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June 7, 2019

Melanie A. Bachman, Esq.
Executive Director/Staff Attorney
Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: **EM-VER-119-170327 – Cellco Partnership d/b/a Verizon Wireless ("Cellco")**
901 France Street (a/k/a North Street), Rocky Hill, Connecticut

Dear Ms. Bachman:

On August 17, 2017, the Siting Council acknowledged receipt of Cellco's notice of intent to modify its existing telecommunications facility at 901 France Street in Rocky Hill, Connecticut. Cellco's modifications involved the replacement of antennas and installation of remote radio heads on the existing tower.

As a condition of its approval, Cellco was required to provide the Council with a letter stating that it had complied with the recommendations made in the February 27, 2017 Structural Modification Report, which was attached to the EM-VER-119-170327 filing. In preparation for construction of the approved modifications it was also discovered that Cellco's antenna mounts would need to be modified.

To avoid any confusion, Cellco had a new structural analysis completed on May 17, 2019, which took into account the modifications approved in EM-VER-119-170327, as well as the modifications to the antenna mounts. A copy of the updated May 17, 2019 structural report is attached. Also attached is the Modification Inspection Report, dated August 21, 2017, verifying that the tower reinforcement, described in the February 27, 2017 Structural Modification Report, was completed.

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Finally, attached is a Notice of Completion Certification from Dewberry Engineers Inc. verifying that all of Cellco's facility modifications were completed in accordance with the May 17, 2019 Structural Analysis Report.

If you have any questions please do not hesitate to contact me.

Sincerely,

Kenneth C. Baldwin

Attachment
Copy to:
Tim Parks

Dewberry

Dewberry Engineers Inc.
600 Parsippany Road, Suite 301
Parsippany, NJ 07054

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Structural Modification Analysis Report and Design Calculations For a Wireless Telecommunications Upgrade

Site Name: Rocky Hill CT
Site Address: France Street, Lot 1
Rocky Hill, CT 06067

Prepared for:

Verizon Wireless
99 East River Drive
East Hartford, CT 06108

May 17, 2019
Rev. 1

Prepared by:
Dewberry Engineers Inc.
600 Parsippany Road, Suite 301
Parsippany, NJ 07054
Dewberry Project Number: 50103542

Prepared by:

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1.0 INTRODUCTION AND PROJECT SUMMARY

The objective of this report is to assess the proposed installation of new antennas on the existing monopole. This report is limited to the analysis of the antenna platform only.

The existing structure is a 140 ft tall tower located in Rocky Hill, CT with antennas attached to the existing platform. There are currently existing antennas and support equipment mounted to the platform in sectors Alpha, Beta, & Gamma. The proposed equipment will be attached at an approximate antenna centerline elevation of 138 ft.

2.0 EXISTING & PROPOSED ANTENNAS & EQUIPMENT

Currently, the tower has the following equipment:

- Two (2) Amphenol BXA-70063-6CF-4 panel antennas*
- One (1) Andrew LNX-6514DS-T6M panel antenna*
- Three (3) Amphenol BXA-171063-12BF-2 panel antennas*
- Four (4) Amphenol LPA-80063-4CF-4 panel antennas measuring 47.44"H x 15.2"W x 13.19"D (20 lb).
- Two (2) Swedcom SC-E 6014 panel antennas measuring 43"H x 8.5"W x 8"D (15 lb).

(*) To be removed

The following antennas and equipment are proposed in each sector:

- Sectors Alpha, Beta & Gamma
  - Two (2) SBNHH-1D65B antenna measuring 72.4"H x 11.9" W x 7.1" D (40.6 lb).
  - One (1) 2x60 700 RRH unit measuring 20.9"H x 11.8"W x 7.5"D (55.6 lb).
  - One (1) OVP measuring 28.93"H x 15.73"W x 10.3"D (32 lb). (*Total one*)
  - Miscellaneous equipment and cables negligible in weight and profile.

For the final configuration, refer to the table below:

Table 1: APPURTENANCE LOADING on TOWER			
Elev.	Status	Carrier	Appurtenance Description
138'	**Final**	**Verizon**	**(6) SBNHH-1D65B**
138'	Final	Verizon	(4) Amphenol LPA-80063-4CF-4
138'	Final	Verizon	(2) Swedcom SC-E 6014
138'	**Final**	**Verizon**	**(3) 2x60 700 RRH**
138'	**Final**	**Verizon**	**(1) OVP**
Verizon			

3.0 CODES, STANDARDS, AND REFERENCES

The structure was analyzed and the proposed installation designed per the provisions of the following Codes and standards:

- *International Building Code (IBC) 2015, International Code Council*
- *2018 Connecticut State Building Code – Amendments to IBC 2015*
- *TIA-222-G-4, Structural Standard for Antenna Supporting Structures and Antennas*
- *Steel Construction Manual 14th Ed, American Institute of Steel Construction.*
- *ASCE 7-10 Minimum Design Loads for Buildings and Other Structures,* American Society of Civil Engineers.
- *Construction Drawings by Dewberry Engineers Inc. (Dated: 09/05/17).*
- *Mount Mapping by Northeast Union Inc. (Dated 1/19/18).*
- *Structural Analysis Report by A-Prime Engineering (Dated 02/12/18).*
- *Structural Analysis Report by Dewberry Engineers (Dated 04/12/18).*

4.0 LOADING AND PERFORMANCE CRITERIA

The following Code-specified strength limit state (LRFD) load combinations were considered in the analysis of the antenna mounts *(TIA-222-G)*:

1. $1.2D + 1.6W$
2. $1.2D + 1.0D_i + 1.0W_i$

The following Code-specified serviceability load combination was considered in the deflection of the antenna mounts (TIA-222-G):

1. $1.0D + 1.0W_{service}$

Where:

D = dead load of structure, steel, and new equipment.
D_i = dead load of ice
W = design wind load
W_i = design ice wind load
$W_{service}$ = service wind load

The following site-specific design parameters were considered in this analysis per the provisions of *TIA-222-G:*

- Class: II
- Exposure: C
- Design Wind Speed: 96 mph
  Converted from 124 mph ASCE 7-10 wind speed
- Design Ice Wind Speed: 50 mph — *Annex B*
- Design Ice Thickness: 1.00 in — *Annex B*
- Serviceability Wind Speed: 60 mph — *Sect. 2.8.3*

5.0 CALCULATIONS

Calculations for this analysis and the design of the installation are included in the Appendices of this report.

6.0 CONCLUSIONS, COMMENTARY, AND RECOMMENDATIONS

Per the original Dewberry evaluation and analysis, it is concluded that the existing mount **does not have sufficient structural capacity** for the proposed installation in regards of stresses and deflections with the most unfavorable loading effects. Under the proposed conditions, the maximum utilization of a single structural member is **110.1%** and the maximum deflection ratio is **208.8%**.

Therefore, Dewberry recommends modifying the existing mount with a proposed support rail kit (P/N SitePro1 HRK14).

With the recommended modifications, the existing mount **does have sufficient structural capacity** for the proposed installation in regards of stresses and deflections with the most unfavorable loading effects. Under the proposed conditions, the maximum utilization of a single structural member is **75.3%** and the maximum deflection ratio is **28.6%**.

Please note this analysis is limited to the existing antenna mount only. If actual field conditions vary from what was assumed in this analysis, the results and conclusions expressed herein are invalid and further evaluation is recommended for any proposed installation to continue.

Dewberry Engineers Inc. reserves the right to add to or modify this report if more information becomes available. The conclusions reached by Dewberry Engineers Inc. in this report are only applicable to the previously mentioned existing structural elements supporting the proposed wireless telecommunications installation. The results of this report are based on the assumption that existing structural elements have been installed per the original design documents, have been well maintained, and are uncompromised. This report does not imply that a thorough inspection of the existing structure has been performed. Any deviation of the support condition, loading, location, placement, equipment configuration, etc., will require Dewberry Engineers Inc. to generate an additional structural analysis.

APPENDIX A

Dewberry®

Job Number	50103542
Made by:	DAP
Date:	5/13/19
Checked by:	BK
Date:	5/16/19

(Rocky Hill CT) - Design Wind Load

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Wind Load Design Criteria

Site Name: Rocky Hill CT

General Information & Design Input from TIA-222-G

Item	Value	Description	Reference
V =	124.00	Design Wind Speed	*ASCE 7-10, ATC Windspeed*
V_{max} =	96.05	$(\sqrt{0.6}) * V$	*Except. #5, Sect. 1609.3.1, Eqn. 16-33, IBC 15*
Location =	CT	Hartford County	*Annex B, TIA*
V_i =	50.00	Design Ice Wind Speed (mph)	
K_d =	0.95	Wind Direction Probability Factor	*Table 2-2, TIA*
Class	II	Structure Classification	*Table 2-1, TIA*
I =	1.00	Importance Factor (Without Ice)	*Table 2-3, TIA*
I_{ice} =	1.00	Importance Factor (Ice Thickness)	*Table 2-3, TIA*
z = h =	138.00	ft. (A.G.L.)	*Max. Center of Appurtenance*
Exp. Cat.	C	Exposure Category	*Sect. 2.6.5.1, TIA*
z_g =	900.00	Exposure Category Coeff.	*Table 2-4, TIA*
α' =	9.50	Exposure Category Coeff.	*Table 2-4, TIA*
$K_{z\,(min)}$ =	0.85	Exposure Category Coeff.	*Table 2-4, TIA*
K_e =	1.00	Exposure Category Coeff.	*Table 2-4, TIA*
K_t =	N/A	Topographic Constant	*Table 2-5, TIA ("N/A" if Topo. Cat. = 1)*
K_z =	1.35	$= 2.01(z/z_g)^{(2/\alpha')}$	*Sect. 2.6.5.2, TIA*
Topo. Cat.	1	Topographic Category (1-5)	*Sect. 2.6.6.2, TIA*
e =	2.72	Natural Logarithmic base	
f =	N/A	Height Attenuation Factor	*Table 2-5, TIA ("N/A" if Topo. Cat. = 1)*
H =	N/A	ft. Height of crest above surrounding terrain	
K_h =	N/A	$e^{((f*z)/H)}$	*Sect. 2.6.6.4, TIA*
K_{zt} =	1.00	$= [1+((K_e*K_t)/K_h)]^2$	*Sect. 2.6.6.4, TIA*
K_{iz} =	1.15	$= (z/33)^{0.10} \leq 1.4$ (Height escalation factor)	*Sect. 2.6.8, TIA*
G_h =	1.00	Gust Effect Factor	*Sect. 2.6.7, TIA*
t_i =	1.00	Design Ice Thickness	*Annex B, TIA*
t_{iz} =	2.3	$= 2\, t_i(I)K_{iz}(K_{zt})^{0.35}$	*Sect. 2.6.8, TIA*
$q_{z\,design}$ =	30.4 psf	$= 0.00256(K_z)(K_{zt})(K_s)(K_e)(K_d)(V^2)(I)$	*Sect. 2.6.9.6, TIA*
$q_{z\,ice}$ =	8.3 psf	$= 0.00256(K_z)(K_{zt})(K_s)(K_e)(K_d)(V_i^2)$	*Sect. 2.6.9.6, TIA*

Design Wind Forces:

Section 2.6.11.2

$F_A = q_{z\,design}G_h(EPA)_A$ *(where $(EPA)_A$ = effective projected area of the appurtenance = $C_a A_a$)*

$F_{Ai} = q_{z\,ice}G_h(EPA)_{Ai}$

(see calculation tables on following pages)

Design Ice Weight:

Section 2.6.10

$F_i = [\pi(t_{iz})(D_c + t_{iz})]*56\ lb/ft^3$ *(where D_c = largest out to out dimension of member)*

(see calculation tables on following pages)

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(Rocky Hill CT) - Design Wind Load

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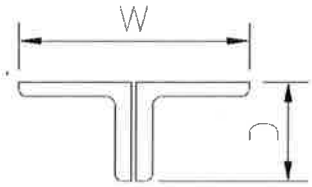
Element Definition

	Description	Dimensions (in.)			Weight	Length /	
		W	D	H	(lb)	# Supports	
	LPA-80063-4CF-F	15.20	13.19	47.44	20.00	2.00	
	SC-E 6014	8.50	8.00	43.00	15.00	2.00	
	SBNHH-1D65B	11.90	7.10	72.90	40.60	2.00	
	SBNHH-1D65B	11.90	7.10	72.90	40.60	2.00	
	2x60 700 RRH	11.80	7.50	20.90	55.60	2.00	
	OVP	15.73	10.30	28.93	32.00	2.00	
	STRUCTURAL MEMBERS						
(Mounting Pipe)	2-3/8" OD Pipe	2.38	2.38	12.00	STAAD	*Pipe*	*(See Note 2)*
	2-3/8" OD Pipe	2.38	2.38	12.00	STAAD	*Pipe*	
	L3x3x1/4	3.00	3.00	12.00	STAAD	*Angle*	
	2L3x3x3/16	6.00	3.00	12.00	STAAD	*Dbl Angle*	
	HSS4x4x1/4	4.00	4.00	12.00	STAAD	*Tube*	

Note:

1) *For Double Angles assume the following:*



2) *For mounting pipes that **do not** support equipment or portions which are not shielded by equipment, create an additional entry below.*

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Design Wind Load

Members	Dimensions (ft.)			Area $(A_a)_n$	Area $(A_a)_t$	Aspect Ratio	Aspect Ratio	C_{an}	C_{at}
	Width (Normal)	Depth (Tangent)	Height (or span)	(normal) (sf)	(tangent) (sf)	(normal)	(tangent)	(normal) Table 2-8	(tangent) Table 2-8
LPA-80063-4CF-F	1.27	1.10	3.95	5.02	4.35	3.11	3.59	1.23	1.25
SC-E 6014	0.71	0.67	3.58	2.54	2.40	5.04	5.34	1.31	1.33
SBNHH-1D65B	0.99	0.59	6.08	6.02	3.59	6.14	10.31	1.36	1.51
SBNHH-1D65B	0.99	0.59	6.08	6.02	3.59	6.14	10.31	1.36	1.51
2x60 700 RRH	0.98	0.63	1.74	1.71	1.10	1.78	2.76	1.20	1.21
OVP	1.31	0.86	2.41	3.16	2.07	1.84	2.80	1.20	1.21
STRUCTURAL MEMBERS									
2-3/8" OD Pipe	0.20	0.20	1.00	0.20	0.20	5.00	5.00	0.76	0.76
2-3/8" OD Pipe	0.20	0.20	1.00	0.20	0.20	5.00	5.00	0.76	0.76
L3x3x1/4	0.25	0.25	1.00	0.25	0.25	4.00	4.00	1.27	1.27
2L3x3x3/16	0.50	0.25	1.00	0.50	0.25	2.00	4.00	1.20	1.27
HSS4x4x1/4	0.33	0.33	1.00	0.33	0.33	3.03	3.03	1.22	1.22

Design Effective Projected Area & Wind Loads

Members	EPA_a @ 0.0° (sf)	EPA_a @ 30.0° (sf)	EPA_a @ 60.0° (sf)	EPA_a @ 90.0° (sf)	F_a @ 0.0° (lb)	F_a @ 30.0° (lb)	F_a @ 60.0° (lb)	F_a @ 90.0° (lb)	Gravity Load @ Support (lb)
LPA-80063-4CF-F	6.17	5.99	5.62	5.44	93.9	91.1	85.5	82.7	10.0
SC-E 6014	3.33	3.29	3.23	3.19	50.6	50.1	49.0	48.5	7.5
SBNHH-1D65B	8.19	7.50	6.11	5.42	124.4	113.9	92.9	82.4	20.3
SBNHH-1D65B	8.19	7.50	6.11	5.42	124.4	113.9	92.9	82.4	20.3
2x60 700 RRH	2.05	1.87	1.51	1.33	31.2	28.5	23.0	20.2	27.8
OVP	3.79	3.47	2.83	2.50	57.6	52.7	43.0	38.1	16.0
STRUCTURAL MEMBERS									
2-3/8" OD Pipe	0.15	-	-	-	4.6	-	-	-	-
2-3/8" OD Pipe	0.15	-	-	-	4.6	-	-	-	-
L3x3x1/4	0.32	-	-	-	9.7	-	-	-	-
2L3x3x3/16	0.60	-	-	-	18.2	-	-	-	-
HSS4x4x1/4	0.40	-	-	-	12.2	-	-	-	-

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(Rocky Hill CT) - Design Wind Load

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Design Ice Wind Load

- Design ice thickness included in tabulated dimensions below.

Members	Dimensions (ft.)			Area $(A_a)_{ni}$	Area $(A_a)_{ti}$	Aspect Ratio	Aspect Ratio	C_{ani}	C_{ati}
	Width (Normal)	Depth (Tangent)	Height (or span)	(normal) (sf)	(tangent) (sf)	(normal)	(tangent)	(normal) Table 2-8	(tangent) Table 2-8
LPA-80063-4CF-F	1.65	1.48	4.34	7.16	6.42	2.63	2.93	1.21	1.22
SC-E 6014	1.09	1.05	3.97	4.33	4.17	3.64	3.78	1.25	1.26
SBNHH-1D65B	1.38	0.98	6.46	8.91	6.33	4.68	6.59	1.30	1.38
SBNHH-1D65B	1.38	0.98	6.46	8.91	6.33	4.68	6.59	1.30	1.38
2x60 700 RRH	1.37	1.01	2.13	2.92	2.15	1.55	2.11	1.20	1.20
OVP	1.69	1.24	2.79	4.72	3.46	1.65	2.25	1.20	1.20
STRUCTURAL MEMBERS									
2-3/8" OD Pipe	0.58	0.58	1.00	0.58	0.58	1.72	1.72	0.70	0.70
2-3/8" OD Pipe	0.58	0.58	1.00	0.58	0.58	1.72	1.72	0.70	0.70
L3x3x1/4	0.63	0.63	1.00	0.63	0.63	1.59	1.59	1.20	1.20
2L3x3x3/16	0.88	0.63	1.00	0.88	0.63	1.14	1.59	1.20	1.20
HSS4x4x1/4	0.72	0.72	1.00	0.72	0.72	1.39	1.39	1.20	1.20

Design Effective Projected Area & Wind Loads with Ice

Members	EPA_{ai} @ 0.0° (sf)	EPA_{ai} @ 30.0° (sf)	EPA_{ai} @ 60.0° (sf)	EPA_{ai} @ 90.0° (sf)	F_{ai} @ 0.0° (lb)	F_{ai} @ 30.0° (lb)	F_{ai} @ 60.0° (lb)	F_{ai} @ 90.0° (lb)	Ice Load F_i @ Support (lb)
LPA-80063-4CF-F	8.66	8.46	8.04	7.83	36.0	35.1	33.4	32.5	124.6
SC-E 6014	5.41	5.37	5.29	5.25	22.5	22.3	22.0	21.8	70.3
SBNHH-1D65B	11.58	10.87	9.45	8.74	48.1	45.1	39.2	36.3	137.9
SBNHH-1D65B	11.58	10.87	9.45	8.74	48.1	45.1	39.2	36.3	137.9
2x60 700 RRH	3.50	3.27	2.81	2.58	14.5	13.6	11.7	10.7	39.8
OVP	5.66	5.29	4.53	4.15	23.5	21.9	18.8	17.2	71.5
STRUCTURAL MEMBERS									
2-3/8" OD Pipe	0.41	-	-	-	3.4	-	-	-	13.2
2-3/8" OD Pipe	0.41	-	-	-	3.4	-	-	-	13.2
L3x3x1/4	0.76	-	-	-	6.3	-	-	-	18.4
2L3x3x3/16	1.06	-	-	-	8.8	-	-	-	23.3
HSS4x4x1/4	0.86	-	-	-	7.2	-	-	-	22.4

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Job Number	50103542
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(Rocky Hill CT) - Serviceability Wind Load

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Serviceability Wind Load Design Criteria

General Information & Design Input from TIA-222-G

Item	Value	Description	Reference
V_s =	60.00	Service Wind Speed (mph)	*Sect. 2.8.3, TIA*
V_m =	30.00	Maintenance Wind Speed (mph)	
K_d =	0.85	Service Wind Direction Probability Factor	*Sect. 2.8.3, TIA*
Class	II	Structure Classification	*Table 2-1, TIA*
z = h =	138.00	ft. (A.G.L.)	*Max. Center of Appurtenance*
Exp. Cat.	C	Exposure Category	*Sect. 2.6.5.1, TIA*
z_g =	900.00	Exposure Category Coeff.	*Table 2-4, TIA*
α' =	9.50	Exposure Category Coeff.	*Table 2-4, TIA*
$K_{z\,(min)}$ =	0.85	Exposure Category Coeff.	*Table 2-4, TIA*
K_e =	1.00	Exposure Category Coeff.	*Table 2-4, TIA*
K_t =	N/A	Topographic Constant	*Table 2-5, TIA ("N/A" if Topo. Cat. = 1)*
K_z =	1.35	$= 2.01(z/z_g)^{(2/\alpha')}$	*Sect. 2.6.5.2, TIA*
Topo. Cat.	1	Topographic Category (1-5)	*Sect. 2.6.6.2, TIA*
e =	2.72	Natural Logarithmic base	
f =	N/A	Height Attenuation Factor	*Table 2-5, TIA ("N/A" if Topo. Cat. = 1)*
H =	N/A	ft. Height of crest above surrounding terrain	
K_h =	N/A	$e^{((f*z)/H)}$	*Sect. 2.6.6.4, TIA*
K_{zt} =	1.00	$= [1+((K_e*K_t)/K_h)]^2$	*Sect. 2.6.6.4, TIA*
G_h =	1.00	Gust Effect Factor	*Sect. 2.6.7, TIA*
$q_{z\,service}$ =	10.7 psf	$= 0.00256(K_z)(K_{zt})(K_d)(V_s^2)$	*Sect. 2.6.9.6, TIA*
$q_{z\,maint}$ =	3.0 psf	$= 0.00256(K_z)(K_{zt})(K_d)(V_m^2)$	

Design Serviceability and Maintenance Wind Forces:

Section 2.6.11.2

$F_{As} = q_{z\,service} G_h (EPA)_A$

$F_{Am} = q_{z\,maint} G_h (EPA)_A$

(see calculation tables on following pages)

(where $(EPA)_A$ = effective projected area of the appurtenance = $C_a A_a$)

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Date:	5/16/19

(Rocky Hill CT) - Serviceability Wind Load

Q:\50095218\50103542\Adm\Reports\MA - Passing\Calculations\50103542 - Rocky Hill CT Analysis Calc. (Co-Location Mount, TIA-222-G).xlsx

Element Definition:

	Description	Dimensions (in.)			Weight	Length /
		W	D	H	(lb)	# Supports
	LPA-80063-4CF-F	15.20	13.19	47.44	20.00	2.00
	SC-E 6014	8.50	8.00	43.00	15.00	2.00
	SBNHH-1D65B	11.90	7.10	72.90	40.60	2.00
	SBNHH-1D65B	11.90	7.10	72.90	40.60	2.00
	2x60 700 RRH	11.80	7.50	20.90	55.60	2.00
	OVP	15.73	10.30	28.93	32.00	2.00
	STRUCTURAL MEMBERS					
(Mounting Pipe)	2-3/8" OD Pipe	2.38	2.38	12.00	STAAD	Pipe
	2-3/8" OD Pipe	2.38	2.38	12.00	STAAD	Pipe
	L3x3x1/4	3.00	3.00	12.00	STAAD	Angle
	2L3x3x3/16	6.00	3.00	12.00	STAAD	Dbl Angle
	HSS4x4x1/4	4.00	4.00	12.00	STAAD	Tube

Service & Maintenance Wind Load

Members	Dimensions (ft.)			Area $(A_a)_n$	Area $(A_a)_t$	Aspect Ratio	Aspect Ratio	C_{an}	C_{at}
	Width *(Normal)*	Depth *(Tangent)*	Height *(or span)*	*(normal)* *(sf)*	*(tangent)* *(sf)*	*(normal)*	*(tangent)*	*(normal)* *Table 2-8*	*(tangent)* *Table 2-8*
LPA-80063-4CF-F	1.27	1.10	3.95	5.02	4.35	3.11	3.59	1.23	1.25
SC-E 6014	0.71	0.67	3.58	2.54	2.40	5.04	5.34	1.31	1.33
SBNHH-1D65B	0.99	0.59	6.08	6.02	3.59	6.14	10.31	1.36	1.51
SBNHH-1D65B	0.99	0.59	6.08	6.02	3.59	6.14	10.31	1.36	1.51
2x60 700 RRH	0.98	0.63	1.74	1.71	1.10	1.78	2.76	1.20	1.21
OVP	1.31	0.86	2.41	3.16	2.07	1.84	2.80	1.20	1.21
STRUCTURAL MEMBERS									
2-3/8" OD Pipe	0.20	0.20	1.00	0.20	0.20	5.00	5.00	0.76	0.76
2-3/8" OD Pipe	0.20	0.20	1.00	0.20	0.20	5.00	5.00	0.76	0.76
L3x3x1/4	0.25	0.25	1.00	0.25	0.25	4.00	4.00	1.27	1.27
2L3x3x3/16	0.50	0.25	1.00	0.50	0.25	2.00	4.00	1.20	1.27
HSS4x4x1/4	0.33	0.33	1.00	0.33	0.33	3.03	3.03	1.22	1.22

Dewberry

Job Number	50103542
Made by:	DAP
Date:	5/13/19
Checked by:	BK
Date:	5/16/19

(Rocky Hill CT) - Serviceability Wind Load

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Service Effective Projected Area & Wind Loads

Members	EPA_{as} @ 0.0° (sf)	EPA_{as} @ 30.0° (sf)	EPA_{as} @ 60.0° (sf)	EPA_{as} @ 90.0° (sf)	F_{as} @ 0.0° (lb)	F_{as} @ 30.0° (lb)	F_{as} @ 60.0° (lb)	F_{as} @ 90.0° (lb)
LPA-80063-4CF-F	6.17	5.99	5.62	5.44	33.0	32.0	30.1	29.1
SC-E 6014	3.33	3.29	3.23	3.19	17.8	17.6	17.3	17.1
SBNHH-1D65B	8.19	7.50	6.11	5.42	43.8	40.1	32.7	29.0
SBNHH-1D65B	8.19	7.50	6.11	5.42	43.8	40.1	32.7	29.0
2x60 700 RRH	2.05	1.87	1.51	1.33	11.0	10.0	8.1	7.1
OVP	3.79	3.47	2.83	2.50	20.3	18.6	15.1	13.4
STRUCTURAL MEMBERS								
2-3/8" OD Pipe	0.15	-	-	-	1.6	-	-	-
2-3/8" OD Pipe	0.15	-	-	-	1.6	-	-	-
L3x3x1/4	0.32	-	-	-	3.4	-	-	-
2L3x3x3/16	0.60	-	-	-	6.4	-	-	-
HSS4x4x1/4	0.40	-	-	-	4.3	-	-	-

Maintenance Effective Projected Area & Wind Loads

Members	EPA_{am} @ 0.0° (sf)	EPA_{am} @ 30.0° (sf)	EPA_{am} @ 60.0° (sf)	EPA_{am} @ 90.0° (sf)	F_{am} @ 0.0° (lb)	F_{am} @ 30.0° (lb)	F_{am} @ 60.0° (lb)	F_{am} @ 90.0° (lb)
LPA-80063-4CF-F	6.17	5.99	5.62	5.44	9.3	9.0	8.4	8.2
SC-E 6014	3.33	3.29	3.23	3.19	5.0	4.9	4.8	4.8
SBNHH-1D65B	8.19	7.50	6.11	5.42	12.3	11.2	9.2	8.1
SBNHH-1D65B	8.19	7.50	6.11	5.42	12.3	11.2	9.2	8.1
2x60 700 RRH	2.05	1.87	1.51	1.33	3.1	2.8	2.3	2.0
OVP	3.79	3.47	2.83	2.50	5.7	5.2	4.2	3.8
STRUCTURAL MEMBERS								
2-3/8" OD Pipe	0.15	-	-	-	0.5	-	-	-
2-3/8" OD Pipe	0.15	-	-	-	0.5	-	-	-
L3x3x1/4	0.32	-	-	-	1.0	-	-	-
2L3x3x3/16	0.60	-	-	-	1.8	-	-	-
HSS4x4x1/4	0.40	-	-	-	1.2	-	-	-

Dewberry

Job Number	50103542
Made by:	DAP
Date:	5/13/19
Checked by:	BK
Date:	5/16/19

(Rocky Hill CT) - Load Input for STAAD Model

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STAAD Gravity Load Input

Equipment	Dead Load *(lb per support)*	Ice Load *(lb per support)*
LPA-80063-4CF-F	10.0	124.6
SC-E 6014	7.5	70.3
SBNHH-1D65B	20.3	137.9
SBNHH-1D65B	20.3	137.9
2x60 700 RRH	27.8	39.8
OVP	16.0	71.5
Platform Grating	8.0	10.7 psf

Maintenance Loads

L_M = 500 lbs - vertical concentrated downward force applied in each mounting pipe location independently

L_V = 250 lbs - vertical concentrated downward force applied at the center of each simply supported horizontal member and at the end of each horizontal cantilevered members independently (excluding tie-backs)

STAAD Wind Load Calculation

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°	Case #5 120°	Case #6 150°	Case #7 180°	Case #8 210°	Case #9 240°	Case #10 270°	Case #11 300°	Case #12 330°
Not Shielded (1 - 4)	z	$-F_0$	Same as Case #4	Same as Case #1	-	Same as Case #1	Same as Case #2	Same as Case #3	Same as Case #4	Same as Case #1	Same as Case #2	Same as Case #3	Same as Case #4
	x	-			F_{90}								
Shielded (1 - 4)	z	N/A			-								
	x	-			F_{90}								
Not Shielded (5 - 12)	z	$-F_{60}$			-								
	x	-			F_{30}								
Shielded (5 - 12)	z	$-F_{60}$			-								
	x	-			N/A								

Dewberry

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(Rocky Hill CT) - Load Input for STAAD Model

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STAAD Load Input (Design Wind Load)

Positions 1 - 4

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-93.9	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			82.7
SBNHH-1D65B	z	-124.4			-
(Not Shielded)	x	-			82.4
SBNHH-1D65B	z	-124.4			-
(Not Shielded)	x	-			82.4
2x60 700 RRH	z	N/A			-
(Shielded)	x	-			20.2
OVP	z	-57.6			-
(Not Shielded)	x	-			38.1

Positions 5 - 12

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-85.5	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			91.1
SC-E 6014	z	-49.0			-
(Not Shielded)	x	-			50.1
SBNHH-1D65B	z	-92.9			-
(Not Shielded)	x	-			113.9
SBNHH-1D65B	z	-92.9			-
(Not Shielded)	x	-			113.9

Dewberry

Job Number	50103542
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(Rocky Hill CT) - Load Input for STAAD Model

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STAAD Load Input (Design Wind on Ice Load)

Positions 1 - 4

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-36.0	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			32.5
SBNHH-1D65B	z	-48.1			-
(Not Shielded)	x	-			36.3
SBNHH-1D65B	z	-48.1			-
(Not Shielded)	x	-			36.3
2x60 700 RRH	z	N/A			-
(Shielded)	x	-			10.7
OVP	z	-23.5			-
(Not Shielded)	x	-			17.2

Positions 5 - 12

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-33.4	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			35.1
SC-E 6014	z	-22.0			-
(Not Shielded)	x	-			22.3
SBNHH-1D65B	z	-39.2			-
(Not Shielded)	x	-			45.1
SBNHH-1D65B	z	-39.2			-
(Not Shielded)	x	-			45.1

Dewberry

Job Number	50103542
Made by:	DAP
Date:	5/13/19
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(Rocky Hill CT) - Load Input for STAAD Model

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STAAD Load Input (Service Wind Load)

Positions 1 - 4

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-33.0	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			29.1
SBNHH-1D65B	z	-43.8			-
(Not Shielded)	x	-			29.0
SBNHH-1D65B	z	-43.8			-
(Not Shielded)	x	-			29.0
2x60 700 RRH	z	N/A			-
(Shielded)	x	-			7.1
OVP	z	-20.3			-
(Not Shielded)	x	-			13.4

Positions 5 - 12

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-30.1	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			32.0
SC-E 6014	z	-17.3			-
(Not Shielded)	x	-			17.6
SBNHH-1D65B	z	-32.7			-
(Not Shielded)	x	-			40.1
SBNHH-1D65B	z	-32.7			-
(Not Shielded)	x	-			40.1

Dewberry

Job Number	50103542
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Date:	5/13/19
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Date:	5/16/19

(Rocky Hill CT) - Load Input for STAAD Model

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STAAD Load Input (Maintenance Wind Load)

Positions 1 - 4

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-9.3	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			8.2
SBNHH-1D65B	z	-12.3			-
(Not Shielded)	x	-			8.1
SBNHH-1D65B	z	-12.3			-
(Not Shielded)	x	-			8.1
2x60 700 RRH	z	N/A			-
(Shielded)	x	-			2.0
OVP	z	-5.7			-
(Not Shielded)	x	-			3.8

Positions 5 - 12

Equipment	Axis	Case #1 0°	Case #2 30°	Case #3 60°	Case #4 90°
LPA-80063-4CF-F	z	-8.4	Same as Case #4	Same as Case #1	-
(Not Shielded)	x	-			9.0
SC-E 6014	z	-4.8			-
(Not Shielded)	x	-			4.9
SBNHH-1D65B	z	-9.2			-
(Not Shielded)	x	-			11.2
SBNHH-1D65B	z	-9.2			-
(Not Shielded)	x	-			11.2

Job Information

	Engineer	Checked	Approved
Name:	DAP	BK	
Date:	5/13/19	5/16/19	

Project ID	
Project Name	

Structure Type	SPACE FRAME

Number of Nodes	97	Highest Node	136
Number of Elements	112	Highest Beam	201
Number of Plates	18	Highest Plate	174

Number of Basic Load Cases	12
Number of Combination Load Cases	18

Included in this printout are data for:

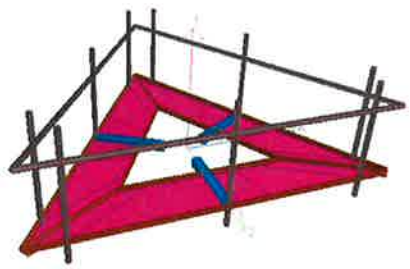
All	The Whole Structure

Included in this printout are results for load cases:

Type	L/C	Name
Primary	1	DEAD
Primary	2	WL#1
Primary	3	WL#4
Primary	4	DI
Primary	5	WLI#1
Primary	6	WLI#4
Primary	7	WLS#1
Primary	8	WLS#4
Primary	9	WLM#1
Primary	10	WLM#4
Primary	11	LM
Primary	12	LV
Combination	13	1.2D+1.6WL#1
Combination	14	1.2D+1.6WL#4
Combination	15	1.2D-1.6WL#1
Combination	16	1.2D-1.6WL#4
Combination	17	1.2D+1.0DI+1.0WI#1
Combination	18	1.2D+1.0DI+1.0WI#4
Combination	19	1.2D+1.0DI-1.0WI#1
Combination	20	1.2D+1.0DI-1.0WI#4
Combination	21	1.0D+1.0WLS#1
Combination	22	1.0D+1.0WLS#4
Combination	23	1.0D-1.0WLS#1
Combination	24	1.0D-1.0WLS#4

Job Information Cont...

Type	L/C	Name
Combination	28	1.2D+1.5LM-1.0WM#1
Combination	29	1.2D+1.5LM-1.0WM#4
Combination	30	1.2D+1.5LV



3D Rendered View

Nodes

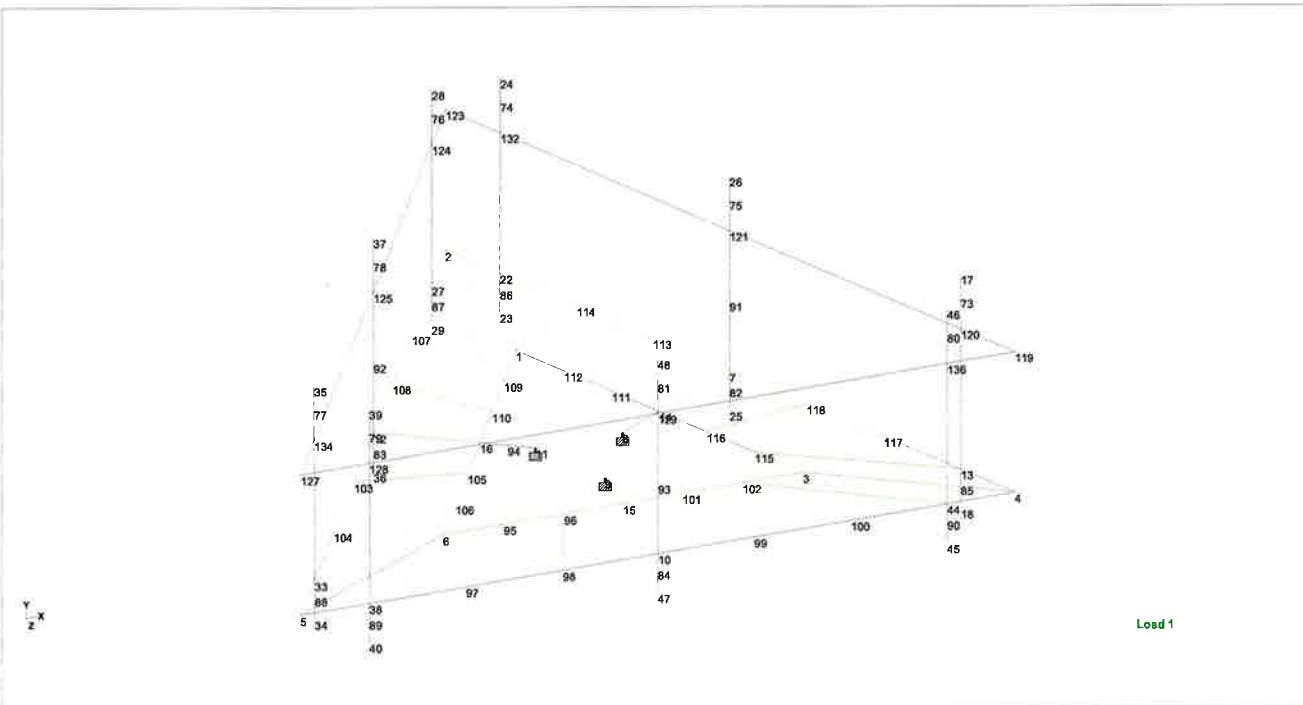
Node	X (ft)	Y (ft)	Z (ft)
1	-0.000	0.000	-4.083
2	-0.000	0.000	-8.083
3	3.536	0.000	2.041
4	7.000	0.000	4.041
5	-7.000	0.000	4.041
6	-3.536	0.000	2.041
7	3.500	0.000	-2.021
8	0.866	0.000	-0.500
9	-0.000	0.000	1.000
10	-0.000	0.000	4.041
11	-0.866	0.000	-0.500
12	-3.500	0.000	-2.021
13	6.333	0.000	2.887
14	1.768	0.000	-1.021
15	-0.000	0.000	2.041
16	-1.768	0.000	-1.021
17	6.333	4.167	2.887
18	6.333	-0.833	2.887
22	0.667	0.000	-6.928
23	0.667	-0.833	-6.928

Nodes Cont...

Node	X (ft)	Y (ft)	Z (ft)
24	0.667	4.167	-6.928
25	3.500	-0.833	-2.021
26	3.500	4.167	-2.021
27	-0.667	0.000	-6.928
28	-0.667	4.167	-6.928
29	-0.667	-0.833	-6.928
33	-6.333	0.000	2.887
34	-6.333	-0.833	2.887
35	-6.333	4.167	2.887
36	-3.500	-0.833	-2.021
37	-3.500	4.167	-2.021
38	-5.667	0.000	4.041
39	-5.667	4.167	4.041
40	-5.667	-0.833	4.041
44	5.667	0.000	4.041
45	5.667	-0.833	4.041
46	5.667	4.167	4.041
47	-0.000	-0.833	4.041
48	-0.000	4.167	4.041
73	6.333	3.667	2.887
74	0.667	3.667	-6.928
75	3.500	3.667	-2.021
76	-0.667	3.667	-6.928
77	-6.333	3.667	2.887
78	-3.500	3.667	-2.021
79	-5.667	3.667	4.041
80	5.667	3.667	4.041
81	-0.000	3.667	4.041
82	3.500	-0.333	-2.021
83	-3.500	-0.333	-2.021
84	-0.000	-0.333	4.041
85	6.333	-0.333	2.887
86	0.667	-0.333	-6.928
87	-0.667	-0.333	-6.928
88	-6.333	-0.333	2.887
89	-5.667	-0.333	4.041
90	5.667	-0.333	4.041
91	3.500	1.500	-2.021
92	-3.500	1.500	-2.021
93	-0.000	1.500	4.041
94	-1.317	0.000	-0.760
95	-2.357	0.000	2.041
96	-1.179	0.000	2.041
97	-3.778	0.000	4.041
98	-1.889	0.000	4.041

Nodes Cont...

Node	X (ft)	Y (ft)	Z (ft)
99	1.889	0.000	4.041
100	3.778	0.000	4.041
101	1.179	0.000	2.041
102	2.357	0.000	2.041
103	-4.444	0.000	-0.385
104	-5.389	0.000	1.251
105	-2.357	0.000	-0.000
106	-2.947	0.000	1.021
107	-1.611	0.000	-5.292
108	-2.556	0.000	-3.657
109	-0.589	0.000	-3.062
110	-1.179	0.000	-2.041
111	1.179	0.000	-2.041
112	0.589	0.000	-3.062
113	2.556	0.000	-3.657
114	1.611	0.000	-5.292
115	2.947	0.000	1.021
116	2.357	0.000	-0.000
117	5.389	0.000	1.251
118	4.444	0.000	-0.385
119	7.000	3.000	4.041
120	6.333	3.000	2.887
121	3.500	3.000	-2.021
123	-0.000	3.000	-8.083
124	-0.667	3.000	-6.928
125	-3.500	3.000	-2.021
127	-7.000	3.000	4.041
128	-5.667	3.000	4.041
129	-0.000	3.000	4.041
132	0.667	3.000	-6.928
134	-6.333	3.000	2.887
136	5.667	3.000	4.041



Node Layout

Beams

Beam	Node A	Node B	Length (ft)	Property	β (degrees)
1	1	2	4.000	4	180
2	3	4	4.000	4	180
3	5	6	4.000	4	180
4	7	14	2.000	2	0
5	9	15	1.041	2	0
6	11	94	0.521	2	0
7	18	85	0.500	5	0
9	23	86	0.500	5	0
10	25	82	0.500	5	0
11	29	87	0.500	5	0
13	34	88	0.500	5	0
14	36	83	0.500	5	0
15	40	89	0.500	5	0
17	45	90	0.500	5	0
18	47	84	0.500	5	0
19	4	13	1.333	3	135
20	7	113	1.889	3	135
21	2	27	1.333	3	135
22	12	103	1.889	3	135
23	5	38	1.333	3	135
24	10	99	1.889	3	135

Beams Cont...

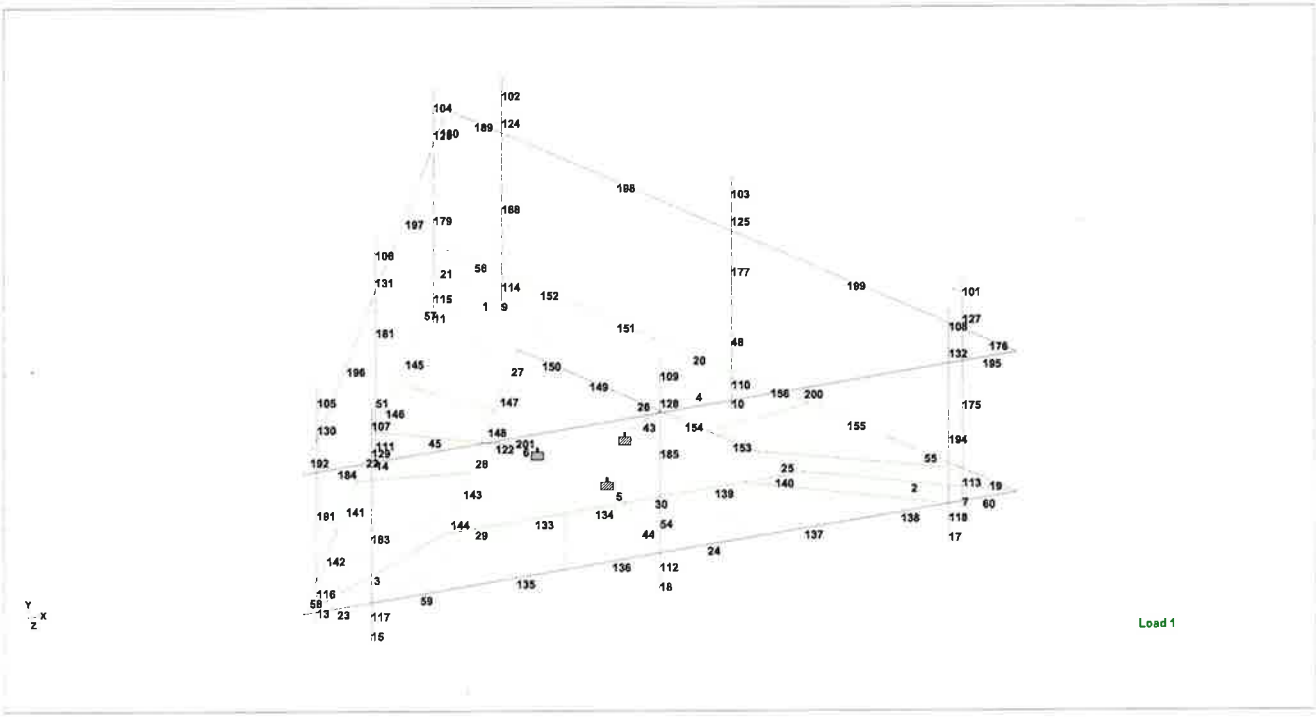
Beam	Node A	Node B	Length (ft)	Property	β (degrees)
25	3	115	1.179	3	225
26	14	111	1.179	3	225
27	1	109	1.179	3	225
28	16	105	1.179	3	225
29	6	95	1.179	3	225
30	15	101	1.179	3	225
43	14	8	1.041	2	0
44	15	10	2.000	2	0
45	16	12	2.000	2	0
48	7	91	1.500	5	0
51	12	92	1.500	5	0
54	10	93	1.500	5	0
55	13	117	1.889	3	135
56	22	2	1.333	3	135
57	27	107	1.889	3	135
58	33	5	1.333	3	135
59	38	97	1.889	3	135
60	44	4	1.333	3	135
101	73	17	0.500	5	0
102	74	24	0.500	5	0
103	75	26	0.500	5	0
104	76	28	0.500	5	0
105	77	35	0.500	5	0
106	78	37	0.500	5	0
107	79	39	0.500	5	0
108	80	46	0.500	5	0
109	81	48	0.500	5	0
110	82	7	0.333	5	0
111	83	12	0.333	5	0
112	84	10	0.333	5	0
113	85	13	0.333	5	0
114	86	22	0.333	5	0
115	87	27	0.333	5	0
116	88	33	0.333	5	0
117	89	38	0.333	5	0
118	90	44	0.333	5	0
122	94	16	0.521	2	0
123	76	124	0.667	5	0
124	74	132	0.667	5	0
125	75	121	0.667	5	0
127	73	120	0.667	5	0
128	81	129	0.667	5	0
129	79	128	0.667	5	0
130	77	134	0.667	5	0
131	78	125	0.667	5	0

Beams Cont...

Beam	Node A	Node B	Length (ft)	Property	β (degrees)
132	80	136	0.667	5	0
133	95	96	1.179	3	225
134	96	15	1.179	3	225
135	97	98	1.889	3	135
136	98	10	1.889	3	135
137	99	100	1.889	3	135
138	100	44	1.889	3	135
139	101	102	1.179	3	225
140	102	3	1.179	3	225
141	103	104	1.889	3	135
142	104	33	1.889	3	135
143	105	106	1.179	3	225
144	106	6	1.179	3	225
145	107	108	1.889	3	135
146	108	12	1.889	3	135
147	109	110	1.179	3	225
148	110	16	1.179	3	225
149	111	112	1.179	3	225
150	112	1	1.179	3	225
151	113	114	1.889	3	135
152	114	22	1.889	3	135
153	115	116	1.179	3	225
154	116	14	1.179	3	225
155	117	118	1.889	3	135
156	118	7	1.889	3	135
175	120	13	3.000	5	0
176	119	120	1.333	5	135
177	121	91	1.500	5	0
179	124	27	3.000	5	0
180	123	124	1.333	5	135
181	125	92	1.500	5	0
183	128	38	3.000	5	0
184	127	128	1.333	5	135
185	129	93	1.500	5	0
188	132	22	3.000	5	0
189	132	123	1.333	5	135
191	134	33	3.000	5	0
192	134	127	1.333	5	135
194	136	44	3.000	5	0
195	136	119	1.333	5	135
196	134	125	5.667	5	0
197	125	124	5.667	5	0
198	132	121	5.667	5	0
199	121	120	5.667	5	0
200	136	129	5.667	5	0

Beams Cont...

Beam	Node A	Node B	Length (ft)	Property	β (degrees)
201	129	128	5.667	5	0



Beam Layout

Plates

Plate	Node A	Node B	Node C	Node D	Property
157	5	38	95	6	1
158	38	98	96	95	1
159	98	10	15	96	1
160	10	99	101	15	1
161	99	44	102	101	1
162	102	44	4	3	1
163	115	3	4	13	1
164	116	115	13	118	1
165	14	116	118	7	1
166	111	14	7	113	1
167	112	111	113	22	1
168	2	1	112	22	1
169	2	27	109	1	1
170	27	108	110	109	1

Plates Cont...

