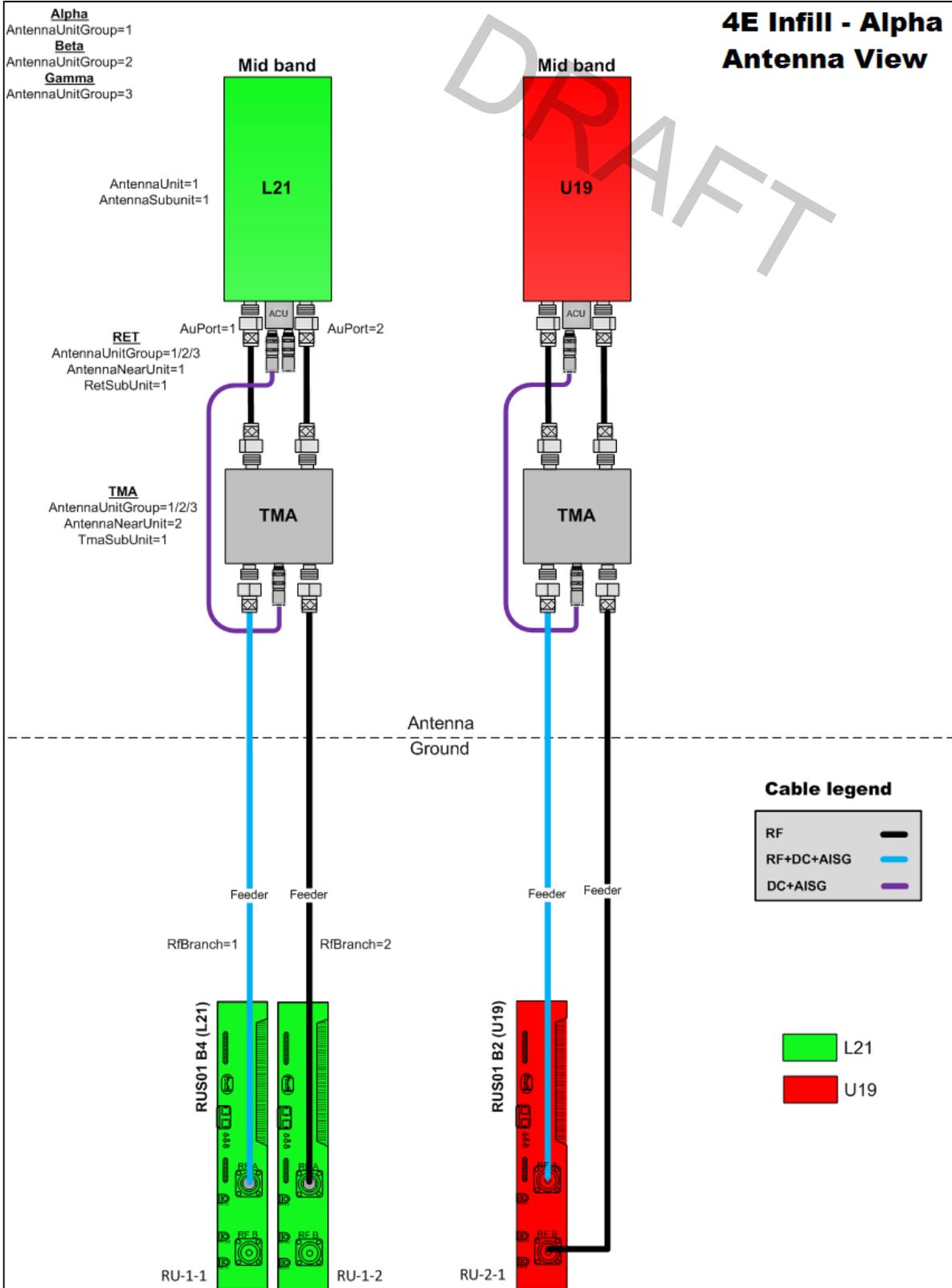
<b>RAN Template:</b> 4E-U19		<b>AL Template:</b> 4E-U19_2DP		
<b>Sector Count:</b> 3	<b>Antenna Count:</b> 3	<b>Coax Line Count:</b> 12	<b>TMA Count:</b> 0	<b>RRU Count:</b> 0