Plate	Node A	Node B	Node C	Node D	Property
171	108	12	16	110	1
172	12	103	105	16	1
173	103	33	106	105	1
174	33	5	6	106	1

Section Properties

Prop	Section	Area (in^2)	I_{yy} (in^4)	I_{zz} (in^4)	J (in^4)	Material
2	HSST4X4X0.25	3.370	7.800	7.800	12.455	STEEL
3	L30304	1.440	1.982	0.506	0.031	STEEL
4	L30303 LD	2.180	3.393	1.928	0.026	STEEL
5	PIPS20	1.020	0.627	0.627	1.262	STEEL

Plate Thickness

Prop	Node A (in)	Node B (in)	Node C (in)	Node D (in)	Material
1	0.062	0.062	0.062	0.062	STEEL

Materials

Mat	Name	E (kip/in^2)	ν	Density (kip/in^3)	α (/°F)
1	STEEL	29E+3	0.300	0.000	6E -6
2	STAINLESSSTEEL	28E+3	0.300	0.000	10E -6
3	ALUMINUM	10E+3	0.330	0.000	13E -6
4	CONCRETE	3.15E+3	0.170	0.000	5E -6

Supports

Node	X (kip/in)	Y (kip/in)	Z (kip/in)	rX (kip-ft/deg)	rY (kip-ft/deg)	rZ (kip-ft/deg)
8	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed
9	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed
11	Fixed	Fixed	Fixed	Fixed	Fixed	Fixed

Releases

There is no data of this type.

Primary Load Cases

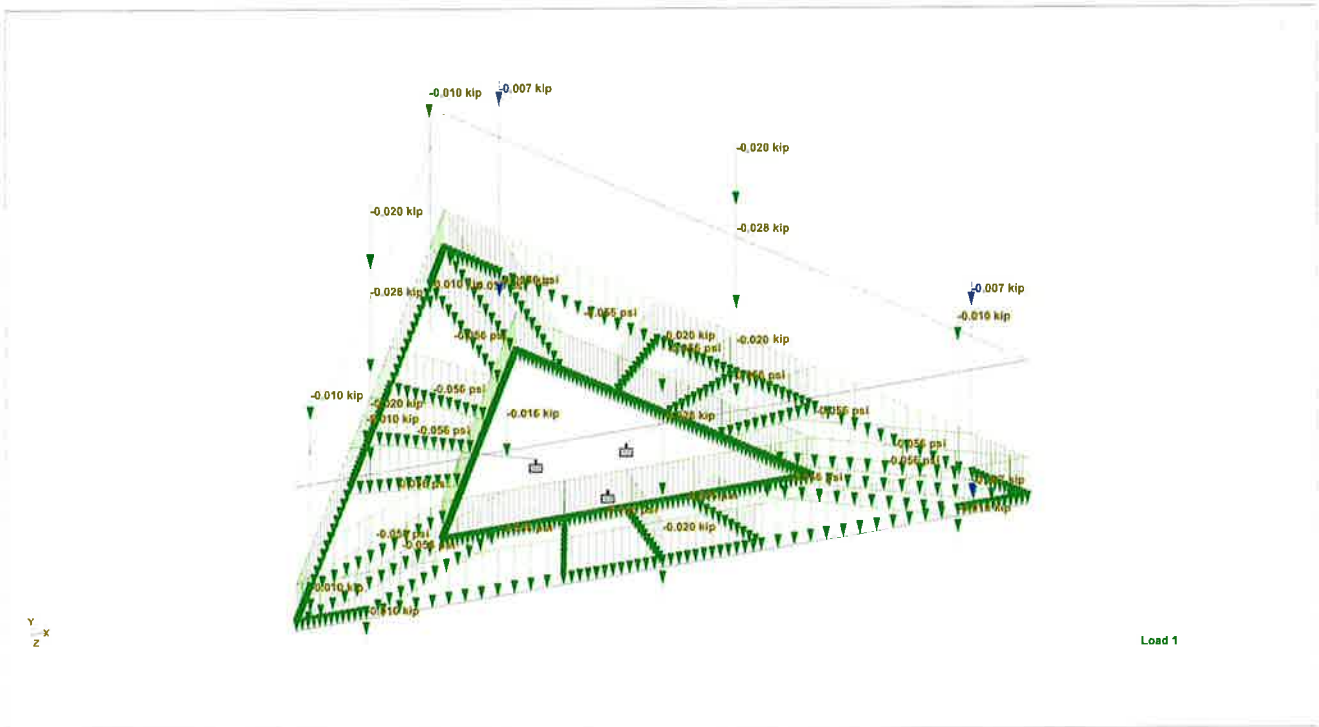
Number	Name	Type
1	DEAD	Dead
2	WL#1	Wind
3	WL#4	Wind
4	DI	Ice
5	WLI#1	Wind on Ice
6	WLI#4	Wind on Ice
7	WLS#1	Wind
8	WLS#4	Wind
9	WLM#1	Wind
10	WLM#4	Wind
11	LM	Live
12	LV	Live

Combination Load Cases

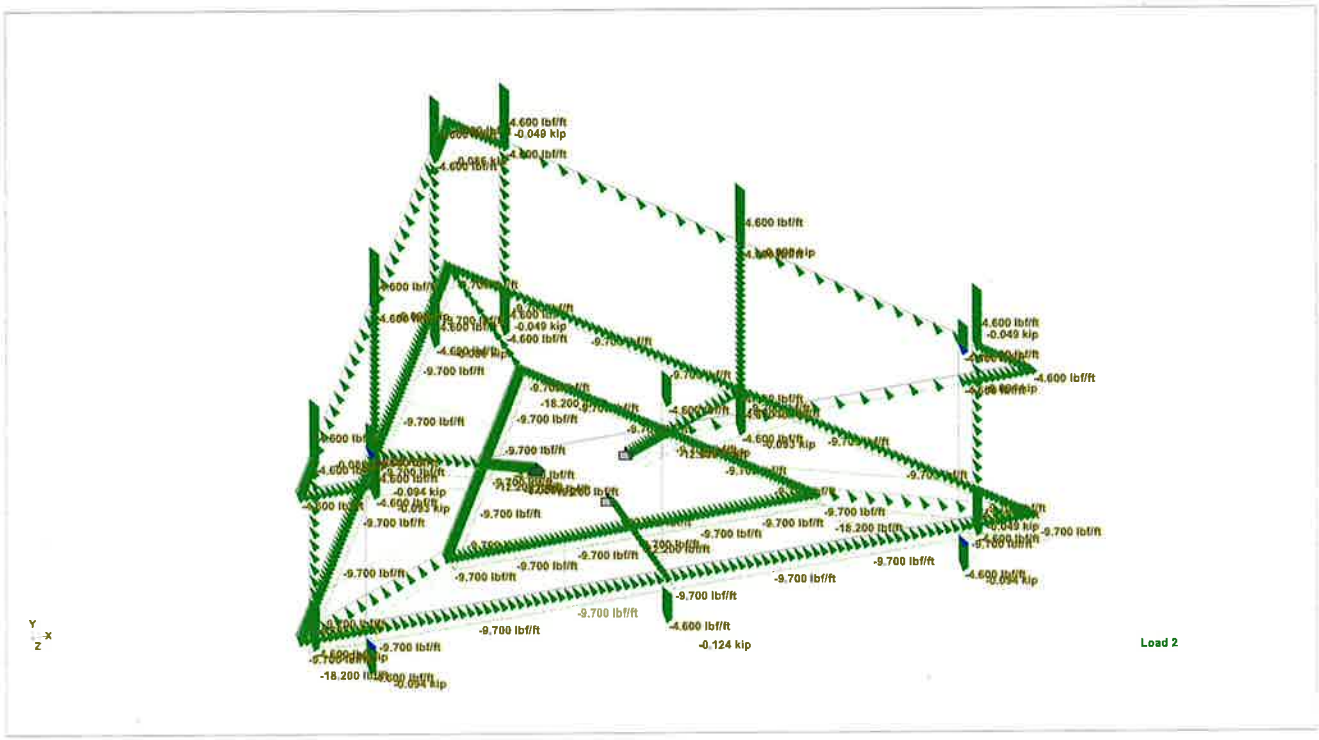
Comb.	Combination L/C Name	Primary	Primary L/C Name	Factor
13	1.2D+1.6WL#1	1	DEAD	1.20
		2	WL#1	1.60
14	1.2D+1.6WL#4	1	DEAD	1.20
		3	WL#4	1.60
15	1.2D-1.6WL#1	1	DEAD	1.20
		2	WL#1	-1.60
16	1.2D-1.6WL#4	1	DEAD	1.20
		3	WL#4	-1.60
17	1.2D+1.0DI+1.0WI#1	1	DEAD	1.20
		5	WLI#1	1.00
		4	DI	1.00
18	1.2D+1.0DI+1.0WI#4	1	DEAD	1.20
		6	WLI#4	1.00
		4	DI	1.00
19	1.2D+1.0DI-1.0WI#1	1	DEAD	1.20
		5	WLI#1	-1.00
		4	DI	1.00
20	1.2D+1.0DI-1.0WI#4	1	DEAD	1.20
		6	WLI#4	-1.00
		4	DI	1.00
21	1.0D+1.0WLS#1	1	DEAD	1.00
		7	WLS#1	1.00
22	1.0D+1.0WLS#4	1	DEAD	1.00
		8	WLS#4	1.00
23	1.0D-1.0WLS#1	1	DEAD	1.00
		7	WLS#1	-1.00
24	1.0D-1.0WLS#4	1	DEAD	1.00

Combination Load Cases Cont...

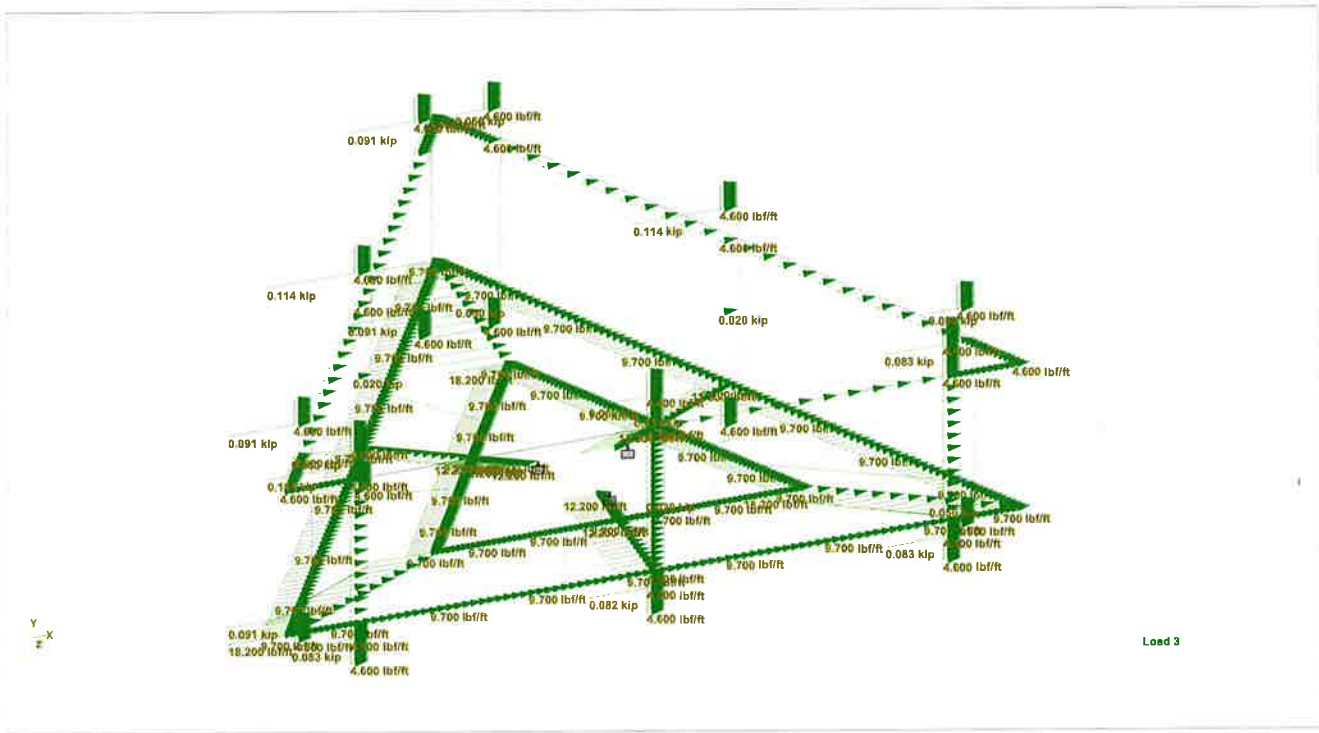
Comb.	Combination L/C Name	Primary	Primary L/C Name	Factor
		8	WLS#4	-1.00
25	1.4D	1	DEAD	1.40
26	1.2D+1.5LM+1.0WM#1	1	DEAD	1.20
		11	LM	1.50
		9	WLM#1	1.00
27	1.2D+1.5LM+1.0WM#4	1	DEAD	1.20
		11	LM	1.50
		10	WLM#4	1.00
28	1.2D+1.5LM-1.0WM#1	1	DEAD	1.20
		11	LM	1.50
		9	WLM#1	-1.00
29	1.2D+1.5LM-1.0WM#4	1	DEAD	1.20
		11	LM	1.50
		10	WLM#4	-1.00
30	1.2D+1.5LV	1	DEAD	1.20
		12	LV	1.50



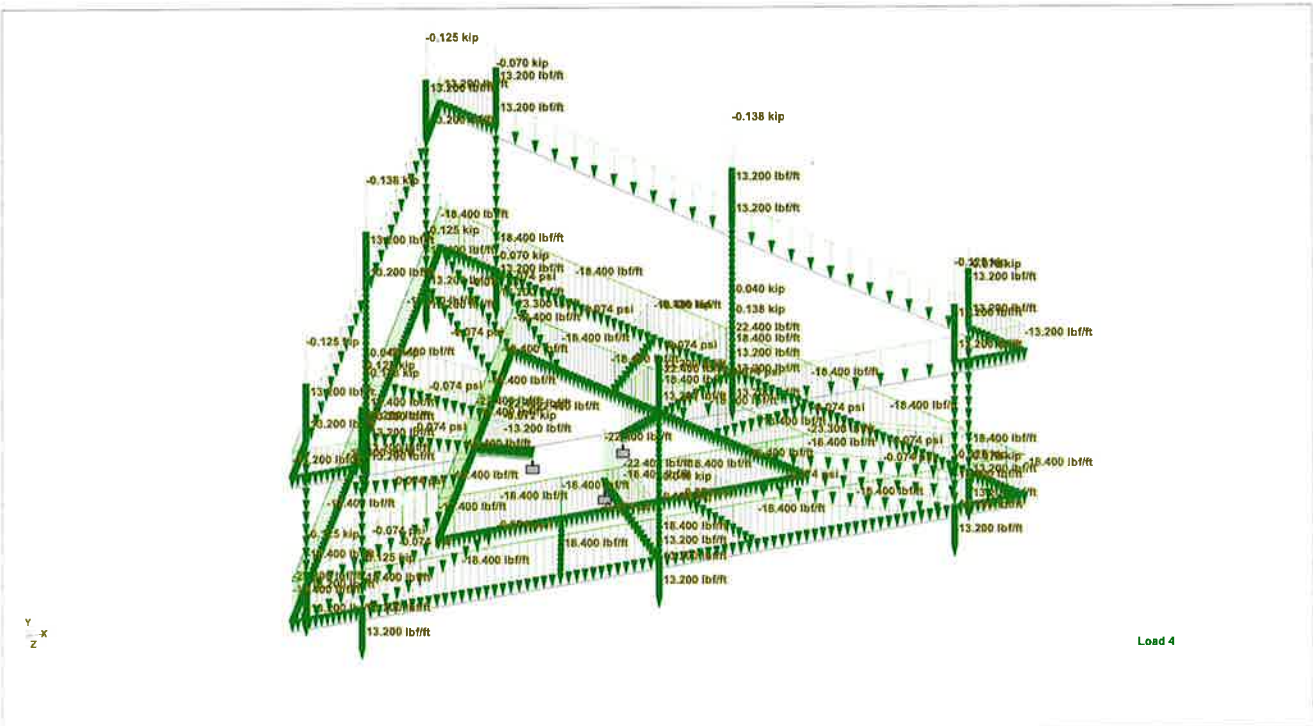
Dead Load



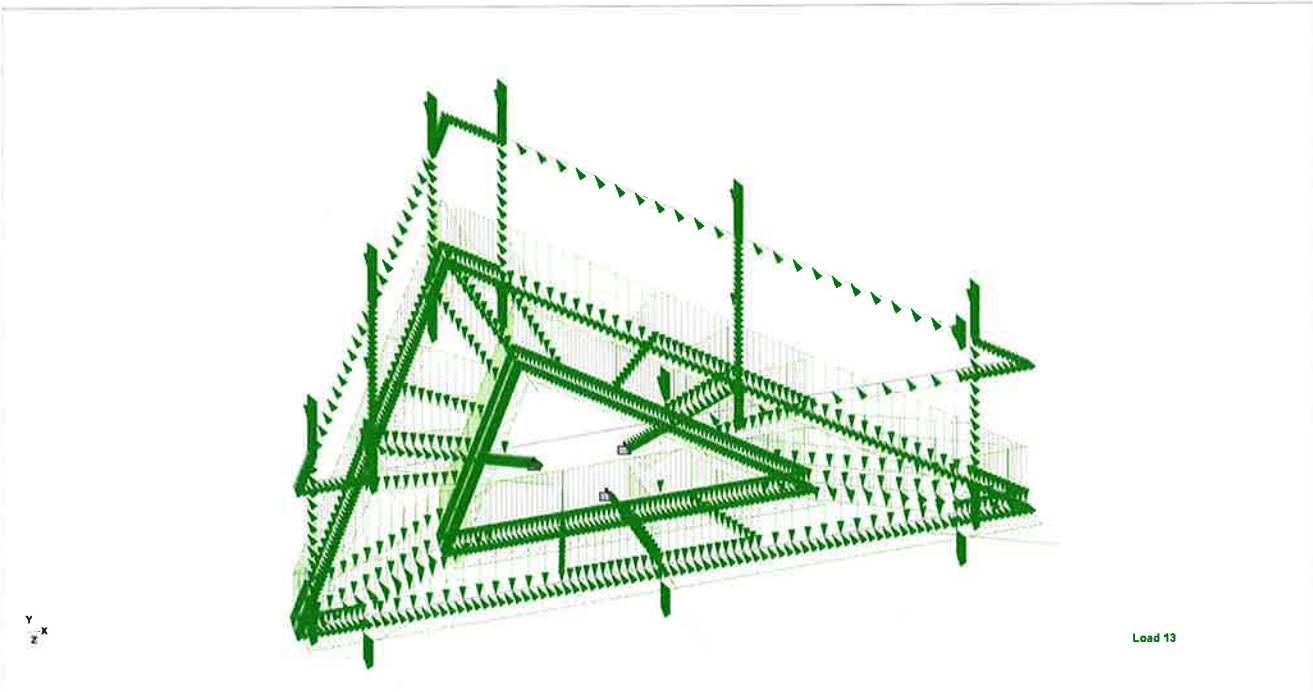
Wind Load (Z-Direction)



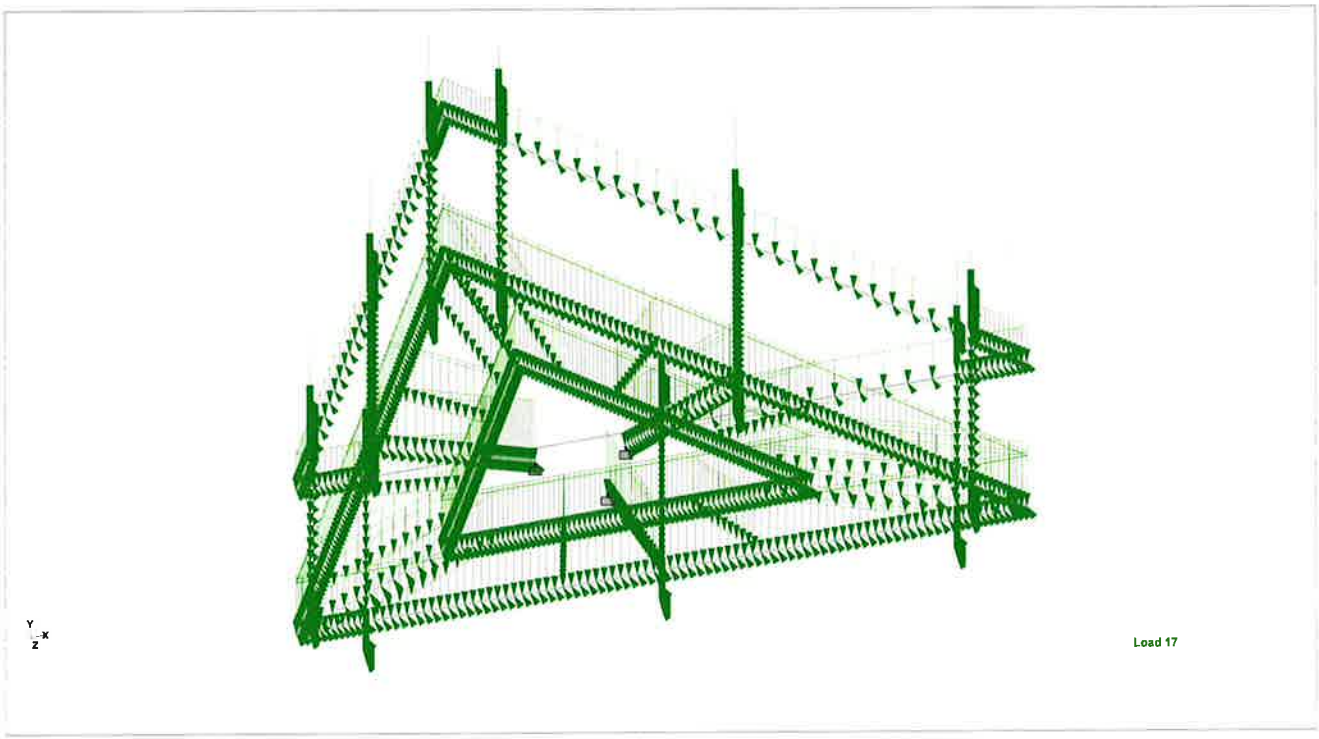
Wind Load (X-Direction)



Dead Ice Load



1.2D+1.6W (TYP.)



1.2D+1.0DI+1.0WI (TYP)

Beam Combined Axial and Bending Stresses Summary

Beam	L/C	Length (ft)	Max Comp Stress (psi)	Max Comp d (ft)	Max Comp Corner	Max Tens Stress (psi)	Max Tens d (ft)	Max Tens Corner
1	1:DEAD	4.000	991.802	4.000	3	-353.213	0.000	3
	2:WL#1	4.000	1.09E+3	0.000	1	-3.06E+3	0.000	3
	3:WL#4	4.000	316.702	0.000	1	-315.272	0.000	2
	4:DI	4.000	5.07E+3	4.000	3	-2.45E+3	0.000	3
	5:WLI#1	4.000	511.276	0.000	1	-1.44E+3	0.000	3
	6:WLI#4	4.000	150.449	0.000	1	-150.021	0.000	2
	7:WLS#1	4.000	381.681	0.000	1	-1.07E+3	0.000	3
	8:WLS#4	4.000	111.344	0.000	1	-110.841	0.000	2
	9:WLM#1	4.000	109.361	0.000	1	-307.732	0.000	3
	10:WLM#4	4.000	31.376	0.000	1	-31.232	0.000	2
	11:LM	4.000	22.756	0.000	3	-8.325	0.000	1
	12:LV	4.000	23.299	4.000	3	-10.939	0.000	3
	13:1.2D+1.6WI	4.000	1.93E+3	0.000	1	-5.32E+3	0.000	3
	14:1.2D+1.6WI	4.000	1.22E+3	4.000	3	-692.582	4.000	2
	15:1.2D-1.6WL	4.000	4.47E+3	0.000	3	-1.55E+3	0.000	1
	16:1.2D-1.6WL	4.000	1.19E+3	4.000	3	-656.131	4.000	1
	17:1.2D+1.0DI·	4.000	6.33E+3	4.000	3	-4.31E+3	0.000	3
	18:1.2D+1.0DI·	4.000	6.27E+3	4.000	3	-2.88E+3	0.000	3
	19:1.2D+1.0DI·	4.000	6.19E+3	4.000	3	-2.19E+3	4.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	20:1.2D+1.0DI·	4.000	6.25E+3	4.000	3	-2.88E+3	0.000	3
	21:1.0D+1.0WI	4.000	1.05E+3	4.000	3	-1.43E+3	0.000	3
	22:1.0D+1.0WI	4.000	998.758	4.000	3	-410.944	4.000	2
	23:1.0D-1.0WL	4.000	1.04E+3	2.667	3	-351.060	2.667	1
	24:1.0D-1.0WL	4.000	991.035	4.000	3	-390.550	4.000	1
	25:1.4D	4.000	1.39E+3	4.000	3	-494.499	0.000	3
	26:1.2D+1.5LM	4.000	1.21E+3	4.000	3	-697.445	0.000	3
	27:1.2D+1.5LM	4.000	1.2E+3	4.000	3	-439.796	4.000	2
	28:1.2D+1.5LM	4.000	1.18E+3	4.000	3	-412.358	4.000	2
	29:1.2D+1.5LM	4.000	1.19E+3	4.000	3	-418.188	4.000	1
	30:1.2D+1.5LV	4.000	1.23E+3	4.000	3	-440.215	0.000	3
2	1:DEAD	4.000	991.329	4.000	3	-352.794	0.000	3
	2:WL#1	4.000	1.55E+3	0.000	3	-797.473	0.000	1
	3:WL#4	4.000	1.08E+3	0.000	1	-2.6E+3	0.000	3
	4:DI	4.000	5.04E+3	4.000	3	-2.42E+3	0.000	3
	5:WLI#1	4.000	725.758	0.000	3	-379.536	0.000	1
	6:WLI#4	4.000	503.925	0.000	1	-1.21E+3	0.000	3
	7:WLS#1	4.000	542.902	0.000	3	-280.184	0.000	1
	8:WLS#4	4.000	378.779	0.000	1	-913.555	0.000	3
	9:WLM#1	4.000	155.479	0.000	3	-79.944	0.000	1
	10:WLM#4	4.000	107.923	0.000	1	-260.467	0.000	3
	11:LM	4.000	46.600	4.000	3	-21.873	0.000	3
	12:LV	4.000	11.378	0.000	3	-4.162	0.000	1
	13:1.2D+1.6WI	4.000	2.05E+3	0.000	3	-1.08E+3	0.000	1
	14:1.2D+1.6WI	4.000	1.92E+3	0.000	1	-4.59E+3	0.000	3
	15:1.2D-1.6WL	4.000	1.47E+3	0.000	1	-2.9E+3	0.000	3
	16:1.2D-1.6WL	4.000	3.74E+3	0.000	3	-1.53E+3	0.000	1
	17:1.2D+1.0DI·	4.000	6.21E+3	4.000	3	-2.22E+3	4.000	1
	18:1.2D+1.0DI·	4.000	6.31E+3	4.000	3	-4.05E+3	0.000	3
	19:1.2D+1.0DI·	4.000	6.27E+3	4.000	3	-3.57E+3	0.000	3
	20:1.2D+1.0DI·	4.000	6.16E+3	4.000	3	-2.2E+3	4.000	1
	21:1.0D+1.0WI	4.000	960.211	4.000	3	-381.467	4.000	1
	22:1.0D+1.0WI	4.000	1.05E+3	4.000	3	-1.27E+3	0.000	3
	23:1.0D-1.0WL	4.000	1.03E+3	4.000	3	-895.478	0.000	3
	24:1.0D-1.0WL	4.000	987.567	3.000	3	-352.983	2.667	1
	25:1.4D	4.000	1.39E+3	4.000	3	-493.911	0.000	3
	26:1.2D+1.5LM	4.000	1.25E+3	4.000	3	-441.738	4.000	1
	27:1.2D+1.5LM	4.000	1.28E+3	4.000	3	-716.629	0.000	3
	28:1.2D+1.5LM	4.000	1.27E+3	4.000	3	-611.283	0.000	3
	29:1.2D+1.5LM	4.000	1.24E+3	4.000	3	-444.121	4.000	2
	30:1.2D+1.5LV	4.000	1.19E+3	4.000	3	-420.034	4.000	2
3	1:DEAD	4.000	1.02E+3	0.000	3	-394.893	4.000	3
	2:WL#1	4.000	1.55E+3	4.000	3	-813.194	4.000	1
	3:WL#4	4.000	2.6E+3	4.000	3	-1.06E+3	4.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	4:DI	4.000	5.42E+3	0.000	3	-3.02E+3	4.000	3
	5:WLI#1	4.000	723.956	4.000	3	-381.615	4.000	1
	6:WLI#4	4.000	1.21E+3	4.000	3	-498.030	4.000	2
	7:WLS#1	4.000	543.913	4.000	3	-285.724	4.000	1
	8:WLS#4	4.000	914.637	4.000	3	-374.017	4.000	2
	9:WLM#1	4.000	155.694	4.000	3	-81.420	4.000	1
	10:WLM#4	4.000	260.772	4.000	3	-106.565	4.000	2
	11:LM	4.000	46.597	0.000	3	-21.874	4.000	3
	12:LV	4.000	23.299	0.000	3	-10.937	4.000	3
	13:1.2D+1.6WI	4.000	2E+3	4.000	3	-1.09E+3	4.000	1
	14:1.2D+1.6WI	4.000	3.69E+3	4.000	3	-1.49E+3	4.000	2
	15:1.2D-1.6WL	4.000	1.51E+3	4.000	1	-2.95E+3	4.000	3
	16:1.2D-1.6WL	4.000	1.92E+3	4.000	2	-4.64E+3	4.000	3
	17:1.2D+1.0DI-	4.000	6.6E+3	0.000	3	-2.79E+3	4.000	3
	18:1.2D+1.0DI-	4.000	6.56E+3	0.000	3	-2.33E+3	0.000	1
	19:1.2D+1.0DI-	4.000	6.68E+3	0.000	3	-4.22E+3	4.000	3
	20:1.2D+1.0DI-	4.000	6.72E+3	0.000	3	-4.7E+3	4.000	3
	21:1.0D+1.0WI	4.000	976.988	0.000	3	-383.331	0.000	1
	22:1.0D+1.0WI	4.000	989.898	1.000	3	-350.637	0.000	1
	23:1.0D-1.0WL	4.000	1.06E+3	0.000	3	-938.806	4.000	3
	24:1.0D-1.0WL	4.000	1.09E+3	0.000	3	-1.31E+3	4.000	3
	25:1.4D	4.000	1.42E+3	0.000	3	-552.850	4.000	3
	26:1.2D+1.5LM	4.000	1.28E+3	0.000	3	-457.092	0.000	1
	27:1.2D+1.5LM	4.000	1.27E+3	0.000	3	-449.047	0.000	1
	28:1.2D+1.5LM	4.000	1.3E+3	0.000	3	-662.282	4.000	3
	29:1.2D+1.5LM	4.000	1.31E+3	0.000	3	-767.451	4.000	3
	30:1.2D+1.5LV	4.000	1.25E+3	0.000	3	-490.277	4.000	3
4	1:DEAD	2.000	2.3E+3	2.000	3	-2.24E+3	2.000	1
	2:WL#1	2.000	2.02E+3	2.000	4	-2.2E+3	2.000	2
	3:WL#4	2.000	3.34E+3	2.000	3	-3.68E+3	2.000	1
	4:DI	2.000	9.28E+3	2.000	3	-9.06E+3	2.000	1
	5:WLI#1	2.000	964.701	2.000	4	-1.05E+3	2.000	2
	6:WLI#4	2.000	1.52E+3	2.000	3	-1.68E+3	2.000	1
	7:WLS#1	2.000	707.843	2.000	4	-772.009	2.000	2
	8:WLS#4	2.000	1.17E+3	2.000	3	-1.29E+3	2.000	1
	9:WLM#1	2.000	202.962	2.000	4	-221.308	2.000	2
	10:WLM#4	2.000	333.915	2.000	3	-367.694	2.000	1
	11:LM	2.000	6.536	0.000	1	-6.225	0.000	3
	12:LV	2.000	3.266	0.000	1	-3.111	0.000	3
	13:1.2D+1.6WI	2.000	5.99E+3	2.000	4	-6.21E+3	2.000	2
	14:1.2D+1.6WI	2.000	8.11E+3	2.000	3	-8.58E+3	2.000	1
	15:1.2D-1.6WL	2.000	2.05E+3	0.000	1	-1.7E+3	0.000	3
	16:1.2D-1.6WL	2.000	3.49E+3	0.000	1	-2.9E+3	0.000	3
	17:1.2D+1.0DI-	2.000	13E+3	2.000	3	-12.8E+3	2.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	18:1.2D+1.0DI·	2.000	13.6E+3	2.000	3	-13.4E+3	2.000	1
	19:1.2D+1.0DI·	2.000	11.3E+3	2.000	3	-10.9E+3	2.000	1
	20:1.2D+1.0DI·	2.000	10.7E+3	2.000	3	-10.2E+3	2.000	1
	21:1.0D+1.0WI	2.000	3.01E+3	2.000	3	-3.01E+3	2.000	1
	22:1.0D+1.0WI	2.000	3.48E+3	2.000	3	-3.54E+3	2.000	1
	23:1.0D-1.0WL	2.000	1.76E+3	2.000	3	-1.63E+3	2.000	1
	24:1.0D-1.0WL	2.000	1.24E+3	2.000	3	-1.06E+3	2.000	1
	25:1.4D	2.000	3.22E+3	2.000	3	-3.14E+3	2.000	1
	26:1.2D+1.5LM	2.000	2.97E+3	2.000	3	-2.92E+3	2.000	1
	27:1.2D+1.5LM	2.000	3.09E+3	2.000	3	-3.05E+3	2.000	1
	28:1.2D+1.5LM	2.000	2.6E+3	2.000	3	-2.51E+3	2.000	1
	29:1.2D+1.5LM	2.000	2.46E+3	2.000	3	-2.36E+3	2.000	1
	30:1.2D+1.5LV	2.000	2.77E+3	2.000	3	-2.69E+3	2.000	1
5	1:DEAD	1.041	4.18E+3	0.000	3	-4.14E+3	0.000	1
	2:WL#1	1.041	5.66E+3	0.000	1	-4.91E+3	0.000	3
	3:WL#4	1.041	1.86E+3	0.000	1	-1.86E+3	0.000	2
	4:DI	1.041	16.5E+3	0.000	3	-16.3E+3	0.000	1
	5:WLI#1	1.041	2.53E+3	0.000	1	-2.17E+3	0.000	3
	6:WLI#4	1.041	965.344	0.000	1	-963.530	0.000	2
	7:WLS#1	1.041	1.99E+3	0.000	1	-1.73E+3	0.000	3
	8:WLS#4	1.041	654.451	0.000	1	-653.364	0.000	2
	9:WLM#1	1.041	567.088	0.000	1	-492.317	0.000	3
	10:WLM#4	1.041	188.073	0.000	1	-187.762	0.000	2
	11:LM	1.041	1.55E+3	0.000	3	-1.55E+3	0.000	1
	12:LV	1.041	3.364	0.000	3	-3.415	0.000	1
	13:1.2D+1.6WI	1.041	4.22E+3	1.041	1	-3E+3	1.041	3
	14:1.2D+1.6WI	1.041	8E+3	0.000	3	-7.95E+3	0.000	1
	15:1.2D-1.6WL	1.041	12.9E+3	0.000	3	-14E+3	0.000	1
	16:1.2D-1.6WL	1.041	7.96E+3	0.000	3	-7.92E+3	0.000	1
	17:1.2D+1.0DI·	1.041	19.5E+3	0.000	3	-18.9E+3	0.000	1
	18:1.2D+1.0DI·	1.041	22.5E+3	0.000	3	-22.3E+3	0.000	1
	19:1.2D+1.0DI·	1.041	23.7E+3	0.000	3	-23.8E+3	0.000	1
	20:1.2D+1.0DI·	1.041	22.5E+3	0.000	3	-22.3E+3	0.000	1
	21:1.0D+1.0WI	1.041	2.59E+3	0.000	3	-2.3E+3	0.000	1
	22:1.0D+1.0WI	1.041	4.83E+3	0.000	3	-4.8E+3	0.000	1
	23:1.0D-1.0WL	1.041	5.91E+3	0.000	3	-6.13E+3	0.000	1
	24:1.0D-1.0WL	1.041	4.82E+3	0.000	3	-4.79E+3	0.000	1
	25:1.4D	1.041	5.85E+3	0.000	3	-5.8E+3	0.000	1
	26:1.2D+1.5LM	1.041	6.89E+3	0.000	3	-6.77E+3	0.000	1
	27:1.2D+1.5LM	1.041	7.53E+3	0.000	3	-7.49E+3	0.000	1
	28:1.2D+1.5LM	1.041	7.83E+3	0.000	3	-7.86E+3	0.000	1
	29:1.2D+1.5LM	1.041	7.53E+3	0.000	3	-7.48E+3	0.000	1
	30:1.2D+1.5LV	1.041	5.02E+3	0.000	3	-4.98E+3	0.000	1
6	1:DEAD	0.521	4.2E+3	0.000	3	-4.17E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	2:WL#1	0.521	4.27E+3	0.000	3	-4.63E+3	0.000	1
	3:WL#4	0.521	5.79E+3	0.000	1	-5.13E+3	0.000	3
	4:DI	0.521	17E+3	0.000	3	-16.9E+3	0.000	1
	5:WLI#1	0.521	1.99E+3	0.000	3	-2.18E+3	0.000	1
	6:WLI#4	0.521	2.64E+3	0.000	1	-2.32E+3	0.000	3
	7:WLS#1	0.521	1.5E+3	0.000	3	-1.63E+3	0.000	1
	8:WLS#4	0.521	2.03E+3	0.000	1	-1.8E+3	0.000	3
	9:WLM#1	0.521	428.400	0.000	3	-465.387	0.000	1
	10:WLM#4	0.521	577.927	0.000	1	-512.061	0.000	3
	11:LM	0.521	6.731	0.000	3	-6.832	0.000	1
	12:LV	0.521	775.729	0.000	3	-775.524	0.000	1
	13:1.2D+1.6WI	0.521	11.9E+3	0.000	3	-12.4E+3	0.000	1
	14:1.2D+1.6WI	0.521	4.26E+3	0.000	1	-3.16E+3	0.000	4
	15:1.2D-1.6WL	0.521	4.52E+3	0.000	1	-3.88E+3	0.000	2
	16:1.2D-1.6WL	0.521	13.2E+3	0.000	3	-14.3E+3	0.000	1
	17:1.2D+1.0DI-	0.521	24.1E+3	0.000	3	-24E+3	0.000	1
	18:1.2D+1.0DI-	0.521	20.7E+3	0.000	3	-20.2E+3	0.000	1
	19:1.2D+1.0DI-	0.521	21.9E+3	0.000	3	-21.5E+3	0.000	1
	20:1.2D+1.0DI-	0.521	24.4E+3	0.000	3	-24.5E+3	0.000	1
	21:1.0D+1.0WI	0.521	5.7E+3	0.000	3	-5.8E+3	0.000	1
	22:1.0D+1.0WI	0.521	3.17E+3	0.000	3	-2.9E+3	0.000	1
	23:1.0D-1.0WL	0.521	4.09E+3	0.000	3	-3.92E+3	0.000	1
	24:1.0D-1.0WL	0.521	6E+3	0.000	3	-6.2E+3	0.000	1
	25:1.4D	0.521	5.89E+3	0.000	3	-5.83E+3	0.000	1
	26:1.2D+1.5LM	0.521	5.48E+3	0.000	3	-5.48E+3	0.000	1
	27:1.2D+1.5LM	0.521	4.76E+3	0.000	3	-4.65E+3	0.000	1
	28:1.2D+1.5LM	0.521	5.01E+3	0.000	3	-4.93E+3	0.000	1
	29:1.2D+1.5LM	0.521	5.56E+3	0.000	3	-5.58E+3	0.000	1
	30:1.2D+1.5LV	0.521	6.21E+3	0.000	3	-6.16E+3	0.000	1
7	1:DEAD	0.500				-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500	0.000	0.000		-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500				-0.000	0.500	
	12:LV	0.500	0.000	0.500				
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500				-2.331	0.500	
	26:1.2D+1.5LM	0.500				-3.421	0.500	
	27:1.2D+1.5LM	0.500				-3.421	0.500	
	28:1.2D+1.5LM	0.500				-3.421	0.500	
	29:1.2D+1.5LM	0.500				-3.421	0.500	
	30:1.2D+1.5LV	0.500				-1.998	0.500	
9	1:DEAD	0.500	0.000	0.000		-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500	0.000	0.000		-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.000				
	12:LV	0.500				-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500	0.000	0.000		-2.331	0.500	
	26:1.2D+1.5LM	0.500				-3.421	0.500	
	27:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	28:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	29:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	30:1.2D+1.5LV	0.500	0.000	0.000		-1.998	0.500	
10	1:DEAD	0.500				-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500	0.000	0.000		-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.500		-0.000	0.500	
	12:LV	0.500	0.000	0.500		-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500				-2.331	0.500	
	26:1.2D+1.5LM	0.500				-3.421	0.500	
	27:1.2D+1.5LM	0.500				-3.421	0.500	
	28:1.2D+1.5LM	0.500				-3.421	0.500	
	29:1.2D+1.5LM	0.500				-3.421	0.500	
	30:1.2D+1.5LV	0.500				-1.998	0.500	
11	1:DEAD	0.500	0.000	0.000		-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500	0.000	0.000		-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.500				
	12:LV	0.500	0.000	0.500		-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500	0.000	0.000		-2.331	0.500	
	26:1.2D+1.5LM	0.500				-3.421	0.500	
	27:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	28:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	29:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	30:1.2D+1.5LV	0.500	0.000	0.000		-1.998	0.500	
13	1:DEAD	0.500	0.000	0.000		-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500				-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500				-0.000	0.458	
	12:LV	0.500				-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500	0.000	0.000		-2.331	0.500	
	26:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	27:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	28:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	29:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	30:1.2D+1.5LV	0.500	0.000	0.000		-1.998	0.500	
14	1:DEAD	0.500	0.000	0.000		-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500	0.000	0.000		-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.500		-0.000	0.500	
	12:LV	0.500	0.000	0.500		-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500	0.000	0.000		-2.331	0.500	
	26:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	27:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	28:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	29:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	30:1.2D+1.5LV	0.500	0.000	0.000		-1.998	0.500	
15	1:DEAD	0.500				-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500				-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.458				