## Section 2 - Existing Template Images

----- This section is intentionally blank. -----

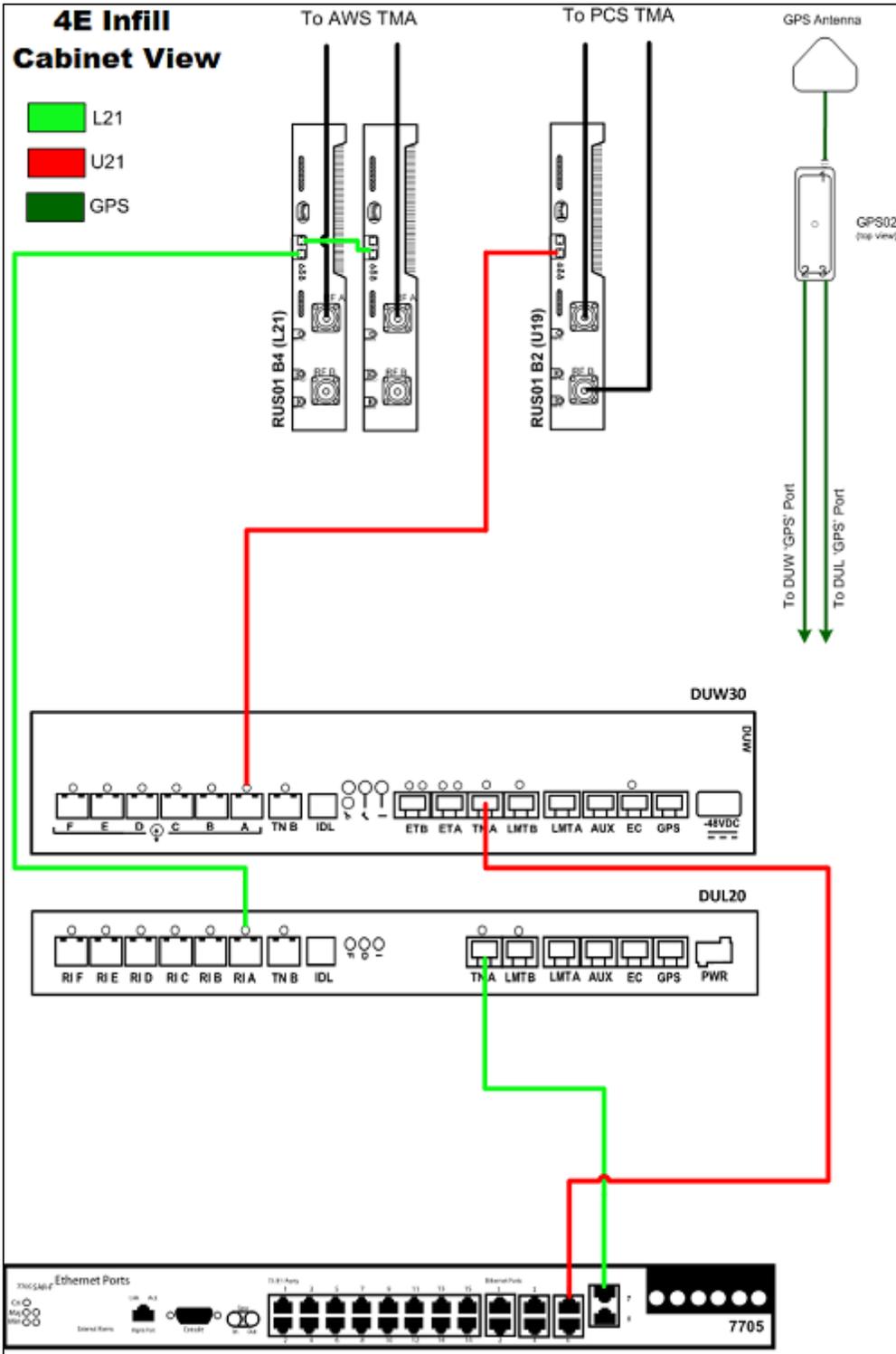
Section 3 - Proposed Template Images

AL\_4E-U19.png



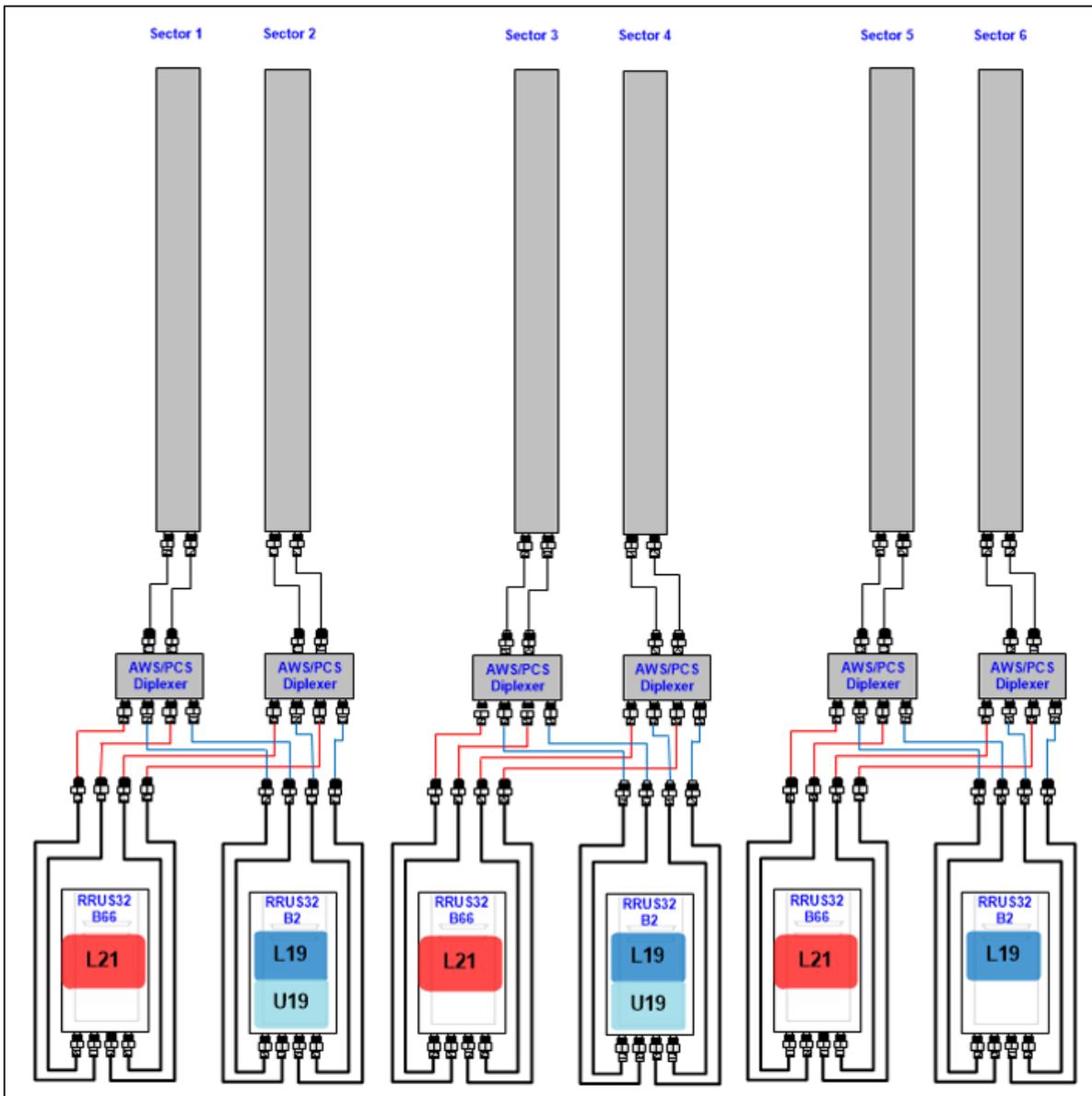
Notes:

RAN\_4E-U19.png



Notes:

HC2 U19.png



Notes:

Section 4 - Siteplan Images

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DRAFT

<b>RAN Template:</b> 4E-U19	<b>A&amp;L Template:</b> 4E-U19_2DP	<b>Power System Template:</b> Custom
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### Section 5 - RAN Equipment

#### Existing RAN Equipment

----- This section is intentionally blank. -----

#### Proposed RAN Equipment

Template: 4E-U19

<b>Enclosure</b>	1	
<b>Enclosure Type</b>	RBS 6201	
<b>Baseband</b>	DUW30 U1900	DUS41 L2100
<b>Radio</b>	RUS01 B2 (x3) U1900	RUS01 B4 (x6) L2100

#### RAN Scope of Work:

<b>RAN Template:</b> 4E-U19	<b>A&amp;L Template:</b> 4E-U19_2DP	<b>Power System Template:</b> Custom
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Section 6 - A&L Equipment