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	12:LV	0.500	0.000	0.500				
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500				-2.331	0.500	
	26:1.2D+1.5LM	0.500				-3.421	0.500	
	27:1.2D+1.5LM	0.500				-3.421	0.500	
	28:1.2D+1.5LM	0.500				-3.421	0.500	
	29:1.2D+1.5LM	0.500				-3.421	0.500	
	30:1.2D+1.5LV	0.500				-1.998	0.500	
17	1:DEAD	0.500	0.000	0.000		-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500	0.000	0.000		-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.458				
	12:LV	0.500				-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500	0.000	0.000		-2.331	0.500	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	26:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	27:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	28:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	29:1.2D+1.5LM	0.500	0.000	0.000		-3.421	0.500	
	30:1.2D+1.5LV	0.500	0.000	0.000		-1.998	0.500	
18	1:DEAD	0.500				-1.665	0.500	
	2:WL#1	0.500	13.096	0.500		-13.096	0.500	
	3:WL#4	0.500	13.096	0.500		-13.096	0.500	
	4:DI	0.500				-6.471	0.500	
	5:WLI#1	0.500	9.679	0.500		-9.679	0.500	
	6:WLI#4	0.500	9.679	0.500		-9.679	0.500	
	7:WLS#1	0.500	4.555	0.500		-4.555	0.500	
	8:WLS#4	0.500	4.555	0.500		-4.555	0.500	
	9:WLM#1	0.500	1.423	0.500		-1.423	0.500	
	10:WLM#4	0.500	1.423	0.500		-1.423	0.500	
	11:LM	0.500	0.000	0.500				
	12:LV	0.500	0.000	0.500		-0.000	0.500	
	13:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	14:1.2D+1.6WI	0.500	18.955	0.500		-22.951	0.500	
	15:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	16:1.2D-1.6WL	0.500	18.955	0.500		-22.951	0.500	
	17:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	18:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	19:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	20:1.2D+1.0DI·	0.500	1.211	0.500		-18.148	0.500	
	21:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	22:1.0D+1.0WI	0.500	2.890	0.500		-6.220	0.500	
	23:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	24:1.0D-1.0WL	0.500	2.890	0.500		-6.220	0.500	
	25:1.4D	0.500				-2.331	0.500	
	26:1.2D+1.5LM	0.500				-3.421	0.500	
	27:1.2D+1.5LM	0.500				-3.421	0.500	
	28:1.2D+1.5LM	0.500				-3.421	0.500	
	29:1.2D+1.5LM	0.500				-3.421	0.500	
	30:1.2D+1.5LV	0.500				-1.998	0.500	
19	1:DEAD	1.333	559.224	1.333	3	-1.04E+3	1.333	2
	2:WL#1	1.333	331.106	1.333	2	-593.401	1.333	1
	3:WL#4	1.333	970.635	1.333	2	-656.302	1.333	1
	4:DI	1.333	2.37E+3	1.333	3	-4.47E+3	1.333	2
	5:WLI#1	1.333	99.494	1.333	3	-262.236	1.333	1
	6:WLI#4	1.333	418.504	1.333	2	-303.835	1.333	1
	7:WLS#1	1.333	116.997	1.333	2	-208.607	1.333	1
	8:WLS#4	1.333	341.108	1.333	2	-230.558	1.333	1
	9:WLM#1	1.333	31.809	1.333	2	-59.402	1.333	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	10:WLM#4	1.333	96.736	1.333	2	-65.683	1.333	1
	11:LM	1.333	16.323	0.000	1	-39.500	0.000	2
	12:LV	1.333	1.941	1.333	3	-3.966	1.333	2
	13:1.2D+1.6WI	1.333	898.271	1.333	3	-764.706	1.333	1
	14:1.2D+1.6WI	1.333	614.700	0.000	1	-1.16E+3	0.000	2
	15:1.2D-1.6WL	1.333	1.13E+3	1.333	1	-1.78E+3	1.333	2
	16:1.2D-1.6WL	1.333	1.23E+3	1.333	1	-2.8E+3	1.333	2
	17:1.2D+1.0DI·	1.333	3.14E+3	1.333	3	-5.62E+3	1.333	2
	18:1.2D+1.0DI·	1.333	2.96E+3	1.333	3	-5.3E+3	1.333	2
	19:1.2D+1.0DI·	1.333	2.94E+3	1.333	3	-5.82E+3	1.333	2
	20:1.2D+1.0DI·	1.333	3.12E+3	1.333	3	-6.14E+3	1.333	2
	21:1.0D+1.0WI	1.333	608.733	1.333	3	-924.827	1.333	2
	22:1.0D+1.0WI	1.333	482.091	1.333	3	-843.325	0.000	2
	23:1.0D-1.0WL	1.333	509.715	1.333	3	-1.16E+3	1.333	2
	24:1.0D-1.0WL	1.333	636.357	1.333	3	-1.38E+3	1.333	2
	25:1.4D	1.333	782.913	1.333	3	-1.46E+3	1.333	2
	26:1.2D+1.5LM	1.333	704.104	1.333	3	-1.26E+3	1.333	2
	27:1.2D+1.5LM	1.333	667.312	1.333	3	-1.19E+3	1.333	2
	28:1.2D+1.5LM	1.333	673.883	1.333	3	-1.32E+3	1.333	2
	29:1.2D+1.5LM	1.333	710.675	1.333	3	-1.39E+3	1.333	2
	30:1.2D+1.5LV	1.333	673.980	1.333	3	-1.26E+3	1.333	2
20	1:DEAD	1.889	1.79E+3	0.000	1	-4.25E+3	0.000	2
	2:WL#1	1.889	1.88E+3	0.000	1	-3.37E+3	0.000	2
	3:WL#4	1.889	2.69E+3	0.000	1	-5.24E+3	0.000	2
	4:DI	1.889	6.8E+3	0.000	1	-16.2E+3	0.000	2
	5:WLI#1	1.889	865.710	0.000	1	-1.53E+3	0.000	2
	6:WLI#4	1.889	1.27E+3	0.000	1	-2.41E+3	0.000	2
	7:WLS#1	1.889	658.887	0.000	1	-1.18E+3	0.000	2
	8:WLS#4	1.889	945.448	0.000	1	-1.84E+3	0.000	2
	9:WLM#1	1.889	188.015	0.000	1	-337.932	0.000	2
	10:WLM#4	1.889	269.999	0.000	1	-524.801	0.000	2
	11:LM	1.889	22.198	0.000	2	-10.875	0.000	1
	12:LV	1.889	13.402	0.000	1	-28.013	0.000	2
	13:1.2D+1.6WI	1.889	5.14E+3	0.000	1	-10.5E+3	0.000	2
	14:1.2D+1.6WI	1.889	6.45E+3	0.000	1	-13.5E+3	0.000	2
	15:1.2D-1.6WL	1.889	2.29E+3	1.889	2	-1.52E+3	1.889	3
	16:1.2D-1.6WL	1.889	3.28E+3	0.000	2	-2.17E+3	0.000	1
	17:1.2D+1.0DI·	1.889	9.81E+3	0.000	1	-22.9E+3	0.000	2
	18:1.2D+1.0DI·	1.889	10.2E+3	0.000	1	-23.8E+3	0.000	2
	19:1.2D+1.0DI·	1.889	8.08E+3	0.000	1	-19.8E+3	0.000	2
	20:1.2D+1.0DI·	1.889	7.68E+3	0.000	1	-18.9E+3	0.000	2
	21:1.0D+1.0WI	1.889	2.44E+3	0.000	1	-5.44E+3	0.000	2
	22:1.0D+1.0WI	1.889	2.73E+3	0.000	1	-6.09E+3	0.000	2
	23:1.0D-1.0WL	1.889	1.13E+3	0.000	1	-3.07E+3	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	24:1.0D-1.0WL	1.889	839.925	0.000	1	-2.41E+3	0.000	2
	25:1.4D	1.889	2.5E+3	0.000	1	-5.95E+3	0.000	2
	26:1.2D+1.5LM	1.889	2.31E+3	0.000	1	-5.41E+3	0.000	2
	27:1.2D+1.5LM	1.889	2.4E+3	0.000	1	-5.6E+3	0.000	2
	28:1.2D+1.5LM	1.889	1.94E+3	0.000	1	-4.73E+3	0.000	2
	29:1.2D+1.5LM	1.889	1.86E+3	0.000	1	-4.55E+3	0.000	2
	30:1.2D+1.5LV	1.889	2.16E+3	0.000	1	-5.15E+3	0.000	2
21	1:DEAD	1.333	539.803	1.333	3	-996.210	1.333	2
	2:WL#1	1.333	609.203	1.333	2	-248.265	1.333	1
	3:WL#4	1.333	874.757	1.333	1	-788.545	1.333	2
	4:DI	1.333	1.89E+3	1.333	3	-4.18E+3	0.000	2
	5:WLI#1	1.333	290.955	1.333	2	-126.473	1.333	1
	6:WLI#4	1.333	393.145	1.333	1	-335.689	1.333	2
	7:WLS#1	1.333	213.895	1.333	2	-87.189	1.333	1
	8:WLS#4	1.333	307.235	1.333	1	-277.024	1.333	2
	9:WLM#1	1.333	61.536	1.333	2	-25.076	1.333	1
	10:WLM#4	1.333	87.330	1.333	1	-78.146	1.333	2
	11:LM	1.333	3.882	1.333	3	-7.933	1.333	2
	12:LV	1.333	8.690	0.000	1	-20.554	0.000	2
	13:1.2D+1.6WI	1.333	529.239	0.000	1	-1.25E+3	0.000	2
	14:1.2D+1.6WI	1.333	1.56E+3	1.333	1	-2.46E+3	1.333	2
	15:1.2D-1.6WL	1.333	1.01E+3	1.333	3	-2.17E+3	1.333	2
	16:1.2D-1.6WL	1.333	667.948	1.333	3	-1.24E+3	1.333	1
	17:1.2D+1.0DI·	1.333	2.43E+3	1.333	3	-5.22E+3	0.000	2
	18:1.2D+1.0DI·	1.333	2.51E+3	1.333	3	-5.2E+3	0.000	2
	19:1.2D+1.0DI·	1.333	2.64E+3	1.333	3	-5.06E+3	0.000	2
	20:1.2D+1.0DI·	1.333	2.56E+3	1.333	3	-5.08E+3	0.000	2
	21:1.0D+1.0WI	1.333	459.693	1.333	3	-864.716	0.000	2
	22:1.0D+1.0WI	1.333	535.436	1.333	3	-1.27E+3	1.333	2
	23:1.0D-1.0WL	1.333	619.913	1.333	3	-1.21E+3	1.333	2
	24:1.0D-1.0WL	1.333	544.170	1.333	3	-753.206	0.000	2
	25:1.4D	1.333	755.724	1.333	3	-1.39E+3	1.333	2
	26:1.2D+1.5LM	1.333	630.510	1.333	3	-1.15E+3	1.333	2
	27:1.2D+1.5LM	1.333	651.892	1.333	3	-1.29E+3	1.333	2
	28:1.2D+1.5LM	1.333	676.663	1.333	3	-1.27E+3	1.333	2
	29:1.2D+1.5LM	1.333	655.281	1.333	3	-1.13E+3	1.333	2
	30:1.2D+1.5LV	1.333	653.019	1.333	3	-1.21E+3	1.333	2
22	1:DEAD	1.889	1.85E+3	0.000	1	-4.4E+3	0.000	2
	2:WL#1	1.889	1.49E+3	0.000	1	-3.02E+3	0.000	2
	3:WL#4	1.889	5.36E+3	0.000	2	-2.9E+3	0.000	1
	4:DI	1.889	7.79E+3	0.000	1	-18.6E+3	0.000	2
	5:WLI#1	1.889	713.713	0.000	1	-1.41E+3	0.000	2
	6:WLI#4	1.889	2.47E+3	0.000	2	-1.36E+3	0.000	1
	7:WLS#1	1.889	523.168	0.000	1	-1.06E+3	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	8:WLS#4	1.889	1.88E+3	0.000	2	-1.02E+3	0.000	1
	9:WLM#1	1.889	150.198	0.000	1	-303.687	0.000	2
	10:WLM#4	1.889	537.127	0.000	2	-290.719	0.000	1
	11:LM	1.889	26.798	0.000	1	-56.018	0.000	2
	12:LV	1.889	42.005	0.000	2	-17.828	0.000	1
	13:1.2D+1.6WI	1.889	4.6E+3	0.000	1	-10.1E+3	0.000	2
	14:1.2D+1.6WI	1.889	3.3E+3	0.000	2	-2.42E+3	0.000	1
	15:1.2D-1.6WL	1.889	648.811	1.889	3	-1.98E+3	1.889	2
	16:1.2D-1.6WL	1.889	6.86E+3	0.000	1	-13.9E+3	0.000	2
	17:1.2D+1.0DI·	1.889	10.7E+3	0.000	1	-25.3E+3	0.000	2
	18:1.2D+1.0DI·	1.889	8.65E+3	0.000	1	-21.4E+3	0.000	2
	19:1.2D+1.0DI·	1.889	9.29E+3	0.000	1	-22.5E+3	0.000	2
	20:1.2D+1.0DI·	1.889	11.4E+3	0.000	1	-26.3E+3	0.000	2
	21:1.0D+1.0WI	1.889	2.37E+3	0.000	1	-5.46E+3	0.000	2
	22:1.0D+1.0WI	1.889	830.347	0.000	1	-2.52E+3	0.000	2
	23:1.0D-1.0WL	1.889	1.33E+3	0.000	1	-3.34E+3	0.000	2
	24:1.0D-1.0WL	1.889	2.87E+3	0.000	1	-6.29E+3	0.000	2
	25:1.4D	1.889	2.59E+3	0.000	1	-6.16E+3	0.000	2
	26:1.2D+1.5LM	1.889	2.41E+3	0.000	1	-5.67E+3	0.000	2
	27:1.2D+1.5LM	1.889	1.97E+3	0.000	1	-4.83E+3	0.000	2
	28:1.2D+1.5LM	1.889	2.11E+3	0.000	1	-5.06E+3	0.000	2
	29:1.2D+1.5LM	1.889	2.55E+3	0.000	1	-5.9E+3	0.000	2
	30:1.2D+1.5LV	1.889	2.19E+3	0.000	1	-5.22E+3	0.000	2
23	1:DEAD	1.333	557.818	1.333	3	-1.02E+3	1.333	2
	2:WL#1	1.333	899.531	1.333	1	-1.07E+3	1.333	2
	3:WL#4	1.333	250.846	0.000	2	-245.187	1.333	1
	4:DI	1.333	2.37E+3	1.333	3	-4.43E+3	0.000	2
	5:WLI#1	1.333	392.201	1.333	1	-435.881	1.333	2
	6:WLI#4	1.333	108.062	0.000	1	-101.887	0.000	3
	7:WLS#1	1.333	316.179	1.333	1	-375.792	1.333	2
	8:WLS#4	1.333	88.181	0.000	2	-86.208	1.333	1
	9:WLM#1	1.333	89.872	1.333	1	-106.085	1.333	2
	10:WLM#4	1.333	24.947	0.000	2	-24.403	1.333	1
	11:LM	1.333	17.301	0.000	1	-41.129	0.000	2
	12:LV	1.333	8.156	0.000	1	-19.754	0.000	2
	13:1.2D+1.6WI	1.333	1.61E+3	1.333	1	-2.94E+3	1.333	2
	14:1.2D+1.6WI	1.333	862.410	1.333	3	-1.26E+3	1.333	2
	15:1.2D-1.6WL	1.333	659.822	0.000	1	-1.26E+3	1.333	1
	16:1.2D-1.6WL	1.333	673.405	0.000	1	-1.39E+3	0.000	2
	17:1.2D+1.0DI·	1.333	3.06E+3	1.333	3	-5.99E+3	1.333	2
	18:1.2D+1.0DI·	1.333	3.12E+3	1.333	3	-5.61E+3	1.333	2
	19:1.2D+1.0DI·	1.333	3.01E+3	1.333	3	-5.4E+3	0.000	2
	20:1.2D+1.0DI·	1.333	2.95E+3	1.333	3	-5.52E+3	0.000	2
	21:1.0D+1.0WI	1.333	597.619	1.333	3	-1.4E+3	1.333	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	22:1.0D+1.0WI	1.333	599.947	1.333	3	-1.03E+3	1.333	2
	23:1.0D-1.0WL	1.333	518.017	1.333	3	-821.902	0.000	2
	24:1.0D-1.0WL	1.333	515.688	1.333	3	-1.01E+3	1.333	2
	25:1.4D	1.333	780.945	1.333	3	-1.43E+3	1.333	2
	26:1.2D+1.5LM	1.333	690.621	1.333	3	-1.36E+3	1.333	2
	27:1.2D+1.5LM	1.333	692.353	1.333	3	-1.26E+3	1.333	2
	28:1.2D+1.5LM	1.333	669.044	1.333	3	-1.15E+3	1.333	2
	29:1.2D+1.5LM	1.333	667.312	1.333	3	-1.26E+3	1.333	2
	30:1.2D+1.5LV	1.333	678.351	1.333	3	-1.25E+3	1.333	2
24	1:DEAD	1.889	1.82E+3	0.000	1	-4.34E+3	0.000	2
	2:WL#1	1.889	6.16E+3	0.000	2	-3.26E+3	0.000	1
	3:WL#4	1.889	737.548	1.889	3	-1.42E+3	1.889	2
	4:DI	1.889	7.4E+3	0.000	1	-17.8E+3	0.000	2
	5:WLI#1	1.889	2.91E+3	0.000	2	-1.57E+3	0.000	1
	6:WLI#4	1.889	339.758	1.889	3	-652.143	1.889	2
	7:WLS#1	1.889	2.16E+3	0.000	2	-1.14E+3	0.000	1
	8:WLS#4	1.889	259.031	1.889	3	-500.023	1.889	2
	9:WLM#1	1.889	620.049	0.000	2	-327.968	0.000	1
	10:WLM#4	1.889	73.733	1.889	3	-142.193	1.889	2
	11:LM	1.889	84.021	0.000	2	-35.659	0.000	1
	12:LV	1.889	11.100	0.000	2	-5.437	0.000	1
	13:1.2D+1.6WI	1.889	4.65E+3	0.000	2	-3.03E+3	0.000	1
	14:1.2D+1.6WI	1.889	2.51E+3	0.000	1	-5.47E+3	0.000	2
	15:1.2D-1.6WL	1.889	7.39E+3	0.000	1	-15.1E+3	0.000	2
	16:1.2D-1.6WL	1.889	1.85E+3	0.000	1	-4.94E+3	0.000	2
	17:1.2D+1.0DI·	1.889	8.01E+3	0.000	1	-20.1E+3	0.000	2
	18:1.2D+1.0DI·	1.889	9.66E+3	0.000	1	-23E+3	0.000	2
	19:1.2D+1.0DI·	1.889	11.1E+3	0.000	1	-25.9E+3	0.000	2
	20:1.2D+1.0DI·	1.889	9.5E+3	0.000	1	-22.9E+3	0.000	2
	21:1.0D+1.0WI	1.889	699.226	0.000	1	-2.18E+3	0.000	2
	22:1.0D+1.0WI	1.889	1.89E+3	0.000	1	-4.4E+3	0.000	2
	23:1.0D-1.0WL	1.889	2.96E+3	0.000	1	-6.5E+3	0.000	2
	24:1.0D-1.0WL	1.889	1.75E+3	0.000	1	-4.28E+3	0.000	2
	25:1.4D	1.889	2.55E+3	0.000	1	-6.07E+3	0.000	2
	26:1.2D+1.5LM	1.889	1.8E+3	0.000	1	-4.46E+3	0.000	2
	27:1.2D+1.5LM	1.889	2.15E+3	0.000	1	-5.1E+3	0.000	2
	28:1.2D+1.5LM	1.889	2.46E+3	0.000	1	-5.7E+3	0.000	2
	29:1.2D+1.5LM	1.889	2.11E+3	0.000	1	-5.07E+3	0.000	2
	30:1.2D+1.5LV	1.889	2.17E+3	0.000	1	-5.19E+3	0.000	2
25	1:DEAD	1.179	690.137	1.179	2	-1.03E+3	1.179	1
	2:WL#1	1.179	1.48E+3	1.179	1	-924.610	1.179	3
	3:WL#4	1.179	1.79E+3	1.179	2	-2.91E+3	1.179	1
	4:DI	1.179	3.19E+3	1.179	2	-5.01E+3	1.179	1
	5:WLI#1	1.179	689.376	1.179	1	-432.082	1.179	3

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	6:WLI#4	1.179	839.920	1.179	2	-1.34E+3	1.179	1
	7:WLS#1	1.179	518.196	1.179	1	-324.743	1.179	3
	8:WLS#4	1.179	628.181	1.179	2	-1.02E+3	1.179	1
	9:WLM#1	1.179	148.389	1.179	1	-93.067	1.179	3
	10:WLM#4	1.179	179.120	1.179	2	-291.039	1.179	1
	11:LM	1.179	24.401	1.179	2	-56.326	1.179	1
	12:LV	1.179	12.274	1.179	1	-5.038	1.179	3
	13:1.2D+1.6WI	1.179	1.66E+3	0.000	1	-1.16E+3	1.179	3
	14:1.2D+1.6WI	1.179	3.69E+3	1.179	2	-5.89E+3	1.179	1
	15:1.2D-1.6WL	1.179	1.79E+3	1.179	3	-3.6E+3	1.179	1
	16:1.2D-1.6WL	1.179	3.41E+3	1.179	1	-2.03E+3	1.179	2
	17:1.2D+1.0DI·	1.179	4.07E+3	1.179	2	-5.56E+3	1.179	1
	18:1.2D+1.0DI·	1.179	4.86E+3	1.179	2	-7.58E+3	1.179	1
	19:1.2D+1.0DI·	1.179	3.97E+3	1.179	2	-6.94E+3	1.179	1
	20:1.2D+1.0DI·	1.179	3.18E+3	1.179	2	-4.91E+3	1.179	1
	21:1.0D+1.0WI	1.179	718.146	1.179	2	-514.974	1.179	1
	22:1.0D+1.0WI	1.179	1.32E+3	1.179	2	-2.05E+3	1.179	1
	23:1.0D-1.0WL	1.179	662.128	1.179	2	-1.55E+3	1.179	1
	24:1.0D-1.0WL	1.179	434.529	0.000	1	-173.414	0.000	2
	25:1.4D	1.179	966.191	1.179	2	-1.45E+3	1.179	1
	26:1.2D+1.5LM	1.179	873.119	1.179	2	-1.18E+3	1.179	1
	27:1.2D+1.5LM	1.179	1.04E+3	1.179	2	-1.62E+3	1.179	1
	28:1.2D+1.5LM	1.179	856.413	1.179	2	-1.47E+3	1.179	1
	29:1.2D+1.5LM	1.179	685.646	1.179	2	-1.03E+3	1.179	1
	30:1.2D+1.5LV	1.179	822.464	1.179	2	-1.22E+3	1.179	1
26	1:DEAD	1.179	1.61E+3	0.000	2	-3.95E+3	0.000	1
	2:WL#1	1.179	2.28E+3	0.000	2	-4.96E+3	0.000	1
	3:WL#4	1.179	571.214	0.000	2	-1.51E+3	0.000	1
	4:DI	1.179	5.6E+3	0.000	2	-13.9E+3	0.000	1
	5:WLI#1	1.179	1.1E+3	0.000	2	-2.33E+3	0.000	1
	6:WLI#4	1.179	270.669	0.000	2	-706.337	0.000	1
	7:WLS#1	1.179	802.288	0.000	2	-1.74E+3	0.000	1
	8:WLS#4	1.179	200.528	0.000	2	-528.831	0.000	1
	9:WLM#1	1.179	229.840	0.000	2	-499.019	0.000	1
	10:WLM#4	1.179	57.462	0.000	2	-151.090	0.000	1
	11:LM	1.179	33.245	0.000	1	-19.171	0.000	2
	12:LV	1.179	27.478	0.000	2	-62.683	0.000	1
	13:1.2D+1.6WI	1.179	5.38E+3	0.000	2	-12.7E+3	0.000	1
	14:1.2D+1.6WI	1.179	2.64E+3	0.000	2	-7.15E+3	0.000	1
	15:1.2D-1.6WL	1.179	4.55E+3	1.179	1	-1.93E+3	0.000	2
	16:1.2D-1.6WL	1.179	1.22E+3	0.000	2	-2.33E+3	0.000	1
	17:1.2D+1.0DI·	1.179	7.98E+3	0.000	2	-20.9E+3	0.000	1
	18:1.2D+1.0DI·	1.179	7.72E+3	0.000	2	-19.3E+3	0.000	1
	19:1.2D+1.0DI·	1.179	7.18E+3	0.000	2	-16.3E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	20:1.2D+1.0DI·	1.179	7.34E+3	0.000	2	-17.9E+3	0.000	1
	21:1.0D+1.0WI	1.179	2.24E+3	0.000	2	-5.69E+3	0.000	1
	22:1.0D+1.0WI	1.179	1.77E+3	0.000	2	-4.48E+3	0.000	1
	23:1.0D-1.0WL	1.179	1.31E+3	0.000	2	-2.2E+3	0.000	1
	24:1.0D-1.0WL	1.179	1.45E+3	0.000	2	-3.42E+3	0.000	1
	25:1.4D	1.179	2.25E+3	0.000	2	-5.53E+3	0.000	1
	26:1.2D+1.5LM	1.179	1.99E+3	0.000	2	-5.18E+3	0.000	1
	27:1.2D+1.5LM	1.179	1.95E+3	0.000	2	-4.84E+3	0.000	1
	28:1.2D+1.5LM	1.179	1.82E+3	0.000	2	-4.19E+3	0.000	1
	29:1.2D+1.5LM	1.179	1.86E+3	0.000	2	-4.53E+3	0.000	1
	30:1.2D+1.5LV	1.179	1.97E+3	0.000	2	-4.83E+3	0.000	1
27	1:DEAD	1.179	696.497	1.179	2	-1.04E+3	1.179	1
	2:WL#1	1.179	1.55E+3	1.179	2	-3.29E+3	1.179	1
	3:WL#4	1.179	419.162	1.179	3	-1.01E+3	1.179	2
	4:DI	1.179	3.28E+3	1.179	2	-5.16E+3	1.179	1
	5:WLI#1	1.179	734.295	1.179	2	-1.54E+3	1.179	1
	6:WLI#4	1.179	196.595	1.179	3	-466.176	1.179	2
	7:WLS#1	1.179	544.086	1.179	2	-1.16E+3	1.179	1
	8:WLS#4	1.179	147.202	1.179	3	-353.998	1.179	2
	9:WLM#1	1.179	155.721	1.179	2	-331.026	1.179	1
	10:WLM#4	1.179	42.051	1.179	3	-100.954	1.179	2
	11:LM	1.179	24.548	1.179	1	-10.078	1.179	3
	12:LV	1.179	4.715	1.179	3	-10.419	1.179	1
	13:1.2D+1.6WI	1.179	3.31E+3	1.179	2	-6.51E+3	1.179	1
	14:1.2D+1.6WI	1.179	989.634	1.179	3	-985.892	1.179	1
	15:1.2D-1.6WL	1.179	4.02E+3	1.080	1	-1.69E+3	0.000	2
	16:1.2D-1.6WL	1.179	2.45E+3	1.179	2	-1.51E+3	1.179	1
	17:1.2D+1.0DI·	1.179	4.85E+3	1.179	2	-7.95E+3	1.179	1
	18:1.2D+1.0DI·	1.179	3.65E+3	1.179	2	-6.33E+3	1.179	1
	19:1.2D+1.0DI·	1.179	3.38E+3	1.179	2	-4.87E+3	1.179	1
	20:1.2D+1.0DI·	1.179	4.58E+3	1.179	2	-6.49E+3	1.179	1
	21:1.0D+1.0WI	1.179	1.24E+3	1.179	2	-2.2E+3	1.179	1
	22:1.0D+1.0WI	1.179	413.014	1.179	3	-983.315	1.179	1
	23:1.0D-1.0WL	1.179	577.400	0.000	1	-213.535	0.000	2
	24:1.0D-1.0WL	1.179	1.05E+3	1.179	2	-1.1E+3	1.179	1
	25:1.4D	1.179	975.096	1.179	2	-1.46E+3	1.179	1
	26:1.2D+1.5LM	1.179	980.116	1.179	2	-1.54E+3	1.179	1
	27:1.2D+1.5LM	1.179	723.441	1.179	2	-1.2E+3	1.179	1
	28:1.2D+1.5LM	1.179	668.674	1.179	2	-881.458	1.179	1
	29:1.2D+1.5LM	1.179	925.348	1.179	2	-1.23E+3	1.179	1
	30:1.2D+1.5LV	1.179	840.339	1.179	2	-1.26E+3	1.179	1
28	1:DEAD	1.179	1.65E+3	0.000	2	-4.04E+3	0.000	1
	2:WL#1	1.179	1.37E+3	1.179	1	-702.027	0.000	2
	3:WL#4	1.179	5.03E+3	0.000	1	-2.3E+3	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	4:DI	1.179	6.17E+3	0.000	2	-15.3E+3	0.000	1
	5:WLI#1	1.179	623.420	1.179	1	-326.211	0.000	2
	6:WLI#4	1.179	2.32E+3	0.000	1	-1.07E+3	0.000	2
	7:WLS#1	1.179	482.715	1.179	1	-246.696	0.000	2
	8:WLS#4	1.179	1.77E+3	0.000	1	-808.569	0.000	2
	9:WLM#1	1.179	137.694	1.179	1	-70.268	0.000	2
	10:WLM#4	1.179	503.277	0.000	1	-230.465	0.000	2
	11:LM	1.179	54.954	0.000	2	-125.376	0.000	1
	12:LV	1.179	4.286	1.179	2	-9.295	0.000	1
	13:1.2D+1.6WI	1.179	2.04E+3	0.000	2	-2.93E+3	0.000	1
	14:1.2D+1.6WI	1.179	4.38E+3	1.179	1	-1.91E+3	0.000	2
	15:1.2D-1.6WL	1.179	2.9E+3	0.000	2	-6.77E+3	0.000	1
	16:1.2D-1.6WL	1.179	5.46E+3	0.000	2	-12.9E+3	0.000	1
	17:1.2D+1.0DI·	1.179	8.18E+3	0.000	2	-19.6E+3	0.000	1
	18:1.2D+1.0DI·	1.179	7.79E+3	0.000	2	-17.8E+3	0.000	1
	19:1.2D+1.0DI·	1.179	8.13E+3	0.000	2	-20.6E+3	0.000	1
	20:1.2D+1.0DI·	1.179	8.53E+3	0.000	2	-22.4E+3	0.000	1
	21:1.0D+1.0WI	1.179	1.67E+3	0.000	2	-3.62E+3	0.000	1
	22:1.0D+1.0WI	1.179	1.35E+3	0.000	2	-2.28E+3	0.000	1
	23:1.0D-1.0WL	1.179	1.73E+3	0.000	2	-4.46E+3	0.000	1
	24:1.0D-1.0WL	1.179	2.29E+3	0.000	2	-5.81E+3	0.000	1
	25:1.4D	1.179	2.31E+3	0.000	2	-5.66E+3	0.000	1
	26:1.2D+1.5LM	1.179	2.07E+3	0.000	2	-4.92E+3	0.000	1
	27:1.2D+1.5LM	1.179	1.98E+3	0.000	2	-4.54E+3	0.000	1
	28:1.2D+1.5LM	1.179	2.06E+3	0.000	2	-5.16E+3	0.000	1
	29:1.2D+1.5LM	1.179	2.15E+3	0.000	2	-5.54E+3	0.000	1
	30:1.2D+1.5LV	1.179	1.99E+3	0.000	2	-4.87E+3	0.000	1
29	1:DEAD	1.179	726.309	1.179	2	-1.09E+3	1.179	1
	2:WL#1	1.179	1.79E+3	1.179	1	-1.69E+3	1.179	2
	3:WL#4	1.179	2.7E+3	1.179	1	-1.13E+3	1.179	3
	4:DI	1.179	3.66E+3	1.179	2	-5.82E+3	1.179	1
	5:WLI#1	1.179	842.171	1.179	1	-780.415	1.179	2
	6:WLI#4	1.179	1.25E+3	1.179	1	-517.735	1.179	3
	7:WLS#1	1.179	630.097	1.179	1	-594.294	1.179	2
	8:WLS#4	1.179	947.378	1.179	1	-397.329	1.179	3
	9:WLM#1	1.179	180.501	1.179	1	-169.826	1.179	2
	10:WLM#4	1.179	270.124	1.179	1	-113.244	1.179	3
	11:LM	1.179	9.448	1.179	3	-20.869	1.179	1
	12:LV	1.179	12.208	1.179	2	-28.160	1.179	1
	13:1.2D+1.6WI	1.179	1.7E+3	0.098	1	-1.84E+3	1.179	2
	14:1.2D+1.6WI	1.179	3.16E+3	0.000	1	-1.48E+3	1.179	3
	15:1.2D-1.6WL	1.179	3.58E+3	1.179	2	-4.17E+3	1.179	1
	16:1.2D-1.6WL	1.179	2.25E+3	1.179	2	-5.62E+3	1.179	1
	17:1.2D+1.0DI·	1.179	3.75E+3	1.179	2	-6.28E+3	1.179	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	18:1.2D+1.0DI·	1.179	4.13E+3	1.179	2	-5.88E+3	1.179	1
	19:1.2D+1.0DI·	1.179	5.31E+3	1.179	2	-7.97E+3	1.179	1
	20:1.2D+1.0DI·	1.179	4.93E+3	1.179	2	-8.37E+3	1.179	1
	21:1.0D+1.0WI	1.179	209.608	1.179	3	-457.061	1.179	1
	22:1.0D+1.0WI	1.179	424.548	1.179	2	-220.443	0.000	2
	23:1.0D-1.0WL	1.179	1.32E+3	1.179	2	-1.72E+3	1.179	1
	24:1.0D-1.0WL	1.179	1.03E+3	1.179	2	-2.03E+3	1.179	1
	25:1.4D	1.179	1.02E+3	1.179	2	-1.52E+3	1.179	1
	26:1.2D+1.5LM	1.179	710.737	1.179	2	-1.16E+3	1.179	1
	27:1.2D+1.5LM	1.179	794.746	1.179	2	-1.07E+3	1.179	1
	28:1.2D+1.5LM	1.179	1.05E+3	1.179	2	-1.52E+3	1.179	1
	29:1.2D+1.5LM	1.179	966.380	1.179	2	-1.61E+3	1.179	1
	30:1.2D+1.5LV	1.179	889.883	1.179	2	-1.35E+3	1.179	1
30	1:DEAD	1.179	1.62E+3	0.000	2	-3.97E+3	0.000	1
	2:WL#1	1.179	3.86E+3	0.000	1	-1.71E+3	0.000	2
	3:WL#4	1.179	1.67E+3	0.000	2	-3.44E+3	0.000	1
	4:DI	1.179	5.72E+3	0.000	2	-14.2E+3	0.000	1
	5:WLI#1	1.179	1.8E+3	0.000	1	-801.475	0.000	2
	6:WLI#4	1.179	795.757	0.000	2	-1.59E+3	0.000	1
	7:WLS#1	1.179	1.36E+3	0.000	1	-600.977	0.000	2
	8:WLS#4	1.179	586.130	0.000	2	-1.21E+3	0.000	1
	9:WLM#1	1.179	388.240	0.000	1	-172.022	0.000	2
	10:WLM#4	1.179	167.260	0.000	2	-343.936	0.000	1
	11:LM	1.179	8.577	1.179	2	-18.597	0.000	1
	12:LV	1.179	16.622	0.000	1	-9.588	0.000	2
	13:1.2D+1.6WI	1.179	2.58E+3	1.179	1	-992.815	0.000	2
	14:1.2D+1.6WI	1.179	4.42E+3	0.000	2	-10.3E+3	0.000	1
	15:1.2D-1.6WL	1.179	4.48E+3	0.000	2	-10.9E+3	0.000	1
	16:1.2D-1.6WL	1.179	2.55E+3	1.179	1	-925.741	0.000	2
	17:1.2D+1.0DI·	1.179	7.34E+3	0.000	2	-17.2E+3	0.000	1
	18:1.2D+1.0DI·	1.179	7.88E+3	0.000	2	-20.6E+3	0.000	1
	19:1.2D+1.0DI·	1.179	7.97E+3	0.000	2	-20.8E+3	0.000	1
	20:1.2D+1.0DI·	1.179	7.49E+3	0.000	2	-17.4E+3	0.000	1
	21:1.0D+1.0WI	1.179	1.38E+3	0.000	2	-2.62E+3	0.000	1
	22:1.0D+1.0WI	1.179	2.04E+3	0.000	2	-5.18E+3	0.000	1
	23:1.0D-1.0WL	1.179	2.05E+3	0.000	2	-5.33E+3	0.000	1
	24:1.0D-1.0WL	1.179	1.45E+3	0.000	2	-2.77E+3	0.000	1
	25:1.4D	1.179	2.27E+3	0.000	2	-5.56E+3	0.000	1
	26:1.2D+1.5LM	1.179	1.89E+3	0.000	2	-4.41E+3	0.000	1
	27:1.2D+1.5LM	1.179	2E+3	0.000	2	-5.14E+3	0.000	1
	28:1.2D+1.5LM	1.179	2.03E+3	0.000	2	-5.18E+3	0.000	1
	29:1.2D+1.5LM	1.179	1.91E+3	0.000	2	-4.45E+3	0.000	1
	30:1.2D+1.5LV	1.179	1.93E+3	0.000	2	-4.74E+3	0.000	1
43	1:DEAD	1.041	4.12E+3	1.041	3	-4.08E+3	1.041	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	2:WL#1	1.041	3.79E+3	1.041	3	-4.14E+3	1.041	1
	3:WL#4	1.041	5.08E+3	1.041	4	-5.71E+3	1.041	2
	4:DI	1.041	15.9E+3	1.041	3	-15.7E+3	1.041	1
	5:WLI#1	1.041	1.87E+3	1.041	3	-2.04E+3	1.041	1
	6:WLI#4	1.041	2.31E+3	1.041	4	-2.62E+3	1.041	2
	7:WLS#1	1.041	1.33E+3	1.041	3	-1.45E+3	1.041	1
	8:WLS#4	1.041	1.78E+3	1.041	4	-2.01E+3	1.041	2
	9:WLM#1	1.041	381.658	1.041	3	-417.054	1.041	1
	10:WLM#4	1.041	507.236	1.041	4	-570.511	1.041	2
	11:LM	1.041	6.728	1.041	3	-6.826	1.041	1
	12:LV	1.041	3.370	1.041	4	-3.425	1.041	2
	13:1.2D+1.6WI	1.041	11E+3	1.041	3	-11.5E+3	1.041	1
	14:1.2D+1.6WI	1.041	13.1E+3	1.041	4	-14E+3	1.041	2
	15:1.2D-1.6WL	1.041	3.69E+3	1.041	4	-3.08E+3	1.041	2
	16:1.2D-1.6WL	1.041	4.28E+3	0.000	1	-3.24E+3	0.000	3
	17:1.2D+1.0DI-	1.041	22.7E+3	1.041	3	-22.7E+3	1.041	1
	18:1.2D+1.0DI-	1.041	23.2E+3	1.041	4	-23.3E+3	1.041	2
	19:1.2D+1.0DI-	1.041	20.6E+3	1.041	4	-20.2E+3	1.041	2
	20:1.2D+1.0DI-	1.041	19.5E+3	1.041	3	-19E+3	1.041	1
	21:1.0D+1.0WI	1.041	5.45E+3	1.041	3	-5.53E+3	1.041	1
	22:1.0D+1.0WI	1.041	5.9E+3	1.041	4	-6.09E+3	1.041	2
	23:1.0D-1.0WL	1.041	3.84E+3	1.041	4	-3.68E+3	1.041	2
	24:1.0D-1.0WL	1.041	3.08E+3	1.041	3	-2.82E+3	1.041	1
	25:1.4D	1.041	5.76E+3	1.041	3	-5.71E+3	1.041	1
	26:1.2D+1.5LM	1.041	5.33E+3	1.041	3	-5.32E+3	1.041	1
	27:1.2D+1.5LM	1.041	5.45E+3	1.041	3	-5.47E+3	1.041	1
	28:1.2D+1.5LM	1.041	4.87E+3	1.041	4	-4.79E+3	1.041	2
	29:1.2D+1.5LM	1.041	4.65E+3	1.041	3	-4.55E+3	1.041	1
	30:1.2D+1.5LV	1.041	4.95E+3	1.041	3	-4.9E+3	1.041	1
44	1:DEAD	2.000	2.34E+3	0.000	3	-2.28E+3	0.000	1
	2:WL#1	2.000	4.15E+3	0.000	1	-3.75E+3	0.000	3
	3:WL#4	2.000	292.491	0.000	2	-292.824	0.000	1
	4:DI	2.000	9.68E+3	0.000	3	-9.44E+3	0.000	1
	5:WLI#1	2.000	1.86E+3	0.000	1	-1.67E+3	0.000	3
	6:WLI#4	2.000	153.954	0.000	2	-153.791	0.000	1
	7:WLS#1	2.000	1.46E+3	0.000	1	-1.32E+3	0.000	3
	8:WLS#4	2.000	102.694	0.000	2	-102.811	0.000	1
	9:WLM#1	2.000	415.982	0.000	1	-376.089	0.000	3
	10:WLM#4	2.000	29.548	0.000	2	-29.581	0.000	1
	11:LM	2.000	38.310	0.000	1	-38.133	0.000	3
	12:LV	2.000	3.267	2.000	2	-3.113	2.000	4
	13:1.2D+1.6WI	2.000	3.94E+3	1.333	1	-3.25E+3	2.000	4
	14:1.2D+1.6WI	2.000	3.28E+3	0.000	3	-3.21E+3	0.000	1
	15:1.2D-1.6WL	2.000	8.82E+3	0.000	3	-9.38E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	16:1.2D-1.6WL	2.000	3.24E+3	0.000	3	-3.16E+3	0.000	1
	17:1.2D+1.0DI·	2.000	10.8E+3	0.000	3	-10.3E+3	0.000	1
	18:1.2D+1.0DI·	2.000	12.6E+3	0.000	3	-12.3E+3	0.000	1
	19:1.2D+1.0DI·	2.000	14.2E+3	0.000	3	-14E+3	0.000	1
	20:1.2D+1.0DI·	2.000	12.6E+3	0.000	3	-12.3E+3	0.000	1
	21:1.0D+1.0WI	2.000	1.04E+3	0.000	3	-839.411	0.000	1
	22:1.0D+1.0WI	2.000	2.45E+3	0.000	3	-2.39E+3	0.000	1
	23:1.0D-1.0WL	2.000	3.66E+3	0.000	3	-3.74E+3	0.000	1
	24:1.0D-1.0WL	2.000	2.43E+3	0.000	3	-2.37E+3	0.000	1
	25:1.4D	2.000	3.28E+3	0.000	3	-3.2E+3	0.000	1
	26:1.2D+1.5LM	2.000	2.38E+3	0.000	3	-2.27E+3	0.000	1
	27:1.2D+1.5LM	2.000	2.78E+3	0.000	3	-2.71E+3	0.000	1
	28:1.2D+1.5LM	2.000	3.13E+3	0.000	3	-3.1E+3	0.000	1
	29:1.2D+1.5LM	2.000	2.78E+3	0.000	3	-2.71E+3	0.000	1
	30:1.2D+1.5LV	2.000	2.81E+3	0.000	3	-2.74E+3	0.000	1
45	1:DEAD	2.000	2.34E+3	0.000	3	-2.28E+3	0.000	1
	2:WL#1	2.000	2.1E+3	0.000	3	-2.29E+3	0.000	1
	3:WL#4	2.000	3.72E+3	0.000	1	-3.38E+3	0.000	3
	4:DI	2.000	9.92E+3	0.000	3	-9.69E+3	0.000	1
	5:WLI#1	2.000	985.317	0.000	3	-1.07E+3	0.000	1
	6:WLI#4	2.000	1.7E+3	0.000	1	-1.54E+3	0.000	3
	7:WLS#1	2.000	738.339	0.000	3	-802.630	0.000	1
	8:WLS#4	2.000	1.31E+3	0.000	1	-1.19E+3	0.000	3
	9:WLM#1	2.000	211.362	0.000	3	-229.734	0.000	1
	10:WLM#4	2.000	372.062	0.000	1	-338.042	0.000	3
	11:LM	2.000	6.534	2.000	1	-6.222	2.000	3
	12:LV	2.000	19.156	0.000	1	-19.069	0.000	3
	13:1.2D+1.6WI	2.000	6.17E+3	0.000	3	-6.39E+3	0.000	1
	14:1.2D+1.6WI	2.000	3.57E+3	2.000	2	-2.97E+3	2.000	4
	15:1.2D-1.6WL	2.000	2.12E+3	2.000	1	-1.76E+3	2.000	3
	16:1.2D-1.6WL	2.000	8.22E+3	0.000	3	-8.69E+3	0.000	1
	17:1.2D+1.0DI·	2.000	13.7E+3	0.000	3	-13.5E+3	0.000	1
	18:1.2D+1.0DI·	2.000	11.3E+3	0.000	3	-10.9E+3	0.000	1
	19:1.2D+1.0DI·	2.000	12E+3	0.000	3	-11.6E+3	0.000	1
	20:1.2D+1.0DI·	2.000	14.2E+3	0.000	3	-14.1E+3	0.000	1
	21:1.0D+1.0WI	2.000	3.08E+3	0.000	3	-3.08E+3	0.000	1
	22:1.0D+1.0WI	2.000	1.27E+3	0.000	3	-1.09E+3	0.000	1
	23:1.0D-1.0WL	2.000	1.8E+3	0.000	3	-1.68E+3	0.000	1
	24:1.0D-1.0WL	2.000	3.53E+3	0.000	3	-3.59E+3	0.000	1
	25:1.4D	2.000	3.28E+3	0.000	3	-3.19E+3	0.000	1
	26:1.2D+1.5LM	2.000	3.03E+3	0.000	3	-2.97E+3	0.000	1
	27:1.2D+1.5LM	2.000	2.51E+3	0.000	3	-2.4E+3	0.000	1
	28:1.2D+1.5LM	2.000	2.65E+3	0.000	3	-2.56E+3	0.000	1
	29:1.2D+1.5LM	2.000	3.14E+3	0.000	3	-3.1E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	30:1.2D+1.5LV	2.000	2.78E+3	0.000	3	-2.71E+3	0.000	1
48	1:DEAD	1.500	1.55E+3	0.000		-1.2E+3	0.000	
	2:WL#1	1.500	14.2E+3	0.000		-14.1E+3	0.000	
	3:WL#4	1.500	16.5E+3	0.000		-16.3E+3	0.000	
	4:DI	1.500	6.57E+3	0.000		-5.09E+3	0.000	
	5:WLI#1	1.500	6.76E+3	0.000		-6.71E+3	0.000	
	6:WLI#4	1.500	7.42E+3	0.000		-7.34E+3	0.000	
	7:WLS#1	1.500	4.99E+3	0.000		-4.95E+3	0.000	
	8:WLS#4	1.500	5.8E+3	0.000		-5.74E+3	0.000	
	9:WLM#1	1.500	1.43E+3	0.000		-1.42E+3	0.000	
	10:WLM#4	1.500	1.65E+3	0.000		-1.63E+3	0.000	
	11:LM	1.500	71.805	0.000		-71.159	0.000	
	12:LV	1.500	35.904	0.000		-35.581	0.000	
	13:1.2D+1.6WI	1.500	22.1E+3	0.000		-21.5E+3	0.000	
	14:1.2D+1.6WI	1.500	25.1E+3	0.000		-24.4E+3	0.000	
	15:1.2D-1.6WL	1.500	23.7E+3	0.000		-23.5E+3	0.000	
	16:1.2D-1.6WL	1.500	27.9E+3	0.000		-27.7E+3	0.000	
	17:1.2D+1.0DI·	1.500	7.8E+3	0.000		-5.86E+3	0.000	
	18:1.2D+1.0DI·	1.500	4.24E+3	0.000		-2.28E+3	0.000	
	19:1.2D+1.0DI·	1.500	13.4E+3	0.000		-11.6E+3	0.000	
	20:1.2D+1.0DI·	1.500	15.4E+3	0.000		-13.6E+3	0.000	
	21:1.0D+1.0WI	1.500	4.55E+3	0.000		-4.17E+3	0.000	
	22:1.0D+1.0WI	1.500	4.76E+3	0.000		-4.36E+3	0.000	
	23:1.0D-1.0WL	1.500	6E+3	0.000		-5.69E+3	0.000	
	24:1.0D-1.0WL	1.500	7.19E+3	0.000		-6.9E+3	0.000	
	25:1.4D	1.500	2.17E+3	0.000		-1.68E+3	0.000	
	26:1.2D+1.5LM	1.500	1.65E+3	0.000		-1.22E+3	0.000	
	27:1.2D+1.5LM	1.500	1.03E+3	0.000		-598.992	0.000	
	28:1.2D+1.5LM	1.500	3.01E+3	0.000		-2.6E+3	0.000	
	29:1.2D+1.5LM	1.500	3.45E+3	0.000		-3.05E+3	0.000	
	30:1.2D+1.5LV	1.500	1.89E+3	0.000		-1.47E+3	0.000	
51	1:DEAD	1.500	1.53E+3	0.000		-1.18E+3	0.000	
	2:WL#1	1.500	14.7E+3	0.000		-14.6E+3	0.000	
	3:WL#4	1.500	16.7E+3	0.000		-16.9E+3	0.000	
	4:DI	1.500	6.19E+3	0.000		-4.65E+3	0.000	
	5:WLI#1	1.500	6.85E+3	0.000		-6.8E+3	0.000	
	6:WLI#4	1.500	7.46E+3	0.000		-7.54E+3	0.000	
	7:WLS#1	1.500	5.18E+3	0.000		-5.14E+3	0.000	
	8:WLS#4	1.500	5.87E+3	0.000		-5.93E+3	0.000	
	9:WLM#1	1.500	1.48E+3	0.000		-1.47E+3	0.000	
	10:WLM#4	1.500	1.67E+3	0.000		-1.68E+3	0.000	
	11:LM	1.500	71.806	0.000		-71.159	0.000	
	12:LV	1.500	52.140	0.000		-53.360	0.000	
	13:1.2D+1.6WI	1.500	22.9E+3	0.000		-22.3E+3	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	14:1.2D+1.6WI	1.500	28.4E+3	0.000		-28.3E+3	0.000	
	15:1.2D-1.6WL	1.500	24.6E+3	0.000		-24.3E+3	0.000	
	16:1.2D-1.6WL	1.500	25.8E+3	0.000		-25.1E+3	0.000	
	17:1.2D+1.0DI·	1.500	7.19E+3	0.000		-5.18E+3	0.000	
	18:1.2D+1.0DI·	1.500	15E+3	0.000		-13.1E+3	0.000	
	19:1.2D+1.0DI·	1.500	13.4E+3	0.000		-11.5E+3	0.000	
	20:1.2D+1.0DI·	1.500	4.74E+3	0.000		-2.71E+3	0.000	
	21:1.0D+1.0WI	1.500	4.72E+3	0.000		-4.34E+3	0.000	
	22:1.0D+1.0WI	1.500	7.29E+3	0.000		-6.99E+3	0.000	
	23:1.0D-1.0WL	1.500	6.19E+3	0.000		-5.87E+3	0.000	
	24:1.0D-1.0WL	1.500	4.93E+3	0.000		-4.52E+3	0.000	
	25:1.4D	1.500	2.14E+3	0.000		-1.65E+3	0.000	
	26:1.2D+1.5LM	1.500	1.63E+3	0.000		-1.19E+3	0.000	
	27:1.2D+1.5LM	1.500	3.46E+3	0.000		-3.05E+3	0.000	
	28:1.2D+1.5LM	1.500	3.05E+3	0.000		-2.63E+3	0.000	
	29:1.2D+1.5LM	1.500	1.07E+3	0.000		-630.044	0.000	
	30:1.2D+1.5LV	1.500	1.91E+3	0.000		-1.49E+3	0.000	
54	1:DEAD	1.500	1.53E+3	0.000		-1.18E+3	0.000	
	2:WL#1	1.500	17.4E+3	0.000		-17.6E+3	0.000	
	3:WL#4	1.500	13.7E+3	0.000		-13.7E+3	0.000	
	4:DI	1.500	6.16E+3	0.000		-4.7E+3	0.000	
	5:WLI#1	1.500	7.38E+3	0.000		-7.47E+3	0.000	
	6:WLI#4	1.500	6.4E+3	0.000		-6.4E+3	0.000	
	7:WLS#1	1.500	6.11E+3	0.000		-6.18E+3	0.000	
	8:WLS#4	1.500	4.81E+3	0.000		-4.81E+3	0.000	
	9:WLM#1	1.500	1.73E+3	0.000		-1.75E+3	0.000	
	10:WLM#4	1.500	1.37E+3	0.000		-1.37E+3	0.000	
	11:LM	1.500	104.277	0.000		-106.717	0.000	
	12:LV	1.500	35.902	0.000		-35.579	0.000	
	13:1.2D+1.6WI	1.500	29.6E+3	0.000		-29.5E+3	0.000	
	14:1.2D+1.6WI	1.500	22.1E+3	0.000		-21.7E+3	0.000	
	15:1.2D-1.6WL	1.500	26.7E+3	0.000		-26E+3	0.000	
	16:1.2D-1.6WL	1.500	22.2E+3	0.000		-21.8E+3	0.000	
	17:1.2D+1.0DI·	1.500	15.3E+3	0.000		-13.5E+3	0.000	
	18:1.2D+1.0DI·	1.500	9.82E+3	0.000		-7.93E+3	0.000	
	19:1.2D+1.0DI·	1.500	1.95E+3	0.000				
	20:1.2D+1.0DI·	1.500	11.1E+3	0.000		-9.17E+3	0.000	
	21:1.0D+1.0WI	1.500	7.64E+3	0.000		-7.35E+3	0.000	
	22:1.0D+1.0WI	1.500	5.13E+3	0.000		-4.78E+3	0.000	
	23:1.0D-1.0WL	1.500	5E+3	0.000		-4.58E+3	0.000	
	24:1.0D-1.0WL	1.500	5.21E+3	0.000		-4.86E+3	0.000	
	25:1.4D	1.500	2.14E+3	0.000		-1.65E+3	0.000	
	26:1.2D+1.5LM	1.500	3.72E+3	0.000		-3.32E+3	0.000	
	27:1.2D+1.5LM	1.500	2.42E+3	0.000		-2.01E+3	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	28:1.2D+1.5LM	1.500	344.203	1.500				
	29:1.2D+1.5LM	1.500	2.49E+3	0.000		-2.07E+3	0.000	
	30:1.2D+1.5LV	1.500	1.86E+3	0.000		-1.44E+3	0.000	
55	1:DEAD	1.889	1.89E+3	0.000	2	-761.121	0.000	1
	2:WL#1	1.889	6.25E+3	0.000	2	-2.38E+3	0.000	1
	3:WL#4	1.889	2.25E+3	0.000	2	-820.477	0.000	1
	4:DI	1.889	7.91E+3	0.000	2	-3.21E+3	0.000	1
	5:WLI#1	1.889	2.96E+3	0.000	2	-1.11E+3	0.000	1
	6:WLI#4	1.889	1.03E+3	0.000	2	-395.980	0.000	1
	7:WLS#1	1.889	2.19E+3	0.000	2	-836.197	0.000	1
	8:WLS#4	1.889	791.986	0.000	2	-288.338	0.000	1
	9:WLM#1	1.889	629.597	0.000	2	-239.685	0.000	1
	10:WLM#4	1.889	225.605	0.000	2	-81.489	0.000	1
	11:LM	1.889	10.519	0.000	2	-12.121	1.889	2
	12:LV	1.889	5.341	1.889	2	-2.073	1.889	1
	13:1.2D+1.6WI	1.889	12.3E+3	0.000	2	-4.72E+3	0.000	1
	14:1.2D+1.6WI	1.889	5.88E+3	0.000	2	-2.23E+3	0.000	1
	15:1.2D-1.6WL	1.889	2.9E+3	0.000	1	-7.73E+3	0.000	2
	16:1.2D-1.6WL	1.889	1.05E+3	1.889	2	-1.33E+3	0.000	2
	17:1.2D+1.0DI·	1.889	13.1E+3	0.000	2	-5.23E+3	0.000	1
	18:1.2D+1.0DI·	1.889	11.2E+3	0.000	2	-4.46E+3	0.000	1
	19:1.2D+1.0DI·	1.889	7.23E+3	0.000	2	-3.02E+3	0.000	1
	20:1.2D+1.0DI·	1.889	9.16E+3	0.000	2	-3.78E+3	0.000	1
	21:1.0D+1.0WI	1.889	4.09E+3	0.000	2	-1.6E+3	0.000	1
	22:1.0D+1.0WI	1.889	2.69E+3	0.000	2	-1.05E+3	0.000	1
	23:1.0D-1.0WL	1.889	257.219	1.889	1	-675.380	1.889	2
	24:1.0D-1.0WL	1.889	1.1E+3	0.000	2	-472.782	0.000	1
	25:1.4D	1.889	2.65E+3	0.000	2	-1.07E+3	0.000	1
	26:1.2D+1.5LM	1.889	2.92E+3	0.000	2	-1.16E+3	0.000	1
	27:1.2D+1.5LM	1.889	2.51E+3	0.000	2	-1E+3	0.000	1
	28:1.2D+1.5LM	1.889	1.66E+3	0.000	2	-680.624	0.000	1
	29:1.2D+1.5LM	1.889	2.06E+3	0.000	2	-838.820	0.000	1
	30:1.2D+1.5LV	1.889	2.28E+3	0.000	2	-915.009	0.000	1
56	1:DEAD	1.333	576.151	0.000	1	-1.03E+3	0.000	2
	2:WL#1	1.333	716.434	0.000	2	-293.022	0.000	1
	3:WL#4	1.333	777.860	0.000	2	-861.595	0.000	1
	4:DI	1.333	2.43E+3	0.000	1	-4.41E+3	0.000	2
	5:WLI#1	1.333	337.765	0.000	2	-142.677	0.000	1
	6:WLI#4	1.333	331.932	0.000	2	-388.543	0.000	1
	7:WLS#1	1.333	251.476	0.000	2	-102.900	0.000	1
	8:WLS#4	1.333	273.271	0.000	2	-302.615	0.000	1
	9:WLM#1	1.333	72.338	0.000	2	-29.502	0.000	1
	10:WLM#4	1.333	77.076	0.000	2	-86.023	0.000	1
	11:LM	1.333	4.204	0.000	1	-7.782	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	12:LV	1.333	8.132	1.333	1	-19.790	1.333	2
	13:1.2D+1.6WI	1.333	583.509	1.333	1	-1.18E+3	1.333	2
	14:1.2D+1.6WI	1.333	719.439	0.000	2	-1.2E+3	0.000	1
	15:1.2D-1.6WL	1.333	978.615	0.000	1	-2.38E+3	0.000	2
	16:1.2D-1.6WL	1.333	1.56E+3	0.000	1	-2.48E+3	0.000	2
	17:1.2D+1.0DI·	1.333	3.04E+3	0.000	1	-5.31E+3	0.000	2
	18:1.2D+1.0DI·	1.333	3.15E+3	0.000	1	-5.32E+3	0.000	2
	19:1.2D+1.0DI·	1.333	3.21E+3	0.000	1	-5.99E+3	0.000	2
	20:1.2D+1.0DI·	1.333	3.1E+3	0.000	1	-5.98E+3	0.000	2
	21:1.0D+1.0WI	1.333	513.205	0.000	1	-844.532	1.333	2
	22:1.0D+1.0WI	1.333	582.249	0.000	3	-758.884	0.000	1
	23:1.0D-1.0WL	1.333	639.097	0.000	1	-1.28E+3	0.000	2
	24:1.0D-1.0WL	1.333	570.053	0.000	1	-1.31E+3	0.000	2
	25:1.4D	1.333	806.612	0.000	1	-1.45E+3	0.000	2
	26:1.2D+1.5LM	1.333	679.459	0.000	1	-1.18E+3	0.000	2
	27:1.2D+1.5LM	1.333	699.887	0.000	1	-1.17E+3	0.000	2
	28:1.2D+1.5LM	1.333	715.916	0.000	1	-1.32E+3	0.000	2
	29:1.2D+1.5LM	1.333	695.488	0.000	1	-1.33E+3	0.000	2
	30:1.2D+1.5LV	1.333	700.238	0.000	1	-1.26E+3	0.000	2
57	1:DEAD	1.889	1.98E+3	0.000	2	-796.996	0.000	1
	2:WL#1	1.889	811.508	1.889	1	-1.92E+3	1.889	2
	3:WL#4	1.889	2.49E+3	0.000	1	-6.56E+3	0.000	2
	4:DI	1.889	9.44E+3	0.000	2	-3.83E+3	0.000	1
	5:WLI#1	1.889	392.181	1.889	1	-894.086	1.889	2
	6:WLI#4	1.889	1.1E+3	0.000	1	-3E+3	0.000	2
	7:WLS#1	1.889	284.928	1.889	1	-674.740	1.889	2
	8:WLS#4	1.889	873.218	0.000	1	-2.3E+3	0.000	2
	9:WLM#1	1.889	81.889	1.889	1	-193.370	1.889	2
	10:WLM#4	1.889	248.158	0.000	1	-655.723	0.000	2
	11:LM	1.889	10.682	1.889	2	-4.147	1.889	1
	12:LV	1.889	13.524	0.000	1	-34.712	0.000	2
	13:1.2D+1.6WI	1.889	1.17E+3	1.889	1	-2.49E+3	1.889	2
	14:1.2D+1.6WI	1.889	3.02E+3	0.000	1	-8.11E+3	0.000	2
	15:1.2D-1.6WL	1.889	4.33E+3	0.000	2	-1.71E+3	0.000	1
	16:1.2D-1.6WL	1.889	12.9E+3	0.000	2	-4.93E+3	0.000	1
	17:1.2D+1.0DI·	1.889	11.2E+3	0.000	2	-4.54E+3	0.000	1
	18:1.2D+1.0DI·	1.889	8.82E+3	0.000	2	-3.69E+3	0.000	1
	19:1.2D+1.0DI·	1.889	12.4E+3	0.000	2	-5.02E+3	0.000	1
	20:1.2D+1.0DI·	1.889	14.8E+3	0.000	2	-5.88E+3	0.000	1
	21:1.0D+1.0WI	1.889	1.56E+3	0.000	2	-632.173	0.000	1
	22:1.0D+1.0WI	1.889	136.889	1.889	1	-440.395	1.889	2
	23:1.0D-1.0WL	1.889	2.41E+3	0.000	2	-961.818	0.000	1
	24:1.0D-1.0WL	1.889	4.29E+3	0.000	2	-1.67E+3	0.000	1
	25:1.4D	1.889	2.78E+3	0.000	2	-1.12E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	26:1.2D+1.5LM	1.889	2.26E+3	0.000	2	-911.896	0.000	1
	27:1.2D+1.5LM	1.889	1.73E+3	0.000	2	-711.565	0.000	1
	28:1.2D+1.5LM	1.889	2.51E+3	0.000	2	-1.01E+3	0.000	1
	29:1.2D+1.5LM	1.889	3.04E+3	0.000	2	-1.21E+3	0.000	1
	30:1.2D+1.5LV	1.889	2.33E+3	0.000	2	-936.110	0.000	1
58	1:DEAD	1.333	557.769	0.000	1	-1.02E+3	0.000	2
	2:WL#1	1.333	453.685	0.000	2	-646.695	0.000	1
	3:WL#4	1.333	674.144	0.000	1	-1.02E+3	0.000	2
	4:DI	1.333	2.04E+3	0.000	1	-4.46E+3	1.333	2
	5:WLI#1	1.333	125.473	0.000	2	-270.081	0.000	1
	6:WLI#4	1.333	308.077	0.000	1	-437.341	0.000	2
	7:WLS#1	1.333	160.042	0.000	2	-227.360	0.000	1
	8:WLS#4	1.333	236.825	0.000	1	-359.136	0.000	2
	9:WLM#1	1.333	43.666	0.000	2	-64.446	0.000	1
	10:WLM#4	1.333	67.466	0.000	1	-101.869	0.000	2
	11:LM	1.333	16.313	1.333	1	-39.508	1.333	2
	12:LV	1.333	8.650	1.333	3	-20.565	1.333	2
	13:1.2D+1.6WI	1.333	797.231	0.000	3	-859.455	0.000	1
	14:1.2D+1.6WI	1.333	1.25E+3	0.000	1	-2.86E+3	0.000	2
	15:1.2D-1.6WL	1.333	1.21E+3	0.000	1	-1.95E+3	0.000	2
	16:1.2D-1.6WL	1.333	615.892	1.333	1	-1.21E+3	1.333	2
	17:1.2D+1.0DI	1.333	2.79E+3	0.000	1	-5.36E+3	1.333	2
	18:1.2D+1.0DI	1.333	2.79E+3	0.000	1	-5.4E+3	1.333	2
	19:1.2D+1.0DI	1.333	2.63E+3	0.000	1	-5.54E+3	1.333	2
	20:1.2D+1.0DI	1.333	2.64E+3	0.000	1	-5.5E+3	1.333	2
	21:1.0D+1.0WI	1.333	585.500	0.000	3	-861.083	0.000	1
	22:1.0D+1.0WI	1.333	630.105	0.000	1	-1.38E+3	0.000	2
	23:1.0D-1.0WL	1.333	530.037	0.000	1	-1.18E+3	0.000	2
	24:1.0D-1.0WL	1.333	485.433	0.000	3	-868.747	1.333	2
	25:1.4D	1.333	780.876	0.000	1	-1.43E+3	0.000	2
	26:1.2D+1.5LM	1.333	696.271	0.000	1	-1.22E+3	0.000	2
	27:1.2D+1.5LM	1.333	707.574	0.000	1	-1.37E+3	0.000	2
	28:1.2D+1.5LM	1.333	678.253	0.000	1	-1.31E+3	0.000	2
	29:1.2D+1.5LM	1.333	666.950	0.000	1	-1.16E+3	0.000	2
	30:1.2D+1.5LV	1.333	674.548	0.000	1	-1.24E+3	0.000	2
59	1:DEAD	1.889	1.99E+3	0.000	2	-800.995	0.000	1
	2:WL#1	1.889	2.02E+3	0.000	1	-5.38E+3	0.000	2
	3:WL#4	1.889	4.08E+3	0.000	2	-1.55E+3	0.000	1
	4:DI	1.889	9.32E+3	0.000	2	-3.79E+3	0.000	1
	5:WLI#1	1.889	872.550	0.000	1	-2.44E+3	0.000	2
	6:WLI#4	1.889	1.93E+3	0.000	2	-734.241	0.000	1
	7:WLS#1	1.889	710.310	0.000	1	-1.89E+3	0.000	2
	8:WLS#4	1.889	1.43E+3	0.000	2	-544.335	0.000	1
	9:WLM#1	1.889	202.138	0.000	1	-539.309	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	10:WLM#4	1.889	409.230	0.000	2	-155.729	0.000	1
	11:LM	1.889	27.045	0.000	1	-69.427	0.000	2
	12:LV	1.889	5.259	0.000	2	-6.061	1.889	2
	13:1.2D+1.6WI	1.889	2.27E+3	0.000	1	-6.21E+3	0.000	2
	14:1.2D+1.6WI	1.889	8.91E+3	0.000	2	-3.44E+3	0.000	1
	15:1.2D-1.6WL	1.889	11E+3	0.000	2	-4.2E+3	0.000	1
	16:1.2D-1.6WL	1.889	1.52E+3	0.000	1	-4.13E+3	0.000	2
	17:1.2D+1.0DI·	1.889	9.27E+3	0.000	2	-3.88E+3	0.000	1
	18:1.2D+1.0DI·	1.889	13.6E+3	0.000	2	-5.48E+3	0.000	1
	19:1.2D+1.0DI·	1.889	14.2E+3	0.000	2	-5.62E+3	0.000	1
	20:1.2D+1.0DI·	1.889	9.78E+3	0.000	2	-4.02E+3	0.000	1
	21:1.0D+1.0WI	1.889	104.001	0.157	2	-90.685	0.000	1
	22:1.0D+1.0WI	1.889	3.42E+3	0.000	2	-1.35E+3	0.000	1
	23:1.0D-1.0WL	1.889	3.88E+3	0.000	2	-1.51E+3	0.000	1
	24:1.0D-1.0WL	1.889	560.244	0.000	2	-524.781	1.889	2
	25:1.4D	1.889	2.79E+3	0.000	2	-1.12E+3	0.000	1
	26:1.2D+1.5LM	1.889	1.75E+3	0.000	2	-718.489	0.000	1
	27:1.2D+1.5LM	1.889	2.69E+3	0.000	2	-1.08E+3	0.000	1
	28:1.2D+1.5LM	1.889	2.82E+3	0.000	2	-1.12E+3	0.000	1
	29:1.2D+1.5LM	1.889	1.88E+3	0.000	2	-764.897	0.000	1
	30:1.2D+1.5LV	1.889	2.4E+3	0.000	2	-964.676	0.000	1
60	1:DEAD	1.333	556.341	0.000	1	-986.576	0.000	2
	2:WL#1	1.333	874.827	0.000	1	-972.646	0.000	2
	3:WL#4	1.333	239.665	1.333	3	-207.412	1.333	2
	4:DI	1.333	1.94E+3	0.000	1	-4.14E+3	1.333	2
	5:WLI#1	1.333	399.385	0.000	1	-417.120	0.000	2
	6:WLI#4	1.333	112.836	1.333	3	-90.245	1.333	2
	7:WLS#1	1.333	307.430	0.000	1	-342.086	0.000	2
	8:WLS#4	1.333	84.175	1.333	3	-72.926	1.333	2
	9:WLM#1	1.333	87.769	0.000	1	-96.876	0.000	2
	10:WLM#4	1.333	24.044	1.333	3	-20.599	1.333	2
	11:LM	1.333	17.340	1.333	3	-41.152	1.333	2
	12:LV	1.333	2.102	0.000	1	-3.892	0.000	2
	13:1.2D+1.6WI	1.333	1.56E+3	0.000	1	-2.74E+3	0.000	2
	14:1.2D+1.6WI	1.333	704.687	1.333	3	-1.29E+3	1.333	2
	15:1.2D-1.6WL	1.333	633.652	1.333	1	-1.24E+3	0.000	1
	16:1.2D-1.6WL	1.333	811.756	0.000	3	-1.35E+3	0.000	1
	17:1.2D+1.0DI·	1.333	2.61E+3	0.000	1	-5.12E+3	1.333	2
	18:1.2D+1.0DI·	1.333	2.55E+3	0.000	1	-5.18E+3	1.333	2
	19:1.2D+1.0DI·	1.333	2.61E+3	0.000	1	-5.06E+3	1.333	2
	20:1.2D+1.0DI·	1.333	2.67E+3	0.000	1	-5E+3	1.333	2
	21:1.0D+1.0WI	1.333	573.881	0.000	1	-1.33E+3	0.000	2
	22:1.0D+1.0WI	1.333	524.944	0.000	1	-949.965	0.000	2
	23:1.0D-1.0WL	1.333	538.802	0.000	3	-786.611	1.333	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	24:1.0D-1.0WL	1.333	587.739	0.000	1	-1.02E+3	0.000	2
	25:1.4D	1.333	778.878	0.000	1	-1.38E+3	0.000	2
	26:1.2D+1.5LM	1.333	682.521	0.000	1	-1.31E+3	0.000	2
	27:1.2D+1.5LM	1.333	668.611	0.000	1	-1.21E+3	0.000	2
	28:1.2D+1.5LM	1.333	673.617	0.000	1	-1.12E+3	0.000	2
	29:1.2D+1.5LM	1.333	687.527	0.000	1	-1.23E+3	0.000	2
	30:1.2D+1.5LV	1.333	670.763	0.000	1	-1.19E+3	0.000	2
101	1:DEAD	0.500	1.665	0.000		-0.000	0.500	
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500				-0.000	0.000	
	12:LV	0.500	0.000	0.000				
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000		-0.000	0.500	
	26:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	27:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	28:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	29:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	30:1.2D+1.5LV	0.500	1.998	0.000		-0.000	0.500	
102	1:DEAD	0.500	1.665	0.000				
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.000		-0.000	0.000	
	12:LV	0.500	0.000	0.000		-0.000	0.000	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000				
	26:1.2D+1.5LM	0.500	3.421	0.000				
	27:1.2D+1.5LM	0.500	3.421	0.000				
	28:1.2D+1.5LM	0.500	3.421	0.000				
	29:1.2D+1.5LM	0.500	3.421	0.000				
	30:1.2D+1.5LV	0.500	1.998	0.000				
103	1:DEAD	0.500	1.665	0.000		-0.000	0.500	
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.000		-0.000	0.000	
	12:LV	0.500	0.000	0.500		-0.000	0.500	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000		-0.000	0.500	
	26:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	27:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	28:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	29:1.2D+1.5LM	0.500	3.421	0.000				
	30:1.2D+1.5LV	0.500	1.998	0.000		-0.000	0.500	
104	1:DEAD	0.500	1.665	0.000		-0.000	0.500	
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.000				
	12:LV	0.500				-0.000	0.458	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000		-0.000	0.500	
	26:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	27:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	28:1.2D+1.5LM	0.500	3.421	0.000				
	29:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	30:1.2D+1.5LV	0.500	1.998	0.000		-0.000	0.500	
105	1:DEAD	0.500	1.665	0.000				
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.000		-0.000	0.000	
	12:LV	0.500				-0.000	0.000	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000				
	26:1.2D+1.5LM	0.500	3.421	0.000				
	27:1.2D+1.5LM	0.500	3.421	0.000				
	28:1.2D+1.5LM	0.500	3.421	0.000				
	29:1.2D+1.5LM	0.500	3.421	0.000				
	30:1.2D+1.5LV	0.500	1.998	0.000				
106	1:DEAD	0.500	1.665	0.000		-0.000	0.500	
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000				
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.000		-0.000	0.000	
	12:LV	0.500	0.000	0.458				
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000		-0.000	0.500	
	26:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	27:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	28:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	29:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	30:1.2D+1.5LV	0.500	1.998	0.000		-0.000	0.500	
107	1:DEAD	0.500	1.665	0.000				
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.500		-0.000	0.500	
	12:LV	0.500				-0.000	0.458	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000				
	26:1.2D+1.5LM	0.500	3.421	0.000				
	27:1.2D+1.5LM	0.500	3.421	0.000				
	28:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	29:1.2D+1.5LM	0.500	3.421	0.000				
	30:1.2D+1.5LV	0.500	1.998	0.000				
108	1:DEAD	0.500	1.665	0.000				
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	4:DI	0.500	6.471	0.000				
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.458		-0.000	0.458	
	12:LV	0.500	0.000	0.500		-0.000	0.500	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000				
	26:1.2D+1.5LM	0.500	3.421	0.000				
	27:1.2D+1.5LM	0.500	3.421	0.000				
	28:1.2D+1.5LM	0.500	3.421	0.000				
	29:1.2D+1.5LM	0.500	3.421	0.000				
	30:1.2D+1.5LV	0.500	1.998	0.000				
109	1:DEAD	0.500	1.665	0.000		-0.000	0.500	
	2:WL#1	0.500	13.096	0.000		-13.096	0.000	
	3:WL#4	0.500	13.096	0.000		-13.096	0.000	
	4:DI	0.500	6.471	0.000		-0.000	0.500	
	5:WLI#1	0.500	9.679	0.000		-9.679	0.000	
	6:WLI#4	0.500	9.679	0.000		-9.679	0.000	
	7:WLS#1	0.500	4.555	0.000		-4.555	0.000	
	8:WLS#4	0.500	4.555	0.000		-4.555	0.000	
	9:WLM#1	0.500	1.423	0.000		-1.423	0.000	
	10:WLM#4	0.500	1.423	0.000		-1.423	0.000	
	11:LM	0.500	0.000	0.000				
	12:LV	0.500	0.000	0.000		-0.000	0.000	
	13:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	14:1.2D+1.6WI	0.500	22.951	0.000		-18.955	0.000	
	15:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	16:1.2D-1.6WL	0.500	22.951	0.000		-18.955	0.000	
	17:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	18:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	19:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	20:1.2D+1.0DI·	0.500	18.148	0.000		-1.211	0.000	
	21:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	22:1.0D+1.0WI	0.500	6.220	0.000		-2.890	0.000	
	23:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	24:1.0D-1.0WL	0.500	6.220	0.000		-2.890	0.000	
	25:1.4D	0.500	2.331	0.000		-0.000	0.500	
	26:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	27:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	28:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	29:1.2D+1.5LM	0.500	3.421	0.000		-0.000	0.500	
	30:1.2D+1.5LV	0.500	1.998	0.000		-0.000	0.500	
110	1:DEAD	0.333				-42.578	0.333	
	2:WL#1	0.333	1.45E+3	0.333		-1.45E+3	0.333	
	3:WL#4	0.333	1.76E+3	0.333		-1.76E+3	0.333	
	4:DI	0.333				-281.176	0.333	
	5:WLI#1	0.333	622.077	0.333		-622.077	0.333	
	6:WLI#4	0.333	707.357	0.333		-707.357	0.333	
	7:WLS#1	0.333	509.150	0.333		-509.150	0.333	
	8:WLS#4	0.333	619.483	0.333		-619.483	0.333	
	9:WLM#1	0.333	143.641	0.333		-143.641	0.333	
	10:WLM#4	0.333	173.375	0.333		-173.375	0.333	
	11:LM	0.333	0.000	0.333		-0.000	0.333	
	12:LV	0.333	0.000	0.333		-0.000	0.333	
	13:1.2D+1.6WI	0.333	2.26E+3	0.333		-2.37E+3	0.333	
	14:1.2D+1.6WI	0.333	2.76E+3	0.333		-2.87E+3	0.333	
	15:1.2D-1.6WL	0.333	2.26E+3	0.333		-2.37E+3	0.333	
	16:1.2D-1.6WL	0.333	2.76E+3	0.333		-2.87E+3	0.333	
	17:1.2D+1.0DI·	0.333	289.806	0.333		-954.347	0.333	
	18:1.2D+1.0DI·	0.333	375.086	0.333		-1.04E+3	0.333	
	19:1.2D+1.0DI·	0.333	289.806	0.333		-954.347	0.333	
	20:1.2D+1.0DI·	0.333	375.086	0.333		-1.04E+3	0.333	
	21:1.0D+1.0WI	0.333	466.572	0.333		-551.728	0.333	
	22:1.0D+1.0WI	0.333	576.904	0.333		-662.061	0.333	
	23:1.0D-1.0WL	0.333	466.572	0.333		-551.728	0.333	
	24:1.0D-1.0WL	0.333	576.904	0.333		-662.061	0.333	
	25:1.4D	0.333				-59.610	0.333	
	26:1.2D+1.5LM	0.333	92.547	0.333		-194.735	0.333	
	27:1.2D+1.5LM	0.333	122.281	0.333		-224.470	0.333	
	28:1.2D+1.5LM	0.333	92.547	0.333		-194.735	0.333	
	29:1.2D+1.5LM	0.333	122.281	0.333		-224.470	0.333	
	30:1.2D+1.5LV	0.333				-51.094	0.333	
111	1:DEAD	0.333				-42.578	0.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	2:WL#1	0.333	1.45E+3	0.333		-1.45E+3	0.333	
	3:WL#4	0.333	1.76E+3	0.333		-1.76E+3	0.333	
	4:DI	0.333				-281.176	0.333	
	5:WLI#1	0.333	622.077	0.333		-622.077	0.333	
	6:WLI#4	0.333	707.357	0.333		-707.357	0.333	
	7:WLS#1	0.333	509.150	0.333		-509.150	0.333	
	8:WLS#4	0.333	619.483	0.333		-619.483	0.333	
	9:WLM#1	0.333	143.641	0.333		-143.641	0.333	
	10:WLM#4	0.333	173.375	0.333		-173.375	0.333	
	11:LM	0.333	0.000	0.333		-0.000	0.333	
	12:LV	0.333	0.000	0.333				
	13:1.2D+1.6WI	0.333	2.26E+3	0.333		-2.37E+3	0.333	
	14:1.2D+1.6WI	0.333	2.76E+3	0.333		-2.87E+3	0.333	
	15:1.2D-1.6WL	0.333	2.26E+3	0.333		-2.37E+3	0.333	
	16:1.2D-1.6WL	0.333	2.76E+3	0.333		-2.87E+3	0.333	
	17:1.2D+1.0DI·	0.333	289.806	0.333		-954.347	0.333	
	18:1.2D+1.0DI·	0.333	375.086	0.333		-1.04E+3	0.333	
	19:1.2D+1.0DI·	0.333	289.806	0.333		-954.347	0.333	
	20:1.2D+1.0DI·	0.333	375.086	0.333		-1.04E+3	0.333	
	21:1.0D+1.0WI	0.333	466.572	0.333		-551.728	0.333	
	22:1.0D+1.0WI	0.333	576.904	0.333		-662.061	0.333	
	23:1.0D-1.0WL	0.333	466.572	0.333		-551.728	0.333	
	24:1.0D-1.0WL	0.333	576.904	0.333		-662.061	0.333	
	25:1.4D	0.333				-59.610	0.333	
	26:1.2D+1.5LM	0.333	92.547	0.333		-194.735	0.333	
	27:1.2D+1.5LM	0.333	122.281	0.333		-224.470	0.333	
	28:1.2D+1.5LM	0.333	92.547	0.333		-194.735	0.333	
	29:1.2D+1.5LM	0.333	122.281	0.333		-224.470	0.333	
	30:1.2D+1.5LV	0.333				-51.094	0.333	
112	1:DEAD	0.333				-42.578	0.333	
	2:WL#1	0.333	1.92E+3	0.333		-1.92E+3	0.333	
	3:WL#4	0.333	1.29E+3	0.333		-1.29E+3	0.333	
	4:DI	0.333				-281.177	0.333	
	5:WLI#1	0.333	752.907	0.333		-752.907	0.333	
	6:WLI#4	0.333	578.045	0.333		-578.045	0.333	
	7:WLS#1	0.333	675.661	0.333		-675.661	0.333	
	8:WLS#4	0.333	452.971	0.333		-452.971	0.333	
	9:WLM#1	0.333	190.077	0.333		-190.077	0.333	
	10:WLM#4	0.333	126.940	0.333		-126.940	0.333	
	11:LM	0.333				-0.000	0.306	
	12:LV	0.333	0.000	0.000		-0.000	0.000	
	13:1.2D+1.6WI	0.333	3.02E+3	0.333		-3.12E+3	0.333	
	14:1.2D+1.6WI	0.333	2.01E+3	0.333		-2.11E+3	0.333	
	15:1.2D-1.6WL	0.333	3.02E+3	0.333		-3.12E+3	0.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	16:1.2D-1.6WL	0.333	2.01E+3	0.333		-2.11E+3	0.333	
	17:1.2D+1.0DI·	0.333	420.636	0.333		-1.09E+3	0.333	
	18:1.2D+1.0DI·	0.333	245.774	0.333		-910.315	0.333	
	19:1.2D+1.0DI·	0.333	420.636	0.333		-1.09E+3	0.333	
	20:1.2D+1.0DI·	0.333	245.774	0.333		-910.316	0.333	
	21:1.0D+1.0WI	0.333	633.083	0.333		-718.240	0.333	
	22:1.0D+1.0WI	0.333	410.393	0.333		-495.550	0.333	
	23:1.0D-1.0WL	0.333	633.083	0.333		-718.240	0.333	
	24:1.0D-1.0WL	0.333	410.393	0.333		-495.550	0.333	
	25:1.4D	0.333				-59.610	0.333	
	26:1.2D+1.5LM	0.333	138.983	0.333		-241.171	0.333	
	27:1.2D+1.5LM	0.333	75.845	0.333		-178.034	0.333	
	28:1.2D+1.5LM	0.333	138.983	0.333		-241.171	0.333	
	29:1.2D+1.5LM	0.333	75.845	0.333		-178.034	0.333	
	30:1.2D+1.5LV	0.333				-51.094	0.333	
113	1:DEAD	0.333				-10.127	0.333	
	2:WL#1	0.333	408.370	0.333		-408.370	0.333	
	3:WL#4	0.333	410.901	0.333		-410.901	0.333	
	4:DI	0.333				-79.706	0.333	
	5:WLI#1	0.333	193.905	0.333		-193.905	0.333	
	6:WLI#4	0.333	191.880	0.333		-191.880	0.333	
	7:WLS#1	0.333	143.989	0.333		-143.989	0.333	
	8:WLS#4	0.333	144.242	0.333		-144.242	0.333	
	9:WLM#1	0.333	40.394	0.333		-40.394	0.333	
	10:WLM#4	0.333	40.521	0.333		-40.521	0.333	
	11:LM	0.333	0.000	0.333		-0.000	0.333	
	12:LV	0.333	0.000	0.306				
	13:1.2D+1.6WI	0.333	641.239	0.333		-665.545	0.333	
	14:1.2D+1.6WI	0.333	645.288	0.333		-669.594	0.333	
	15:1.2D-1.6WL	0.333	641.239	0.333		-665.545	0.333	
	16:1.2D-1.6WL	0.333	645.288	0.333		-669.594	0.333	
	17:1.2D+1.0DI·	0.333	102.046	0.333		-285.763	0.333	
	18:1.2D+1.0DI·	0.333	100.021	0.333		-283.739	0.333	
	19:1.2D+1.0DI·	0.333	102.046	0.333		-285.763	0.333	
	20:1.2D+1.0DI·	0.333	100.021	0.333		-283.739	0.333	
	21:1.0D+1.0WI	0.333	133.862	0.333		-154.117	0.333	
	22:1.0D+1.0WI	0.333	134.115	0.333		-154.370	0.333	
	23:1.0D-1.0WL	0.333	133.862	0.333		-154.117	0.333	
	24:1.0D-1.0WL	0.333	134.115	0.333		-154.370	0.333	
	25:1.4D	0.333				-14.178	0.333	
	26:1.2D+1.5LM	0.333	28.241	0.333		-52.547	0.333	
	27:1.2D+1.5LM	0.333	28.368	0.333		-52.674	0.333	
	28:1.2D+1.5LM	0.333	28.241	0.333		-52.547	0.333	
	29:1.2D+1.5LM	0.333	28.368	0.333		-52.674	0.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	30:1.2D+1.5LV	0.333				-12.153	0.333	
114	1:DEAD	0.333				-10.127	0.333	
	2:WL#1	0.333	408.370	0.333		-408.370	0.333	
	3:WL#4	0.333	410.901	0.333		-410.901	0.333	
	4:DI	0.333				-79.706	0.333	
	5:WLI#1	0.333	193.905	0.333		-193.905	0.333	
	6:WLI#4	0.333	191.880	0.333		-191.880	0.333	
	7:WLS#1	0.333	143.989	0.333		-143.989	0.333	
	8:WLS#4	0.333	144.242	0.333		-144.242	0.333	
	9:WLM#1	0.333	40.394	0.333		-40.394	0.333	
	10:WLM#4	0.333	40.521	0.333		-40.521	0.333	
	11:LM	0.333	0.000	0.333				
	12:LV	0.333				-0.000	0.000	
	13:1.2D+1.6WI	0.333	641.239	0.333		-665.545	0.333	
	14:1.2D+1.6WI	0.333	645.288	0.333		-669.594	0.333	
	15:1.2D-1.6WL	0.333	641.239	0.333		-665.545	0.333	
	16:1.2D-1.6WL	0.333	645.288	0.333		-669.594	0.333	
	17:1.2D+1.0DI·	0.333	102.046	0.333		-285.763	0.333	
	18:1.2D+1.0DI·	0.333	100.021	0.333		-283.739	0.333	
	19:1.2D+1.0DI·	0.333	102.046	0.333		-285.763	0.333	
	20:1.2D+1.0DI·	0.333	100.021	0.333		-283.739	0.333	
	21:1.0D+1.0WI	0.333	133.862	0.333		-154.117	0.333	
	22:1.0D+1.0WI	0.333	134.115	0.333		-154.370	0.333	
	23:1.0D-1.0WL	0.333	133.862	0.333		-154.117	0.333	
	24:1.0D-1.0WL	0.333	134.115	0.333		-154.370	0.333	
	25:1.4D	0.333				-14.178	0.333	
	26:1.2D+1.5LM	0.333	28.241	0.333		-52.547	0.333	
	27:1.2D+1.5LM	0.333	28.368	0.333		-52.674	0.333	
	28:1.2D+1.5LM	0.333	28.241	0.333		-52.547	0.333	
	29:1.2D+1.5LM	0.333	28.368	0.333		-52.674	0.333	
	30:1.2D+1.5LV	0.333				-12.153	0.333	
115	1:DEAD	0.333				-12.578	0.333	
	2:WL#1	0.333	685.467	0.333		-685.467	0.333	
	3:WL#4	0.333	722.160	0.333		-722.160	0.333	
	4:DI	0.333				-132.941	0.333	
	5:WLI#1	0.333	280.450	0.333		-280.450	0.333	
	6:WLI#4	0.333	289.054	0.333		-289.054	0.333	
	7:WLS#1	0.333	241.163	0.333		-241.163	0.333	
	8:WLS#4	0.333	253.563	0.333		-253.563	0.333	
	9:WLM#1	0.333	67.724	0.333		-67.724	0.333	
	10:WLM#4	0.333	71.647	0.333		-71.647	0.333	
	11:LM	0.333	0.000	0.333		-0.000	0.333	
	12:LV	0.333	0.000	0.333		-0.000	0.333	
	13:1.2D+1.6WI	0.333	1.08E+3	0.333		-1.11E+3	0.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	14:1.2D+1.6WI	0.333	1.14E+3	0.333		-1.17E+3	0.333	
	15:1.2D-1.6WL	0.333	1.08E+3	0.333		-1.11E+3	0.333	
	16:1.2D-1.6WL	0.333	1.14E+3	0.333		-1.17E+3	0.333	
	17:1.2D+1.0DI·	0.333	132.415	0.333		-428.485	0.333	
	18:1.2D+1.0DI·	0.333	141.019	0.333		-437.089	0.333	
	19:1.2D+1.0DI·	0.333	132.415	0.333		-428.485	0.333	
	20:1.2D+1.0DI·	0.333	141.019	0.333		-437.089	0.333	
	21:1.0D+1.0WI	0.333	228.585	0.333		-253.741	0.333	
	22:1.0D+1.0WI	0.333	240.984	0.333		-266.141	0.333	
	23:1.0D-1.0WL	0.333	228.585	0.333		-253.741	0.333	
	24:1.0D-1.0WL	0.333	240.984	0.333		-266.141	0.333	
	25:1.4D	0.333				-17.610	0.333	
	26:1.2D+1.5LM	0.333	52.630	0.333		-82.818	0.333	
	27:1.2D+1.5LM	0.333	56.553	0.333		-86.741	0.333	
	28:1.2D+1.5LM	0.333	52.630	0.333		-82.818	0.333	
	29:1.2D+1.5LM	0.333	56.553	0.333		-86.741	0.333	
	30:1.2D+1.5LV	0.333				-15.094	0.333	
116	1:DEAD	0.333				-12.578	0.333	
	2:WL#1	0.333	685.467	0.333		-685.467	0.333	
	3:WL#4	0.333	722.160	0.333		-722.160	0.333	
	4:DI	0.333				-132.941	0.333	
	5:WLI#1	0.333	280.450	0.333		-280.450	0.333	
	6:WLI#4	0.333	289.054	0.333		-289.054	0.333	
	7:WLS#1	0.333	241.163	0.333		-241.163	0.333	
	8:WLS#4	0.333	253.563	0.333		-253.563	0.333	
	9:WLM#1	0.333	67.724	0.333		-67.724	0.333	
	10:WLM#4	0.333	71.647	0.333		-71.647	0.333	
	11:LM	0.333				-0.000	0.333	
	12:LV	0.333				-0.000	0.333	
	13:1.2D+1.6WI	0.333	1.08E+3	0.333		-1.11E+3	0.333	
	14:1.2D+1.6WI	0.333	1.14E+3	0.333		-1.17E+3	0.333	
	15:1.2D-1.6WL	0.333	1.08E+3	0.333		-1.11E+3	0.333	
	16:1.2D-1.6WL	0.333	1.14E+3	0.333		-1.17E+3	0.333	
	17:1.2D+1.0DI·	0.333	132.415	0.333		-428.485	0.333	
	18:1.2D+1.0DI·	0.333	141.019	0.333		-437.089	0.333	
	19:1.2D+1.0DI·	0.333	132.415	0.333		-428.485	0.333	
	20:1.2D+1.0DI·	0.333	141.019	0.333		-437.089	0.333	
	21:1.0D+1.0WI	0.333	228.585	0.333		-253.741	0.333	
	22:1.0D+1.0WI	0.333	240.984	0.333		-266.141	0.333	
	23:1.0D-1.0WL	0.333	228.585	0.333		-253.741	0.333	
	24:1.0D-1.0WL	0.333	240.984	0.333		-266.141	0.333	
	25:1.4D	0.333				-17.610	0.333	
	26:1.2D+1.5LM	0.333	52.630	0.333		-82.818	0.333	
	27:1.2D+1.5LM	0.333	56.553	0.333		-86.741	0.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	28:1.2D+1.5LM	0.333	52.630	0.333		-82.818	0.333	
	29:1.2D+1.5LM	0.333	56.553	0.333		-86.741	0.333	
	30:1.2D+1.5LV	0.333				-15.094	0.333	
117	1:DEAD	0.333				-12.578	0.333	
	2:WL#1	0.333	743.417	0.333		-743.417	0.333	
	3:WL#4	0.333	664.210	0.333		-664.210	0.333	
	4:DI	0.333				-132.941	0.333	
	5:WLI#1	0.333	295.886	0.333		-295.886	0.333	
	6:WLI#4	0.333	273.618	0.333		-273.618	0.333	
	7:WLS#1	0.333	261.154	0.333		-261.154	0.333	
	8:WLS#4	0.333	233.571	0.333		-233.571	0.333	
	9:WLM#1	0.333	73.924	0.333		-73.924	0.333	
	10:WLM#4	0.333	66.206	0.333		-66.206	0.333	
	11:LM	0.333	0.000	0.333				
	12:LV	0.333	0.000	0.333		-0.000	0.333	
	13:1.2D+1.6WI	0.333	1.17E+3	0.333		-1.2E+3	0.333	
	14:1.2D+1.6WI	0.333	1.05E+3	0.333		-1.08E+3	0.333	
	15:1.2D-1.6WL	0.333	1.17E+3	0.333		-1.2E+3	0.333	
	16:1.2D-1.6WL	0.333	1.05E+3	0.333		-1.08E+3	0.333	
	17:1.2D+1.0DI-	0.333	147.851	0.333		-443.922	0.333	
	18:1.2D+1.0DI-	0.333	125.582	0.333		-421.653	0.333	
	19:1.2D+1.0DI-	0.333	147.851	0.333		-443.922	0.333	
	20:1.2D+1.0DI-	0.333	125.582	0.333		-421.653	0.333	
	21:1.0D+1.0WI	0.333	248.576	0.333		-273.733	0.333	
	22:1.0D+1.0WI	0.333	220.993	0.333		-246.150	0.333	
	23:1.0D-1.0WL	0.333	248.576	0.333		-273.733	0.333	
	24:1.0D-1.0WL	0.333	220.993	0.333		-246.150	0.333	
	25:1.4D	0.333				-17.610	0.333	
	26:1.2D+1.5LM	0.333	58.830	0.333		-89.018	0.333	
	27:1.2D+1.5LM	0.333	51.112	0.333		-81.300	0.333	
	28:1.2D+1.5LM	0.333	58.830	0.333		-89.018	0.333	
	29:1.2D+1.5LM	0.333	51.112	0.333		-81.300	0.333	
	30:1.2D+1.5LV	0.333				-15.094	0.333	
118	1:DEAD	0.333				-12.578	0.333	
	2:WL#1	0.333	743.417	0.333		-743.417	0.333	
	3:WL#4	0.333	664.210	0.333		-664.210	0.333	
	4:DI	0.333				-132.941	0.333	
	5:WLI#1	0.333	295.886	0.333		-295.886	0.333	
	6:WLI#4	0.333	273.618	0.333		-273.618	0.333	
	7:WLS#1	0.333	261.154	0.333		-261.154	0.333	
	8:WLS#4	0.333	233.571	0.333		-233.571	0.333	
	9:WLM#1	0.333	73.924	0.333		-73.924	0.333	
	10:WLM#4	0.333	66.206	0.333		-66.206	0.333	
	11:LM	0.333	0.000	0.333				