Existing Template: Custom  
Proposed Template: 4E-U19\_2DP

Sector 1 (Proposed) view from behind

<b>Coverage Type</b>	A - Outdoor Macro	
<b>Antenna</b>	1	
<b>Antenna Model</b>	RFS - APX16DWV-16DWV-S-E-A20 (Quad)	
<b>Azimuth</b>	40	
<b>M. Tilt</b>	0	
<b>Height</b>	35	
<b>Ports</b>	<b>P1</b>	<b>P2</b>
<b>Active Tech.</b>	L2100	U1900
<b>Dark Tech.</b>		
<b>Restricted Tech.</b>		
<b>Decomm. Tech.</b>		
<b>E. Tilt</b>	2	2
<b>Cables</b>	1/2" Coax - 40 ft. (x2)	1/2" Coax - 40 ft. (x2)
<b>TMA's</b>		
<b>Diplexers / Combiners</b>		
<b>Radio</b>		
<b>Sector Equipment</b>		
<b>Unconnected Equipment:</b>		
<b>Scope of Work:</b>		

<b>RAN Template:</b> 4E-U19	<b>A&amp;L Template:</b> 4E-U19_2DP	<b>Power System Template:</b> Custom
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Sector 2 (Proposed) view from behind		
<b>Coverage Type</b>	A - Outdoor Macro	
<b>Antenna</b>	1	
<b>Antenna Model</b>	RFS - APX16DWV-16DWV-S-E-A20 (Quad)	
<b>Azimuth</b>	180	
<b>M. Tilt</b>	0	
<b>Height</b>	35	
<b>Ports</b>	<b>P1</b>	<b>P2</b>
<b>Active Tech.</b>	L2100	U1900
<b>Dark Tech.</b>		
<b>Restricted Tech.</b>		
<b>Decomm. Tech.</b>		
<b>E. Tilt</b>	2	2
<b>Cables</b>	1/2" Coax - 40 ft. (x2)	1/2" Coax - 40 ft. (x2)
<b>TMA's</b>		
<b>Diplexers / Combiners</b>		
<b>Radio</b>		
<b>Sector Equipment</b>		
<b>Unconnected Equipment:</b>		
<b>Scope of Work:</b>		

<b>RAN Template:</b> 4E-U19	<b>A&amp;L Template:</b> 4E-U19_2DP	<b>Power System Template:</b> Custom
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**Sector 3 (Proposed) view from behind**

<b>Coverage Type</b>	A - Outdoor Macro	
<b>Antenna</b>	1	
<b>Antenna Model</b>	RFS - APX16DWV-16DWV-S-E-A20 (Quad)	
<b>Azimuth</b>	280	
<b>M. Tilt</b>	0	
<b>Height</b>	35	
<b>Ports</b>	<b>P1</b>	<b>P2</b>
<b>Active Tech.</b>	L2100	U1900
<b>Dark Tech.</b>		
<b>Restricted Tech.</b>		
<b>Decomm. Tech.</b>		
<b>E. Tilt</b>	1	1
<b>Cables</b>	1/2" Coax - 40 ft. (x2)	1/2" Coax - 40 ft. (x2)
<b>TMA's</b>		
<b>Diplexers / Combiners</b>		
<b>Radio</b>		
<b>Sector Equipment</b>		
<b>Unconnected Equipment:</b>		
<b>Scope of Work:</b>		

<b>RAN Template:</b> 4E-U19	<b>A&amp;L Template:</b> 4E-U19_2DP	<b>Power System Template:</b> Custom
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**Section 7 - Power Systems Equipment**

**Existing Power Systems Equipment**

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**Proposed Power Systems Equipment**



Optimizer® Side-by-Side Dual Polarized Antenna, 1710-2200, 65deg, 18.4dBi, 1.4m, VET, 0-10deg RET

**Product Description**

A combination of two X-Polarized antennas in a single radome, this pair of variable tilt antennas provides exceptional suppression of all upper sidelobes at all downtilt angles. It also features a wide downtilt range. This antenna is optimized for performance across the entire frequency band (1710-2200 MHz). The antenna comes pre-connected with two antenna control units (ACU).

**Features/Benefits**

- Variable electrical downtilt - provides enhanced precision in controlling intercell interference. The tilt is infield adjustable 0-10 deg.
- High Suppression of all Upper Sidelobes (Typically <-20dB).
- Gain tracking – difference between AWS UL (1710-1755 MHz) and DL (2110-2155 MHz) <1dB.
- Two X-Polarised panels in a single radome.
- Azimuth horizontal beamwidth difference <4deg between AWS UL (1710-1755 MHz) and DL (2110-2155 MHz).
- Low profile for low visual impact.
- Dual polarization; Broadband design.
- Includes (2) AISG 2.0 Compatible ACU-A20-N antenna control units.