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	12:LV	0.333	0.000	0.333				
	13:1.2D+1.6WI	0.333	1.17E+3	0.333		-1.2E+3	0.333	
	14:1.2D+1.6WI	0.333	1.05E+3	0.333		-1.08E+3	0.333	
	15:1.2D-1.6WL	0.333	1.17E+3	0.333		-1.2E+3	0.333	
	16:1.2D-1.6WL	0.333	1.05E+3	0.333		-1.08E+3	0.333	
	17:1.2D+1.0DI·	0.333	147.851	0.333		-443.922	0.333	
	18:1.2D+1.0DI·	0.333	125.582	0.333		-421.653	0.333	
	19:1.2D+1.0DI·	0.333	147.851	0.333		-443.922	0.333	
	20:1.2D+1.0DI·	0.333	125.582	0.333		-421.653	0.333	
	21:1.0D+1.0WI	0.333	248.576	0.333		-273.733	0.333	
	22:1.0D+1.0WI	0.333	220.993	0.333		-246.150	0.333	
	23:1.0D-1.0WL	0.333	248.576	0.333		-273.733	0.333	
	24:1.0D-1.0WL	0.333	220.993	0.333		-246.150	0.333	
	25:1.4D	0.333				-17.610	0.333	
	26:1.2D+1.5LM	0.333	58.830	0.333		-89.018	0.333	
	27:1.2D+1.5LM	0.333	51.112	0.333		-81.300	0.333	
	28:1.2D+1.5LM	0.333	58.830	0.333		-89.018	0.333	
	29:1.2D+1.5LM	0.333	51.112	0.333		-81.300	0.333	
	30:1.2D+1.5LV	0.333				-15.094	0.333	
122	1:DEAD	0.521	3.25E+3	0.000	3	-3.21E+3	0.000	1
	2:WL#1	0.521	2.99E+3	0.521	4	-3.34E+3	0.521	2
	3:WL#4	0.521	4.42E+3	0.521	1	-3.79E+3	0.521	3
	4:DI	0.521	13.4E+3	0.000	3	-13.2E+3	0.000	1
	5:WLI#1	0.521	1.41E+3	0.521	4	-1.58E+3	0.521	2
	6:WLI#4	0.521	2.03E+3	0.521	1	-1.72E+3	0.521	3
	7:WLS#1	0.521	1.05E+3	0.521	4	-1.17E+3	0.521	2
	8:WLS#4	0.521	1.55E+3	0.521	1	-1.33E+3	0.521	3
	9:WLM#1	0.521	300.722	0.521	4	-335.571	0.521	2
	10:WLM#4	0.521	441.587	0.521	1	-378.450	0.521	3
	11:LM	0.521	4.434	0.000	2	-4.534	0.000	1
	12:LV	0.521	378.271	0.000	3	-378.066	0.000	1
	13:1.2D+1.6WI	0.521	7.63E+3	0.000	3	-8.14E+3	0.000	1
	14:1.2D+1.6WI	0.521	4.32E+3	0.521	1	-3.26E+3	0.521	3
	15:1.2D-1.6WL	0.521	2.59E+3	0.521	2	-1.99E+3	0.521	4
	16:1.2D-1.6WL	0.521	9.88E+3	0.000	3	-10.8E+3	0.000	1
	17:1.2D+1.0DI·	0.521	18.4E+3	0.000	3	-18.3E+3	0.000	1
	18:1.2D+1.0DI·	0.521	15.8E+3	0.000	3	-15.2E+3	0.000	1
	19:1.2D+1.0DI·	0.521	16.5E+3	0.000	3	-16.1E+3	0.000	1
	20:1.2D+1.0DI·	0.521	19E+3	0.000	3	-19E+3	0.000	1
	21:1.0D+1.0WI	0.521	4.07E+3	0.000	3	-4.15E+3	0.000	1
	22:1.0D+1.0WI	0.521	2.06E+3	0.000	3	-1.8E+3	0.000	1
	23:1.0D-1.0WL	0.521	2.65E+3	0.000	3	-2.49E+3	0.000	1
	24:1.0D-1.0WL	0.521	4.56E+3	0.000	3	-4.75E+3	0.000	1
	25:1.4D	0.521	4.55E+3	0.000	3	-4.5E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	26:1.2D+1.5LM	0.521	4.14E+3	0.000	3	-4.13E+3	0.000	1
	27:1.2D+1.5LM	0.521	3.57E+3	0.000	3	-3.46E+3	0.000	1
	28:1.2D+1.5LM	0.521	3.73E+3	0.000	3	-3.65E+3	0.000	1
	29:1.2D+1.5LM	0.521	4.27E+3	0.000	3	-4.29E+3	0.000	1
	30:1.2D+1.5LV	0.521	4.47E+3	0.000	3	-4.42E+3	0.000	1
123	1:DEAD	0.667	13.688	0.667				
	2:WL#1	0.667	1.37E+3	0.667		-1.37E+3	0.667	
	3:WL#4	0.667	1.43E+3	0.667		-1.43E+3	0.667	
	4:DI	0.667	137.255	0.667				
	5:WLI#1	0.667	559.828	0.667		-559.828	0.667	
	6:WLI#4	0.667	568.432	0.667		-568.432	0.667	
	7:WLS#1	0.667	481.823	0.667		-481.823	0.667	
	8:WLS#4	0.667	502.573	0.667		-502.573	0.667	
	9:WLM#1	0.667	135.291	0.667		-135.291	0.667	
	10:WLM#4	0.667	141.871	0.667		-141.871	0.667	
	11:LM	0.667	0.000	0.667		-0.000	0.667	
	12:LV	0.667				-0.000	0.000	
	13:1.2D+1.6WI	0.667	2.21E+3	0.667		-2.17E+3	0.667	
	14:1.2D+1.6WI	0.667	2.31E+3	0.667		-2.27E+3	0.667	
	15:1.2D-1.6WL	0.667	2.21E+3	0.667		-2.17E+3	0.667	
	16:1.2D-1.6WL	0.667	2.31E+3	0.667		-2.27E+3	0.667	
	17:1.2D+1.0DI·	0.667	713.509	0.667		-406.147	0.667	
	18:1.2D+1.0DI·	0.667	722.113	0.667		-414.751	0.667	
	19:1.2D+1.0DI·	0.667	713.509	0.667		-406.147	0.667	
	20:1.2D+1.0DI·	0.667	722.113	0.667		-414.751	0.667	
	21:1.0D+1.0WI	0.667	495.511	0.667		-468.134	0.667	
	22:1.0D+1.0WI	0.667	516.262	0.667		-488.885	0.667	
	23:1.0D-1.0WL	0.667	495.511	0.667		-468.134	0.667	
	24:1.0D-1.0WL	0.667	516.262	0.667		-488.885	0.667	
	25:1.4D	0.667	19.164	0.667				
	26:1.2D+1.5LM	0.667	151.717	0.667		-118.865	0.667	
	27:1.2D+1.5LM	0.667	158.297	0.667		-125.445	0.667	
	28:1.2D+1.5LM	0.667	151.717	0.667		-118.865	0.667	
	29:1.2D+1.5LM	0.667	158.297	0.667		-125.445	0.667	
	30:1.2D+1.5LV	0.667	16.426	0.667				
124	1:DEAD	0.667	11.237	0.667				
	2:WL#1	0.667	815.290	0.667		-815.290	0.667	
	3:WL#4	0.667	808.710	0.667		-808.710	0.667	
	4:DI	0.667	84.020	0.667				
	5:WLI#1	0.667	386.736	0.667		-386.736	0.667	
	6:WLI#4	0.667	374.083	0.667		-374.083	0.667	
	7:WLS#1	0.667	287.474	0.667		-287.474	0.667	
	8:WLS#4	0.667	283.931	0.667		-283.931	0.667	
	9:WLM#1	0.667	80.631	0.667		-80.631	0.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	10:WLM#4	0.667	79.618	0.667		-79.618	0.667	
	11:LM	0.667	0.000	0.667		-0.000	0.667	
	12:LV	0.667	0.000	0.667		-0.000	0.667	
	13:1.2D+1.6WI	0.667	1.32E+3	0.667		-1.29E+3	0.667	
	14:1.2D+1.6WI	0.667	1.31E+3	0.667		-1.28E+3	0.667	
	15:1.2D-1.6WL	0.667	1.32E+3	0.667		-1.29E+3	0.667	
	16:1.2D-1.6WL	0.667	1.31E+3	0.667		-1.28E+3	0.667	
	17:1.2D+1.0DI·	0.667	484.240	0.667		-289.232	0.667	
	18:1.2D+1.0DI·	0.667	471.587	0.667		-276.579	0.667	
	19:1.2D+1.0DI·	0.667	484.240	0.667		-289.232	0.667	
	20:1.2D+1.0DI·	0.667	471.587	0.667		-276.579	0.667	
	21:1.0D+1.0WI	0.667	298.711	0.667		-276.237	0.667	
	22:1.0D+1.0WI	0.667	295.168	0.667		-272.694	0.667	
	23:1.0D-1.0WL	0.667	298.711	0.667		-276.237	0.667	
	24:1.0D-1.0WL	0.667	295.168	0.667		-272.694	0.667	
	25:1.4D	0.667	15.732	0.667				
	26:1.2D+1.5LM	0.667	94.115	0.667		-67.146	0.667	
	27:1.2D+1.5LM	0.667	93.103	0.667		-66.134	0.667	
	28:1.2D+1.5LM	0.667	94.115	0.667		-67.146	0.667	
	29:1.2D+1.5LM	0.667	93.103	0.667		-66.134	0.667	
	30:1.2D+1.5LV	0.667	13.485	0.667				
125	1:DEAD	0.667	43.688	0.667				
	2:WL#1	0.667	2.89E+3	0.667		-2.89E+3	0.667	
	3:WL#4	0.667	3.51E+3	0.667		-3.51E+3	0.667	
	4:DI	0.667	285.490	0.667				
	5:WLI#1	0.667	1.24E+3	0.667		-1.24E+3	0.667	
	6:WLI#4	0.667	1.41E+3	0.667		-1.41E+3	0.667	
	7:WLS#1	0.667	1.02E+3	0.667		-1.02E+3	0.667	
	8:WLS#4	0.667	1.23E+3	0.667		-1.23E+3	0.667	
	9:WLM#1	0.667	287.126	0.667		-287.126	0.667	
	10:WLM#4	0.667	345.329	0.667		-345.329	0.667	
	11:LM	0.667	0.000	0.611		-0.000	0.611	
	12:LV	0.667	0.000	0.667		-0.000	0.667	
	13:1.2D+1.6WI	0.667	4.68E+3	0.667		-4.58E+3	0.667	
	14:1.2D+1.6WI	0.667	5.66E+3	0.667		-5.56E+3	0.667	
	15:1.2D-1.6WL	0.667	4.68E+3	0.667		-4.58E+3	0.667	
	16:1.2D-1.6WL	0.667	5.66E+3	0.667		-5.56E+3	0.667	
	17:1.2D+1.0DI·	0.667	1.58E+3	0.667		-905.169	0.667	
	18:1.2D+1.0DI·	0.667	1.74E+3	0.667		-1.07E+3	0.667	
	19:1.2D+1.0DI·	0.667	1.58E+3	0.667		-905.169	0.667	
	20:1.2D+1.0DI·	0.667	1.74E+3	0.667		-1.07E+3	0.667	
	21:1.0D+1.0WI	0.667	1.06E+3	0.667		-974.111	0.667	
	22:1.0D+1.0WI	0.667	1.28E+3	0.667		-1.19E+3	0.667	
	23:1.0D-1.0WL	0.667	1.06E+3	0.667		-974.111	0.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	24:1.0D-1.0WL	0.667	1.28E+3	0.667		-1.19E+3	0.667	
	25:1.4D	0.667	61.164	0.667				
	26:1.2D+1.5LM	0.667	339.552	0.667		-234.700	0.667	
	27:1.2D+1.5LM	0.667	397.755	0.667		-292.904	0.667	
	28:1.2D+1.5LM	0.667	339.552	0.667		-234.700	0.667	
	29:1.2D+1.5LM	0.667	397.755	0.667		-292.904	0.667	
	30:1.2D+1.5LV	0.667	52.426	0.667				
127	1:DEAD	0.667	11.237	0.667				
	2:WL#1	0.667	815.290	0.667		-815.290	0.667	
	3:WL#4	0.667	808.710	0.667		-808.710	0.667	
	4:DI	0.667	84.020	0.667				
	5:WLI#1	0.667	386.736	0.667		-386.736	0.667	
	6:WLI#4	0.667	374.083	0.667		-374.083	0.667	
	7:WLS#1	0.667	287.474	0.667		-287.474	0.667	
	8:WLS#4	0.667	283.931	0.667		-283.931	0.667	
	9:WLM#1	0.667	80.631	0.667		-80.631	0.667	
	10:WLM#4	0.667	79.618	0.667		-79.618	0.667	
	11:LM	0.667	0.000	0.611		-0.000	0.611	
	12:LV	0.667	0.000	0.611				
	13:1.2D+1.6WI	0.667	1.32E+3	0.667		-1.29E+3	0.667	
	14:1.2D+1.6WI	0.667	1.31E+3	0.667		-1.28E+3	0.667	
	15:1.2D-1.6WL	0.667	1.32E+3	0.667		-1.29E+3	0.667	
	16:1.2D-1.6WL	0.667	1.31E+3	0.667		-1.28E+3	0.667	
	17:1.2D+1.0DI·	0.667	484.240	0.667		-289.232	0.667	
	18:1.2D+1.0DI·	0.667	471.587	0.667		-276.579	0.667	
	19:1.2D+1.0DI·	0.667	484.240	0.667		-289.232	0.667	
	20:1.2D+1.0DI·	0.667	471.587	0.667		-276.579	0.667	
	21:1.0D+1.0WI	0.667	298.711	0.667		-276.237	0.667	
	22:1.0D+1.0WI	0.667	295.168	0.667		-272.694	0.667	
	23:1.0D-1.0WL	0.667	298.711	0.667		-276.237	0.667	
	24:1.0D-1.0WL	0.667	295.168	0.667		-272.694	0.667	
	25:1.4D	0.667	15.732	0.667				
	26:1.2D+1.5LM	0.667	94.115	0.667		-67.146	0.667	
	27:1.2D+1.5LM	0.667	93.103	0.667		-66.134	0.667	
	28:1.2D+1.5LM	0.667	94.115	0.667		-67.146	0.667	
	29:1.2D+1.5LM	0.667	93.103	0.667		-66.134	0.667	
	30:1.2D+1.5LV	0.667	13.485	0.667				
128	1:DEAD	0.667	43.688	0.667				
	2:WL#1	0.667	3.83E+3	0.667		-3.83E+3	0.667	
	3:WL#4	0.667	2.57E+3	0.667		-2.57E+3	0.667	
	4:DI	0.667	285.490	0.667				
	5:WLI#1	0.667	1.5E+3	0.667		-1.5E+3	0.667	
	6:WLI#4	0.667	1.16E+3	0.667		-1.16E+3	0.667	
	7:WLS#1	0.667	1.35E+3	0.667		-1.35E+3	0.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	8:WLS#4	0.667	905.442	0.667		-905.442	0.667	
	9:WLM#1	0.667	378.733	0.667		-378.733	0.667	
	10:WLM#4	0.667	253.722	0.667		-253.722	0.667	
	11:LM	0.667				-0.000	0.000	
	12:LV	0.667	0.000	0.000		-0.000	0.000	
	13:1.2D+1.6WI	0.667	6.17E+3	0.667		-6.07E+3	0.667	
	14:1.2D+1.6WI	0.667	4.17E+3	0.667		-4.07E+3	0.667	
	15:1.2D-1.6WL	0.667	6.17E+3	0.667		-6.07E+3	0.667	
	16:1.2D-1.6WL	0.667	4.17E+3	0.667		-4.07E+3	0.667	
	17:1.2D+1.0DI·	0.667	1.83E+3	0.667		-1.16E+3	0.667	
	18:1.2D+1.0DI·	0.667	1.49E+3	0.667		-817.104	0.667	
	19:1.2D+1.0DI·	0.667	1.83E+3	0.667		-1.16E+3	0.667	
	20:1.2D+1.0DI·	0.667	1.49E+3	0.667		-817.104	0.667	
	21:1.0D+1.0WI	0.667	1.39E+3	0.667		-1.3E+3	0.667	
	22:1.0D+1.0WI	0.667	949.130	0.667		-861.754	0.667	
	23:1.0D-1.0WL	0.667	1.39E+3	0.667		-1.3E+3	0.667	
	24:1.0D-1.0WL	0.667	949.130	0.667		-861.754	0.667	
	25:1.4D	0.667	61.164	0.667				
	26:1.2D+1.5LM	0.667	431.159	0.667		-326.307	0.667	
	27:1.2D+1.5LM	0.667	306.148	0.667		-201.297	0.667	
	28:1.2D+1.5LM	0.667	431.159	0.667		-326.307	0.667	
	29:1.2D+1.5LM	0.667	306.148	0.667		-201.296	0.667	
	30:1.2D+1.5LV	0.667	52.426	0.667				
129	1:DEAD	0.667	13.688	0.667				
	2:WL#1	0.667	1.47E+3	0.667		-1.47E+3	0.667	
	3:WL#4	0.667	1.33E+3	0.667		-1.33E+3	0.667	
	4:DI	0.667	137.255	0.667				
	5:WLI#1	0.667	582.097	0.667		-582.097	0.667	
	6:WLI#4	0.667	546.163	0.667		-546.163	0.667	
	7:WLS#1	0.667	517.757	0.667		-517.757	0.667	
	8:WLS#4	0.667	466.639	0.667		-466.639	0.667	
	9:WLM#1	0.667	146.426	0.667		-146.426	0.667	
	10:WLM#4	0.667	132.255	0.667		-132.255	0.667	
	11:LM	0.667				-0.000	0.667	
	12:LV	0.667	0.000	0.611		-0.000	0.611	
	13:1.2D+1.6WI	0.667	2.37E+3	0.667		-2.34E+3	0.667	
	14:1.2D+1.6WI	0.667	2.14E+3	0.667		-2.11E+3	0.667	
	15:1.2D-1.6WL	0.667	2.37E+3	0.667		-2.34E+3	0.667	
	16:1.2D-1.6WL	0.667	2.14E+3	0.667		-2.11E+3	0.667	
	17:1.2D+1.0DI·	0.667	735.778	0.667		-428.416	0.667	
	18:1.2D+1.0DI·	0.667	699.844	0.667		-392.482	0.667	
	19:1.2D+1.0DI·	0.667	735.778	0.667		-428.416	0.667	
	20:1.2D+1.0DI·	0.667	699.844	0.667		-392.482	0.667	
	21:1.0D+1.0WI	0.667	531.445	0.667		-504.069	0.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	22:1.0D+1.0WI	0.667	480.327	0.667		-452.951	0.667	
	23:1.0D-1.0WL	0.667	531.445	0.667		-504.069	0.667	
	24:1.0D-1.0WL	0.667	480.327	0.667		-452.951	0.667	
	25:1.4D	0.667	19.164	0.667				
	26:1.2D+1.5LM	0.667	162.852	0.667		-130.000	0.667	
	27:1.2D+1.5LM	0.667	148.680	0.667		-115.829	0.667	
	28:1.2D+1.5LM	0.667	162.852	0.667		-130.000	0.667	
	29:1.2D+1.5LM	0.667	148.680	0.667		-115.829	0.667	
	30:1.2D+1.5LV	0.667	16.426	0.667				
130	1:DEAD	0.667	13.688	0.667				
	2:WL#1	0.667	1.37E+3	0.667		-1.37E+3	0.667	
	3:WL#4	0.667	1.43E+3	0.667		-1.43E+3	0.667	
	4:DI	0.667	137.255	0.667				
	5:WLI#1	0.667	559.828	0.667		-559.828	0.667	
	6:WLI#4	0.667	568.432	0.667		-568.432	0.667	
	7:WLS#1	0.667	481.823	0.667		-481.823	0.667	
	8:WLS#4	0.667	502.573	0.667		-502.573	0.667	
	9:WLM#1	0.667	135.291	0.667		-135.291	0.667	
	10:WLM#4	0.667	141.871	0.667		-141.871	0.667	
	11:LM	0.667	0.000	0.611		-0.000	0.611	
	12:LV	0.667				-0.000	0.611	
	13:1.2D+1.6WI	0.667	2.21E+3	0.667		-2.17E+3	0.667	
	14:1.2D+1.6WI	0.667	2.31E+3	0.667		-2.27E+3	0.667	
	15:1.2D-1.6WL	0.667	2.21E+3	0.667		-2.17E+3	0.667	
	16:1.2D-1.6WL	0.667	2.31E+3	0.667		-2.27E+3	0.667	
	17:1.2D+1.0DI	0.667	713.509	0.667		-406.147	0.667	
	18:1.2D+1.0DI	0.667	722.113	0.667		-414.751	0.667	
	19:1.2D+1.0DI	0.667	713.509	0.667		-406.147	0.667	
	20:1.2D+1.0DI	0.667	722.113	0.667		-414.751	0.667	
	21:1.0D+1.0WI	0.667	495.511	0.667		-468.134	0.667	
	22:1.0D+1.0WI	0.667	516.262	0.667		-488.885	0.667	
	23:1.0D-1.0WL	0.667	495.511	0.667		-468.134	0.667	
	24:1.0D-1.0WL	0.667	516.262	0.667		-488.885	0.667	
	25:1.4D	0.667	19.164	0.667				
	26:1.2D+1.5LM	0.667	151.717	0.667		-118.865	0.667	
	27:1.2D+1.5LM	0.667	158.297	0.667		-125.445	0.667	
	28:1.2D+1.5LM	0.667	151.717	0.667		-118.865	0.667	
	29:1.2D+1.5LM	0.667	158.297	0.667		-125.445	0.667	
	30:1.2D+1.5LV	0.667	16.426	0.667				
131	1:DEAD	0.667	43.688	0.667				
	2:WL#1	0.667	2.89E+3	0.667		-2.89E+3	0.667	
	3:WL#4	0.667	3.51E+3	0.667		-3.51E+3	0.667	
	4:DI	0.667	285.490	0.667				
	5:WLI#1	0.667	1.24E+3	0.667		-1.24E+3	0.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	6:WLI#4	0.667	1.41E+3	0.667		-1.41E+3	0.667	
	7:WLS#1	0.667	1.02E+3	0.667		-1.02E+3	0.667	
	8:WLS#4	0.667	1.23E+3	0.667		-1.23E+3	0.667	
	9:WLM#1	0.667	287.126	0.667		-287.126	0.667	
	10:WLM#4	0.667	345.329	0.667		-345.329	0.667	
	11:LM	0.667	0.000	0.667		-0.000	0.667	
	12:LV	0.667				-0.000	0.667	
	13:1.2D+1.6WI	0.667	4.68E+3	0.667		-4.58E+3	0.667	
	14:1.2D+1.6WI	0.667	5.66E+3	0.667		-5.56E+3	0.667	
	15:1.2D-1.6WL	0.667	4.68E+3	0.667		-4.58E+3	0.667	
	16:1.2D-1.6WL	0.667	5.66E+3	0.667		-5.56E+3	0.667	
	17:1.2D+1.0DI·	0.667	1.58E+3	0.667		-905.169	0.667	
	18:1.2D+1.0DI·	0.667	1.74E+3	0.667		-1.07E+3	0.667	
	19:1.2D+1.0DI·	0.667	1.58E+3	0.667		-905.169	0.667	
	20:1.2D+1.0DI·	0.667	1.74E+3	0.667		-1.07E+3	0.667	
	21:1.0D+1.0WI	0.667	1.06E+3	0.667		-974.111	0.667	
	22:1.0D+1.0WI	0.667	1.28E+3	0.667		-1.19E+3	0.667	
	23:1.0D-1.0WL	0.667	1.06E+3	0.667		-974.111	0.667	
	24:1.0D-1.0WL	0.667	1.28E+3	0.667		-1.19E+3	0.667	
	25:1.4D	0.667	61.164	0.667				
	26:1.2D+1.5LM	0.667	339.552	0.667		-234.700	0.667	
	27:1.2D+1.5LM	0.667	397.755	0.667		-292.904	0.667	
	28:1.2D+1.5LM	0.667	339.552	0.667		-234.700	0.667	
	29:1.2D+1.5LM	0.667	397.755	0.667		-292.904	0.667	
	30:1.2D+1.5LV	0.667	52.426	0.667				
132	1:DEAD	0.667	13.688	0.667				
	2:WL#1	0.667	1.47E+3	0.667		-1.47E+3	0.667	
	3:WL#4	0.667	1.33E+3	0.667		-1.33E+3	0.667	
	4:DI	0.667	137.255	0.667				
	5:WLI#1	0.667	582.097	0.667		-582.097	0.667	
	6:WLI#4	0.667	546.163	0.667		-546.163	0.667	
	7:WLS#1	0.667	517.757	0.667		-517.757	0.667	
	8:WLS#4	0.667	466.639	0.667		-466.639	0.667	
	9:WLM#1	0.667	146.426	0.667		-146.426	0.667	
	10:WLM#4	0.667	132.255	0.667		-132.255	0.667	
	11:LM	0.667	0.000	0.667				
	12:LV	0.667	0.000	0.000		-0.000	0.000	
	13:1.2D+1.6WI	0.667	2.37E+3	0.667		-2.34E+3	0.667	
	14:1.2D+1.6WI	0.667	2.14E+3	0.667		-2.11E+3	0.667	
	15:1.2D-1.6WL	0.667	2.37E+3	0.667		-2.34E+3	0.667	
	16:1.2D-1.6WL	0.667	2.14E+3	0.667		-2.11E+3	0.667	
	17:1.2D+1.0DI·	0.667	735.778	0.667		-428.416	0.667	
	18:1.2D+1.0DI·	0.667	699.844	0.667		-392.482	0.667	
	19:1.2D+1.0DI·	0.667	735.778	0.667		-428.416	0.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	20:1.2D+1.0DI·	0.667	699.844	0.667		-392.482	0.667	
	21:1.0D+1.0WI	0.667	531.445	0.667		-504.069	0.667	
	22:1.0D+1.0WI	0.667	480.327	0.667		-452.951	0.667	
	23:1.0D-1.0WL	0.667	531.445	0.667		-504.069	0.667	
	24:1.0D-1.0WL	0.667	480.327	0.667		-452.951	0.667	
	25:1.4D	0.667	19.164	0.667				
	26:1.2D+1.5LM	0.667	162.852	0.667		-130.000	0.667	
	27:1.2D+1.5LM	0.667	148.680	0.667		-115.829	0.667	
	28:1.2D+1.5LM	0.667	162.852	0.667		-130.000	0.667	
	29:1.2D+1.5LM	0.667	148.680	0.667		-115.829	0.667	
	30:1.2D+1.5LV	0.667	16.426	0.667				
133	1:DEAD	1.179	1.03E+3	1.179	2	-2.3E+3	1.179	1
	2:WL#1	1.179	2.86E+3	1.179	1	-1.6E+3	1.179	2
	3:WL#4	1.179	3.13E+3	1.179	1	-1.11E+3	1.179	2
	4:DI	1.179	4.55E+3	1.179	2	-9.97E+3	1.179	1
	5:WLI#1	1.179	1.34E+3	1.179	1	-749.279	1.179	2
	6:WLI#4	1.179	1.45E+3	1.179	1	-508.580	1.179	2
	7:WLS#1	1.179	1E+3	1.179	1	-562.114	1.179	2
	8:WLS#4	1.179	1.1E+3	1.179	1	-389.179	1.179	2
	9:WLM#1	1.179	287.382	1.179	1	-160.887	1.179	2
	10:WLM#4	1.179	312.997	1.179	1	-110.723	1.179	2
	11:LM	1.179	9.399	0.000	2	-19.138	1.179	1
	12:LV	1.179	18.192	1.179	2	-45.050	1.179	1
	13:1.2D+1.6WI	1.179	2.37E+3	0.000	1	-1.32E+3	1.179	2
	14:1.2D+1.6WI	1.179	2.99E+3	0.000	1	-1.14E+3	0.000	2
	15:1.2D-1.6WL	1.179	3.8E+3	1.179	2	-7.34E+3	1.179	1
	16:1.2D-1.6WL	1.179	3.01E+3	1.179	2	-7.77E+3	1.179	1
	17:1.2D+1.0DI·	1.179	5.04E+3	1.179	2	-11.4E+3	1.179	1
	18:1.2D+1.0DI·	1.179	5.28E+3	1.179	2	-11.3E+3	1.179	1
	19:1.2D+1.0DI·	1.179	6.54E+3	1.179	2	-14.1E+3	1.179	1
	20:1.2D+1.0DI·	1.179	6.3E+3	1.179	2	-14.2E+3	1.179	1
	21:1.0D+1.0WI	1.179	642.177	1.179	3	-1.3E+3	1.179	1
	22:1.0D+1.0WI	1.179	643.151	1.179	2	-1.21E+3	1.179	1
	23:1.0D-1.0WL	1.179	1.59E+3	1.179	2	-3.31E+3	1.179	1
	24:1.0D-1.0WL	1.179	1.42E+3	1.179	2	-3.4E+3	1.179	1
	25:1.4D	1.179	1.45E+3	1.179	2	-3.23E+3	1.179	1
	26:1.2D+1.5LM	1.179	1.09E+3	1.179	2	-2.51E+3	1.179	1
	27:1.2D+1.5LM	1.179	1.14E+3	1.179	2	-2.48E+3	1.179	1
	28:1.2D+1.5LM	1.179	1.41E+3	1.179	2	-3.08E+3	1.179	1
	29:1.2D+1.5LM	1.179	1.36E+3	1.179	2	-3.11E+3	1.179	1
	30:1.2D+1.5LV	1.179	1.27E+3	1.179	2	-2.83E+3	1.179	1
134	1:DEAD	1.179	1.65E+3	1.179	3	-4.04E+3	1.179	1
	2:WL#1	1.179	3.83E+3	1.179	1	-1.66E+3	1.179	2
	3:WL#4	1.179	3.49E+3	1.179	1	-1.69E+3	1.179	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	4:DI	1.179	6.18E+3	1.179	3	-15.3E+3	1.179	1
	5:WLI#1	1.179	1.8E+3	1.179	1	-789.107	1.179	2
	6:WLI#4	1.179	1.61E+3	1.179	1	-803.014	1.179	2
	7:WLS#1	1.179	1.35E+3	1.179	1	-584.130	1.179	2
	8:WLS#4	1.179	1.23E+3	1.179	1	-592.478	1.179	2
	9:WLM#1	1.179	385.518	1.179	1	-167.345	1.179	2
	10:WLM#4	1.179	349.354	1.179	1	-169.066	1.179	2
	11:LM	1.179	8.573	0.000	2	-18.591	1.179	1
	12:LV	1.179	27.474	1.179	3	-62.689	1.179	1
	13:1.2D+1.6WI	1.179	2.4E+3	0.000	1	-885.748	1.179	2
	14:1.2D+1.6WI	1.179	2.54E+3	0.000	1	-924.391	1.179	2
	15:1.2D-1.6WL	1.179	4.44E+3	1.179	2	-11E+3	1.179	1
	16:1.2D-1.6WL	1.179	4.47E+3	1.179	2	-10.4E+3	1.179	1
	17:1.2D+1.0DI·	1.179	7.81E+3	1.179	3	-18.3E+3	1.179	1
	18:1.2D+1.0DI·	1.179	7.99E+3	1.179	3	-18.5E+3	1.179	1
	19:1.2D+1.0DI·	1.179	8.52E+3	1.179	3	-21.9E+3	1.179	1
	20:1.2D+1.0DI·	1.179	8.34E+3	1.179	2	-21.8E+3	1.179	1
	21:1.0D+1.0WI	1.179	1.36E+3	1.179	3	-2.7E+3	1.179	1
	22:1.0D+1.0WI	1.179	1.48E+3	1.179	3	-2.82E+3	1.179	1
	23:1.0D-1.0WL	1.179	2.06E+3	1.179	2	-5.39E+3	1.179	1
	24:1.0D-1.0WL	1.179	2.07E+3	1.179	2	-5.27E+3	1.179	1
	25:1.4D	1.179	2.31E+3	1.179	3	-5.66E+3	1.179	1
	26:1.2D+1.5LM	1.179	1.91E+3	1.179	3	-4.5E+3	1.179	1
	27:1.2D+1.5LM	1.179	1.95E+3	1.179	3	-4.53E+3	1.179	1
	28:1.2D+1.5LM	1.179	2.08E+3	1.179	3	-5.27E+3	1.179	1
	29:1.2D+1.5LM	1.179	2.04E+3	1.179	3	-5.23E+3	1.179	1
	30:1.2D+1.5LV	1.179	2.02E+3	1.179	3	-4.95E+3	1.179	1
135	1:DEAD	1.889	784.539	1.889	3	-1.48E+3	1.889	2
	2:WL#1	1.889	2.7E+3	1.889	2	-1.69E+3	1.889	3
	3:WL#4	1.889	2.84E+3	0.000	2	-988.057	0.000	1
	4:DI	1.889	3.78E+3	1.889	3	-7.38E+3	1.889	2
	5:WLI#1	1.889	1.27E+3	1.889	2	-828.960	1.889	3
	6:WLI#4	1.889	1.33E+3	0.000	2	-461.392	0.000	1
	7:WLS#1	1.889	947.046	1.889	2	-593.629	1.889	3
	8:WLS#4	1.889	995.587	0.000	2	-346.937	0.000	1
	9:WLM#1	1.889	271.429	1.889	2	-170.486	1.889	3
	10:WLM#4	1.889	284.321	0.000	2	-99.026	0.000	1
	11:LM	1.889	32.479	1.889	2	-18.474	0.000	2
	12:LV	1.889	8.238	1.889	3	-17.381	1.889	2
	13:1.2D+1.6WI	1.889	2.54E+3	1.889	2	-1.76E+3	1.889	3
	14:1.2D+1.6WI	1.889	5.1E+3	0.000	2	-1.74E+3	0.000	1
	15:1.2D-1.6WL	1.889	3.65E+3	1.889	3	-6.09E+3	1.889	2
	16:1.2D-1.6WL	1.889	1.51E+3	1.889	3	-4.33E+3	1.889	2
	17:1.2D+1.0DI·	1.889	3.89E+3	1.889	3	-7.88E+3	1.889	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	18:1.2D+1.0DI·	1.889	4.57E+3	1.889	3	-8.42E+3	1.889	2
	19:1.2D+1.0DI·	1.889	5.55E+3	1.889	3	-10.4E+3	1.889	2
	20:1.2D+1.0DI·	1.889	4.88E+3	1.889	3	-9.88E+3	1.889	2
	21:1.0D+1.0WI	1.889	190.909	1.889	3	-532.136	1.889	2
	22:1.0D+1.0WI	1.889	1.47E+3	0.000	2	-918.724	1.889	2
	23:1.0D-1.0WL	1.889	1.38E+3	1.889	3	-2.43E+3	1.889	2
	24:1.0D-1.0WL	1.889	908.393	1.889	3	-2.04E+3	1.889	2
	25:1.4D	1.889	1.1E+3	1.889	3	-2.07E+3	1.889	2
	26:1.2D+1.5LM	1.889	748.869	1.889	3	-1.45E+3	1.889	2
	27:1.2D+1.5LM	1.889	884.374	1.889	3	-1.57E+3	1.889	2
	28:1.2D+1.5LM	1.889	1.09E+3	1.889	3	-2E+3	1.889	2
	29:1.2D+1.5LM	1.889	954.337	1.889	3	-1.89E+3	1.889	2
	30:1.2D+1.5LV	1.889	953.804	1.889	3	-1.8E+3	1.889	2
136	1:DEAD	1.889	1.85E+3	1.889	3	-4.4E+3	1.889	2
	2:WL#1	1.889	6.34E+3	1.889	2	-3.34E+3	1.889	3
	3:WL#4	1.889	1.46E+3	0.000	2	-733.185	0.000	1
	4:DI	1.889	7.72E+3	1.889	3	-18.4E+3	1.889	2
	5:WLI#1	1.889	2.94E+3	1.889	2	-1.58E+3	1.889	3
	6:WLI#4	1.889	662.116	0.000	2	-336.371	0.000	1
	7:WLS#1	1.889	2.23E+3	1.889	2	-1.17E+3	1.889	3
	8:WLS#4	1.889	511.660	0.000	2	-257.499	0.000	1
	9:WLM#1	1.889	637.358	1.889	2	-336.335	1.889	3
	10:WLM#4	1.889	145.500	0.000	2	-73.294	0.000	1
	11:LM	1.889	84.014	1.889	2	-35.653	1.889	3
	12:LV	1.889	13.401	1.889	3	-28.009	1.889	2
	13:1.2D+1.6WI	1.889	4.86E+3	1.889	2	-3.13E+3	1.889	3
	14:1.2D+1.6WI	1.889	1.85E+3	1.889	1	-4.91E+3	1.889	2
	15:1.2D-1.6WL	1.889	7.57E+3	1.889	3	-15.4E+3	1.889	2
	16:1.2D-1.6WL	1.889	2.59E+3	1.889	3	-5.66E+3	1.889	2
	17:1.2D+1.0DI·	1.889	8.36E+3	1.889	3	-20.8E+3	1.889	2
	18:1.2D+1.0DI·	1.889	9.85E+3	1.889	3	-23.6E+3	1.889	2
	19:1.2D+1.0DI·	1.889	11.5E+3	1.889	3	-26.6E+3	1.889	2
	20:1.2D+1.0DI·	1.889	10E+3	1.889	3	-23.8E+3	1.889	2
	21:1.0D+1.0WI	1.889	698.284	1.889	1	-2.18E+3	1.889	2
	22:1.0D+1.0WI	1.889	1.77E+3	1.889	3	-4.32E+3	1.889	2
	23:1.0D-1.0WL	1.889	3.02E+3	1.889	3	-6.63E+3	1.889	2
	24:1.0D-1.0WL	1.889	1.93E+3	1.889	3	-4.49E+3	1.889	2
	25:1.4D	1.889	2.59E+3	1.889	3	-6.17E+3	1.889	2
	26:1.2D+1.5LM	1.889	1.83E+3	1.889	3	-4.52E+3	1.889	2
	27:1.2D+1.5LM	1.889	2.14E+3	1.889	3	-5.14E+3	1.889	2
	28:1.2D+1.5LM	1.889	2.5E+3	1.889	3	-5.8E+3	1.889	2
	29:1.2D+1.5LM	1.889	2.19E+3	1.889	3	-5.18E+3	1.889	2
	30:1.2D+1.5LV	1.889	2.24E+3	1.889	3	-5.33E+3	1.889	2
137	1:DEAD	1.889	765.005	0.000	1	-1.44E+3	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	2:WL#1	1.889	2.68E+3	0.000	2	-1.67E+3	0.000	1
	3:WL#4	1.889	992.660	1.889	1	-2.84E+3	1.889	2
	4:DI	1.889	3.53E+3	0.000	1	-6.87E+3	0.000	2
	5:WLI#1	1.889	1.28E+3	0.000	2	-827.638	0.000	1
	6:WLI#4	1.889	462.749	1.889	1	-1.33E+3	1.889	2
	7:WLS#1	1.889	940.881	0.000	2	-586.260	0.000	1
	8:WLS#4	1.889	348.553	1.889	1	-997.860	1.889	2
	9:WLM#1	1.889	269.883	0.000	2	-168.549	0.000	1
	10:WLM#4	1.889	99.488	1.889	1	-284.975	1.889	2
	11:LM	1.889	32.477	0.000	2	-18.482	1.889	2
	12:LV	1.889	8.119	0.000	2	-3.296	0.000	1
	13:1.2D+1.6WI	1.889	2.56E+3	0.000	2	-1.75E+3	0.000	1
	14:1.2D+1.6WI	1.889	1.46E+3	1.889	1	-4.24E+3	0.000	2
	15:1.2D-1.6WL	1.889	3.59E+3	0.000	1	-6.01E+3	0.000	2
	16:1.2D-1.6WL	1.889	5.13E+3	1.889	2	-1.75E+3	1.889	1
	17:1.2D+1.0DI·	1.889	3.62E+3	0.000	1	-7.32E+3	0.000	2
	18:1.2D+1.0DI·	1.889	4.6E+3	0.000	1	-9.32E+3	0.000	2
	19:1.2D+1.0DI·	1.889	5.28E+3	0.000	1	-9.88E+3	0.000	2
	20:1.2D+1.0DI·	1.889	4.3E+3	0.000	1	-7.88E+3	0.000	2
	21:1.0D+1.0WI	1.889	178.745	0.000	1	-498.654	0.000	2
	22:1.0D+1.0WI	1.889	884.654	0.000	1	-1.99E+3	0.000	2
	23:1.0D-1.0WL	1.889	1.35E+3	0.000	1	-2.38E+3	0.000	2
	24:1.0D-1.0WL	1.889	1.48E+3	1.889	2	-888.288	0.000	2
	25:1.4D	1.889	1.07E+3	0.000	1	-2.02E+3	0.000	2
	26:1.2D+1.5LM	1.889	727.356	0.000	1	-1.41E+3	0.000	2
	27:1.2D+1.5LM	1.889	929.690	0.000	1	-1.84E+3	0.000	2
	28:1.2D+1.5LM	1.889	1.06E+3	0.000	1	-1.95E+3	0.000	2
	29:1.2D+1.5LM	1.889	862.121	0.000	1	-1.52E+3	0.000	2
	30:1.2D+1.5LV	1.889	913.433	0.000	1	-1.72E+3	0.000	2
138	1:DEAD	1.889	1.98E+3	1.889	2	-799.789	1.889	3
	2:WL#1	1.889	1.93E+3	1.889	3	-5.09E+3	1.889	2
	3:WL#4	1.889	1.55E+3	1.889	3	-4.11E+3	1.889	2
	4:DI	1.889	9.55E+3	1.889	2	-3.89E+3	1.889	3
	5:WLI#1	1.889	865.244	1.889	1	-2.4E+3	1.889	2
	6:WLI#4	1.889	733.382	1.889	3	-1.94E+3	1.889	2
	7:WLS#1	1.889	679.015	1.889	3	-1.79E+3	1.889	2
	8:WLS#4	1.889	545.743	1.889	3	-1.44E+3	1.889	2
	9:WLM#1	1.889	193.844	1.889	3	-512.550	1.889	2
	10:WLM#4	1.889	156.132	1.889	3	-413.155	1.889	2
	11:LM	1.889	27.045	1.889	3	-69.441	1.889	2
	12:LV	1.889	5.334	0.000	2	-2.092	0.000	1
	13:1.2D+1.6WI	1.889	2.13E+3	1.889	3	-5.77E+3	1.889	2
	14:1.2D+1.6WI	1.889	1.53E+3	1.889	3	-4.2E+3	1.889	2
	15:1.2D-1.6WL	1.889	10.5E+3	1.889	2	-4.05E+3	1.889	3

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	16:1.2D-1.6WL	1.889	8.96E+3	1.889	2	-3.45E+3	1.889	3
	17:1.2D+1.0DI·	1.889	9.54E+3	1.889	2	-3.98E+3	1.889	3
	18:1.2D+1.0DI·	1.889	9.99E+3	1.889	2	-4.11E+3	1.889	3
	19:1.2D+1.0DI·	1.889	14.3E+3	1.889	2	-5.71E+3	1.889	3
	20:1.2D+1.0DI·	1.889	13.9E+3	1.889	2	-5.58E+3	1.889	3
	21:1.0D+1.0WI	1.889	193.202	1.889	2	-120.774	1.889	3
	22:1.0D+1.0WI	1.889	538.436	1.889	2	-511.255	0.000	2
	23:1.0D-1.0WL	1.889	3.77E+3	1.889	2	-1.48E+3	1.889	3
	24:1.0D-1.0WL	1.889	3.43E+3	1.889	2	-1.35E+3	1.889	3
	25:1.4D	1.889	2.78E+3	1.889	2	-1.12E+3	1.889	3
	26:1.2D+1.5LM	1.889	1.76E+3	1.889	2	-725.335	1.889	3
	27:1.2D+1.5LM	1.889	1.86E+3	1.889	2	-763.047	1.889	3
	28:1.2D+1.5LM	1.889	2.79E+3	1.889	2	-1.11E+3	1.889	3
	29:1.2D+1.5LM	1.889	2.69E+3	1.889	2	-1.08E+3	1.889	3
	30:1.2D+1.5LV	1.889	2.38E+3	1.889	2	-961.449	1.889	3
139	1:DEAD	1.179	997.339	0.000	2	-2.25E+3	0.000	1
	2:WL#1	1.179	2.9E+3	0.000	1	-1.57E+3	0.000	2
	3:WL#4	1.179	1.07E+3	0.000	2	-3.1E+3	0.000	1
	4:DI	1.179	4.13E+3	0.000	2	-9.12E+3	0.000	1
	5:WLI#1	1.179	1.35E+3	0.000	1	-742.324	0.000	2
	6:WLI#4	1.179	489.699	0.000	2	-1.44E+3	0.000	1
	7:WLS#1	1.179	1.02E+3	0.000	1	-551.339	0.000	2
	8:WLS#4	1.179	375.004	0.000	2	-1.09E+3	0.000	1
	9:WLM#1	1.179	291.080	0.000	1	-157.966	0.000	2
	10:WLM#4	1.179	106.684	0.000	2	-310.039	0.000	1
	11:LM	1.179	9.426	1.179	2	-19.147	0.000	1
	12:LV	1.179	14.261	0.000	1	-5.779	0.000	2
	13:1.2D+1.6WI	1.179	2.5E+3	1.179	1	-1.31E+3	0.000	2
	14:1.2D+1.6WI	1.179	2.91E+3	0.000	2	-7.65E+3	0.000	1
	15:1.2D-1.6WL	1.179	3.71E+3	0.000	2	-7.33E+3	0.000	1
	16:1.2D-1.6WL	1.179	3E+3	1.179	1	-1.22E+3	1.179	2
	17:1.2D+1.0DI·	1.179	4.59E+3	0.000	2	-10.5E+3	0.000	1
	18:1.2D+1.0DI·	1.179	5.82E+3	0.000	2	-13.3E+3	0.000	1
	19:1.2D+1.0DI·	1.179	6.07E+3	0.000	2	-13.2E+3	0.000	1
	20:1.2D+1.0DI·	1.179	4.84E+3	0.000	2	-10.4E+3	0.000	1
	21:1.0D+1.0WI	1.179	643.038	0.000	2	-1.23E+3	0.000	1
	22:1.0D+1.0WI	1.179	1.37E+3	0.000	2	-3.34E+3	0.000	1
	23:1.0D-1.0WL	1.179	1.55E+3	0.000	2	-3.26E+3	0.000	1
	24:1.0D-1.0WL	1.179	622.335	0.000	2	-1.16E+3	0.000	1
	25:1.4D	1.179	1.4E+3	0.000	2	-3.15E+3	0.000	1
	26:1.2D+1.5LM	1.179	1.05E+3	0.000	2	-2.43E+3	0.000	1
	27:1.2D+1.5LM	1.179	1.31E+3	0.000	2	-3.04E+3	0.000	1
	28:1.2D+1.5LM	1.179	1.36E+3	0.000	2	-3.02E+3	0.000	1
	29:1.2D+1.5LM	1.179	1.1E+3	0.000	2	-2.42E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	30:1.2D+1.5LV	1.179	1.19E+3	0.000	2	-2.68E+3	0.000	1
140	1:DEAD	1.179	712.606	0.000	2	-1.03E+3	0.000	1
	2:WL#1	1.179	1.8E+3	0.000	1	-1.7E+3	0.000	2
	3:WL#4	1.179	1.14E+3	0.000	2	-2.64E+3	0.000	1
	4:DI	1.179	3.34E+3	0.000	2	-5.1E+3	0.000	1
	5:WLI#1	1.179	839.006	0.000	1	-798.269	0.000	2
	6:WLI#4	1.179	522.970	0.000	2	-1.22E+3	0.000	1
	7:WLS#1	1.179	633.551	0.000	1	-597.420	0.000	2
	8:WLS#4	1.179	401.438	0.000	2	-927.184	0.000	1
	9:WLM#1	1.179	181.342	0.000	1	-171.047	0.000	2
	10:WLM#4	1.179	114.418	0.000	2	-264.374	0.000	1
	11:LM	1.179	9.480	0.000	2	-20.894	0.000	1
	12:LV	1.179	12.357	0.000	1	-5.038	0.000	2
	13:1.2D+1.6WI	1.179	1.8E+3	1.080	1	-1.87E+3	0.000	2
	14:1.2D+1.6WI	1.179	2.3E+3	0.000	2	-5.46E+3	0.000	1
	15:1.2D-1.6WL	1.179	3.58E+3	0.000	2	-4.13E+3	0.000	1
	16:1.2D-1.6WL	1.179	3.17E+3	1.179	1	-1.51E+3	0.000	2
	17:1.2D+1.0DI-	1.179	3.4E+3	0.000	2	-5.5E+3	0.000	1
	18:1.2D+1.0DI-	1.179	4.62E+3	0.000	2	-7.56E+3	0.000	1
	19:1.2D+1.0DI-	1.179	5E+3	0.000	2	-7.18E+3	0.000	1
	20:1.2D+1.0DI-	1.179	3.78E+3	0.000	2	-5.12E+3	0.000	1
	21:1.0D+1.0WI	1.179	211.486	0.000	2	-399.539	0.000	1
	22:1.0D+1.0WI	1.179	1.03E+3	0.000	2	-1.96E+3	0.000	1
	23:1.0D-1.0WL	1.179	1.31E+3	0.000	2	-1.67E+3	0.000	1
	24:1.0D-1.0WL	1.179	421.150	1.179	1	-236.142	1.179	2
	25:1.4D	1.179	997.649	0.000	2	-1.45E+3	0.000	1
	26:1.2D+1.5LM	1.179	693.107	0.000	2	-1.09E+3	0.000	1
	27:1.2D+1.5LM	1.179	954.319	0.000	2	-1.54E+3	0.000	1
	28:1.2D+1.5LM	1.179	1.04E+3	0.000	2	-1.45E+3	0.000	1
	29:1.2D+1.5LM	1.179	773.989	0.000	2	-1.01E+3	0.000	1
	30:1.2D+1.5LV	1.179	849.651	0.000	2	-1.22E+3	0.000	1
141	1:DEAD	1.889	784.254	0.000	1	-1.48E+3	0.000	2
	2:WL#1	1.889	3.3E+3	1.889	2	-1.12E+3	1.889	1
	3:WL#4	1.889	3.07E+3	0.000	2	-1.6E+3	0.000	1
	4:DI	1.889	3.77E+3	0.000	1	-7.39E+3	0.000	2
	5:WLI#1	1.889	1.53E+3	1.889	2	-523.111	1.889	3
	6:WLI#4	1.889	1.43E+3	0.000	2	-774.309	0.000	1
	7:WLS#1	1.889	1.16E+3	1.889	2	-392.365	1.889	1
	8:WLS#4	1.889	1.08E+3	0.000	2	-562.541	0.000	1
	9:WLM#1	1.889	331.906	1.889	2	-112.441	1.889	1
	10:WLM#4	1.889	307.799	0.000	2	-160.933	0.000	1
	11:LM	1.889	16.476	0.000	1	-34.761	0.000	2
	12:LV	1.889	16.239	0.000	2	-9.238	1.889	2
	13:1.2D+1.6WI	1.889	5.85E+3	1.889	2	-2.01E+3	1.889	3

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	14:1.2D+1.6WI	1.889	3.14E+3	0.000	2	-1.62E+3	0.000	1
	15:1.2D-1.6WL	1.889	1.57E+3	1.889	1	-4.72E+3	1.889	2
	16:1.2D-1.6WL	1.889	3.5E+3	0.000	1	-6.69E+3	0.000	2
	17:1.2D+1.0DI·	1.889	4.99E+3	0.000	1	-9.15E+3	0.000	2
	18:1.2D+1.0DI·	1.889	3.94E+3	0.000	1	-7.74E+3	0.000	2
	19:1.2D+1.0DI·	1.889	4.43E+3	0.000	1	-9.19E+3	0.000	2
	20:1.2D+1.0DI·	1.889	5.49E+3	0.000	1	-10.6E+3	0.000	2
	21:1.0D+1.0WI	1.889	1.63E+3	1.889	2	-1.45E+3	0.000	2
	22:1.0D+1.0WI	1.889	568.800	1.889	2	-399.457	0.000	2
	23:1.0D-1.0WL	1.889	596.621	0.000	1	-1.5E+3	0.000	2
	24:1.0D-1.0WL	1.889	1.35E+3	0.000	1	-2.56E+3	0.000	2
	25:1.4D	1.889	1.1E+3	0.000	1	-2.07E+3	0.000	2
	26:1.2D+1.5LM	1.889	1.02E+3	0.000	1	-1.82E+3	0.000	2
	27:1.2D+1.5LM	1.889	804.886	0.000	1	-1.52E+3	0.000	2
	28:1.2D+1.5LM	1.889	911.615	0.000	1	-1.83E+3	0.000	2
	29:1.2D+1.5LM	1.889	1.13E+3	0.000	1	-2.13E+3	0.000	2
	30:1.2D+1.5LV	1.889	930.059	0.000	1	-1.75E+3	0.000	2
142	1:DEAD	1.889	1.99E+3	1.889	2	-800.684	1.889	3
	2:WL#1	1.889	6.42E+3	1.889	2	-2.42E+3	1.889	3
	3:WL#4	1.889	867.881	1.889	3	-2.32E+3	1.889	2
	4:DI	1.889	9.62E+3	1.889	2	-3.92E+3	1.889	3
	5:WLI#1	1.889	2.97E+3	1.889	2	-1.1E+3	1.889	3
	6:WLI#4	1.889	399.138	1.889	1	-1.05E+3	1.889	2
	7:WLS#1	1.889	2.25E+3	1.889	2	-851.221	1.889	3
	8:WLS#4	1.889	304.988	1.889	3	-814.615	1.889	2
	9:WLM#1	1.889	645.247	1.889	2	-243.457	1.889	3
	10:WLM#4	1.889	86.229	1.889	3	-232.045	1.889	2
	11:LM	1.889	10.518	1.889	2	-12.121	0.000	2
	12:LV	1.889	13.523	1.889	3	-34.714	1.889	2
	13:1.2D+1.6WI	1.889	12.7E+3	1.889	2	-4.84E+3	1.889	3
	14:1.2D+1.6WI	1.889	1.01E+3	0.000	2	-1.32E+3	1.889	2
	15:1.2D-1.6WL	1.889	2.92E+3	1.889	3	-7.88E+3	1.889	2
	16:1.2D-1.6WL	1.889	6.1E+3	1.889	2	-2.35E+3	1.889	3
	17:1.2D+1.0DI·	1.889	15E+3	1.889	2	-5.98E+3	1.889	3
	18:1.2D+1.0DI·	1.889	11E+3	1.889	2	-4.52E+3	1.889	3
	19:1.2D+1.0DI·	1.889	9.03E+3	1.889	2	-3.78E+3	1.889	3
	20:1.2D+1.0DI·	1.889	13.1E+3	1.889	2	-5.23E+3	1.889	3
	21:1.0D+1.0WI	1.889	4.24E+3	1.889	2	-1.65E+3	1.889	3
	22:1.0D+1.0WI	1.889	1.18E+3	1.889	2	-495.696	1.889	3
	23:1.0D-1.0WL	1.889	256.002	0.000	1	-688.827	0.000	2
	24:1.0D-1.0WL	1.889	2.8E+3	1.889	2	-1.11E+3	1.889	3
	25:1.4D	1.889	2.79E+3	1.889	2	-1.12E+3	1.889	3
	26:1.2D+1.5LM	1.889	3.05E+3	1.889	2	-1.21E+3	1.889	3
	27:1.2D+1.5LM	1.889	2.17E+3	1.889	2	-881.555	1.889	3