**Technical Specifications**

**Electrical Specifications**

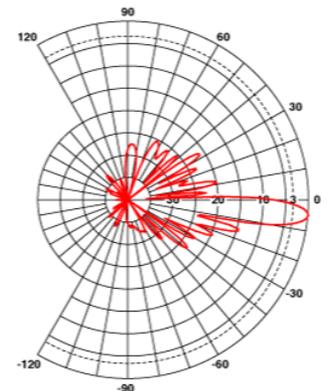
Frequency Range, MHz	1710-2200
Horizontal Beamwidth, deg	65
Vertical Beamwidth, deg	5.9 to 7.7
Electrical Downtilt, deg	0-10
Gain, dBi (dBd)	18.4 (16.3)
1st Upper Sidelobe Suppression, dB	> 18 (typically > 20)
Upper Sidelobe Suppression, dB	> 18 all (typically > 20)
Front-To-Back Ratio, dB	>26 (typically 28)
Polarization	Dual pol +/-45°
VSWR	< 1.5:1
Isolation between Ports, dB	> 30
3rd Order IMP @ 2 x 43 dBm, dBc	> 150 (155 Typical)
Impedance, Ohms	50
Maximum Power Input, W	300
Lightning Protection	Direct Ground
Connector Type	(4) 7-16 Long Neck Female

**Mechanical Specifications**

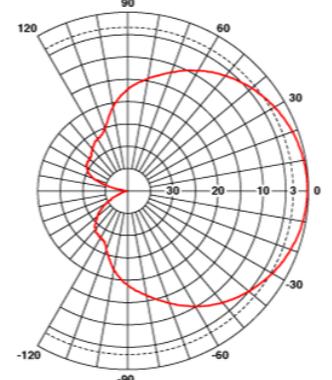
Dimensions - HxWxD, mm (in)	1420 x 331 x 80 (55.9 x 13 x 3.15)
Weight w/o Mtg Hardware, kg (lb)	18.5 (40.7)
Survival Wind Speed, km/h (mph)	200 (125)
Rated Wind Speed, km/h (mph)	160 (100)
Max Wind Loading Area, m <sup>2</sup> (ft <sup>2</sup> )	0.47 (5.03)
Front Thrust @ Rated Wind, N (lbf)	756 (170)
Maximum Thrust @ Rated Wind, N (lbf)	756 (170)
Wind Load - Side @ Rated Wind, N (lbf)	231 (52)
Wind Load - Rear @ Rated Wind, N (lbf)	408 (92)
Radome Material	Fiberglass
Radome Color	Light Grey RAL7035
Mounting Hardware Material	Diecasted Aluminum
Shipping Weight, kg (lb)	24.5 (53.9)
Packing Dimensions, HxWxD, mm (in)	1520 x 408 x 198 (59.8 x 16 x 7.8)

**Ordering Information**

Mounting Hardware APM40-2 + APM40-E2



Vertical Pattern



Horizontal Pattern

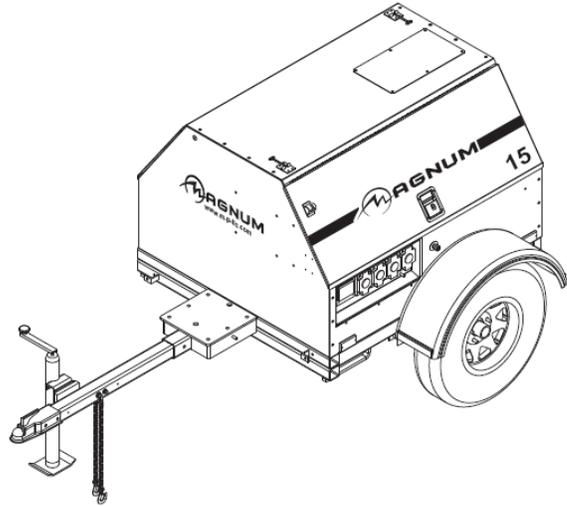
All information contained in the present datasheet is subject to confirmation at time of ordering

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## Magnum Mobile Lite Generator – MLG15 Specifications

### ENGINE

- Mitsubishi® S4L2-Y461ML - naturally aspirated, diesel engine
  - Prime - 22.3 hp @ 1800 rpm
  - 4 cylinder
  - 1.8 L displacement
  - Interim Tier IV approved
- Polyethylene fuel tank
  - 56 gal. capacity
  - 43 hr. run time – full load
  - 3 ½" fill port
- Fuel consumption at prime:
  - 100% - 1.30 gph (4.92 Lph)
  - 75% - 0.98 gph (3.71 Lph)
  - 50% - 0.65 gph (2.46 Lph)
- Cooling system capable of operating at 120°F ambient
- Rubber vibration dampers isolate engine/generator from frame
- Full flow oil filter, spin on type
- Fuel filter with replaceable element
- Dry type cartridge air filter
- 60 Hz engine/generator



### ENGINE CONTROLS

- Engraved aluminum punched and anodized control panel
- Four position keyed switch – glow plugs (preheat, off, run, start)
- Hour meter
- Automatic low oil/high temperature shutdown system

### GENERATOR

- Marathon Electric®
  - Brushless
  - 4 pole
  - Class H insulation
- Single phase output
  - Prime - 13 kW / 13 kVA (54A @ 240V)
  - Standby - 14 kW / 14 kVA (58A @ 240V)
- Voltage regulation +/- 1% with Marathon SE350 Voltage Regulator

## ELECTRICAL SYSTEM AND CONTROLS

- 70A start limit breaker (assures no load condition exists before starting)
- Convenience receptacles with individual breakers
  - (2) 120V 20 Amp GFCI duplex outlets (Nema 5-20R type)
  - (2) 240V 30 Amp twistlock outlets (Nema L6-30R type)
  - (2) 240V 50 Amp twistlock outlets (Non-Nema 6369)
- 440 CCA wet cell battery

## ENCLOSURE

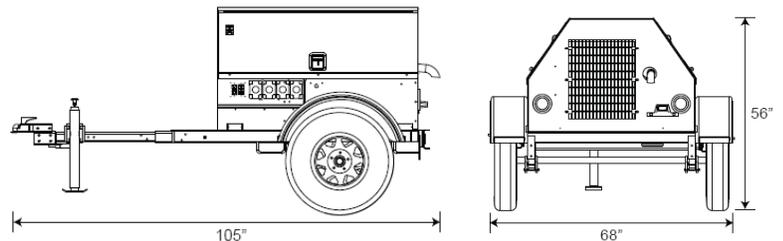
- Steel, 14-gauge, sound attenuated enclosure
  - UV & fade resistant, high temperature cured, white polyester powder paint
  - Insulated and baffled
  - 70 dB(A) at 23 feet – prime power
- Fully lockable enclosure
- Stainless steel hinges, door latches and exterior hardware
- Emergency stop switch located on front panel
- License plate holder with light
- Multi-lingual operating/safety decals
- Document holder with operating/parts manuals including AC/DC wiring diagrams

## TRAILER

- DOT approved tail, side, brake, and directional lights
  - Recessed rear lights
- Transportation tie downs
- Safety chains with spring loaded safety hooks
- Single wall polyethylene fenders
- 2" ball hitch
- 2200 lb. leaf spring axle
- 2000 lb. tongue jack with footplate
- ST205/75R15 tubeless tires – 6 ply
- 48" track width

## WEIGHTS & DIMENSIONS

- Dry weight: 1425 lbs (646 kg)
- Operating weight: 1823 lbs (827 kg)
- 105 x 68 x 56 in  
(2.67 x 1.73 x 1.42 m)



## WARRANTY

- Engine and generator covered under OEM warranty – consult factory for details

## CERTIFICATIONS

- CSA certified



## MLG15 Options

### ENGINE OPTIONS

- ◆ Heated fuel filter
- ◆ Lower radiator hose – engine heater
- ◆ Oil drain valve kit

### ELECTRICAL CONTROLS OPTIONS

- ◆ 720 CCA gel cell battery
- ◆ 720 CCA wet cell battery
- ◆ 685 CCA gel cell battery
- ◆ Battery disconnect
- ◆ Battery charger – 2A trickle

### VOLTAGE OUTPUT OPTIONS

- ◆ Alternative receptacle panel – consult factory for configurations

### COOLANT OPTIONS

- ◆ 60/40 Coolant – cold weather applications

### ENCLOSURE OPTIONS

- ◆ Interior cabinet light
- ◆ Level indicator
- ◆ Tamper pack
- ◆ Liquid containment / Quiet pack
- ◆ Lift structure

### FUEL TANK OPTIONS

- ◆ 56 gal. fuel tank
- ◆ Tethered fuel tank cap

### TRAILER OPTIONS

- ◆ 6 pin or 7 spade electrical connectors
- ◆ Outrigger package
- ◆ Tube and sleeve jack
- ◆ Spare tire/wheel kit

### HITCH OPTIONS

- ◆ 2.5” lunette ring
- ◆ 3” lunette ring
- ◆ 3” HD lunette ring
- ◆ 2 5/16” ball
- ◆ Combination hitch – 2.5” lunette ring / 2” ball