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	28:1.2D+1.5LM	1.889	1.76E+3	1.889	2	-724.326	1.889	3
	29:1.2D+1.5LM	1.889	2.64E+3	1.889	2	-1.05E+3	1.889	3
	30:1.2D+1.5LV	1.889	2.34E+3	1.889	2	-940.538	1.889	3
143	1:DEAD	1.179	1.03E+3	0.000	2	-2.3E+3	0.000	1
	2:WL#1	1.179	1.36E+3	0.000	1	-758.770	1.179	2
	3:WL#4	1.179	4.02E+3	0.000	1	-1.91E+3	0.000	2
	4:DI	1.179	4.53E+3	0.000	2	-9.96E+3	0.000	1
	5:WLI#1	1.179	618.672	0.000	1	-357.021	1.179	2
	6:WLI#4	1.179	1.85E+3	0.000	1	-889.260	0.000	2
	7:WLS#1	1.179	479.119	0.000	1	-266.510	1.179	2
	8:WLS#4	1.179	1.41E+3	0.000	1	-669.747	0.000	2
	9:WLM#1	1.179	136.775	0.000	1	-76.443	1.179	2
	10:WLM#4	1.179	401.935	0.000	1	-191.034	0.000	2
	11:LM	1.179	36.382	0.000	2	-90.098	0.000	1
	12:LV	1.179	4.702	1.179	2	-9.569	0.000	1
	13:1.2D+1.6WI	1.179	948.868	0.000	2	-874.152	1.179	2
	14:1.2D+1.6WI	1.179	4.06E+3	1.179	1	-1.81E+3	0.000	2
	15:1.2D-1.6WL	1.179	1.89E+3	0.000	2	-4.95E+3	0.000	1
	16:1.2D-1.6WL	1.179	4.29E+3	0.000	2	-9.19E+3	0.000	1
	17:1.2D+1.0DI-	1.179	5.69E+3	0.000	2	-12.1E+3	0.000	1
	18:1.2D+1.0DI-	1.179	4.88E+3	0.000	2	-10.9E+3	0.000	1
	19:1.2D+1.0DI-	1.179	5.84E+3	0.000	2	-13.3E+3	0.000	1
	20:1.2D+1.0DI-	1.179	6.66E+3	0.000	2	-14.6E+3	0.000	1
	21:1.0D+1.0WI	1.179	968.241	0.000	2	-1.83E+3	0.000	1
	22:1.0D+1.0WI	1.179	518.477	0.000	2	-893.690	0.000	1
	23:1.0D-1.0WL	1.179	1.1E+3	0.000	2	-2.78E+3	0.000	1
	24:1.0D-1.0WL	1.179	1.7E+3	0.000	2	-3.71E+3	0.000	1
	25:1.4D	1.179	1.44E+3	0.000	2	-3.23E+3	0.000	1
	26:1.2D+1.5LM	1.179	1.27E+3	0.000	2	-2.76E+3	0.000	1
	27:1.2D+1.5LM	1.179	1.1E+3	0.000	2	-2.5E+3	0.000	1
	28:1.2D+1.5LM	1.179	1.31E+3	0.000	2	-3.04E+3	0.000	1
	29:1.2D+1.5LM	1.179	1.48E+3	0.000	2	-3.3E+3	0.000	1
	30:1.2D+1.5LV	1.179	1.24E+3	0.000	2	-2.78E+3	0.000	1
144	1:DEAD	1.179	726.061	0.000	2	-1.09E+3	0.000	1
	2:WL#1	1.179	1.51E+3	0.000	1	-890.800	0.000	3
	3:WL#4	1.179	2.89E+3	0.000	1	-1.86E+3	0.000	2
	4:DI	1.179	3.63E+3	0.000	2	-5.82E+3	0.000	1
	5:WLI#1	1.179	691.677	0.000	1	-419.075	0.000	3
	6:WLI#4	1.179	1.33E+3	0.000	1	-867.721	0.000	2
	7:WLS#1	1.179	529.535	0.000	1	-312.893	0.000	3
	8:WLS#4	1.179	1.01E+3	0.000	1	-653.979	0.000	2
	9:WLM#1	1.179	151.395	0.000	1	-89.685	0.000	3
	10:WLM#4	1.179	288.912	0.000	1	-186.469	0.000	2
	11:LM	1.179	24.415	0.000	2	-56.318	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	12:LV	1.179	4.725	0.000	2	-10.435	0.000	1
	13:1.2D+1.6WI	1.179	1.66E+3	1.179	1	-1.09E+3	0.000	3
	14:1.2D+1.6WI	1.179	3.32E+3	0.196	1	-2.11E+3	0.000	2
	15:1.2D-1.6WL	1.179	1.76E+3	0.000	2	-3.72E+3	0.000	1
	16:1.2D-1.6WL	1.179	3.85E+3	0.000	2	-5.92E+3	0.000	1
	17:1.2D+1.0DI·	1.179	4.54E+3	0.000	2	-6.43E+3	0.000	1
	18:1.2D+1.0DI·	1.179	3.63E+3	0.000	2	-5.8E+3	0.000	1
	19:1.2D+1.0DI·	1.179	4.46E+3	0.000	2	-7.81E+3	0.000	1
	20:1.2D+1.0DI·	1.179	5.37E+3	0.000	2	-8.45E+3	0.000	1
	21:1.0D+1.0WI	1.179	755.489	0.000	2	-557.228	0.000	1
	22:1.0D+1.0WI	1.179	398.869	1.179	1	-166.089	1.179	3
	23:1.0D-1.0WL	1.179	696.634	0.000	2	-1.62E+3	0.000	1
	24:1.0D-1.0WL	1.179	1.38E+3	0.000	2	-2.1E+3	0.000	1
	25:1.4D	1.179	1.02E+3	0.000	2	-1.52E+3	0.000	1
	26:1.2D+1.5LM	1.179	916.485	0.000	2	-1.24E+3	0.000	1
	27:1.2D+1.5LM	1.179	721.427	0.000	2	-1.1E+3	0.000	1
	28:1.2D+1.5LM	1.179	899.306	0.000	2	-1.54E+3	0.000	1
	29:1.2D+1.5LM	1.179	1.09E+3	0.000	2	-1.68E+3	0.000	1
	30:1.2D+1.5LV	1.179	875.771	0.000	2	-1.32E+3	0.000	1
145	1:DEAD	1.889	766.253	1.889	3	-1.44E+3	1.889	2
	2:WL#1	1.889	1.14E+3	1.889	3	-2.74E+3	1.889	2
	3:WL#4	1.889	1.47E+3	1.889	2	-2.64E+3	0.000	2
	4:DI	1.889	3.52E+3	1.889	3	-6.84E+3	1.889	2
	5:WLI#1	1.889	550.564	1.889	3	-1.29E+3	1.889	2
	6:WLI#4	1.889	690.770	1.889	2	-1.22E+3	0.000	2
	7:WLS#1	1.889	398.908	1.889	3	-962.127	1.889	2
	8:WLS#4	1.889	514.971	1.889	2	-927.766	0.000	2
	9:WLM#1	1.889	114.446	1.889	3	-275.550	1.889	2
	10:WLM#4	1.889	147.224	1.889	2	-264.306	0.000	2
	11:LM	1.889	16.261	1.889	2	-6.570	1.889	1
	12:LV	1.889	16.241	1.889	2	-9.236	0.000	2
	13:1.2D+1.6WI	1.889	2.74E+3	1.889	3	-6.11E+3	1.889	2
	14:1.2D+1.6WI	1.889	1.25E+3	0.000	1	-3.64E+3	0.000	2
	15:1.2D-1.6WL	1.889	3.66E+3	0.000	2	-1.42E+3	0.000	1
	16:1.2D-1.6WL	1.889	4.81E+3	0.000	2	-4.07E+3	1.889	2
	17:1.2D+1.0DI·	1.889	4.99E+3	1.889	3	-9.86E+3	1.889	2
	18:1.2D+1.0DI·	1.889	3.83E+3	1.889	3	-7.87E+3	1.889	2
	19:1.2D+1.0DI·	1.889	3.89E+3	1.889	3	-7.27E+3	1.889	2
	20:1.2D+1.0DI·	1.889	5.06E+3	1.889	3	-9.25E+3	1.889	2
	21:1.0D+1.0WI	1.889	1.17E+3	1.889	3	-2.4E+3	1.889	2
	22:1.0D+1.0WI	1.889	330.505	1.889	3	-923.222	1.889	2
	23:1.0D-1.0WL	1.889	1.16E+3	0.000	2	-476.066	1.889	2
	24:1.0D-1.0WL	1.889	1.42E+3	0.000	2	-1.95E+3	1.889	2
	25:1.4D	1.889	1.07E+3	1.889	3	-2.01E+3	1.889	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	26:1.2D+1.5LM	1.889	1.02E+3	1.889	3	-1.98E+3	1.889	2
	27:1.2D+1.5LM	1.889	785.436	1.889	3	-1.55E+3	1.889	2
	28:1.2D+1.5LM	1.889	795.947	1.889	3	-1.43E+3	1.889	2
	29:1.2D+1.5LM	1.889	1.04E+3	1.889	3	-1.85E+3	1.889	2
	30:1.2D+1.5LV	1.889	908.457	1.889	3	-1.7E+3	1.889	2
146	1:DEAD	1.889	1.82E+3	1.889	3	-4.34E+3	1.889	2
	2:WL#1	1.889	1.8E+3	1.889	3	-3.19E+3	1.889	2
	3:WL#4	1.889	5.21E+3	1.889	2	-2.68E+3	1.889	3
	4:DI	1.889	7.35E+3	1.889	3	-17.7E+3	1.889	2
	5:WLI#1	1.889	856.424	1.889	3	-1.5E+3	1.889	2
	6:WLI#4	1.889	2.4E+3	1.889	2	-1.26E+3	1.889	3
	7:WLS#1	1.889	632.526	1.889	3	-1.12E+3	1.889	2
	8:WLS#4	1.889	1.83E+3	1.889	2	-939.209	1.889	3
	9:WLM#1	1.889	180.959	1.889	3	-320.252	1.889	2
	10:WLM#4	1.889	521.169	1.889	2	-268.224	1.889	3
	11:LM	1.889	22.199	1.889	2	-10.870	1.889	3
	12:LV	1.889	42.002	1.889	2	-17.825	1.889	3
	13:1.2D+1.6WI	1.889	5.06E+3	1.889	3	-10.3E+3	1.889	2
	14:1.2D+1.6WI	1.889	3.12E+3	1.889	2	-2.1E+3	1.889	3
	15:1.2D-1.6WL	1.889	2.16E+3	0.000	2	-1.47E+3	0.000	1
	16:1.2D-1.6WL	1.889	6.46E+3	1.889	3	-13.5E+3	1.889	2
	17:1.2D+1.0DI·	1.889	10.4E+3	1.889	3	-24.4E+3	1.889	2
	18:1.2D+1.0DI·	1.889	8.27E+3	1.889	3	-20.5E+3	1.889	2
	19:1.2D+1.0DI·	1.889	8.67E+3	1.889	3	-21.4E+3	1.889	2
	20:1.2D+1.0DI·	1.889	10.8E+3	1.889	3	-25.3E+3	1.889	2
	21:1.0D+1.0WI	1.889	2.45E+3	1.889	3	-5.46E+3	1.889	2
	22:1.0D+1.0WI	1.889	879.461	1.889	1	-2.51E+3	1.889	2
	23:1.0D-1.0WL	1.889	1.19E+3	1.889	1	-3.22E+3	1.889	2
	24:1.0D-1.0WL	1.889	2.76E+3	1.889	3	-6.17E+3	1.889	2
	25:1.4D	1.889	2.55E+3	1.889	3	-6.07E+3	1.889	2
	26:1.2D+1.5LM	1.889	2.35E+3	1.889	3	-5.49E+3	1.889	2
	27:1.2D+1.5LM	1.889	1.9E+3	1.889	3	-4.65E+3	1.889	2
	28:1.2D+1.5LM	1.889	1.99E+3	1.889	3	-4.85E+3	1.889	2
	29:1.2D+1.5LM	1.889	2.43E+3	1.889	3	-5.69E+3	1.889	2
	30:1.2D+1.5LV	1.889	2.16E+3	1.889	3	-5.14E+3	1.889	2
147	1:DEAD	1.179	1E+3	1.179	2	-2.25E+3	1.179	1
	2:WL#1	1.179	1.73E+3	1.179	2	-4.19E+3	1.179	1
	3:WL#4	1.179	850.681	1.179	1	-792.236	1.179	2
	4:DI	1.179	4.15E+3	1.179	2	-9.15E+3	1.179	1
	5:WLI#1	1.179	817.495	1.179	2	-1.96E+3	1.179	1
	6:WLI#4	1.179	401.843	1.179	1	-373.912	1.179	2
	7:WLS#1	1.179	607.981	1.179	2	-1.47E+3	1.179	1
	8:WLS#4	1.179	298.658	1.179	1	-278.122	1.179	2
	9:WLM#1	1.179	174.062	1.179	2	-421.116	1.179	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	10:WLM#4	1.179	85.371	1.179	1	-79.482	1.179	2
	11:LM	1.179	28.545	1.179	1	-11.595	1.179	3
	12:LV	1.179	4.677	0.000	2	-9.555	1.179	1
	13:1.2D+1.6WI	1.179	3.97E+3	1.179	2	-9.4E+3	1.179	1
	14:1.2D+1.6WI	1.179	997.485	1.179	3	-1.34E+3	1.179	1
	15:1.2D-1.6WL	1.179	4.45E+3	0.000	1	-1.57E+3	1.179	2
	16:1.2D-1.6WL	1.179	2.47E+3	1.179	2	-4.06E+3	1.179	1
	17:1.2D+1.0DI·	1.179	6.17E+3	1.179	2	-13.8E+3	1.179	1
	18:1.2D+1.0DI·	1.179	4.97E+3	1.179	2	-11.4E+3	1.179	1
	19:1.2D+1.0DI·	1.179	4.53E+3	1.179	2	-9.88E+3	1.179	1
	20:1.2D+1.0DI·	1.179	5.72E+3	1.179	2	-12.3E+3	1.179	1
	21:1.0D+1.0WI	1.179	1.61E+3	1.179	2	-3.72E+3	1.179	1
	22:1.0D+1.0WI	1.179	827.598	1.179	3	-1.95E+3	1.179	1
	23:1.0D-1.0WL	1.179	442.760	1.179	3	-776.350	1.179	1
	24:1.0D-1.0WL	1.179	1.28E+3	1.179	2	-2.55E+3	1.179	1
	25:1.4D	1.179	1.4E+3	1.179	2	-3.15E+3	1.179	1
	26:1.2D+1.5LM	1.179	1.36E+3	1.179	2	-3.07E+3	1.179	1
	27:1.2D+1.5LM	1.179	1.11E+3	1.179	2	-2.57E+3	1.179	1
	28:1.2D+1.5LM	1.179	1.01E+3	1.179	2	-2.23E+3	1.179	1
	29:1.2D+1.5LM	1.179	1.26E+3	1.179	2	-2.74E+3	1.179	1
	30:1.2D+1.5LV	1.179	1.21E+3	1.179	2	-2.71E+3	1.179	1
148	1:DEAD	1.179	1.62E+3	1.179	3	-3.97E+3	1.179	1
	2:WL#1	1.179	2.35E+3	1.179	2	-4.97E+3	1.179	1
	3:WL#4	1.179	1.49E+3	1.179	1	-561.860	1.179	2
	4:DI	1.179	5.73E+3	1.179	3	-14.3E+3	1.179	1
	5:WLI#1	1.179	1.12E+3	1.179	2	-2.33E+3	1.179	1
	6:WLI#4	1.179	701.426	1.179	1	-267.662	1.179	2
	7:WLS#1	1.179	825.458	1.179	2	-1.75E+3	1.179	1
	8:WLS#4	1.179	523.013	1.179	1	-197.244	1.179	2
	9:WLM#1	1.179	236.282	1.179	2	-499.612	1.179	1
	10:WLM#4	1.179	149.432	1.179	1	-56.528	1.179	2
	11:LM	1.179	33.251	1.179	1	-19.179	1.179	3
	12:LV	1.179	4.287	0.000	2	-9.288	1.179	1
	13:1.2D+1.6WI	1.179	5.5E+3	1.179	2	-12.7E+3	1.179	1
	14:1.2D+1.6WI	1.179	1.24E+3	1.179	3	-2.38E+3	1.179	1
	15:1.2D-1.6WL	1.179	4.62E+3	0.000	1	-2.02E+3	1.179	2
	16:1.2D-1.6WL	1.179	2.65E+3	1.179	2	-7.15E+3	1.179	1
	17:1.2D+1.0DI·	1.179	8.21E+3	1.179	2	-21.4E+3	1.179	1
	18:1.2D+1.0DI·	1.179	7.49E+3	1.179	3	-18.4E+3	1.179	1
	19:1.2D+1.0DI·	1.179	7.37E+3	1.179	3	-16.7E+3	1.179	1
	20:1.2D+1.0DI·	1.179	7.86E+3	1.179	3	-19.8E+3	1.179	1
	21:1.0D+1.0WI	1.179	2.28E+3	1.179	2	-5.72E+3	1.179	1
	22:1.0D+1.0WI	1.179	1.47E+3	1.179	3	-3.45E+3	1.179	1
	23:1.0D-1.0WL	1.179	1.37E+3	1.179	3	-2.23E+3	1.179	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	24:1.0D-1.0WL	1.179	1.77E+3	1.179	3	-4.5E+3	1.179	1
	25:1.4D	1.179	2.27E+3	1.179	3	-5.56E+3	1.179	1
	26:1.2D+1.5LM	1.179	1.99E+3	1.179	2	-5.22E+3	1.179	1
	27:1.2D+1.5LM	1.179	1.87E+3	1.179	3	-4.57E+3	1.179	1
	28:1.2D+1.5LM	1.179	1.84E+3	1.179	3	-4.22E+3	1.179	1
	29:1.2D+1.5LM	1.179	1.96E+3	1.179	3	-4.87E+3	1.179	1
	30:1.2D+1.5LV	1.179	1.95E+3	1.179	3	-4.78E+3	1.179	1
149	1:DEAD	1.179	988.750	0.000	2	-2.23E+3	0.000	1
	2:WL#1	1.179	1.72E+3	0.000	2	-4.18E+3	0.000	1
	3:WL#4	1.179	792.001	0.000	2	-864.651	0.000	1
	4:DI	1.179	4.02E+3	0.000	2	-8.9E+3	0.000	1
	5:WLI#1	1.179	807.300	0.000	2	-1.96E+3	0.000	1
	6:WLI#4	1.179	375.047	0.000	2	-405.587	0.000	1
	7:WLS#1	1.179	603.098	0.000	2	-1.47E+3	0.000	1
	8:WLS#4	1.179	278.036	0.000	2	-303.567	0.000	1
	9:WLM#1	1.179	172.586	0.000	2	-420.395	0.000	1
	10:WLM#4	1.179	79.443	0.000	2	-86.773	0.000	1
	11:LM	1.179	28.524	0.000	1	-11.555	0.000	2
	12:LV	1.179	18.232	0.000	2	-45.032	0.000	1
	13:1.2D+1.6WI	1.179	3.93E+3	0.000	2	-9.36E+3	0.000	1
	14:1.2D+1.6WI	1.179	2.45E+3	0.000	2	-4.06E+3	0.000	1
	15:1.2D-1.6WL	1.179	4.4E+3	1.179	1	-1.56E+3	0.000	2
	16:1.2D-1.6WL	1.179	971.658	0.000	3	-1.29E+3	0.000	1
	17:1.2D+1.0DI·	1.179	6.01E+3	0.000	2	-13.5E+3	0.000	1
	18:1.2D+1.0DI·	1.179	5.58E+3	0.000	2	-12E+3	0.000	1
	19:1.2D+1.0DI·	1.179	4.39E+3	0.000	2	-9.61E+3	0.000	1
	20:1.2D+1.0DI·	1.179	4.83E+3	0.000	2	-11.2E+3	0.000	1
	21:1.0D+1.0WI	1.179	1.59E+3	0.000	2	-3.7E+3	0.000	1
	22:1.0D+1.0WI	1.179	1.27E+3	0.000	2	-2.53E+3	0.000	1
	23:1.0D-1.0WL	1.179	412.912	0.000	2	-763.240	0.000	1
	24:1.0D-1.0WL	1.179	817.798	0.000	2	-1.93E+3	0.000	1
	25:1.4D	1.179	1.38E+3	0.000	2	-3.12E+3	0.000	1
	26:1.2D+1.5LM	1.179	1.34E+3	0.000	2	-3.06E+3	0.000	1
	27:1.2D+1.5LM	1.179	1.25E+3	0.000	2	-2.72E+3	0.000	1
	28:1.2D+1.5LM	1.179	998.303	0.000	2	-2.21E+3	0.000	1
	29:1.2D+1.5LM	1.179	1.09E+3	0.000	2	-2.55E+3	0.000	1
	30:1.2D+1.5LV	1.179	1.21E+3	0.000	2	-2.75E+3	0.000	1
150	1:DEAD	1.179	706.815	0.000	2	-1.03E+3	0.000	1
	2:WL#1	1.179	1.61E+3	0.000	2	-3.24E+3	0.000	1
	3:WL#4	1.179	988.815	0.000	2	-396.353	0.000	1
	4:DI	1.179	3.27E+3	0.000	2	-5.01E+3	0.000	1
	5:WLI#1	1.179	763.725	0.000	2	-1.52E+3	0.000	1
	6:WLI#4	1.179	460.188	0.000	2	-190.000	0.000	1
	7:WLS#1	1.179	565.758	0.000	2	-1.14E+3	0.000	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	8:WLS#4	1.179	347.216	0.000	2	-139.190	0.000	1
	9:WLM#1	1.179	161.844	0.000	2	-325.846	0.000	1
	10:WLM#4	1.179	99.029	0.000	2	-39.766	0.000	1
	11:LM	1.179	24.713	0.000	1	-10.076	0.000	2
	12:LV	1.179	12.156	0.000	2	-28.201	0.000	1
	13:1.2D+1.6WI	1.179	3.42E+3	0.000	2	-6.41E+3	0.000	1
	14:1.2D+1.6WI	1.179	2.43E+3	0.000	2	-1.51E+3	0.000	1
	15:1.2D-1.6WL	1.179	3.95E+3	0.196	1	-1.83E+3	1.179	3
	16:1.2D-1.6WL	1.179	951.044	0.000	3	-951.828	0.000	1
	17:1.2D+1.0DI·	1.179	4.88E+3	0.000	2	-7.76E+3	0.000	1
	18:1.2D+1.0DI·	1.179	4.58E+3	0.000	2	-6.33E+3	0.000	1
	19:1.2D+1.0DI·	1.179	3.36E+3	0.000	2	-4.73E+3	0.000	1
	20:1.2D+1.0DI·	1.179	3.66E+3	0.000	2	-6.16E+3	0.000	1
	21:1.0D+1.0WI	1.179	1.27E+3	0.000	2	-2.16E+3	0.000	1
	22:1.0D+1.0WI	1.179	1.05E+3	0.000	2	-1.09E+3	0.000	1
	23:1.0D-1.0WL	1.179	569.216	1.179	1	-220.600	1.179	2
	24:1.0D-1.0WL	1.179	403.256	0.000	2	-964.871	0.000	1
	25:1.4D	1.179	989.541	0.000	2	-1.44E+3	0.000	1
	26:1.2D+1.5LM	1.179	999.071	0.000	2	-1.52E+3	0.000	1
	27:1.2D+1.5LM	1.179	936.256	0.000	2	-1.21E+3	0.000	1
	28:1.2D+1.5LM	1.179	675.384	0.000	2	-868.523	0.000	1
	29:1.2D+1.5LM	1.179	738.199	0.000	2	-1.18E+3	0.000	1
	30:1.2D+1.5LV	1.179	866.413	0.000	2	-1.27E+3	0.000	1
151	1:DEAD	1.889	752.068	0.000	1	-1.41E+3	0.000	2
	2:WL#1	1.889	1.16E+3	0.000	1	-2.79E+3	0.000	2
	3:WL#4	1.889	2.67E+3	1.889	2	-1.47E+3	0.000	2
	4:DI	1.889	3.32E+3	0.000	1	-6.4E+3	0.000	2
	5:WLI#1	1.889	551.467	0.000	1	-1.3E+3	0.000	2
	6:WLI#4	1.889	1.22E+3	1.889	2	-691.385	0.000	1
	7:WLS#1	1.889	406.217	0.000	1	-979.409	0.000	2
	8:WLS#4	1.889	937.999	1.889	2	-515.749	0.000	2
	9:WLM#1	1.889	116.375	0.000	1	-280.249	0.000	2
	10:WLM#4	1.889	267.219	1.889	2	-147.446	0.000	2
	11:LM	1.889	16.238	0.000	2	-6.593	0.000	1
	12:LV	1.889	8.243	0.000	1	-17.374	0.000	2
	13:1.2D+1.6WI	1.889	2.75E+3	0.000	1	-6.15E+3	0.000	2
	14:1.2D+1.6WI	1.889	4.82E+3	1.889	2	-4.04E+3	0.000	2
	15:1.2D-1.6WL	1.889	3.5E+3	1.889	2	-1.38E+3	1.889	1
	16:1.2D-1.6WL	1.889	1.28E+3	1.889	3	-3.73E+3	1.889	2
	17:1.2D+1.0DI·	1.889	4.78E+3	0.000	1	-9.4E+3	0.000	2
	18:1.2D+1.0DI·	1.889	4.84E+3	0.000	1	-8.78E+3	0.000	2
	19:1.2D+1.0DI·	1.889	3.67E+3	0.000	1	-6.79E+3	0.000	2
	20:1.2D+1.0DI·	1.889	3.61E+3	0.000	1	-7.4E+3	0.000	2
	21:1.0D+1.0WI	1.889	1.16E+3	0.000	1	-2.39E+3	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	22:1.0D+1.0WI	1.889	1.39E+3	1.889	2	-1.93E+3	0.000	2
	23:1.0D-1.0WL	1.889	1.1E+3	1.889	2	-432.176	0.000	2
	24:1.0D-1.0WL	1.889	319.600	0.000	1	-895.836	0.000	2
	25:1.4D	1.889	1.05E+3	0.000	1	-1.98E+3	0.000	2
	26:1.2D+1.5LM	1.889	1.01E+3	0.000	1	-1.95E+3	0.000	2
	27:1.2D+1.5LM	1.889	1.02E+3	0.000	1	-1.82E+3	0.000	2
	28:1.2D+1.5LM	1.889	776.962	0.000	1	-1.39E+3	0.000	2
	29:1.2D+1.5LM	1.889	769.317	0.000	1	-1.52E+3	0.000	2
	30:1.2D+1.5LV	1.889	914.846	0.000	1	-1.72E+3	0.000	2
152	1:DEAD	1.889	1.89E+3	1.889	2	-763.806	1.889	3
	2:WL#1	1.889	794.468	0.000	1	-1.85E+3	0.000	2
	3:WL#4	1.889	6.62E+3	1.889	2	-2.51E+3	1.889	3
	4:DI	1.889	7.92E+3	1.889	2	-3.22E+3	1.889	3
	5:WLI#1	1.889	392.456	0.000	1	-888.914	0.000	2
	6:WLI#4	1.889	3.01E+3	1.889	2	-1.11E+3	1.889	3
	7:WLS#1	1.889	278.911	0.000	1	-648.264	0.000	2
	8:WLS#4	1.889	2.32E+3	1.889	2	-880.212	1.889	3
	9:WLM#1	1.889	80.345	0.000	1	-186.380	0.000	2
	10:WLM#4	1.889	661.770	1.889	2	-250.153	1.889	3
	11:LM	1.889	10.667	0.000	2	-4.184	0.000	1
	12:LV	1.889	5.261	1.889	2	-6.056	0.000	2
	13:1.2D+1.6WI	1.889	1.16E+3	0.000	1	-2.41E+3	0.000	2
	14:1.2D+1.6WI	1.889	12.9E+3	1.889	2	-4.93E+3	1.889	3
	15:1.2D-1.6WL	1.889	3.9E+3	1.889	2	-1.57E+3	1.889	3
	16:1.2D-1.6WL	1.889	3.09E+3	1.889	3	-8.31E+3	1.889	2
	17:1.2D+1.0DI-	1.889	9.64E+3	1.889	2	-3.91E+3	1.889	3
	18:1.2D+1.0DI-	1.889	13.2E+3	1.889	2	-5.25E+3	1.889	3
	19:1.2D+1.0DI-	1.889	10.7E+3	1.889	2	-4.37E+3	1.889	3
	20:1.2D+1.0DI-	1.889	7.18E+3	1.889	2	-3.04E+3	1.889	3
	21:1.0D+1.0WI	1.889	1.54E+3	1.889	2	-621.284	1.889	3
	22:1.0D+1.0WI	1.889	4.22E+3	1.889	2	-1.64E+3	1.889	3
	23:1.0D-1.0WL	1.889	2.25E+3	1.889	2	-906.327	1.889	3
	24:1.0D-1.0WL	1.889	159.441	1.889	1	-482.118	0.000	2
	25:1.4D	1.889	2.65E+3	1.889	2	-1.07E+3	1.889	3
	26:1.2D+1.5LM	1.889	2.18E+3	1.889	2	-878.025	1.889	3
	27:1.2D+1.5LM	1.889	2.94E+3	1.889	2	-1.17E+3	1.889	3
	28:1.2D+1.5LM	1.889	2.38E+3	1.889	2	-961.922	1.889	3
	29:1.2D+1.5LM	1.889	1.62E+3	1.889	2	-669.820	1.889	3
	30:1.2D+1.5LV	1.889	2.28E+3	1.889	2	-920.023	1.889	3
153	1:DEAD	1.179	991.972	1.179	2	-2.23E+3	1.179	1
	2:WL#1	1.179	1.31E+3	1.179	1	-817.548	0.000	2
	3:WL#4	1.179	1.86E+3	1.179	2	-4.03E+3	1.179	1
	4:DI	1.179	4.01E+3	1.179	2	-8.84E+3	1.179	1
	5:WLI#1	1.179	609.226	1.179	1	-377.149	0.000	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	6:WLI#4	1.179	870.095	1.179	2	-1.86E+3	1.179	1
	7:WLS#1	1.179	459.822	1.179	1	-287.135	0.000	2
	8:WLS#4	1.179	652.787	1.179	2	-1.42E+3	1.179	1
	9:WLM#1	1.179	131.536	1.179	1	-82.258	0.000	2
	10:WLM#4	1.179	186.202	1.179	2	-403.420	1.179	1
	11:LM	1.179	36.371	1.179	2	-90.105	1.179	1
	12:LV	1.179	14.273	1.179	1	-5.797	1.179	3
	13:1.2D+1.6WI	1.179	895.437	1.179	2	-984.947	0.000	2
	14:1.2D+1.6WI	1.179	4.16E+3	1.179	2	-9.12E+3	1.179	1
	15:1.2D-1.6WL	1.179	1.94E+3	1.179	3	-4.77E+3	1.179	1
	16:1.2D-1.6WL	1.179	4.11E+3	0.000	1	-1.78E+3	1.179	2
	17:1.2D+1.0DI·	1.179	5.13E+3	1.179	2	-10.9E+3	1.179	1
	18:1.2D+1.0DI·	1.179	6.08E+3	1.179	2	-13.4E+3	1.179	1
	19:1.2D+1.0DI·	1.179	5.28E+3	1.179	2	-12.1E+3	1.179	1
	20:1.2D+1.0DI·	1.179	4.33E+3	1.179	2	-9.66E+3	1.179	1
	21:1.0D+1.0WI	1.179	927.105	1.179	2	-1.77E+3	1.179	1
	22:1.0D+1.0WI	1.179	1.64E+3	1.179	2	-3.64E+3	1.179	1
	23:1.0D-1.0WL	1.179	1.06E+3	1.179	2	-2.69E+3	1.179	1
	24:1.0D-1.0WL	1.179	484.244	1.179	3	-813.237	1.179	1
	25:1.4D	1.179	1.39E+3	1.179	2	-3.12E+3	1.179	1
	26:1.2D+1.5LM	1.179	1.23E+3	1.179	2	-2.68E+3	1.179	1
	27:1.2D+1.5LM	1.179	1.43E+3	1.179	2	-3.21E+3	1.179	1
	28:1.2D+1.5LM	1.179	1.26E+3	1.179	2	-2.94E+3	1.179	1
	29:1.2D+1.5LM	1.179	1.06E+3	1.179	2	-2.41E+3	1.179	1
	30:1.2D+1.5LV	1.179	1.18E+3	1.179	2	-2.65E+3	1.179	1
154	1:DEAD	1.179	1.61E+3	1.179	3	-3.94E+3	1.179	1
	2:WL#1	1.179	1.28E+3	0.000	1	-628.285	1.179	2
	3:WL#4	1.179	2.28E+3	1.179	2	-5.03E+3	1.179	1
	4:DI	1.179	5.56E+3	1.179	3	-13.8E+3	1.179	1
	5:WLI#1	1.179	601.482	0.000	1	-306.266	1.179	2
	6:WLI#4	1.179	1.07E+3	1.179	2	-2.32E+3	1.179	1
	7:WLS#1	1.179	448.980	0.000	1	-220.795	1.179	2
	8:WLS#4	1.179	801.699	1.179	2	-1.77E+3	1.179	1
	9:WLM#1	1.179	128.418	0.000	1	-63.086	1.179	2
	10:WLM#4	1.179	228.505	1.179	2	-503.362	1.179	1
	11:LM	1.179	54.952	1.179	3	-125.380	1.179	1
	12:LV	1.179	16.626	1.179	1	-9.592	1.179	3
	13:1.2D+1.6WI	1.179	1.76E+3	1.179	3	-2.91E+3	1.179	1
	14:1.2D+1.6WI	1.179	5.38E+3	1.179	2	-12.8E+3	1.179	1
	15:1.2D-1.6WL	1.179	2.73E+3	1.179	2	-6.55E+3	1.179	1
	16:1.2D-1.6WL	1.179	4.46E+3	0.000	1	-1.92E+3	1.179	2
	17:1.2D+1.0DI·	1.179	7.48E+3	1.179	3	-18E+3	1.179	1
	18:1.2D+1.0DI·	1.179	7.93E+3	1.179	2	-20.9E+3	1.179	1
	19:1.2D+1.0DI·	1.179	7.5E+3	1.179	3	-19.1E+3	1.179	1

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	20:1.2D+1.0DI·	1.179	7.12E+3	1.179	3	-16.2E+3	1.179	1
	21:1.0D+1.0WI	1.179	1.57E+3	1.179	3	-3.54E+3	1.179	1
	22:1.0D+1.0WI	1.179	2.24E+3	1.179	2	-5.71E+3	1.179	1
	23:1.0D-1.0WL	1.179	1.66E+3	1.179	2	-4.34E+3	1.179	1
	24:1.0D-1.0WL	1.179	1.31E+3	1.179	3	-2.18E+3	1.179	1
	25:1.4D	1.179	2.25E+3	1.179	3	-5.52E+3	1.179	1
	26:1.2D+1.5LM	1.179	2E+3	1.179	3	-4.81E+3	1.179	1
	27:1.2D+1.5LM	1.179	2.1E+3	1.179	3	-5.42E+3	1.179	1
	28:1.2D+1.5LM	1.179	2.02E+3	1.179	3	-5.04E+3	1.179	1
	29:1.2D+1.5LM	1.179	1.93E+3	1.179	3	-4.42E+3	1.179	1
	30:1.2D+1.5LV	1.179	1.92E+3	1.179	3	-4.71E+3	1.179	1
155	1:DEAD	1.889	753.212	1.889	3	-1.41E+3	1.889	2
	2:WL#1	1.889	3.22E+3	0.000	2	-1.11E+3	0.000	1
	3:WL#4	1.889	1.6E+3	1.889	3	-3.08E+3	1.889	2
	4:DI	1.889	3.32E+3	1.889	3	-6.37E+3	1.889	2
	5:WLI#1	1.889	1.53E+3	0.000	2	-529.173	0.000	1
	6:WLI#4	1.889	773.305	1.889	3	-1.43E+3	1.889	2
	7:WLS#1	1.889	1.13E+3	0.000	2	-390.330	0.000	1
	8:WLS#4	1.889	561.204	1.889	3	-1.08E+3	1.889	2
	9:WLM#1	1.889	324.776	0.000	2	-112.067	0.000	1
	10:WLM#4	1.889	160.554	1.889	3	-308.627	1.889	2
	11:LM	1.889	16.476	1.889	3	-34.762	1.889	2
	12:LV	1.889	8.130	1.889	2	-3.285	1.889	1
	13:1.2D+1.6WI	1.889	5.71E+3	0.000	2	-1.99E+3	0.000	1
	14:1.2D+1.6WI	1.889	3.46E+3	1.889	3	-6.62E+3	1.889	2
	15:1.2D-1.6WL	1.889	1.56E+3	0.000	1	-4.61E+3	0.000	2
	16:1.2D-1.6WL	1.889	3.24E+3	1.889	2	-1.65E+3	1.889	3
	17:1.2D+1.0DI·	1.889	4.49E+3	1.889	3	-8.04E+3	1.889	2
	18:1.2D+1.0DI·	1.889	5E+3	1.889	3	-9.49E+3	1.889	2
	19:1.2D+1.0DI·	1.889	3.95E+3	1.889	3	-8.09E+3	1.889	2
	20:1.2D+1.0DI·	1.889	3.45E+3	1.889	3	-6.63E+3	1.889	2
	21:1.0D+1.0WI	1.889	1.59E+3	0.000	2	-1.38E+3	1.889	2
	22:1.0D+1.0WI	1.889	1.31E+3	1.889	3	-2.49E+3	1.889	2
	23:1.0D-1.0WL	1.889	576.533	1.889	3	-1.44E+3	1.889	2
	24:1.0D-1.0WL	1.889	567.917	0.000	2	-327.936	1.889	2
	25:1.4D	1.889	1.05E+3	1.889	3	-1.97E+3	1.889	2
	26:1.2D+1.5LM	1.889	979.749	1.889	3	-1.74E+3	1.889	2
	27:1.2D+1.5LM	1.889	1.09E+3	1.889	3	-2.05E+3	1.889	2
	28:1.2D+1.5LM	1.889	877.388	1.889	3	-1.75E+3	1.889	2
	29:1.2D+1.5LM	1.889	768.014	1.889	3	-1.44E+3	1.889	2
	30:1.2D+1.5LV	1.889	899.298	1.889	3	-1.68E+3	1.889	2
156	1:DEAD	1.889	1.79E+3	1.889	3	-4.25E+3	1.889	2
	2:WL#1	1.889	1.44E+3	1.889	3	-2.9E+3	1.889	2
	3:WL#4	1.889	2.9E+3	1.889	3	-5.35E+3	1.889	2