## RADIO FREQUENCY EMISSIONS ANALYSIS REPORT EVALUATION OF HUMAN EXPOSURE POTENTIAL TO NON-IONIZING EMISSIONS

T-Mobile Existing Facility

Site ID: CTCOW01A

Woodstock Fair  
281 Route 169  
Woodstock, CT 06281

**July 9, 2019**

**EBI Project Number: 6218004968**

Site Compliance Summary	
Compliance Status:	<b>COMPLIANT</b>
Site total MPE% of FCC general population allowable limit:	<b>8.79 %</b>



July 9, 2019

T-Mobile USA  
Attn: Jason Overbey, RF Manager  
35 Griffin Road South  
Bloomfield, CT 06002

## Emissions Analysis for Site: **CTCOW01A – Woodstock Fair**

EBI Consulting was directed to analyze the proposed T-Mobile facility located at **281 Route 169, Woodstock, CT**, for the purpose of determining whether the emissions from the Proposed T-Mobile Antenna Installation located on this property are within specified federal limits.

All information used in this report was analyzed as a percentage of current Maximum Permissible Exposure (% MPE) as listed in the FCC OET Bulletin 65 Edition 97-01 and ANSI/IEEE Std C95.1. The FCC regulates Maximum Permissible Exposure in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The number of  $\mu\text{W}/\text{cm}^2$  calculated at each sample point is called the power density. The exposure limit for power density varies depending upon the frequencies being utilized. Wireless Carriers and Paging Services use different frequency bands each with different exposure limits, therefore it is necessary to report results and limits in terms of percent MPE rather than power density.

All results were compared to the FCC (Federal Communications Commission) radio frequency exposure rules, 47 CFR 1.1307(b)(1) – (b)(3), to determine compliance with the Maximum Permissible Exposure (MPE) limits for General Population/Uncontrolled environments as defined below.

General population/uncontrolled exposure limits apply to situations in which the general population may be exposed or in which persons who are exposed as a consequence of their employment may not be made fully aware of the potential for exposure or cannot exercise control over their exposure. Therefore, members of the general population would always be considered under this category when exposure is not employment related, for example, in the case of a telecommunications tower that exposes persons in a nearby residential area.

Public exposure to radio frequencies is regulated and enforced in units of microwatts per square centimeter ( $\mu\text{W}/\text{cm}^2$ ). The general population exposure limit for the 1900 MHz (PCS) and 2100 MHz (AWS) bands is  $1000 \mu\text{W}/\text{cm}^2$ . Because each carrier will be using different frequency bands, and each frequency band has different exposure limits, it is necessary to report percent of MPE rather than power density.



Occupational/controlled exposure limits apply to situations in which persons are exposed as a consequence of their employment and in which those persons who are exposed have been made fully aware of the potential for exposure and can exercise control over their exposure. Occupational/controlled exposure limits also apply where exposure is of a transient nature as a result of incidental passage through a location where exposure levels may be above general population/uncontrolled limits (see below), as long as the exposed person has been made fully aware of the potential for exposure and can exercise control over his or her exposure by leaving the area or by some other appropriate means.

Additional details can be found in FCC OET 65.

## CALCULATIONS

Calculations were done for the proposed T-Mobile Wireless antenna facility located at **281 Route 169, Woodstock, CT**, using the equipment information listed below. All calculations were performed per the specifications under FCC OET 65. Since T-Mobile is proposing highly focused directional panel antennas, which project most of the emitted energy out toward the horizon, all calculations were performed assuming a lobe representing the maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was focused at the base of the tower. For this report the sample point is the top of a 6-foot person standing at the base of the tower.

For all calculations, all equipment was calculated using the following assumptions:

- 1) 2 UMTS channels (PCS Band - 1900 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 30 Watts per Channel.
- 2) 2 LTE channels (AWS Band – 2100 MHz) were considered for each sector of the proposed installation. These Channels have a transmit power of 60 Watts per Channel
- 3) Since all radios are ground mounted there are additional cabling losses accounted for. For each ground mounted RF path, the following losses were calculated. 1.24 dB of additional cable loss for all ground mounted 1900 MHz radios and 1.32 dB of additional cable loss for all ground mounted 2100 MHz channels were factored into the calculations used for this analysis. This is based on manufacturers Specifications for 40 feet of 1/2" coax cable on each path.
- 4) All radios at the proposed installation were considered to be running at full power and were uncombined in their RF transmissions paths per carrier prescribed configuration. Per FCC OET Bulletin No. 65 - Edition 97-01 recommendations to achieve the maximum anticipated value at each sample point, all power levels emitting from the proposed antenna installation are increased by a factor of 2.56 to account for possible in-phase reflections from the surrounding environment. This is rarely the case, and if so, is never continuous.



- 5) For the following calculations the sample point was the top of a 6-foot person standing at the base of the tower. The maximum gain of the antenna per the antenna manufactures supplied specifications minus 10 dB was used in this direction. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 6) The antennas used in this modeling are the **RFS APX16DWV-16DWVS-E-A20** for 1900 MHz (PCS) and 2100 MHz (AWS) channels. This is based on feedback from the carrier with regards to anticipated antenna selection. The **RFS APX16DWV-16DWVS-E-A20** has a maximum gain of **16.3 dBd** at its main lobe at 1900 MHz and 2100 MHz. The maximum gain of the antenna per the antenna manufactures supplied specifications, minus 10 dB, was used for all calculations. This value is a very conservative estimate as gain reductions for these particular antennas are typically much higher in this direction.
- 7) The antenna mounting height centerline of the proposed antennas is **60 feet** above ground level (AGL).
- 8) Emissions values for additional carriers were taken from the Connecticut Siting Council active database. Values in this database are provided by the individual carriers themselves.
- 9) All calculations were done with respect to uncontrolled / general population threshold limits.



## T-Mobile Site Inventory and Power Data

Sector:	A	Sector:	B	Sector:	C
Antenna #:	1	Antenna #:	1	Antenna #:	1
Make / Model:	RFS APX16DWV- 16DWVS-E-A20	Make / Model:	RFS APX16DWV- 16DWVS-E-A20	Make / Model:	RFS APX16DWV- 16DWVS-E-A20
Gain:	16.3 dBd	Gain:	16.3 dBd	Gain:	16.3 dBd
Height (AGL):	60	Height (AGL):	60	Height (AGL):	60
Frequency Bands	1900 MHz (PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz (PCS) / 2100 MHz (AWS)	Frequency Bands	1900 MHz (PCS) / 2100 MHz (AWS)
Channel Count	4	Channel Count	4	Channel Count	4
Total TX Power(W):	180	Total TX Power(W):	180	Total TX Power(W):	180
ERP (W):	7,127.55	ERP (W):	7,127.55	ERP (W):	7,127.55
Antenna A1 MPE%	8.79	Antenna B1 MPE%	8.79	Antenna C1 MPE%	8.79

Site Composite MPE%	
Carrier	MPE%
T-Mobile (Per Sector Max)	8.79 %
No Additional Carriers Located at This Site	NA
<b>Site Total MPE %:</b>	<b>8.79 %</b>

T-Mobile Sector A Total:	8.79 %
T-Mobile Sector B Total:	8.79 %
T-Mobile Sector C Total:	8.79 %
<b>Site Total:</b>	<b>8.79 %</b>

T-Mobile _Max Power Values (per sector)	# Channels	Watts ERP (Per Channel)	Height (feet)	Total Power Density ( $\mu\text{W}/\text{cm}^2$ )	Frequency (MHz)	Allowable MPE ( $\mu\text{W}/\text{cm}^2$ )	Calculated % MPE
T-Mobile AWS - 2100 MHz LTE	2	2,372.20	60	58.49	AWS - 2100 MHz	1000	5.85%
T-Mobile PCS - 1900 MHz UMTS	2	1,191.57	60	29.38	PCS - 1900 MHz	1000	2.94%
<b>Total:</b>							<b>8.79%</b>



## Summary

All calculations performed for this analysis yielded results that were **within** the allowable limits for general population exposure to RF Emissions.

The anticipated maximum composite contributions from the T-Mobile facility as well as the site composite emissions value with regards to compliance with FCC's allowable limits for general population exposure to RF Emissions are shown here:

T-Mobile Sector	Power Density Value (%)
Sector A:	8.79 %
Sector B:	8.79 %
Sector C:	8.79 %
T-Mobile Per Sector Maximum:	8.79 %
Site Total:	8.79 %
Site Compliance Status:	<b>COMPLIANT</b>

The anticipated composite MPE value for this site assuming all carriers present is **8.79%** of the allowable FCC established general population limit sampled at the ground level. This is based upon values listed in the Connecticut Siting Council database for existing carrier emissions.

FCC guidelines state that if a site is found to be out of compliance (over allowable thresholds), that carriers over a 5% contribution to the composite value will require measures to bring the site into compliance. For this facility, the composite values calculated were well within the allowable 100% threshold standard per the federal government.

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