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	4:DI	1.889	6.79E+3	1.889	3	-16.2E+3	1.889	2
	5:WLI#1	1.889	707.405	1.889	3	-1.39E+3	1.889	2
	6:WLI#4	1.889	1.36E+3	1.889	3	-2.46E+3	1.889	2
	7:WLS#1	1.889	504.327	1.889	3	-1.02E+3	1.889	2
	8:WLS#4	1.889	1.02E+3	1.889	3	-1.88E+3	1.889	2
	9:WLM#1	1.889	145.155	1.889	3	-292.654	1.889	2
	10:WLM#4	1.889	290.101	1.889	3	-535.999	1.889	2
	11:LM	1.889	26.798	1.889	3	-56.016	1.889	2
	12:LV	1.889	11.099	1.889	2	-5.436	1.889	3
	13:1.2D+1.6WI	1.889	4.44E+3	1.889	3	-9.74E+3	1.889	2
	14:1.2D+1.6WI	1.889	6.78E+3	1.889	3	-13.7E+3	1.889	2
	15:1.2D-1.6WL	1.889	644.111	0.000	1	-1.94E+3	0.000	2
	16:1.2D-1.6WL	1.889	3.46E+3	1.889	2	-2.49E+3	1.889	3
	17:1.2D+1.0DI·	1.889	9.64E+3	1.889	3	-22.7E+3	1.889	2
	18:1.2D+1.0DI·	1.889	10.3E+3	1.889	3	-23.8E+3	1.889	2
	19:1.2D+1.0DI·	1.889	8.22E+3	1.889	3	-19.9E+3	1.889	2
	20:1.2D+1.0DI·	1.889	7.57E+3	1.889	3	-18.9E+3	1.889	2
	21:1.0D+1.0WI	1.889	2.29E+3	1.889	3	-5.27E+3	1.889	2
	22:1.0D+1.0WI	1.889	2.8E+3	1.889	3	-6.13E+3	1.889	2
	23:1.0D-1.0WL	1.889	1.28E+3	1.889	3	-3.23E+3	1.889	2
	24:1.0D-1.0WL	1.889	768.665	1.889	1	-2.37E+3	1.889	2
	25:1.4D	1.889	2.5E+3	1.889	3	-5.95E+3	1.889	2
	26:1.2D+1.5LM	1.889	2.33E+3	1.889	3	-5.48E+3	1.889	2
	27:1.2D+1.5LM	1.889	2.47E+3	1.889	3	-5.72E+3	1.889	2
	28:1.2D+1.5LM	1.889	2.04E+3	1.889	3	-4.89E+3	1.889	2
	29:1.2D+1.5LM	1.889	1.89E+3	1.889	3	-4.65E+3	1.889	2
	30:1.2D+1.5LV	1.889	2.13E+3	1.889	3	-5.09E+3	1.889	2
175	1:DEAD	3.000	3.11E+3	3.000		-3.12E+3	3.000	
	2:WL#1	3.000	6.59E+3	3.000		-6.43E+3	3.000	
	3:WL#4	3.000	1.46E+3	0.000		-1.25E+3	0.000	
	4:DI	3.000	13.2E+3	3.000		-13.1E+3	3.000	
	5:WLI#1	3.000	3.19E+3	3.000		-3.12E+3	3.000	
	6:WLI#4	3.000	676.992	0.000		-583.668	0.000	
	7:WLS#1	3.000	2.32E+3	3.000		-2.26E+3	3.000	
	8:WLS#4	3.000	512.903	0.000		-439.884	0.000	
	9:WLM#1	3.000	665.317	3.000		-648.950	3.000	
	10:WLM#4	3.000	145.951	0.000		-125.190	0.000	
	11:LM	3.000	39.613	3.000		-37.424	3.000	
	12:LV	3.000	8.469	0.000		-8.785	0.000	
	13:1.2D+1.6WI	3.000	14.3E+3	3.000		-14E+3	3.000	
	14:1.2D+1.6WI	3.000	5.61E+3	3.000		-5.28E+3	3.000	
	15:1.2D-1.6WL	3.000	7.4E+3	0.000		-7.69E+3	0.000	
	16:1.2D-1.6WL	3.000	2.08E+3	3.000		-2.41E+3	3.000	
	17:1.2D+1.0DI·	3.000	20.1E+3	3.000		-20E+3	3.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	18:1.2D+1.0DI·	3.000	17.5E+3	3.000		-17.3E+3	3.000	
	19:1.2D+1.0DI·	3.000	13.8E+3	3.000		-13.8E+3	3.000	
	20:1.2D+1.0DI·	3.000	16.4E+3	3.000		-16.4E+3	3.000	
	21:1.0D+1.0WI	3.000	5.43E+3	3.000		-5.37E+3	3.000	
	22:1.0D+1.0WI	3.000	3.52E+3	3.000		-3.45E+3	3.000	
	23:1.0D-1.0WL	3.000	824.882	3.000		-885.144	3.000	
	24:1.0D-1.0WL	3.000	2.72E+3	3.000		-2.8E+3	3.000	
	25:1.4D	3.000	4.36E+3	3.000		-4.36E+3	3.000	
	26:1.2D+1.5LM	3.000	4.46E+3	3.000		-4.44E+3	3.000	
	27:1.2D+1.5LM	3.000	3.91E+3	3.000		-3.89E+3	3.000	
	28:1.2D+1.5LM	3.000	3.13E+3	3.000		-3.15E+3	3.000	
	29:1.2D+1.5LM	3.000	3.68E+3	3.000		-3.7E+3	3.000	
	30:1.2D+1.5LV	3.000	3.75E+3	3.000		-3.75E+3	3.000	
176	1:DEAD	1.333	654.587	0.000		-667.731	0.000	
	2:WL#1	1.333	4.87E+3	0.000		-4.6E+3	0.000	
	3:WL#4	1.333	2.49E+3	0.000		-2.45E+3	0.000	
	4:DI	1.333	2.71E+3	0.000		-2.76E+3	0.000	
	5:WLI#1	1.333	2.13E+3	0.000		-2.01E+3	0.000	
	6:WLI#4	1.333	1.14E+3	0.000		-1.12E+3	0.000	
	7:WLS#1	1.333	1.71E+3	0.000		-1.61E+3	0.000	
	8:WLS#4	1.333	872.968	0.000		-858.916	0.000	
	9:WLM#1	1.333	487.341	0.000		-459.871	0.000	
	10:WLM#4	1.333	248.100	0.000		-244.095	0.000	
	11:LM	1.333	36.447	1.333		-38.086	1.333	
	12:LV	1.333	6.131	0.000		-6.254	0.000	
	13:1.2D+1.6WI	1.333	7.59E+3	0.000		-7.17E+3	0.000	
	14:1.2D+1.6WI	1.333	4.55E+3	0.000		-4.5E+3	0.000	
	15:1.2D-1.6WL	1.333	7.61E+3	0.000		-8.07E+3	0.000	
	16:1.2D-1.6WL	1.333	3.41E+3	0.000		-3.49E+3	0.000	
	17:1.2D+1.0DI·	1.333	3.77E+3	0.000		-3.72E+3	0.000	
	18:1.2D+1.0DI·	1.333	4.38E+3	0.000		-4.43E+3	0.000	
	19:1.2D+1.0DI·	1.333	4.32E+3	0.000		-4.5E+3	0.000	
	20:1.2D+1.0DI·	1.333	2.8E+3	0.000		-2.89E+3	0.000	
	21:1.0D+1.0WI	1.333	1.64E+3	0.000		-1.56E+3	0.000	
	22:1.0D+1.0WI	1.333	1.41E+3	0.000		-1.41E+3	0.000	
	23:1.0D-1.0WL	1.333	1.9E+3	0.000		-2.01E+3	0.000	
	24:1.0D-1.0WL	1.333	617.300	0.000		-644.497	0.000	
	25:1.4D	1.333	916.421	0.000		-934.823	0.000	
	26:1.2D+1.5LM	1.333	852.355	0.000		-843.117	0.000	
	27:1.2D+1.5LM	1.333	1.02E+3	0.000		-1.04E+3	0.000	
	28:1.2D+1.5LM	1.333	1.05E+3	0.000		-1.1E+3	0.000	
	29:1.2D+1.5LM	1.333	684.360	0.000		-706.597	0.000	
	30:1.2D+1.5LV	1.333	794.700	0.000		-810.657	0.000	
177	1:DEAD	1.500	919.853	1.500		-637.279	1.500	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	2:WL#1	1.500	7.1E+3	0.000		-7E+3	0.000	
	3:WL#4	1.500	7.3E+3	1.500		-7.13E+3	1.500	
	4:DI	1.500	3.99E+3	1.500		-2.63E+3	1.500	
	5:WLI#1	1.500	3.43E+3	0.000		-3.38E+3	0.000	
	6:WLI#4	1.500	3.2E+3	1.500		-3.13E+3	1.500	
	7:WLS#1	1.500	2.49E+3	0.000		-2.46E+3	0.000	
	8:WLS#4	1.500	2.56E+3	1.500		-2.51E+3	1.500	
	9:WLM#1	1.500	716.689	0.000		-706.829	0.000	
	10:WLM#4	1.500	726.422	1.500		-710.013	1.500	
	11:LM	1.500	28.845	1.500		-28.198	1.500	
	12:LV	1.500	14.423	1.500		-14.099	1.500	
	13:1.2D+1.6WI	1.500	11.5E+3	0.000		-11E+3	0.000	
	14:1.2D+1.6WI	1.500	10.9E+3	1.500		-10.3E+3	1.500	
	15:1.2D-1.6WL	1.500	11.4E+3	0.000		-11.2E+3	0.000	
	16:1.2D-1.6WL	1.500	12.5E+3	1.500		-12.4E+3	1.500	
	17:1.2D+1.0DI·	1.500	4.44E+3	0.000		-2.75E+3	0.000	
	18:1.2D+1.0DI·	1.500	3.02E+3	0.000		-1.3E+3	0.000	
	19:1.2D+1.0DI·	1.500	6.92E+3	1.500		-5.26E+3	1.500	
	20:1.2D+1.0DI·	1.500	8.19E+3	1.500		-6.57E+3	1.500	
	21:1.0D+1.0WI	1.500	2.64E+3	0.000		-2.33E+3	0.000	
	22:1.0D+1.0WI	1.500	1.94E+3	1.500		-1.6E+3	1.500	
	23:1.0D-1.0WL	1.500	2.6E+3	0.000		-2.36E+3	0.000	
	24:1.0D-1.0WL	1.500	3.42E+3	1.500		-3.19E+3	1.500	
	25:1.4D	1.500	1.29E+3	1.500		-892.190	1.500	
	26:1.2D+1.5LM	1.500	884.856	0.000		-546.924	0.000	
	27:1.2D+1.5LM	1.500	668.706	0.000		-324.225	0.000	
	28:1.2D+1.5LM	1.500	1.54E+3	1.500		-1.21E+3	1.500	
	29:1.2D+1.5LM	1.500	1.84E+3	1.500		-1.52E+3	1.500	
	30:1.2D+1.5LV	1.500	1.12E+3	1.500		-781.148	1.500	
179	1:DEAD	3.000	3.16E+3	3.000		-3.16E+3	3.000	
	2:WL#1	3.000	2.63E+3	3.000		-2.54E+3	3.000	
	3:WL#4	3.000	6.23E+3	3.000		-6.47E+3	3.000	
	4:DI	3.000	13.7E+3	3.000		-13.6E+3	3.000	
	5:WLI#1	3.000	1.2E+3	3.000		-1.15E+3	3.000	
	6:WLI#4	3.000	2.86E+3	3.000		-2.97E+3	3.000	
	7:WLS#1	3.000	923.957	3.000		-892.009	3.000	
	8:WLS#4	3.000	2.19E+3	3.000		-2.27E+3	3.000	
	9:WLM#1	3.000	264.213	3.000		-255.087	3.000	
	10:WLM#4	3.000	623.334	3.000		-647.924	3.000	
	11:LM	3.000	16.937	0.000		-17.569	0.000	
	12:LV	3.000	26.751	0.000		-27.243	0.000	
	13:1.2D+1.6WI	3.000	1.54E+3	0.000		-1.42E+3	0.000	
	14:1.2D+1.6WI	3.000	7.11E+3	0.000		-7.53E+3	0.000	
	15:1.2D-1.6WL	3.000	7.84E+3	3.000		-7.99E+3	3.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	16:1.2D-1.6WL	3.000	14.1E+3	3.000		-13.7E+3	3.000	
	17:1.2D+1.0DI·	3.000	16.3E+3	3.000		-16.2E+3	3.000	
	18:1.2D+1.0DI·	3.000	14.5E+3	3.000		-14.5E+3	3.000	
	19:1.2D+1.0DI·	3.000	18.6E+3	3.000		-18.6E+3	3.000	
	20:1.2D+1.0DI·	3.000	20.4E+3	3.000		-20.2E+3	3.000	
	21:1.0D+1.0WI	3.000	2.28E+3	3.000		-2.25E+3	3.000	
	22:1.0D+1.0WI	3.000	970.940	3.000		-1.06E+3	3.000	
	23:1.0D-1.0WL	3.000	4.05E+3	3.000		-4.08E+3	3.000	
	24:1.0D-1.0WL	3.000	5.42E+3	3.000		-5.33E+3	3.000	
	25:1.4D	3.000	4.42E+3	3.000		-4.43E+3	3.000	
	26:1.2D+1.5LM	3.000	3.56E+3	3.000		-3.55E+3	3.000	
	27:1.2D+1.5LM	3.000	3.17E+3	3.000		-3.2E+3	3.000	
	28:1.2D+1.5LM	3.000	4.07E+3	3.000		-4.08E+3	3.000	
	29:1.2D+1.5LM	3.000	4.45E+3	3.000		-4.43E+3	3.000	
	30:1.2D+1.5LV	3.000	3.76E+3	3.000		-3.76E+3	3.000	
180	1:DEAD	1.333	654.445	0.000		-668.102	0.000	
	2:WL#1	1.333	1.82E+3	0.000		-1.86E+3	0.000	
	3:WL#4	1.333	4.57E+3	0.000		-4.78E+3	0.000	
	4:DI	1.333	2.72E+3	0.000		-2.78E+3	0.000	
	5:WLI#1	1.333	891.337	0.000		-915.800	0.000	
	6:WLI#4	1.333	2.06E+3	0.000		-2.16E+3	0.000	
	7:WLS#1	1.333	639.114	0.000		-652.122	0.000	
	8:WLS#4	1.333	1.61E+3	0.000		-1.68E+3	0.000	
	9:WLM#1	1.333	183.517	0.000		-187.340	0.000	
	10:WLM#4	1.333	456.508	0.000		-477.181	0.000	
	11:LM	1.333	12.262	0.000		-12.507	0.000	
	12:LV	1.333	18.631	0.000		-18.395	0.000	
	13:1.2D+1.6WI	1.333	3.68E+3	0.000		-3.75E+3	0.000	
	14:1.2D+1.6WI	1.333	7.25E+3	0.000		-7.59E+3	0.000	
	15:1.2D-1.6WL	1.333	2.2E+3	0.000		-2.16E+3	0.000	
	16:1.2D-1.6WL	1.333	7.79E+3	0.000		-7.47E+3	0.000	
	17:1.2D+1.0DI·	1.333	4.38E+3	0.000		-4.48E+3	0.000	
	18:1.2D+1.0DI·	1.333	3.89E+3	0.000		-4.06E+3	0.000	
	19:1.2D+1.0DI·	1.333	2.64E+3	0.000		-2.69E+3	0.000	
	20:1.2D+1.0DI·	1.333	4.28E+3	0.000		-4.26E+3	0.000	
	21:1.0D+1.0WI	1.333	1.28E+3	0.000		-1.31E+3	0.000	
	22:1.0D+1.0WI	1.333	1.64E+3	0.000		-1.73E+3	0.000	
	23:1.0D-1.0WL	1.333	371.774	1.333		-368.800	1.333	
	24:1.0D-1.0WL	1.333	1.88E+3	0.000		-1.82E+3	0.000	
	25:1.4D	1.333	916.223	0.000		-935.343	0.000	
	26:1.2D+1.5LM	1.333	982.720	0.000		-1E+3	0.000	
	27:1.2D+1.5LM	1.333	862.352	0.000		-899.783	0.000	
	28:1.2D+1.5LM	1.333	627.352	0.000		-640.286	0.000	
	29:1.2D+1.5LM	1.333	991.219	0.000		-987.303	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	30:1.2D+1.5LV	1.333	810.078	0.000		-826.114	0.000	
181	1:DEAD	1.500	906.890	1.500		-620.064	1.500	
	2:WL#1	1.500	7.55E+3	0.000		-7.45E+3	0.000	
	3:WL#4	1.500	7.31E+3	1.500		-7.47E+3	1.500	
	4:DI	1.500	3.76E+3	1.500		-2.34E+3	1.500	
	5:WLI#1	1.500	3.5E+3	0.000		-3.46E+3	0.000	
	6:WLI#4	1.500	3.18E+3	1.500		-3.26E+3	1.500	
	7:WLS#1	1.500	2.65E+3	0.000		-2.62E+3	0.000	
	8:WLS#4	1.500	2.57E+3	1.500		-2.63E+3	1.500	
	9:WLM#1	1.500	758.999	0.000		-749.129	0.000	
	10:WLM#4	1.500	728.005	1.500		-744.251	1.500	
	11:LM	1.500	28.845	1.500		-28.198	1.500	
	12:LV	1.500	30.306	1.500		-31.526	1.500	
	13:1.2D+1.6WI	1.500	12.2E+3	0.000		-11.7E+3	0.000	
	14:1.2D+1.6WI	1.500	12.8E+3	1.500		-12.7E+3	1.500	
	15:1.2D-1.6WL	1.500	12.1E+3	0.000		-11.9E+3	0.000	
	16:1.2D-1.6WL	1.500	11.2E+3	1.500		-10.6E+3	1.500	
	17:1.2D+1.0DI·	1.500	4.39E+3	0.000		-2.63E+3	0.000	
	18:1.2D+1.0DI·	1.500	7.99E+3	1.500		-6.29E+3	1.500	
	19:1.2D+1.0DI·	1.500	6.73E+3	1.500		-5.01E+3	1.500	
	20:1.2D+1.0DI·	1.500	3.16E+3	0.000		-1.36E+3	0.000	
	21:1.0D+1.0WI	1.500	2.79E+3	0.000		-2.48E+3	0.000	
	22:1.0D+1.0WI	1.500	3.47E+3	1.500		-3.24E+3	1.500	
	23:1.0D-1.0WL	1.500	2.76E+3	0.000		-2.52E+3	0.000	
	24:1.0D-1.0WL	1.500	2.03E+3	1.500		-1.68E+3	1.500	
	25:1.4D	1.500	1.27E+3	1.500		-868.090	1.500	
	26:1.2D+1.5LM	1.500	918.793	0.000		-575.749	0.000	
	27:1.2D+1.5LM	1.500	1.84E+3	1.500		-1.51E+3	1.500	
	28:1.2D+1.5LM	1.500	1.54E+3	1.500		-1.2E+3	1.500	
	29:1.2D+1.5LM	1.500	685.833	0.000		-336.411	0.000	
	30:1.2D+1.5LV	1.500	1.13E+3	1.500		-791.361	1.500	
183	1:DEAD	3.000	3.2E+3	3.000		-3.2E+3	3.000	
	2:WL#1	3.000	4.5E+3	0.000		-4.78E+3	0.000	
	3:WL#4	3.000	4.95E+3	3.000		-4.91E+3	3.000	
	4:DI	3.000	14.5E+3	3.000		-14.5E+3	3.000	
	5:WLI#1	3.000	2.08E+3	0.000		-2.2E+3	0.000	
	6:WLI#4	3.000	2.36E+3	3.000		-2.34E+3	3.000	
	7:WLS#1	3.000	1.58E+3	0.000		-1.68E+3	0.000	
	8:WLS#4	3.000	1.74E+3	3.000		-1.72E+3	3.000	
	9:WLM#1	3.000	452.446	0.000		-480.023	0.000	
	10:WLM#4	3.000	497.477	3.000		-493.020	3.000	
	11:LM	3.000	53.500	0.000		-54.485	0.000	
	12:LV	3.000	19.809	3.000		-18.714	3.000	
	13:1.2D+1.6WI	3.000	4.54E+3	0.000		-5E+3	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	14:1.2D+1.6WI	3.000	11.8E+3	3.000		-11.7E+3	3.000	
	15:1.2D-1.6WL	3.000	11.2E+3	3.000		-10.7E+3	3.000	
	16:1.2D-1.6WL	3.000	4.4E+3	0.000		-4.5E+3	0.000	
	17:1.2D+1.0DI·	3.000	16.3E+3	3.000		-16.3E+3	3.000	
	18:1.2D+1.0DI·	3.000	20.7E+3	3.000		-20.7E+3	3.000	
	19:1.2D+1.0DI·	3.000	20.5E+3	3.000		-20.3E+3	3.000	
	20:1.2D+1.0DI·	3.000	16E+3	3.000		-16E+3	3.000	
	21:1.0D+1.0WI	3.000	1.65E+3	3.000		-1.75E+3	3.000	
	22:1.0D+1.0WI	3.000	4.94E+3	3.000		-4.92E+3	3.000	
	23:1.0D-1.0WL	3.000	4.8E+3	3.000		-4.7E+3	3.000	
	24:1.0D-1.0WL	3.000	1.46E+3	3.000		-1.48E+3	3.000	
	25:1.4D	3.000	4.48E+3	3.000		-4.48E+3	3.000	
	26:1.2D+1.5LM	3.000	3.31E+3	3.000		-3.34E+3	3.000	
	27:1.2D+1.5LM	3.000	4.26E+3	3.000		-4.26E+3	3.000	
	28:1.2D+1.5LM	3.000	4.22E+3	3.000		-4.19E+3	3.000	
	29:1.2D+1.5LM	3.000	3.27E+3	3.000		-3.27E+3	3.000	
	30:1.2D+1.5LV	3.000	3.87E+3	3.000		-3.87E+3	3.000	
184	1:DEAD	1.333	647.610	0.000		-660.928	0.000	
	2:WL#1	1.333	4.03E+3	0.000		-4.22E+3	0.000	
	3:WL#4	1.333	3.58E+3	0.000		-3.37E+3	0.000	
	4:DI	1.333	2.63E+3	0.000		-2.69E+3	0.000	
	5:WLI#1	1.333	1.75E+3	0.000		-1.83E+3	0.000	
	6:WLI#4	1.333	1.57E+3	0.000		-1.48E+3	0.000	
	7:WLS#1	1.333	1.42E+3	0.000		-1.48E+3	0.000	
	8:WLS#4	1.333	1.26E+3	0.000		-1.18E+3	0.000	
	9:WLM#1	1.333	402.761	0.000		-421.403	0.000	
	10:WLM#4	1.333	357.236	0.000		-336.075	0.000	
	11:LM	1.333	37.263	0.000		-36.792	0.000	
	12:LV	1.333	18.224	1.333		-19.044	1.333	
	13:1.2D+1.6WI	1.333	6.24E+3	0.000		-6.55E+3	0.000	
	14:1.2D+1.6WI	1.333	5.34E+3	0.000		-5.02E+3	0.000	
	15:1.2D-1.6WL	1.333	7.04E+3	0.000		-6.75E+3	0.000	
	16:1.2D-1.6WL	1.333	5.84E+3	0.000		-6.19E+3	0.000	
	17:1.2D+1.0DI·	1.333	3.36E+3	0.000		-3.52E+3	0.000	
	18:1.2D+1.0DI·	1.333	3.04E+3	0.000		-3.02E+3	0.000	
	19:1.2D+1.0DI·	1.333	4.28E+3	0.000		-4.27E+3	0.000	
	20:1.2D+1.0DI·	1.333	4.3E+3	0.000		-4.46E+3	0.000	
	21:1.0D+1.0WI	1.333	1.35E+3	0.000		-1.42E+3	0.000	
	22:1.0D+1.0WI	1.333	1.06E+3	0.000		-1E+3	0.000	
	23:1.0D-1.0WL	1.333	1.8E+3	0.000		-1.75E+3	0.000	
	24:1.0D-1.0WL	1.333	1.62E+3	0.000		-1.71E+3	0.000	
	25:1.4D	1.333	906.654	0.000		-925.299	0.000	
	26:1.2D+1.5LM	1.333	801.967	0.000		-835.884	0.000	
	27:1.2D+1.5LM	1.333	705.419	0.000		-699.532	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	28:1.2D+1.5LM	1.333	1.03E+3	0.000		-1.03E+3	0.000	
	29:1.2D+1.5LM	1.333	1.05E+3	0.000		-1.09E+3	0.000	
	30:1.2D+1.5LV	1.333	801.872	0.000		-819.082	0.000	
185	1:DEAD	1.500	906.547	1.500		-619.709	1.500	
	2:WL#1	1.500	8.7E+3	1.500		-8.89E+3	1.500	
	3:WL#4	1.500	8.17E+3	0.000		-8.17E+3	0.000	
	4:DI	1.500	3.7E+3	1.500		-2.35E+3	1.500	
	5:WLI#1	1.500	3.68E+3	1.500		-3.77E+3	1.500	
	6:WLI#4	1.500	3.79E+3	0.000		-3.79E+3	0.000	
	7:WLS#1	1.500	3.06E+3	1.500		-3.13E+3	1.500	
	8:WLS#4	1.500	2.87E+3	0.000		-2.87E+3	0.000	
	9:WLM#1	1.500	867.914	1.500		-887.332	1.500	
	10:WLM#4	1.500	820.152	0.000		-820.109	0.000	
	11:LM	1.500	60.612	1.500		-63.052	1.500	
	12:LV	1.500	14.422	1.500		-14.099	1.500	
	13:1.2D+1.6WI	1.500	15E+3	1.500		-15E+3	1.500	
	14:1.2D+1.6WI	1.500	13.2E+3	0.000		-12.9E+3	0.000	
	15:1.2D-1.6WL	1.500	13.5E+3	1.500		-12.8E+3	1.500	
	16:1.2D-1.6WL	1.500	13.2E+3	0.000		-12.9E+3	0.000	
	17:1.2D+1.0DI	1.500	8.46E+3	1.500		-6.87E+3	1.500	
	18:1.2D+1.0DI	1.500	4.88E+3	1.500		-3.19E+3	1.500	
	19:1.2D+1.0DI	1.500	1.97E+3	0.000		-241.921	0.000	
	20:1.2D+1.0DI	1.500	5.41E+3	0.000		-3.77E+3	0.000	
	21:1.0D+1.0WI	1.500	3.97E+3	1.500		-3.75E+3	1.500	
	22:1.0D+1.0WI	1.500	3.01E+3	0.000		-2.73E+3	0.000	
	23:1.0D-1.0WL	1.500	2.51E+3	1.500		-2.15E+3	1.500	
	24:1.0D-1.0WL	1.500	3.02E+3	0.000		-2.74E+3	0.000	
	25:1.4D	1.500	1.27E+3	1.500		-867.593	1.500	
	26:1.2D+1.5LM	1.500	2.05E+3	1.500		-1.73E+3	1.500	
	27:1.2D+1.5LM	1.500	1.2E+3	1.500		-855.570	1.500	
	28:1.2D+1.5LM	1.500	399.845	0.000		-51.869	0.000	
	29:1.2D+1.5LM	1.500	1.21E+3	1.500		-872.433	1.500	
	30:1.2D+1.5LV	1.500	1.1E+3	1.500		-760.291	1.500	
188	1:DEAD	3.000	3.11E+3	3.000		-3.12E+3	3.000	
	2:WL#1	3.000	2.26E+3	3.000		-2.16E+3	3.000	
	3:WL#4	3.000	6.41E+3	0.000		-6.16E+3	0.000	
	4:DI	3.000	13.2E+3	3.000		-13.1E+3	3.000	
	5:WLI#1	3.000	1.13E+3	3.000		-1.09E+3	3.000	
	6:WLI#4	3.000	2.95E+3	0.000		-2.84E+3	0.000	
	7:WLS#1	3.000	794.352	3.000		-758.323	3.000	
	8:WLS#4	3.000	2.25E+3	0.000		-2.16E+3	0.000	
	9:WLM#1	3.000	229.080	3.000		-218.823	3.000	
	10:WLM#4	3.000	641.889	0.000		-617.094	0.000	
	11:LM	3.000	16.942	0.000		-17.572	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	12:LV	3.000	19.815	3.000		-18.721	3.000	
	13:1.2D+1.6WI	3.000	1.07E+3	0.000		-934.987	0.000	
	14:1.2D+1.6WI	3.000	13.9E+3	3.000		-13.5E+3	3.000	
	15:1.2D-1.6WL	3.000	7.19E+3	3.000		-7.35E+3	3.000	
	16:1.2D-1.6WL	3.000	7.29E+3	0.000		-7.71E+3	0.000	
	17:1.2D+1.0DI·	3.000	15.8E+3	3.000		-15.7E+3	3.000	
	18:1.2D+1.0DI·	3.000	19.8E+3	3.000		-19.7E+3	3.000	
	19:1.2D+1.0DI·	3.000	18E+3	3.000		-18E+3	3.000	
	20:1.2D+1.0DI·	3.000	14E+3	3.000		-14E+3	3.000	
	21:1.0D+1.0WI	3.000	2.36E+3	3.000		-2.33E+3	3.000	
	22:1.0D+1.0WI	3.000	5.34E+3	3.000		-5.26E+3	3.000	
	23:1.0D-1.0WL	3.000	3.87E+3	3.000		-3.91E+3	3.000	
	24:1.0D-1.0WL	3.000	921.628	3.000		-1.01E+3	3.000	
	25:1.4D	3.000	4.36E+3	3.000		-4.36E+3	3.000	
	26:1.2D+1.5LM	3.000	3.54E+3	3.000		-3.53E+3	3.000	
	27:1.2D+1.5LM	3.000	4.39E+3	3.000		-4.37E+3	3.000	
	28:1.2D+1.5LM	3.000	3.97E+3	3.000		-3.99E+3	3.000	
	29:1.2D+1.5LM	3.000	3.12E+3	3.000		-3.15E+3	3.000	
	30:1.2D+1.5LV	3.000	3.77E+3	3.000		-3.77E+3	3.000	
189	1:DEAD	1.333	654.789	1.333		-667.959	1.333	
	2:WL#1	1.333	1.81E+3	1.333		-1.91E+3	1.333	
	3:WL#4	1.333	4.96E+3	1.333		-4.72E+3	1.333	
	4:DI	1.333	2.73E+3	1.333		-2.79E+3	1.333	
	5:WLI#1	1.333	891.522	1.333		-933.166	1.333	
	6:WLI#4	1.333	2.21E+3	1.333		-2.1E+3	1.333	
	7:WLS#1	1.333	637.258	1.333		-670.611	1.333	
	8:WLS#4	1.333	1.74E+3	1.333		-1.66E+3	1.333	
	9:WLM#1	1.333	183.041	1.333		-192.562	1.333	
	10:WLM#4	1.333	495.187	1.333		-470.617	1.333	
	11:LM	1.333	12.262	1.333		-12.509	1.333	
	12:LV	1.333	18.222	0.000		-19.042	0.000	
	13:1.2D+1.6WI	1.333	3.66E+3	1.333		-3.83E+3	1.333	
	14:1.2D+1.6WI	1.333	7.94E+3	1.333		-7.57E+3	1.333	
	15:1.2D-1.6WL	1.333	2.3E+3	1.333		-2.17E+3	1.333	
	16:1.2D-1.6WL	1.333	7.6E+3	1.333		-8.01E+3	1.333	
	17:1.2D+1.0DI·	1.333	4.38E+3	1.333		-4.49E+3	1.333	
	18:1.2D+1.0DI·	1.333	4.21E+3	1.333		-4.17E+3	1.333	
	19:1.2D+1.0DI·	1.333	2.69E+3	1.333		-2.72E+3	1.333	
	20:1.2D+1.0DI·	1.333	4.03E+3	1.333		-4.21E+3	1.333	
	21:1.0D+1.0WI	1.333	1.28E+3	1.333		-1.32E+3	1.333	
	22:1.0D+1.0WI	1.333	1.84E+3	1.333		-1.77E+3	1.333	
	23:1.0D-1.0WL	1.333	260.079	0.000		-236.272	0.000	
	24:1.0D-1.0WL	1.333	1.8E+3	1.333		-1.89E+3	1.333	
	25:1.4D	1.333	916.705	1.333		-935.143	1.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	26:1.2D+1.5LM	1.333	980.189	1.333		-1.01E+3	1.333	
	27:1.2D+1.5LM	1.333	934.315	1.333		-925.919	1.333	
	28:1.2D+1.5LM	1.333	632.161	1.333		-638.814	1.333	
	29:1.2D+1.5LM	1.333	939.158	1.333		-979.901	1.333	
	30:1.2D+1.5LV	1.333	810.481	1.333		-827.514	1.333	
191	1:DEAD	3.000	3.2E+3	3.000		-3.2E+3	3.000	
	2:WL#1	3.000	6.87E+3	3.000		-6.7E+3	3.000	
	3:WL#4	3.000	1.42E+3	0.000		-1.63E+3	0.000	
	4:DI	3.000	14.2E+3	3.000		-14E+3	3.000	
	5:WLI#1	3.000	3.25E+3	3.000		-3.17E+3	3.000	
	6:WLI#4	3.000	625.641	0.000		-720.853	0.000	
	7:WLS#1	3.000	2.41E+3	3.000		-2.35E+3	3.000	
	8:WLS#4	3.000	498.350	0.000		-573.448	0.000	
	9:WLM#1	3.000	692.111	3.000		-674.573	3.000	
	10:WLM#4	3.000	141.431	0.000		-162.783	0.000	
	11:LM	3.000	39.618	3.000		-37.429	3.000	
	12:LV	3.000	26.750	0.000		-27.243	0.000	
	13:1.2D+1.6WI	3.000	14.8E+3	3.000		-14.5E+3	3.000	
	14:1.2D+1.6WI	3.000	2.53E+3	3.000		-2.87E+3	3.000	
	15:1.2D-1.6WL	3.000	7.43E+3	0.000		-7.73E+3	0.000	
	16:1.2D-1.6WL	3.000	5.58E+3	3.000		-5.24E+3	3.000	
	17:1.2D+1.0DI·	3.000	21.2E+3	3.000		-21E+3	3.000	
	18:1.2D+1.0DI·	3.000	17.5E+3	3.000		-17.5E+3	3.000	
	19:1.2D+1.0DI·	3.000	14.8E+3	3.000		-14.7E+3	3.000	
	20:1.2D+1.0DI·	3.000	18.5E+3	3.000		-18.3E+3	3.000	
	21:1.0D+1.0WI	3.000	5.61E+3	3.000		-5.54E+3	3.000	
	22:1.0D+1.0WI	3.000	2.86E+3	3.000		-2.94E+3	3.000	
	23:1.0D-1.0WL	3.000	826.752	3.000		-889.174	3.000	
	24:1.0D-1.0WL	3.000	3.56E+3	3.000		-3.48E+3	3.000	
	25:1.4D	3.000	4.48E+3	3.000		-4.48E+3	3.000	
	26:1.2D+1.5LM	3.000	4.59E+3	3.000		-4.57E+3	3.000	
	27:1.2D+1.5LM	3.000	3.8E+3	3.000		-3.82E+3	3.000	
	28:1.2D+1.5LM	3.000	3.21E+3	3.000		-3.22E+3	3.000	
	29:1.2D+1.5LM	3.000	4E+3	3.000		-3.98E+3	3.000	
	30:1.2D+1.5LV	3.000	3.8E+3	3.000		-3.8E+3	3.000	
192	1:DEAD	1.333	647.605	1.333		-660.906	1.333	
	2:WL#1	1.333	4.79E+3	1.333		-4.53E+3	1.333	
	3:WL#4	1.333	2.79E+3	1.333		-2.86E+3	1.333	
	4:DI	1.333	2.62E+3	1.333		-2.67E+3	1.333	
	5:WLI#1	1.333	2.05E+3	1.333		-1.95E+3	1.333	
	6:WLI#4	1.333	1.23E+3	1.333		-1.26E+3	1.333	
	7:WLS#1	1.333	1.68E+3	1.333		-1.59E+3	1.333	
	8:WLS#4	1.333	979.387	1.333		-1E+3	1.333	
	9:WLM#1	1.333	478.363	1.333		-452.392	1.333	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	10:WLM#4	1.333	278.361	1.333		-285.554	1.333	
	11:LM	1.333	36.449	0.000		-38.088	0.000	
	12:LV	1.333	18.632	1.333		-18.396	1.333	
	13:1.2D+1.6WI	1.333	7.55E+3	1.333		-7.15E+3	1.333	
	14:1.2D+1.6WI	1.333	3.91E+3	1.333		-4.04E+3	1.333	
	15:1.2D-1.6WL	1.333	7.43E+3	1.333		-7.87E+3	1.333	
	16:1.2D-1.6WL	1.333	5.18E+3	1.333		-5.08E+3	1.333	
	17:1.2D+1.0DI·	1.333	3.78E+3	1.333		-3.74E+3	1.333	
	18:1.2D+1.0DI·	1.333	2.64E+3	1.333		-2.74E+3	1.333	
	19:1.2D+1.0DI·	1.333	4.08E+3	1.333		-4.25E+3	1.333	
	20:1.2D+1.0DI·	1.333	4.38E+3	1.333		-4.42E+3	1.333	
	21:1.0D+1.0WI	1.333	1.68E+3	1.333		-1.6E+3	1.333	
	22:1.0D+1.0WI	1.333	755.119	0.000		-791.579	0.000	
	23:1.0D-1.0WL	1.333	1.83E+3	1.333		-1.93E+3	1.333	
	24:1.0D-1.0WL	1.333	1.55E+3	1.333		-1.53E+3	1.333	
	25:1.4D	1.333	906.647	1.333		-925.269	1.333	
	26:1.2D+1.5LM	1.333	886.709	1.333		-879.158	1.333	
	27:1.2D+1.5LM	1.333	639.512	1.333		-665.126	1.333	
	28:1.2D+1.5LM	1.333	1E+3	1.333		-1.05E+3	1.333	
	29:1.2D+1.5LM	1.333	1.06E+3	1.333		-1.07E+3	1.333	
	30:1.2D+1.5LV	1.333	801.822	1.333		-817.430	1.333	
194	1:DEAD	3.000	3.16E+3	3.000		-3.16E+3	3.000	
	2:WL#1	3.000	4.26E+3	0.000		-4.52E+3	0.000	
	3:WL#4	3.000	5.03E+3	3.000		-5.07E+3	3.000	
	4:DI	3.000	13.8E+3	3.000		-13.7E+3	3.000	
	5:WLI#1	3.000	2.03E+3	0.000		-2.15E+3	0.000	
	6:WLI#4	3.000	2.38E+3	3.000		-2.4E+3	3.000	
	7:WLS#1	3.000	1.5E+3	0.000		-1.59E+3	0.000	
	8:WLS#4	3.000	1.77E+3	3.000		-1.78E+3	3.000	
	9:WLM#1	3.000	429.126	0.000		-455.150	0.000	
	10:WLM#4	3.000	505.488	3.000		-509.762	3.000	
	11:LM	3.000	53.498	0.000		-54.483	0.000	
	12:LV	3.000	8.471	0.000		-8.785	0.000	
	13:1.2D+1.6WI	3.000	4.16E+3	0.000		-4.6E+3	0.000	
	14:1.2D+1.6WI	3.000	4.69E+3	0.000		-4.78E+3	0.000	
	15:1.2D-1.6WL	3.000	10.8E+3	3.000		-10.4E+3	3.000	
	16:1.2D-1.6WL	3.000	11.9E+3	3.000		-11.8E+3	3.000	
	17:1.2D+1.0DI·	3.000	15.5E+3	3.000		-15.5E+3	3.000	
	18:1.2D+1.0DI·	3.000	15.2E+3	3.000		-15.1E+3	3.000	
	19:1.2D+1.0DI·	3.000	19.7E+3	3.000		-19.4E+3	3.000	
	20:1.2D+1.0DI·	3.000	20E+3	3.000		-19.8E+3	3.000	
	21:1.0D+1.0WI	3.000	1.68E+3	3.000		-1.77E+3	3.000	
	22:1.0D+1.0WI	3.000	1.38E+3	3.000		-1.4E+3	3.000	
	23:1.0D-1.0WL	3.000	4.69E+3	3.000		-4.6E+3	3.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	24:1.0D-1.0WL	3.000	4.94E+3	3.000		-4.93E+3	3.000	
	25:1.4D	3.000	4.42E+3	3.000		-4.43E+3	3.000	
	26:1.2D+1.5LM	3.000	3.28E+3	3.000		-3.31E+3	3.000	
	27:1.2D+1.5LM	3.000	3.21E+3	3.000		-3.22E+3	3.000	
	28:1.2D+1.5LM	3.000	4.15E+3	3.000		-4.13E+3	3.000	
	29:1.2D+1.5LM	3.000	4.23E+3	3.000		-4.23E+3	3.000	
	30:1.2D+1.5LV	3.000	3.8E+3	3.000		-3.8E+3	3.000	
195	1:DEAD	1.333	654.336	1.333		-668.014	1.333	
	2:WL#1	1.333	4.02E+3	1.333		-4.18E+3	1.333	
	3:WL#4	1.333	2.99E+3	1.333		-3.13E+3	1.333	
	4:DI	1.333	2.7E+3	1.333		-2.76E+3	1.333	
	5:WLI#1	1.333	1.78E+3	1.333		-1.86E+3	1.333	
	6:WLI#4	1.333	1.36E+3	1.333		-1.43E+3	1.333	
	7:WLS#1	1.333	1.41E+3	1.333		-1.47E+3	1.333	
	8:WLS#4	1.333	1.05E+3	1.333		-1.1E+3	1.333	
	9:WLM#1	1.333	402.512	1.333		-419.082	1.333	
	10:WLM#4	1.333	298.033	1.333		-312.116	1.333	
	11:LM	1.333	37.265	1.333		-36.794	1.333	
	12:LV	1.333	6.131	1.333		-6.254	1.333	
	13:1.2D+1.6WI	1.333	6.12E+3	1.333		-6.4E+3	1.333	
	14:1.2D+1.6WI	1.333	5.2E+3	1.333		-5.44E+3	1.333	
	15:1.2D-1.6WL	1.333	7.06E+3	1.333		-6.82E+3	1.333	
	16:1.2D-1.6WL	1.333	4.66E+3	1.333		-4.46E+3	1.333	
	17:1.2D+1.0DI·	1.333	3.33E+3	1.333		-3.49E+3	1.333	
	18:1.2D+1.0DI·	1.333	4.32E+3	1.333		-4.47E+3	1.333	
	19:1.2D+1.0DI·	1.333	4.45E+3	1.333		-4.45E+3	1.333	
	20:1.2D+1.0DI·	1.333	3.06E+3	1.333		-3.07E+3	1.333	
	21:1.0D+1.0WI	1.333	1.27E+3	1.333		-1.34E+3	1.333	
	22:1.0D+1.0WI	1.333	1.48E+3	1.333		-1.54E+3	1.333	
	23:1.0D-1.0WL	1.333	1.85E+3	1.333		-1.8E+3	1.333	
	24:1.0D-1.0WL	1.333	967.183	1.333		-931.439	1.333	
	25:1.4D	1.333	916.071	1.333		-935.219	1.333	
	26:1.2D+1.5LM	1.333	761.495	1.333		-793.771	1.333	
	27:1.2D+1.5LM	1.333	1.02E+3	1.333		-1.05E+3	1.333	
	28:1.2D+1.5LM	1.333	1.07E+3	1.333		-1.07E+3	1.333	
	29:1.2D+1.5LM	1.333	734.078	1.333		-735.702	1.333	
	30:1.2D+1.5LV	1.333	794.400	1.333		-810.998	1.333	
196	1:DEAD	5.667	2.81E+3	5.667		-2.98E+3	5.667	
	2:WL#1	5.667	6.87E+3	5.667		-6.76E+3	5.667	
	3:WL#4	5.667	986.100	4.250		-876.156	4.250	
	4:DI	5.667	12.2E+3	5.667		-12.9E+3	5.667	
	5:WLI#1	5.667	3.15E+3	5.667		-3.11E+3	5.667	
	6:WLI#4	5.667	429.783	3.306		-376.825	3.306	
	7:WLS#1	5.667	2.41E+3	5.667		-2.38E+3	5.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	8:WLS#4	5.667	347.178	4.250		-308.631	4.250	
	9:WLM#1	5.667	689.514	5.667		-678.967	5.667	
	10:WLM#4	5.667	97.402	4.250		-86.233	4.250	
	11:LM	5.667	23.210	0.000		-26.695	0.000	
	12:LV	5.667	36.372	5.667		-34.662	5.667	
	13:1.2D+1.6WI	5.667	14.3E+3	5.667		-14.3E+3	5.667	
	14:1.2D+1.6WI	5.667	3.5E+3	0.000		-3.56E+3	0.000	
	15:1.2D-1.6WL	5.667	7.68E+3	0.000		-7.98E+3	0.000	
	16:1.2D-1.6WL	5.667	3.93E+3	5.667		-4.32E+3	5.667	
	17:1.2D+1.0DI·	5.667	18.6E+3	5.667		-19.5E+3	5.667	
	18:1.2D+1.0DI·	5.667	15.4E+3	5.667		-16.3E+3	5.667	
	19:1.2D+1.0DI·	5.667	12.5E+3	5.667		-13.5E+3	5.667	
	20:1.2D+1.0DI·	5.667	15.7E+3	5.667		-16.7E+3	5.667	
	21:1.0D+1.0WI	5.667	5.19E+3	5.667		-5.32E+3	5.667	
	22:1.0D+1.0WI	5.667	2.75E+3	5.667		-2.88E+3	5.667	
	23:1.0D-1.0WL	5.667	790.137	0.000		-980.190	0.000	
	24:1.0D-1.0WL	5.667	2.9E+3	5.667		-3.11E+3	5.667	
	25:1.4D	5.667	3.94E+3	5.667		-4.17E+3	5.667	
	26:1.2D+1.5LM	5.667	4.05E+3	5.667		-4.25E+3	5.667	
	27:1.2D+1.5LM	5.667	3.36E+3	5.667		-3.55E+3	5.667	
	28:1.2D+1.5LM	5.667	2.72E+3	5.667		-2.93E+3	5.667	
	29:1.2D+1.5LM	5.667	3.4E+3	5.667		-3.62E+3	5.667	
	30:1.2D+1.5LV	5.667	3.35E+3	5.667		-3.55E+3	5.667	
197	1:DEAD	5.667	2.82E+3	0.000		-2.99E+3	0.000	
	2:WL#1	5.667	3.41E+3	5.667		-3.49E+3	5.667	
	3:WL#4	5.667	6.05E+3	0.000		-6.02E+3	0.000	
	4:DI	5.667	12.3E+3	0.000		-13E+3	0.000	
	5:WLI#1	5.667	1.53E+3	5.667		-1.57E+3	5.667	
	6:WLI#4	5.667	2.73E+3	0.000		-2.72E+3	0.000	
	7:WLS#1	5.667	1.2E+3	5.667		-1.23E+3	5.667	
	8:WLS#4	5.667	2.13E+3	0.000		-2.11E+3	0.000	
	9:WLM#1	5.667	340.851	5.667		-349.079	5.667	
	10:WLM#4	5.667	604.110	0.000		-601.319	0.000	
	11:LM	5.667	28.299	0.000		-29.433	0.000	
	12:LV	5.667	36.373	0.000		-34.663	0.000	
	13:1.2D+1.6WI	5.667	2.87E+3	5.667		-3.2E+3	5.667	
	14:1.2D+1.6WI	5.667	6.13E+3	0.000		-6.29E+3	0.000	
	15:1.2D-1.6WL	5.667	8.5E+3	0.000		-8.5E+3	0.000	
	16:1.2D-1.6WL	5.667	13E+3	0.000		-13.2E+3	0.000	
	17:1.2D+1.0DI·	5.667	14.3E+3	0.000		-15.3E+3	0.000	
	18:1.2D+1.0DI·	5.667	13E+3	0.000		-13.9E+3	0.000	
	19:1.2D+1.0DI·	5.667	17.1E+3	0.000		-17.9E+3	0.000	
	20:1.2D+1.0DI·	5.667	18.4E+3	0.000		-19.3E+3	0.000	
	21:1.0D+1.0WI	5.667	1.75E+3	0.000		-1.96E+3	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

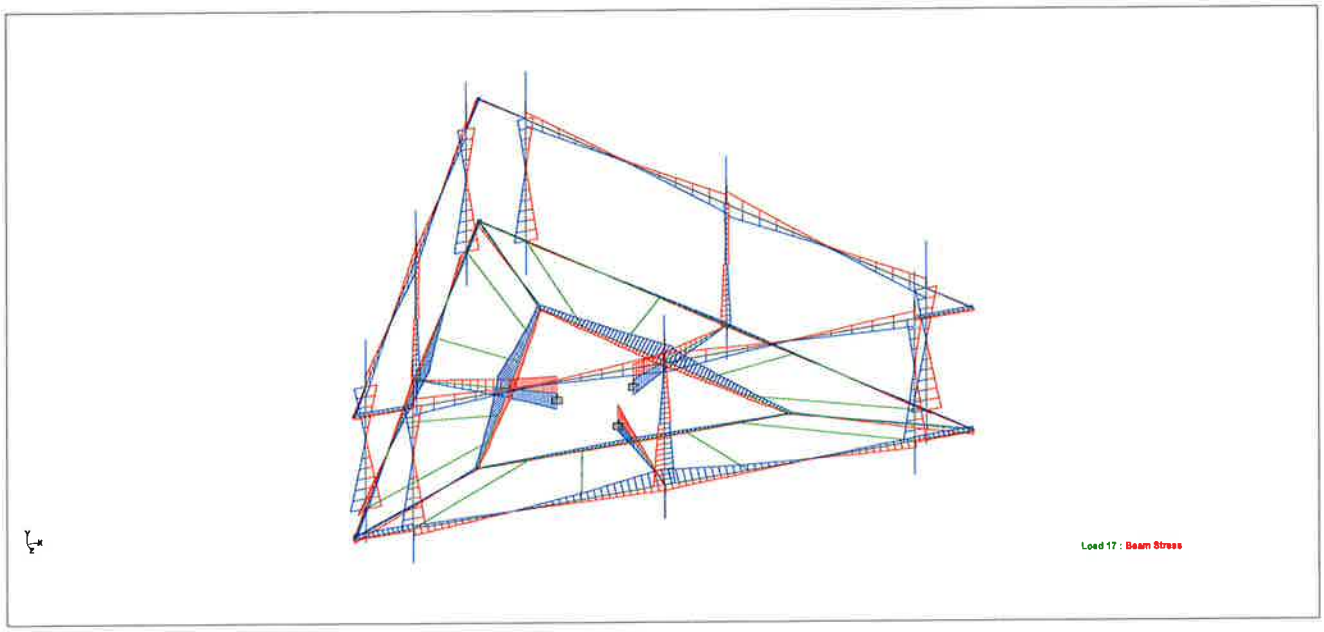
			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	22:1.0D+1.0WI	5.667	750.529	0.000		-907.356	0.000	
	23:1.0D-1.0WL	5.667	3.93E+3	0.000		-4.05E+3	0.000	
	24:1.0D-1.0WL	5.667	4.93E+3	0.000		-5.1E+3	0.000	
	25:1.4D	5.667	3.95E+3	0.000		-4.18E+3	0.000	
	26:1.2D+1.5LM	5.667	3.11E+3	0.000		-3.33E+3	0.000	
	27:1.2D+1.5LM	5.667	2.83E+3	0.000		-3.03E+3	0.000	
	28:1.2D+1.5LM	5.667	3.74E+3	0.000		-3.92E+3	0.000	
	29:1.2D+1.5LM	5.667	4.02E+3	0.000		-4.22E+3	0.000	
	30:1.2D+1.5LV	5.667	3.36E+3	0.000		-3.56E+3	0.000	
198	1:DEAD	5.667	2.75E+3	5.667		-2.91E+3	5.667	
	2:WL#1	5.667	3.15E+3	0.000		-3.25E+3	0.000	
	3:WL#4	5.667	5.99E+3	5.667		-6.02E+3	5.667	
	4:DI	5.667	11.2E+3	5.667		-11.9E+3	5.667	
	5:WLI#1	5.667	1.48E+3	0.000		-1.53E+3	0.000	
	6:WLI#4	5.667	2.71E+3	5.667		-2.72E+3	5.667	
	7:WLS#1	5.667	1.11E+3	0.000		-1.14E+3	0.000	
	8:WLS#4	5.667	2.1E+3	5.667		-2.11E+3	5.667	
	9:WLM#1	5.667	316.679	0.000		-326.048	0.000	
	10:WLM#4	5.667	597.947	5.667		-600.810	5.667	
	11:LM	5.667	28.295	5.667		-29.429	5.667	
	12:LV	5.667	11.606	0.000		-13.348	0.000	
	13:1.2D+1.6WI	5.667	2.72E+3	0.000		-3.06E+3	0.000	
	14:1.2D+1.6WI	5.667	12.9E+3	5.667		-13.1E+3	5.667	
	15:1.2D-1.6WL	5.667	8.05E+3	5.667		-8.03E+3	5.667	
	16:1.2D-1.6WL	5.667	6.17E+3	5.667		-6.32E+3	5.667	
	17:1.2D+1.0DI·	5.667	13.1E+3	5.667		-14.1E+3	5.667	
	18:1.2D+1.0DI·	5.667	17.2E+3	5.667		-18.1E+3	5.667	
	19:1.2D+1.0DI·	5.667	15.9E+3	5.667		-16.7E+3	5.667	
	20:1.2D+1.0DI·	5.667	11.8E+3	5.667		-12.7E+3	5.667	
	21:1.0D+1.0WI	5.667	1.77E+3	5.667		-1.98E+3	5.667	
	22:1.0D+1.0WI	5.667	4.84E+3	5.667		-5.02E+3	5.667	
	23:1.0D-1.0WL	5.667	3.78E+3	5.667		-3.89E+3	5.667	
	24:1.0D-1.0WL	5.667	696.397	5.667		-849.918	5.667	
	25:1.4D	5.667	3.85E+3	5.667		-4.08E+3	5.667	
	26:1.2D+1.5LM	5.667	3.05E+3	5.667		-3.26E+3	5.667	
	27:1.2D+1.5LM	5.667	3.93E+3	5.667		-4.13E+3	5.667	
	28:1.2D+1.5LM	5.667	3.63E+3	5.667		-3.81E+3	5.667	
	29:1.2D+1.5LM	5.667	2.75E+3	5.667		-2.94E+3	5.667	
	30:1.2D+1.5LV	5.667	3.3E+3	5.667		-3.5E+3	5.667	
199	1:DEAD	5.667	2.75E+3	0.000		-2.91E+3	0.000	
	2:WL#1	5.667	6.64E+3	0.000		-6.57E+3	0.000	
	3:WL#4	5.667	906.032	0.472		-1.01E+3	0.000	
	4:DI	5.667	11.2E+3	0.000		-11.9E+3	0.000	
	5:WLI#1	5.667	3.11E+3	0.000		-3.08E+3	0.000	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	6:WLI#4	5.667	349.012	1.889		-401.578	1.889	
	7:WLS#1	5.667	2.33E+3	0.000		-2.31E+3	0.000	
	8:WLS#4	5.667	320.370	0.000		-358.923	0.000	
	9:WLM#1	5.667	667.127	0.000		-660.270	0.000	
	10:WLM#4	5.667	86.935	0.472		-97.937	0.472	
	11:LM	5.667	23.211	5.667		-26.696	5.667	
	12:LV	5.667	14.150	0.000		-14.717	0.000	
	13:1.2D+1.6WI	5.667	13.8E+3	0.000		-13.9E+3	0.000	
	14:1.2D+1.6WI	5.667	4.07E+3	0.000		-4.44E+3	0.000	
	15:1.2D-1.6WL	5.667	7.31E+3	5.667		-7.54E+3	5.667	
	16:1.2D-1.6WL	5.667	3.14E+3	5.667		-3.2E+3	5.667	
	17:1.2D+1.0DI·	5.667	17.5E+3	0.000		-18.4E+3	0.000	
	18:1.2D+1.0DI·	5.667	14.7E+3	0.000		-15.6E+3	0.000	
	19:1.2D+1.0DI·	5.667	11.5E+3	0.000		-12.5E+3	0.000	
	20:1.2D+1.0DI·	5.667	14.4E+3	0.000		-15.2E+3	0.000	
	21:1.0D+1.0WI	5.667	5.04E+3	0.000		-5.18E+3	0.000	
	22:1.0D+1.0WI	5.667	2.88E+3	0.000		-3.09E+3	0.000	
	23:1.0D-1.0WL	5.667	787.383	5.667		-959.474	5.667	
	24:1.0D-1.0WL	5.667	2.65E+3	0.000		-2.78E+3	0.000	
	25:1.4D	5.667	3.85E+3	0.000		-4.08E+3	0.000	
	26:1.2D+1.5LM	5.667	3.95E+3	0.000		-4.15E+3	0.000	
	27:1.2D+1.5LM	5.667	3.34E+3	0.000		-3.55E+3	0.000	
	28:1.2D+1.5LM	5.667	2.66E+3	0.000		-2.87E+3	0.000	
	29:1.2D+1.5LM	5.667	3.27E+3	0.000		-3.46E+3	0.000	
	30:1.2D+1.5LV	5.667	3.32E+3	0.000		-3.52E+3	0.000	
200	1:DEAD	5.667	2.82E+3	5.667		-2.99E+3	5.667	
	2:WL#1	5.667	3.74E+3	5.667		-3.66E+3	5.667	
	3:WL#4	5.667	5.32E+3	5.667		-5.43E+3	5.667	
	4:DI	5.667	12.1E+3	5.667		-12.9E+3	5.667	
	5:WLI#1	5.667	1.73E+3	5.667		-1.69E+3	5.667	
	6:WLI#4	5.667	2.44E+3	5.667		-2.51E+3	5.667	
	7:WLS#1	5.667	1.31E+3	5.667		-1.29E+3	5.667	
	8:WLS#4	5.667	1.87E+3	5.667		-1.91E+3	5.667	
	9:WLM#1	5.667	375.103	5.667		-366.839	5.667	
	10:WLM#4	5.667	531.297	5.667		-543.409	5.667	
	11:LM	5.667	72.743	5.667		-69.323	5.667	
	12:LV	5.667	14.147	5.667		-14.714	5.667	
	13:1.2D+1.6WI	5.667	2.42E+3	5.667		-2.49E+3	5.667	
	14:1.2D+1.6WI	5.667	5.95E+3	0.000		-6.25E+3	0.000	
	15:1.2D-1.6WL	5.667	9.23E+3	5.667		-9.56E+3	5.667	
	16:1.2D-1.6WL	5.667	12E+3	5.667		-12E+3	5.667	
	17:1.2D+1.0DI·	5.667	13.8E+3	5.667		-14.7E+3	5.667	
	18:1.2D+1.0DI·	5.667	13.1E+3	5.667		-14.1E+3	5.667	
	19:1.2D+1.0DI·	5.667	17.2E+3	5.667		-18.2E+3	5.667	

Beam Combined Axial and Bending Stresses Summary Cont...

			Max Comp			Max Tens		
Beam	L/C	Length (ft)	Stress (psi)	d (ft)	Corner	Stress (psi)	d (ft)	Corner
	20:1.2D+1.0DI-	5.667	18E+3	5.667		-18.8E+3	5.667	
	21:1.0D+1.0WI	5.667	1.54E+3	5.667		-1.68E+3	5.667	
	22:1.0D+1.0WI	5.667	1.07E+3	5.667		-1.27E+3	5.667	
	23:1.0D-1.0WL	5.667	4.1E+3	5.667		-4.3E+3	5.667	
	24:1.0D-1.0WL	5.667	4.69E+3	5.667		-4.82E+3	5.667	
	25:1.4D	5.667	3.95E+3	5.667		-4.18E+3	5.667	
	26:1.2D+1.5LM	5.667	2.97E+3	5.667		-3.16E+3	5.667	
	27:1.2D+1.5LM	5.667	2.82E+3	5.667		-3.03E+3	5.667	
	28:1.2D+1.5LM	5.667	3.71E+3	5.667		-3.91E+3	5.667	
	29:1.2D+1.5LM	5.667	3.87E+3	5.667		-4.05E+3	5.667	
	30:1.2D+1.5LV	5.667	3.4E+3	5.667		-3.6E+3	5.667	
201	1:DEAD	5.667	2.81E+3	0.000		-2.98E+3	0.000	
	2:WL#1	5.667	3.88E+3	0.000		-3.81E+3	0.000	
	3:WL#4	5.667	5.47E+3	5.667		-5.33E+3	5.667	
	4:DI	5.667	11.4E+3	0.000		-12.2E+3	0.000	
	5:WLI#1	5.667	1.75E+3	0.000		-1.71E+3	0.000	
	6:WLI#4	5.667	2.5E+3	0.000		-2.45E+3	0.000	
	7:WLS#1	5.667	1.37E+3	0.000		-1.34E+3	0.000	
	8:WLS#4	5.667	1.92E+3	5.667		-1.87E+3	5.667	
	9:WLM#1	5.667	389.047	0.000		-381.635	0.000	
	10:WLM#4	5.667	547.272	5.667		-532.778	5.667	
	11:LM	5.667	72.742	0.000		-69.323	0.000	
	12:LV	5.667	11.605	5.667		-13.348	5.667	
	13:1.2D+1.6WI	5.667	2.66E+3	0.000		-2.74E+3	0.000	
	14:1.2D+1.6WI	5.667	12E+3	0.000		-12E+3	0.000	
	15:1.2D-1.6WL	5.667	9.47E+3	0.000		-9.79E+3	0.000	
	16:1.2D-1.6WL	5.667	6.04E+3	5.667		-6.48E+3	5.667	
	17:1.2D+1.0DI-	5.667	13.1E+3	0.000		-14E+3	0.000	
	18:1.2D+1.0DI-	5.667	17.2E+3	0.000		-18.1E+3	0.000	
	19:1.2D+1.0DI-	5.667	16.5E+3	0.000		-17.5E+3	0.000	
	20:1.2D+1.0DI-	5.667	12.4E+3	0.000		-13.4E+3	0.000	
	21:1.0D+1.0WI	5.667	1.48E+3	0.000		-1.62E+3	0.000	
	22:1.0D+1.0WI	5.667	4.68E+3	0.000		-4.8E+3	0.000	
	23:1.0D-1.0WL	5.667	4.15E+3	0.000		-4.34E+3	0.000	
	24:1.0D-1.0WL	5.667	1.07E+3	0.000		-1.29E+3	0.000	
	25:1.4D	5.667	3.94E+3	0.000		-4.17E+3	0.000	
	26:1.2D+1.5LM	5.667	2.95E+3	0.000		-3.14E+3	0.000	
	27:1.2D+1.5LM	5.667	3.86E+3	0.000		-4.04E+3	0.000	
	28:1.2D+1.5LM	5.667	3.71E+3	0.000		-3.92E+3	0.000	
	29:1.2D+1.5LM	5.667	2.81E+3	0.000		-3.03E+3	0.000	
	30:1.2D+1.5LV	5.667	3.38E+3	0.000		-3.58E+3	0.000	



Stresses

Utilization Ratio

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in^2)	Iz (in^4)	Iy (in^4)	Ix (in^4)
1	L30303 LD	L30303 LD	0.197	1.000	0.197	LRFD-H1-1B-	17	2.186	1.928	3.396	0.026
2	L30303 LD	L30303 LD	0.196	1.000	0.196	LRFD-H1-1B-	19	2.186	1.928	3.396	0.026
3	L30303 LD	L30303 LD	0.208	1.000	0.208	LRFD-H1-1B-	20	2.186	1.928	3.396	0.026
4	HSST4X4X0	HSST4X4X0	0.421	1.000	0.421	HSS T+SH+F	18	3.370	7.800	7.800	12.800
5	HSST4X4X0	HSST4X4X0	0.742	1.000	0.742	HSS T+SH+F	19	3.370	7.800	7.800	12.800
6	HSST4X4X0	HSST4X4X0	0.753	1.000	0.753	HSS T+SH+F	20	3.370	7.800	7.800	12.800
7	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
9	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
10	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
11	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
13	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
14	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
15	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
17	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
18	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
19	L30304	L30304	0.141	1.000	0.141	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
20	L30304	L30304	0.543	1.000	0.543	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
21	L30304	L30304	0.115	1.000	0.115	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
22	L30304	L30304	0.602	1.000	0.602	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
23	L30304	L30304	0.137	1.000	0.137	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
24	L30304	L30304	0.592	1.000	0.592	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
25	L30304	L30304	0.169	1.000	0.169	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
26	L30304	L30304	0.467	1.000	0.467	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
27	L30304	L30304	0.177	1.000	0.177	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030

Utilization Ratio Cont...

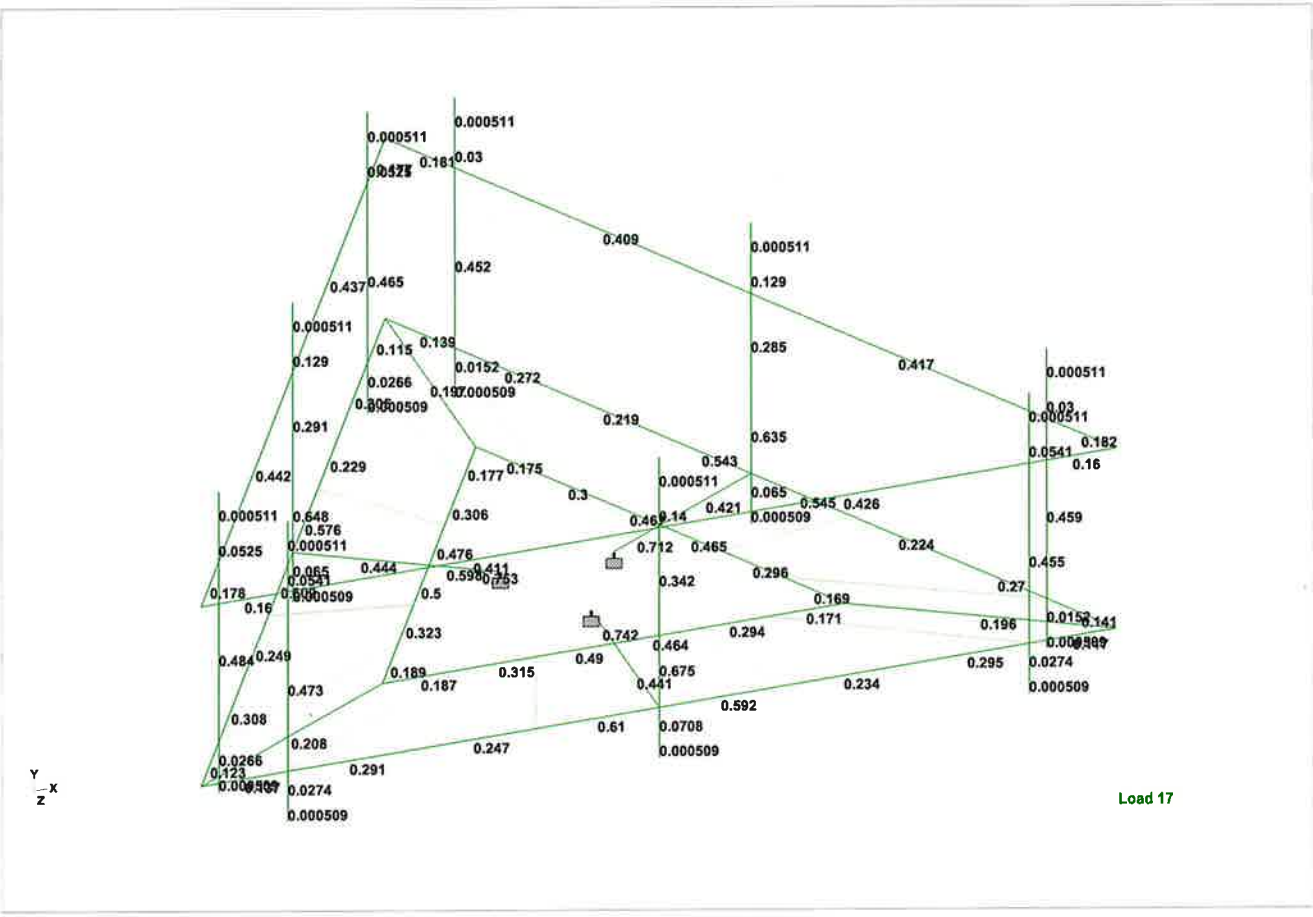
Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in^2)	Iz (in^4)	Iy (in^4)	Ix (in^4)
28	L30304	L30304	0.500	1.000	0.500	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
29	L30304	L30304	0.187	1.000	0.187	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
30	L30304	L30304	0.464	1.000	0.464	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
43	HSST4X4X0	HSST4X4X0	0.712	1.000	0.712	HSS T+SH+F	18	3.370	7.800	7.800	12.800
44	HSST4X4X0	HSST4X4X0	0.441	1.000	0.441	HSS T+SH+F	19	3.370	7.800	7.800	12.800
45	HSST4X4X0	HSST4X4X0	0.444	1.000	0.444	HSS T+SH+F	20	3.370	7.800	7.800	12.800
48	PIPS20	PIPS20	0.635	1.000	0.635	LRFD-H1-1B-	16	1.000	0.627	0.627	1.254
51	PIPS20	PIPS20	0.648	1.000	0.648	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
54	PIPS20	PIPS20	0.675	1.000	0.675	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
55	L30304	L30304	0.270	1.000	0.270	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
56	L30304	L30304	0.139	1.000	0.139	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
57	L30304	L30304	0.305	1.000	0.305	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
58	L30304	L30304	0.123	1.000	0.123	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
59	L30304	L30304	0.291	1.000	0.291	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
60	L30304	L30304	0.117	1.000	0.117	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
101	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
102	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
103	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
104	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
105	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
106	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
107	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
108	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
109	PIPS20	PIPS20	0.001	1.000	0.001	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
110	PIPS20	PIPS20	0.065	1.000	0.065	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
111	PIPS20	PIPS20	0.065	1.000	0.065	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
112	PIPS20	PIPS20	0.071	1.000	0.071	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
113	PIPS20	PIPS20	0.015	1.000	0.015	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
114	PIPS20	PIPS20	0.015	1.000	0.015	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
115	PIPS20	PIPS20	0.027	1.000	0.027	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
116	PIPS20	PIPS20	0.027	1.000	0.027	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
117	PIPS20	PIPS20	0.027	1.000	0.027	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
118	PIPS20	PIPS20	0.027	1.000	0.027	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
122	HSST4X4X0	HSST4X4X0	0.598	1.000	0.598	HSS T+SH+F	20	3.370	7.800	7.800	12.800
123	PIPS20	PIPS20	0.053	1.000	0.053	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
124	PIPS20	PIPS20	0.030	1.000	0.030	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
125	PIPS20	PIPS20	0.129	1.000	0.129	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
127	PIPS20	PIPS20	0.030	1.000	0.030	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
128	PIPS20	PIPS20	0.140	1.000	0.140	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
129	PIPS20	PIPS20	0.054	1.000	0.054	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
130	PIPS20	PIPS20	0.053	1.000	0.053	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
131	PIPS20	PIPS20	0.129	1.000	0.129	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
132	PIPS20	PIPS20	0.054	1.000	0.054	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
133	L30304	L30304	0.315	1.000	0.315	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
134	L30304	L30304	0.490	1.000	0.490	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030

Utilization Ratio Cont...

Beam	Analysis Property	Design Property	Actual Ratio	Allowable Ratio	Ratio (Act./Allow.)	Clause	L/C	Ax (in^2)	Iz (in^4)	Iy (in^4)	Ix (in^4)
135	L30304	L30304	0.247	1.000	0.247	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
136	L30304	L30304	0.610	1.000	0.610	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
137	L30304	L30304	0.234	1.000	0.234	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
138	L30304	L30304	0.295	1.000	0.295	LRFD-H1-1B-	19	1.438	0.504	1.985	0.030
139	L30304	L30304	0.294	1.000	0.294	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
140	L30304	L30304	0.171	1.000	0.171	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
141	L30304	L30304	0.249	1.000	0.249	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
142	L30304	L30304	0.308	1.000	0.308	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
143	L30304	L30304	0.323	1.000	0.323	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
144	L30304	L30304	0.189	1.000	0.189	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
145	L30304	L30304	0.229	1.000	0.229	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
146	L30304	L30304	0.576	1.000	0.576	LRFD-H1-1B-	20	1.438	0.504	1.985	0.030
147	L30304	L30304	0.306	1.000	0.306	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
148	L30304	L30304	0.476	1.000	0.476	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
149	L30304	L30304	0.300	1.000	0.300	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
150	L30304	L30304	0.175	1.000	0.175	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
151	L30304	L30304	0.219	1.000	0.219	LRFD-H1-1B-	17	1.438	0.504	1.985	0.030
152	L30304	L30304	0.272	1.000	0.272	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
153	L30304	L30304	0.296	1.000	0.296	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
154	L30304	L30304	0.465	1.000	0.465	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
155	L30304	L30304	0.224	1.000	0.224	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
156	L30304	L30304	0.545	1.000	0.545	LRFD-H1-1B-	18	1.438	0.504	1.985	0.030
175	PIPS20	PIPS20	0.459	1.000	0.459	LRFD-H1-1B-	17	1.000	0.627	0.627	1.254
176	PIPS20	PIPS20	0.182	1.000	0.182	LRFD-H1-1B-	15	1.000	0.627	0.627	1.254
177	PIPS20	PIPS20	0.285	1.000	0.285	LRFD-H1-1B-	16	1.000	0.627	0.627	1.254
179	PIPS20	PIPS20	0.465	1.000	0.465	LRFD-H1-1B-	20	1.000	0.627	0.627	1.254
180	PIPS20	PIPS20	0.177	1.000	0.177	LRFD-H1-1B-	16	1.000	0.627	0.627	1.254
181	PIPS20	PIPS20	0.291	1.000	0.291	LRFD-H1-1B-	14	1.000	0.627	0.627	1.254
183	PIPS20	PIPS20	0.473	1.000	0.473	LRFD-H1-1B-	18	1.000	0.627	0.627	1.254
184	PIPS20	PIPS20	0.160	1.000	0.160	LRFD-H1-1B-	15	1.000	0.627	0.627	1.254
185	PIPS20	PIPS20	0.342	1.000	0.342	LRFD-H1-1B-	13	1.000	0.627	0.627	1.254
188	PIPS20	PIPS20	0.452	1.000	0.452	LRFD-H1-1B-	18	1.000	0.627	0.627	1.254
189	PIPS20	PIPS20	0.181	1.000	0.181	LRFD-H1-1B-	16	1.000	0.627	0.627	1.254
191	PIPS20	PIPS20	0.484	1.000	0.484	LRFD-H1-1B-	17	1.000	0.627	0.627	1.254
192	PIPS20	PIPS20	0.178	1.000	0.178	LRFD-H1-1B-	15	1.000	0.627	0.627	1.254
194	PIPS20	PIPS20	0.455	1.000	0.455	LRFD-H1-1B-	20	1.000	0.627	0.627	1.254
195	PIPS20	PIPS20	0.160	1.000	0.160	LRFD-H1-1B-	15	1.000	0.627	0.627	1.254
196	PIPS20	PIPS20	0.442	1.000	0.442	LRFD-H1-1B-	17	1.000	0.627	0.627	1.254
197	PIPS20	PIPS20	0.437	1.000	0.437	LRFD-H1-1B-	20	1.000	0.627	0.627	1.254
198	PIPS20	PIPS20	0.409	1.000	0.409	LRFD-H1-1B-	18	1.000	0.627	0.627	1.254
199	PIPS20	PIPS20	0.417	1.000	0.417	LRFD-H1-1B-	17	1.000	0.627	0.627	1.254
200	PIPS20	PIPS20	0.426	1.000	0.426	LRFD-H1-1B-	20	1.000	0.627	0.627	1.254
201	PIPS20	PIPS20	0.411	1.000	0.411	LRFD-H1-1B-	18	1.000	0.627	0.627	1.254

Failed Members

There is no data of this type.



Utility Ratio

Node Displacements

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
1	21:1.0D+1.0WI	0.000	-0.074	-0.001	0.074	-0.003	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.040	0.000	0.040	-0.001	-0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.007	0.001	0.007	-0.000	0.000	-0.000
	24:1.0D-1.0WL	-0.001	-0.040	0.000	0.040	-0.001	0.000	-0.000
2	21:1.0D+1.0WI	0.000	-0.216	-0.001	0.216	-0.003	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.106	0.000	0.106	-0.001	-0.000	-0.001
	23:1.0D-1.0WL	-0.000	0.005	0.001	0.005	0.001	0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.105	0.000	0.105	-0.001	0.000	0.001
3	21:1.0D+1.0WI	-0.000	-0.024	-0.001	0.024	0.000	0.000	-0.001
	22:1.0D+1.0WI	0.001	-0.069	0.000	0.069	0.001	0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.057	0.000	0.057	0.001	-0.000	-0.002
	24:1.0D-1.0WL	-0.001	-0.012	-0.000	0.012	0.000	-0.000	-0.000
4	21:1.0D+1.0WI	-0.000	-0.050	-0.001	0.050	-0.001	0.000	-0.001
	22:1.0D+1.0WI	0.001	-0.199	-0.000	0.199	0.001	-0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.161	0.001	0.161	0.002	-0.000	-0.001
	24:1.0D-1.0WL	-0.001	-0.012	0.000	0.012	0.000	-0.000	0.001
5	21:1.0D+1.0WI	-0.000	-0.055	-0.001	0.055	-0.001	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.016	-0.000	0.016	0.000	0.000	-0.001
	23:1.0D-1.0WL	0.000	-0.166	0.001	0.166	0.002	0.000	0.001
	24:1.0D-1.0WL	-0.001	-0.205	-0.000	0.205	0.001	-0.000	0.003
6	21:1.0D+1.0WI	-0.000	-0.025	-0.001	0.025	0.000	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.013	-0.000	0.013	0.000	0.000	0.000
	23:1.0D-1.0WL	0.000	-0.059	0.001	0.059	0.001	0.000	0.002
	24:1.0D-1.0WL	-0.001	-0.071	0.000	0.071	0.001	-0.000	0.002
7	21:1.0D+1.0WI	-0.000	-0.037	-0.000	0.037	-0.001	0.000	-0.001
	22:1.0D+1.0WI	0.000	-0.043	0.000	0.043	-0.001	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.021	0.001	0.021	0.000	-0.000	-0.001
	24:1.0D-1.0WL	-0.000	-0.015	-0.000	0.015	-0.001	0.000	-0.000
8	21:1.0D+1.0WI	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	22:1.0D+1.0WI	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	23:1.0D-1.0WL	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	24:1.0D-1.0WL	0.000	0.000	0.000	0.000	0.000	0.000	0.000
9	21:1.0D+1.0WI	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	22:1.0D+1.0WI	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	23:1.0D-1.0WL	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	24:1.0D-1.0WL	0.000	0.000	0.000	0.000	0.000	0.000	0.000
10	21:1.0D+1.0WI	-0.000	-0.013	-0.000	0.013	0.000	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.030	-0.000	0.030	0.001	0.000	-0.001
	23:1.0D-1.0WL	0.000	-0.046	0.000	0.046	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.030	-0.000	0.030	0.001	-0.000	0.001
11	21:1.0D+1.0WI	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	22:1.0D+1.0WI	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	23:1.0D-1.0WL	0.000	0.000	0.000	0.000	0.000	0.000	0.000
	24:1.0D-1.0WL	0.000	0.000	0.000	0.000	0.000	0.000	0.000
12	21:1.0D+1.0WI	0.000	-0.038	-0.001	0.038	-0.001	-0.000	0.001

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
	22:1.0D+1.0WI	0.000	-0.015	-0.000	0.015	-0.001	-0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.022	0.001	0.022	0.000	0.000	0.001
	24:1.0D-1.0WL	-0.000	-0.044	0.000	0.044	-0.001	0.000	0.002
13	21:1.0D+1.0WI	-0.000	-0.056	-0.001	0.056	-0.001	-0.000	-0.001
	22:1.0D+1.0WI	0.001	-0.170	-0.000	0.170	0.001	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.127	0.001	0.127	0.002	-0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.013	-0.000	0.014	0.001	-0.000	0.002
14	21:1.0D+1.0WI	-0.000	-0.006	-0.000	0.006	-0.001	0.000	-0.001
	22:1.0D+1.0WI	0.000	-0.007	0.000	0.007	-0.000	-0.000	-0.001
	23:1.0D-1.0WL	0.000	-0.004	0.000	0.004	-0.000	-0.000	-0.001
	24:1.0D-1.0WL	-0.000	-0.003	-0.000	0.003	-0.000	0.000	-0.000
15	21:1.0D+1.0WI	-0.000	-0.003	-0.000	0.003	0.000	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.005	-0.000	0.005	0.001	0.000	-0.000
	23:1.0D-1.0WL	0.000	-0.007	0.000	0.007	0.001	0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.005	-0.000	0.005	0.001	-0.000	0.000
16	21:1.0D+1.0WI	0.000	-0.006	-0.001	0.006	-0.001	-0.000	0.001
	22:1.0D+1.0WI	0.000	-0.003	-0.000	0.003	-0.000	-0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.004	0.001	0.004	-0.000	0.000	0.001
	24:1.0D-1.0WL	-0.000	-0.007	0.000	0.007	-0.000	0.000	0.001
17	21:1.0D+1.0WI	0.015	-0.056	-0.095	0.111	-0.002	-0.001	-0.001
	22:1.0D+1.0WI	0.108	-0.170	-0.001	0.201	0.000	-0.001	-0.002
	23:1.0D-1.0WL	-0.008	-0.127	0.098	0.161	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.101	-0.013	0.005	0.102	0.000	0.000	0.002
18	21:1.0D+1.0WI	-0.008	-0.056	0.010	0.058	-0.001	-0.000	-0.001
	22:1.0D+1.0WI	-0.023	-0.170	-0.006	0.171	0.001	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.127	-0.022	0.129	0.002	-0.000	0.000
	24:1.0D-1.0WL	0.015	-0.013	-0.006	0.021	0.001	-0.000	0.002
22	21:1.0D+1.0WI	0.000	-0.178	-0.001	0.178	-0.002	0.000	0.000
	22:1.0D+1.0WI	0.001	-0.101	0.000	0.101	-0.001	0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.005	0.001	0.005	0.001	0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.082	-0.000	0.082	-0.000	0.000	0.002
23	21:1.0D+1.0WI	0.003	-0.178	0.024	0.180	-0.002	0.000	0.000
	22:1.0D+1.0WI	-0.015	-0.101	0.011	0.103	-0.001	0.000	-0.002
	23:1.0D-1.0WL	0.004	-0.005	-0.011	0.012	0.001	0.000	0.000
	24:1.0D-1.0WL	0.022	-0.082	0.002	0.085	-0.000	0.000	0.002
24	21:1.0D+1.0WI	0.005	-0.178	-0.101	0.205	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.108	-0.101	-0.017	0.149	-0.001	0.001	-0.002
	23:1.0D-1.0WL	-0.005	-0.005	0.093	0.094	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.107	-0.082	0.009	0.135	0.000	-0.000	0.002
25	21:1.0D+1.0WI	-0.010	-0.037	0.012	0.040	-0.001	0.000	-0.001
	22:1.0D+1.0WI	-0.017	-0.043	0.006	0.047	-0.001	-0.000	-0.002
	23:1.0D-1.0WL	-0.009	-0.021	-0.001	0.023	0.000	-0.000	-0.001
	24:1.0D-1.0WL	-0.003	-0.015	0.005	0.016	-0.001	0.000	-0.000
26	21:1.0D+1.0WI	0.045	-0.037	-0.120	0.134	-0.002	0.000	-0.001
	22:1.0D+1.0WI	0.165	-0.043	-0.040	0.175	-0.001	-0.000	-0.004

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
	23:1.0D-1.0WL	-0.000	-0.021	0.095	0.097	0.002	-0.000	0.001
	24:1.0D-1.0WL	-0.120	-0.015	0.014	0.122	0.001	0.000	0.003
27	21:1.0D+1.0WI	0.000	-0.178	-0.001	0.178	-0.002	-0.000	-0.000
	22:1.0D+1.0WI	0.001	-0.083	-0.000	0.083	-0.000	-0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.006	0.001	0.006	0.001	-0.000	-0.000
	24:1.0D-1.0WL	-0.001	-0.101	0.000	0.101	-0.001	-0.000	0.002
28	21:1.0D+1.0WI	-0.005	-0.178	-0.103	0.206	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.107	-0.083	0.009	0.135	0.000	0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.006	0.095	0.095	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.112	-0.101	-0.017	0.152	-0.001	-0.001	0.002
29	21:1.0D+1.0WI	-0.002	-0.178	0.023	0.180	-0.002	-0.000	-0.000
	22:1.0D+1.0WI	-0.021	-0.083	0.002	0.085	-0.000	-0.000	-0.002
	23:1.0D-1.0WL	-0.004	-0.006	-0.010	0.012	0.001	-0.000	-0.000
	24:1.0D-1.0WL	0.015	-0.101	0.011	0.103	-0.001	-0.000	0.002
33	21:1.0D+1.0WI	-0.000	-0.061	-0.001	0.061	-0.001	0.000	0.001
	22:1.0D+1.0WI	0.001	-0.017	-0.000	0.017	0.001	0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.131	0.001	0.131	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.174	-0.000	0.174	0.001	0.000	0.002
34	21:1.0D+1.0WI	0.009	-0.061	0.009	0.062	-0.001	0.000	0.001
	22:1.0D+1.0WI	-0.014	-0.017	-0.006	0.023	0.001	0.000	-0.002
	23:1.0D-1.0WL	0.001	-0.131	-0.022	0.133	0.002	0.000	0.000
	24:1.0D-1.0WL	0.024	-0.174	-0.007	0.176	0.001	0.000	0.002
35	21:1.0D+1.0WI	-0.019	-0.061	-0.099	0.117	-0.002	0.001	0.001
	22:1.0D+1.0WI	0.102	-0.017	0.006	0.104	0.000	-0.000	-0.002
	23:1.0D-1.0WL	0.007	-0.131	0.103	0.167	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.114	-0.175	-0.002	0.209	0.000	0.001	0.003
36	21:1.0D+1.0WI	0.010	-0.038	0.012	0.041	-0.001	-0.000	0.001
	22:1.0D+1.0WI	0.003	-0.015	0.005	0.017	-0.001	-0.000	0.000
	23:1.0D-1.0WL	0.009	-0.022	-0.001	0.024	0.000	0.000	0.001
	24:1.0D-1.0WL	0.017	-0.044	0.006	0.047	-0.001	0.000	0.002
37	21:1.0D+1.0WI	-0.048	-0.038	-0.123	0.137	-0.002	-0.000	0.001
	22:1.0D+1.0WI	0.121	-0.016	0.016	0.123	0.001	-0.000	-0.003
	23:1.0D-1.0WL	-0.001	-0.022	0.098	0.100	0.002	0.000	-0.001
	24:1.0D-1.0WL	-0.170	-0.044	-0.041	0.180	-0.001	0.000	0.004
38	21:1.0D+1.0WI	-0.000	-0.044	-0.001	0.044	-0.002	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.026	-0.000	0.026	0.000	-0.000	-0.001
	23:1.0D-1.0WL	0.000	-0.148	0.001	0.148	0.002	-0.000	0.001
	24:1.0D-1.0WL	-0.001	-0.165	0.000	0.165	-0.000	-0.000	0.002
39	21:1.0D+1.0WI	-0.001	-0.044	-0.112	0.120	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.087	-0.026	0.008	0.091	0.000	-0.000	-0.001
	23:1.0D-1.0WL	-0.008	-0.148	0.120	0.191	0.003	-0.001	0.001
	24:1.0D-1.0WL	-0.097	-0.165	0.000	0.191	0.000	-0.000	0.002
40	21:1.0D+1.0WI	0.005	-0.044	0.019	0.048	-0.002	-0.000	0.001
	22:1.0D+1.0WI	-0.007	-0.026	-0.002	0.027	0.000	-0.000	-0.001
	23:1.0D-1.0WL	0.011	-0.148	-0.021	0.150	0.002	-0.000	0.001

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
	24:1.0D-1.0WL	0.022	-0.165	0.001	0.167	-0.000	-0.000	0.002
44	21:1.0D+1.0WI	-0.000	-0.041	-0.001	0.041	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.001	-0.161	0.000	0.161	-0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.143	0.001	0.143	0.002	0.000	-0.001
	24:1.0D-1.0WL	-0.001	-0.023	-0.000	0.023	0.000	0.000	0.001
45	21:1.0D+1.0WI	-0.004	-0.041	0.019	0.045	-0.002	0.000	-0.000
	22:1.0D+1.0WI	-0.022	-0.161	0.000	0.163	-0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.010	-0.143	-0.021	0.145	0.002	0.000	-0.001
	24:1.0D-1.0WL	0.007	-0.023	-0.002	0.024	0.000	0.000	0.001
46	21:1.0D+1.0WI	-0.003	-0.041	-0.110	0.117	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.094	-0.161	0.001	0.186	0.000	0.000	-0.002
	23:1.0D-1.0WL	0.007	-0.143	0.118	0.186	0.002	0.001	-0.001
	24:1.0D-1.0WL	-0.090	-0.023	0.007	0.093	0.000	0.000	0.001
47	21:1.0D+1.0WI	-0.000	-0.013	-0.004	0.014	0.000	-0.000	0.000
	22:1.0D+1.0WI	-0.006	-0.030	-0.011	0.032	0.001	0.000	-0.001
	23:1.0D-1.0WL	0.000	-0.046	-0.019	0.050	0.002	0.000	0.000
	24:1.0D-1.0WL	0.006	-0.030	-0.011	0.032	0.001	-0.000	0.001
48	21:1.0D+1.0WI	-0.002	-0.013	-0.137	0.137	-0.004	0.000	0.000
	22:1.0D+1.0WI	0.091	-0.030	0.028	0.100	0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.001	-0.046	0.191	0.197	0.004	-0.000	0.000
	24:1.0D-1.0WL	-0.094	-0.030	0.026	0.102	0.000	-0.000	0.002
73	21:1.0D+1.0WI	0.012	-0.056	-0.086	0.103	-0.002	-0.001	-0.001
	22:1.0D+1.0WI	0.094	-0.170	-0.002	0.194	0.000	-0.001	-0.002
	23:1.0D-1.0WL	-0.007	-0.127	0.087	0.154	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.089	-0.013	0.004	0.090	0.000	0.000	0.002
74	21:1.0D+1.0WI	0.004	-0.178	-0.088	0.199	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.096	-0.101	-0.012	0.140	-0.001	0.001	-0.002
	23:1.0D-1.0WL	-0.003	-0.005	0.083	0.083	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.095	-0.082	0.007	0.126	0.000	-0.000	0.002
75	21:1.0D+1.0WI	0.038	-0.037	-0.106	0.118	-0.002	0.000	-0.001
	22:1.0D+1.0WI	0.142	-0.043	-0.034	0.153	-0.001	-0.000	-0.004
	23:1.0D-1.0WL	0.003	-0.021	0.082	0.085	0.002	-0.000	0.001
	24:1.0D-1.0WL	-0.101	-0.015	0.010	0.103	0.001	0.000	0.003
76	21:1.0D+1.0WI	-0.004	-0.178	-0.090	0.200	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.094	-0.083	0.007	0.126	0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.001	-0.006	0.085	0.085	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.099	-0.101	-0.012	0.142	-0.001	-0.001	0.002
77	21:1.0D+1.0WI	-0.016	-0.061	-0.089	0.109	-0.002	0.001	0.001
	22:1.0D+1.0WI	0.089	-0.017	0.004	0.091	0.000	-0.000	-0.002
	23:1.0D-1.0WL	0.006	-0.131	0.091	0.160	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.099	-0.175	-0.002	0.201	0.000	0.001	0.003
78	21:1.0D+1.0WI	-0.041	-0.038	-0.108	0.121	-0.002	-0.000	0.001
	22:1.0D+1.0WI	0.102	-0.016	0.011	0.103	0.001	-0.000	-0.003
	23:1.0D-1.0WL	-0.004	-0.022	0.085	0.088	0.002	0.000	-0.001
	24:1.0D-1.0WL	-0.147	-0.044	-0.034	0.157	-0.001	0.000	0.004

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
79	21:1.0D+1.0WI	-0.001	-0.044	-0.098	0.107	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.079	-0.026	0.007	0.083	0.000	-0.000	-0.001
	23:1.0D-1.0WL	-0.005	-0.148	0.105	0.182	0.003	-0.001	0.001
	24:1.0D-1.0WL	-0.085	-0.165	-0.000	0.186	0.000	-0.000	0.002
80	21:1.0D+1.0WI	-0.002	-0.041	-0.096	0.104	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.083	-0.161	0.001	0.181	0.000	0.000	-0.002
	23:1.0D-1.0WL	0.004	-0.143	0.103	0.177	0.002	0.001	-0.001
	24:1.0D-1.0WL	-0.081	-0.023	0.006	0.084	0.000	0.000	0.001
81	21:1.0D+1.0WI	-0.002	-0.013	-0.114	0.114	-0.004	0.000	0.000
	22:1.0D+1.0WI	0.081	-0.030	0.026	0.090	0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.001	-0.046	0.164	0.171	0.004	-0.000	0.000
	24:1.0D-1.0WL	-0.083	-0.030	0.025	0.092	0.000	-0.000	0.002
82	21:1.0D+1.0WI	-0.004	-0.037	0.005	0.037	-0.001	0.000	-0.001
	22:1.0D+1.0WI	-0.006	-0.043	0.003	0.044	-0.001	-0.000	-0.002
	23:1.0D-1.0WL	-0.004	-0.021	-0.000	0.022	0.000	-0.000	-0.001
	24:1.0D-1.0WL	-0.001	-0.015	0.002	0.015	-0.001	0.000	-0.000
83	21:1.0D+1.0WI	0.004	-0.038	0.004	0.038	-0.001	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.015	0.002	0.016	-0.001	-0.000	0.000
	23:1.0D-1.0WL	0.004	-0.022	0.000	0.022	0.000	0.000	0.001
	24:1.0D-1.0WL	0.007	-0.044	0.003	0.044	-0.001	0.000	0.002
84	21:1.0D+1.0WI	-0.000	-0.013	-0.002	0.013	0.000	-0.000	0.000
	22:1.0D+1.0WI	-0.002	-0.030	-0.005	0.030	0.001	0.000	-0.001
	23:1.0D-1.0WL	0.000	-0.046	-0.007	0.047	0.002	0.000	0.000
	24:1.0D-1.0WL	0.002	-0.030	-0.005	0.030	0.001	-0.000	0.001
85	21:1.0D+1.0WI	-0.003	-0.056	0.003	0.056	-0.001	-0.000	-0.001
	22:1.0D+1.0WI	-0.009	-0.170	-0.003	0.170	0.001	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.127	-0.008	0.127	0.002	-0.000	0.000
	24:1.0D-1.0WL	0.006	-0.013	-0.002	0.015	0.001	-0.000	0.002
86	21:1.0D+1.0WI	0.001	-0.178	0.009	0.178	-0.002	0.000	0.000
	22:1.0D+1.0WI	-0.005	-0.101	0.005	0.101	-0.001	0.000	-0.002
	23:1.0D-1.0WL	0.001	-0.005	-0.004	0.007	0.001	0.000	0.000
	24:1.0D-1.0WL	0.008	-0.082	0.001	0.082	-0.000	0.000	0.002
87	21:1.0D+1.0WI	-0.001	-0.178	0.009	0.179	-0.002	-0.000	-0.000
	22:1.0D+1.0WI	-0.008	-0.083	0.001	0.083	-0.000	-0.000	-0.002
	23:1.0D-1.0WL	-0.002	-0.006	-0.004	0.007	0.001	-0.000	-0.000
	24:1.0D-1.0WL	0.005	-0.101	0.005	0.101	-0.001	-0.000	0.002
88	21:1.0D+1.0WI	0.003	-0.061	0.003	0.061	-0.001	0.000	0.001
	22:1.0D+1.0WI	-0.005	-0.017	-0.003	0.018	0.001	0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.131	-0.008	0.131	0.002	0.000	0.000
	24:1.0D-1.0WL	0.009	-0.174	-0.003	0.175	0.001	0.000	0.002
89	21:1.0D+1.0WI	0.002	-0.044	0.007	0.045	-0.002	-0.000	0.001
	22:1.0D+1.0WI	-0.002	-0.026	-0.001	0.027	0.000	-0.000	-0.001
	23:1.0D-1.0WL	0.005	-0.148	-0.008	0.148	0.002	-0.000	0.001
	24:1.0D-1.0WL	0.009	-0.165	0.000	0.165	-0.000	-0.000	0.002
90	21:1.0D+1.0WI	-0.002	-0.041	0.007	0.041	-0.002	0.000	-0.000

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
	22:1.0D+1.0WI	-0.008	-0.161	0.000	0.161	-0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.004	-0.143	-0.008	0.144	0.002	0.000	-0.001
	24:1.0D-1.0WL	0.002	-0.023	-0.001	0.023	0.000	0.000	0.001
91	21:1.0D+1.0WI	0.014	-0.037	-0.039	0.056	-0.003	0.000	-0.001
	22:1.0D+1.0WI	0.048	-0.043	-0.010	0.066	-0.001	-0.000	-0.003
	23:1.0D-1.0WL	0.011	-0.021	0.024	0.034	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.023	-0.015	-0.004	0.028	0.000	0.000	0.002
92	21:1.0D+1.0WI	-0.015	-0.038	-0.040	0.057	-0.003	-0.000	0.001
	22:1.0D+1.0WI	0.024	-0.016	-0.004	0.029	0.000	-0.000	-0.002
	23:1.0D-1.0WL	-0.011	-0.022	0.026	0.036	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.050	-0.044	-0.010	0.067	-0.001	0.000	0.003
93	21:1.0D+1.0WI	-0.001	-0.013	-0.024	0.027	-0.003	0.000	0.000
	22:1.0D+1.0WI	0.030	-0.030	0.015	0.045	0.001	0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.046	0.053	0.071	0.004	-0.000	0.000
	24:1.0D-1.0WL	-0.031	-0.030	0.015	0.045	0.001	-0.000	0.002
94	21:1.0D+1.0WI	0.000	-0.002	-0.000	0.002	-0.000	-0.000	0.000
	22:1.0D+1.0WI	0.000	-0.001	-0.000	0.001	-0.000	-0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.001	0.000	0.001	-0.000	0.000	0.000
	24:1.0D-1.0WL	-0.000	-0.002	0.000	0.002	-0.000	0.000	0.001
95	21:1.0D+1.0WI	-0.000	-0.015	-0.001	0.015	0.000	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.009	-0.000	0.009	0.000	-0.000	0.000
	23:1.0D-1.0WL	0.000	-0.034	0.000	0.034	0.001	-0.000	0.002
	24:1.0D-1.0WL	-0.001	-0.041	0.000	0.041	0.001	-0.000	0.002
96	21:1.0D+1.0WI	-0.000	-0.007	-0.000	0.007	0.000	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.005	-0.000	0.005	0.001	-0.000	0.000
	23:1.0D-1.0WL	0.000	-0.015	0.000	0.015	0.001	-0.000	0.001
	24:1.0D-1.0WL	-0.001	-0.017	-0.000	0.017	0.001	-0.000	0.001
97	21:1.0D+1.0WI	-0.000	-0.032	-0.001	0.032	-0.001	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.032	0.000	0.032	0.001	0.000	0.000
	23:1.0D-1.0WL	0.000	-0.112	0.001	0.112	0.002	0.000	0.002
	24:1.0D-1.0WL	-0.001	-0.111	0.000	0.111	0.000	0.000	0.002
98	21:1.0D+1.0WI	-0.000	-0.020	-0.000	0.020	-0.000	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.027	-0.000	0.027	0.001	-0.000	0.000
	23:1.0D-1.0WL	0.000	-0.069	0.000	0.069	0.002	-0.000	0.002
	24:1.0D-1.0WL	-0.001	-0.062	0.000	0.062	0.001	-0.000	0.002
99	21:1.0D+1.0WI	-0.000	-0.019	-0.000	0.019	-0.000	0.000	-0.000
	22:1.0D+1.0WI	0.001	-0.061	-0.000	0.061	0.001	0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.067	0.000	0.067	0.002	0.000	-0.002
	24:1.0D-1.0WL	-0.001	-0.026	-0.000	0.026	0.001	0.000	-0.000
100	21:1.0D+1.0WI	-0.000	-0.030	-0.001	0.030	-0.001	0.000	-0.000
	22:1.0D+1.0WI	0.001	-0.109	0.000	0.109	0.000	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.109	0.001	0.109	0.002	-0.000	-0.002
	24:1.0D-1.0WL	-0.001	-0.030	0.000	0.030	0.001	-0.000	-0.000
101	21:1.0D+1.0WI	-0.000	-0.006	-0.000	0.006	0.000	0.000	-0.000
	22:1.0D+1.0WI	0.001	-0.017	-0.000	0.017	0.001	0.000	-0.001

Node Displacements Cont...

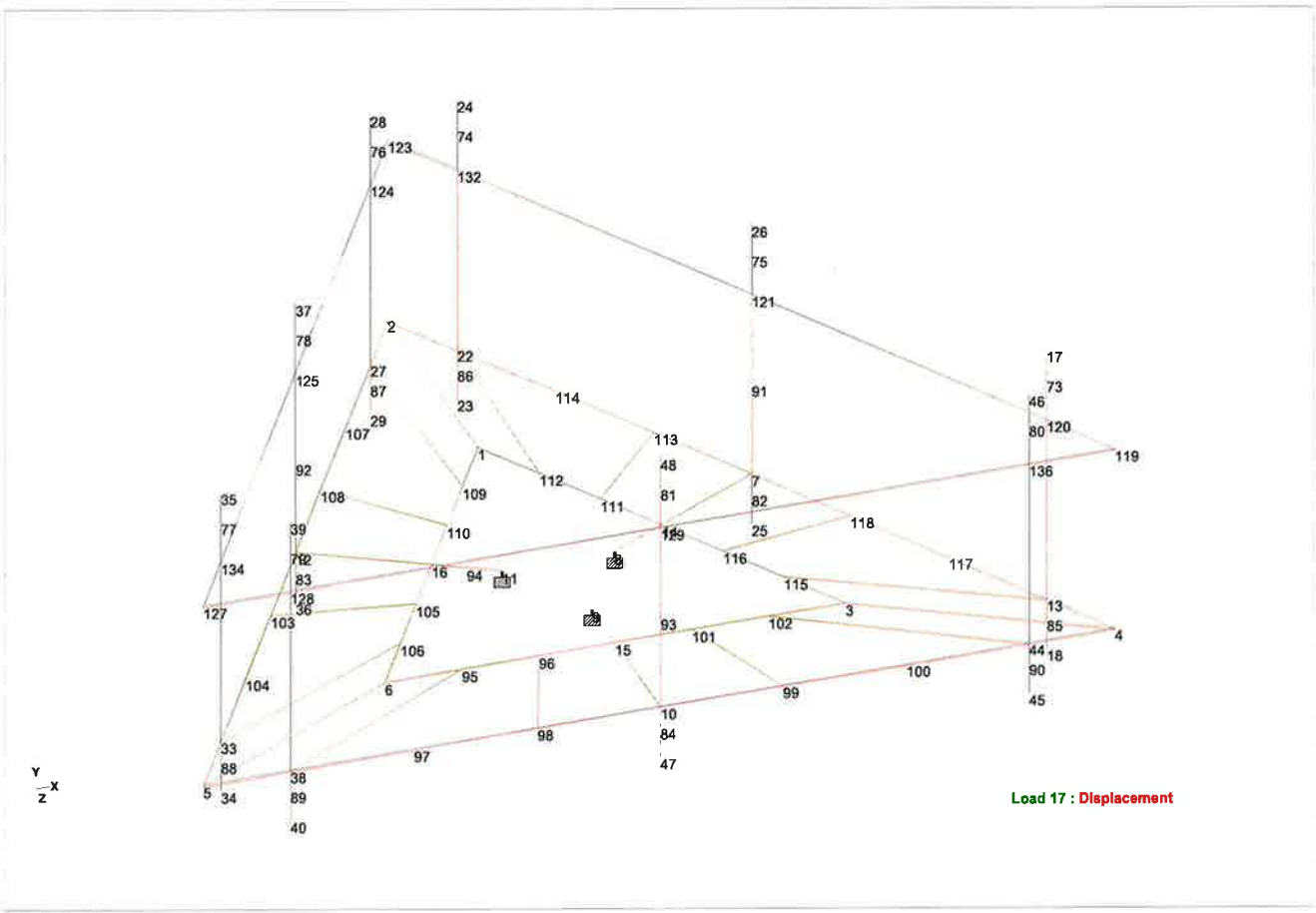
Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
	23:1.0D-1.0WL	0.000	-0.015	0.000	0.015	0.001	0.000	-0.001
	24:1.0D-1.0WL	-0.001	-0.004	-0.000	0.004	0.001	0.000	-0.000
102	21:1.0D+1.0WI	-0.000	-0.014	-0.000	0.014	0.000	0.000	-0.001
	22:1.0D+1.0WI	0.001	-0.040	0.000	0.040	0.001	0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.033	0.000	0.033	0.001	0.000	-0.002
	24:1.0D-1.0WL	-0.001	-0.008	-0.000	0.008	0.000	0.000	-0.000
103	21:1.0D+1.0WI	0.000	-0.041	-0.001	0.041	-0.000	0.000	0.002
	22:1.0D+1.0WI	0.000	-0.015	-0.000	0.015	0.000	0.000	-0.000
	23:1.0D-1.0WL	-0.000	-0.048	0.001	0.048	0.001	0.000	0.001
	24:1.0D-1.0WL	-0.000	-0.074	0.000	0.074	0.001	0.000	0.003
104	21:1.0D+1.0WI	-0.000	-0.057	-0.001	0.057	-0.000	-0.000	0.001
	22:1.0D+1.0WI	0.001	-0.018	-0.000	0.018	0.001	0.000	-0.001
	23:1.0D-1.0WL	-0.000	-0.087	0.001	0.087	0.002	0.000	0.001
	24:1.0D-1.0WL	-0.001	-0.125	-0.000	0.125	0.001	-0.000	0.003
105	21:1.0D+1.0WI	0.000	-0.008	-0.001	0.008	-0.000	0.000	0.001
	22:1.0D+1.0WI	0.000	-0.004	-0.000	0.004	0.000	0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.015	0.001	0.015	0.001	0.000	0.001
	24:1.0D-1.0WL	-0.000	-0.018	0.000	0.018	0.000	0.000	0.002
106	21:1.0D+1.0WI	0.000	-0.015	-0.001	0.015	0.000	0.000	0.001
	22:1.0D+1.0WI	0.001	-0.009	-0.000	0.009	0.000	0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.034	0.001	0.034	0.001	0.000	0.002
	24:1.0D-1.0WL	-0.000	-0.041	0.000	0.041	0.001	0.000	0.002
107	21:1.0D+1.0WI	0.000	-0.124	-0.001	0.124	-0.003	0.000	-0.000
	22:1.0D+1.0WI	0.001	-0.055	-0.000	0.055	-0.001	-0.000	-0.001
	23:1.0D-1.0WL	-0.000	-0.015	0.001	0.015	0.000	0.000	-0.000
	24:1.0D-1.0WL	-0.001	-0.084	0.000	0.084	-0.002	0.000	0.001
108	21:1.0D+1.0WI	0.000	-0.070	-0.001	0.070	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.031	-0.000	0.031	-0.001	-0.000	-0.001
	23:1.0D-1.0WL	-0.000	-0.016	0.001	0.016	-0.000	-0.000	0.000
	24:1.0D-1.0WL	-0.000	-0.055	0.000	0.055	-0.002	-0.000	0.001
109	21:1.0D+1.0WI	0.000	-0.042	-0.001	0.042	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.024	-0.000	0.024	-0.001	-0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.005	0.001	0.005	-0.000	-0.000	0.000
	24:1.0D-1.0WL	-0.000	-0.024	0.000	0.024	-0.001	-0.000	0.000
110	21:1.0D+1.0WI	0.000	-0.018	-0.001	0.018	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.000	-0.010	-0.000	0.010	-0.001	-0.000	0.000
	23:1.0D-1.0WL	-0.000	-0.003	0.001	0.003	-0.000	-0.000	0.000
	24:1.0D-1.0WL	-0.000	-0.011	0.000	0.011	-0.001	-0.000	0.000
111	21:1.0D+1.0WI	-0.000	-0.018	-0.001	0.018	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.000	-0.011	0.000	0.011	-0.001	0.000	-0.000
	23:1.0D-1.0WL	0.000	-0.003	0.001	0.003	-0.000	0.000	-0.000
	24:1.0D-1.0WL	-0.000	-0.010	-0.000	0.010	-0.001	0.000	-0.000
112	21:1.0D+1.0WI	-0.000	-0.042	-0.001	0.042	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.000	-0.024	0.000	0.024	-0.001	0.000	-0.000
	23:1.0D-1.0WL	-0.000	-0.005	0.001	0.005	-0.000	0.000	-0.000

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
	24:1.0D-1.0WL	-0.001	-0.024	-0.000	0.024	-0.001	0.000	-0.000
113	21:1.0D+1.0WI	-0.000	-0.070	-0.000	0.070	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.000	-0.055	0.000	0.055	-0.002	0.000	-0.001
	23:1.0D-1.0WL	-0.000	-0.016	0.001	0.016	-0.000	0.000	-0.000
	24:1.0D-1.0WL	-0.001	-0.031	-0.000	0.031	-0.001	0.000	0.001
114	21:1.0D+1.0WI	0.000	-0.123	-0.001	0.123	-0.003	-0.000	0.000
	22:1.0D+1.0WI	0.001	-0.083	0.000	0.083	-0.002	-0.000	-0.001
	23:1.0D-1.0WL	-0.000	-0.015	0.001	0.015	0.000	-0.000	0.000
	24:1.0D-1.0WL	-0.001	-0.054	-0.000	0.054	-0.000	0.000	0.001
115	21:1.0D+1.0WI	-0.000	-0.014	-0.000	0.014	0.000	-0.000	-0.001
	22:1.0D+1.0WI	0.000	-0.040	0.000	0.040	0.001	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.033	0.000	0.033	0.001	-0.000	-0.002
	24:1.0D-1.0WL	-0.001	-0.008	-0.000	0.008	0.000	-0.000	-0.000
116	21:1.0D+1.0WI	-0.000	-0.007	-0.000	0.007	-0.000	-0.000	-0.001
	22:1.0D+1.0WI	0.000	-0.017	0.000	0.017	0.000	-0.000	-0.002
	23:1.0D-1.0WL	0.000	-0.014	0.000	0.014	0.001	-0.000	-0.001
	24:1.0D-1.0WL	-0.000	-0.004	-0.000	0.004	0.000	-0.000	-0.000
117	21:1.0D+1.0WI	-0.000	-0.054	-0.001	0.054	-0.000	0.000	-0.001
	22:1.0D+1.0WI	0.001	-0.122	-0.000	0.122	0.001	0.000	-0.003
	23:1.0D-1.0WL	0.000	-0.084	0.001	0.084	0.002	0.000	-0.001
	24:1.0D-1.0WL	-0.001	-0.016	-0.000	0.016	0.000	-0.000	0.001
118	21:1.0D+1.0WI	-0.000	-0.039	-0.000	0.039	-0.000	-0.000	-0.001
	22:1.0D+1.0WI	0.000	-0.072	0.000	0.072	0.001	-0.000	-0.003
	23:1.0D-1.0WL	0.000	-0.046	0.001	0.046	0.001	-0.000	-0.001
	24:1.0D-1.0WL	-0.000	-0.014	-0.000	0.014	0.000	-0.000	0.000
119	21:1.0D+1.0WI	-0.001	-0.039	-0.068	0.079	-0.002	-0.001	-0.000
	22:1.0D+1.0WI	0.068	-0.191	0.001	0.202	0.000	-0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.152	0.069	0.167	0.002	0.001	-0.000
	24:1.0D-1.0WL	-0.069	0.000	-0.000	0.069	0.000	0.000	0.002
120	21:1.0D+1.0WI	0.008	-0.056	-0.074	0.093	-0.001	-0.001	-0.001
	22:1.0D+1.0WI	0.075	-0.170	-0.003	0.186	0.000	-0.001	-0.002
	23:1.0D-1.0WL	-0.006	-0.127	0.073	0.146	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.073	-0.013	0.002	0.074	0.000	0.000	0.002
121	21:1.0D+1.0WI	0.030	-0.037	-0.086	0.099	-0.002	0.000	-0.001
	22:1.0D+1.0WI	0.113	-0.043	-0.025	0.124	-0.001	-0.000	-0.004
	23:1.0D-1.0WL	0.007	-0.021	0.065	0.069	0.002	-0.000	0.001
	24:1.0D-1.0WL	-0.076	-0.015	0.004	0.077	0.001	0.000	0.003
123	21:1.0D+1.0WI	0.001	-0.207	-0.070	0.219	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.069	-0.096	0.000	0.118	-0.000	0.001	-0.002
	23:1.0D-1.0WL	-0.003	0.017	0.071	0.073	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.071	-0.095	0.001	0.119	-0.000	-0.001	0.002
124	21:1.0D+1.0WI	-0.003	-0.178	-0.072	0.192	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.077	-0.083	0.005	0.114	0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.002	-0.006	0.071	0.071	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.083	-0.101	-0.006	0.131	-0.001	-0.001	0.002

Node Displacements Cont...

Node	L/C	X (in)	Y (in)	Z (in)	Resultant (in)	rX (rad)	rY (rad)	rZ (rad)
125	21:1.0D+1.0WI	-0.032	-0.038	-0.088	0.101	-0.002	-0.000	0.001
	22:1.0D+1.0WI	0.076	-0.016	0.004	0.078	0.001	-0.000	-0.003
	23:1.0D-1.0WL	-0.009	-0.022	0.067	0.071	0.002	0.000	-0.001
	24:1.0D-1.0WL	-0.116	-0.044	-0.025	0.127	-0.001	0.000	0.004
127	21:1.0D+1.0WI	-0.002	-0.043	-0.071	0.083	-0.002	0.001	0.000
	22:1.0D+1.0WI	0.067	-0.004	-0.001	0.067	0.000	-0.000	-0.002
	23:1.0D-1.0WL	-0.001	-0.157	0.072	0.173	0.002	-0.001	0.000
	24:1.0D-1.0WL	-0.070	-0.196	0.002	0.208	0.000	0.000	0.002
128	21:1.0D+1.0WI	-0.002	-0.044	-0.080	0.091	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.067	-0.026	0.006	0.073	0.000	-0.000	-0.001
	23:1.0D-1.0WL	-0.001	-0.148	0.085	0.171	0.002	-0.001	0.001
	24:1.0D-1.0WL	-0.070	-0.165	-0.001	0.179	0.000	-0.000	0.002
129	21:1.0D+1.0WI	-0.002	-0.013	-0.083	0.084	-0.004	0.000	0.000
	22:1.0D+1.0WI	0.067	-0.030	0.023	0.077	0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.046	0.129	0.137	0.004	-0.000	0.000
	24:1.0D-1.0WL	-0.069	-0.030	0.022	0.079	0.000	-0.000	0.002
132	21:1.0D+1.0WI	0.003	-0.178	-0.071	0.192	-0.002	0.000	-0.000
	22:1.0D+1.0WI	0.080	-0.101	-0.007	0.129	-0.001	0.001	-0.002
	23:1.0D-1.0WL	-0.001	-0.005	0.070	0.070	0.002	0.000	0.000
	24:1.0D-1.0WL	-0.079	-0.082	0.005	0.114	0.000	-0.000	0.002
134	21:1.0D+1.0WI	-0.011	-0.061	-0.076	0.098	-0.002	0.001	0.001
	22:1.0D+1.0WI	0.073	-0.017	0.003	0.075	0.000	-0.000	-0.002
	23:1.0D-1.0WL	0.005	-0.131	0.076	0.151	0.002	-0.000	-0.000
	24:1.0D-1.0WL	-0.079	-0.175	-0.003	0.192	0.000	0.001	0.002
136	21:1.0D+1.0WI	-0.001	-0.041	-0.078	0.088	-0.002	-0.000	0.000
	22:1.0D+1.0WI	0.068	-0.161	-0.000	0.175	0.000	0.000	-0.002
	23:1.0D-1.0WL	-0.000	-0.143	0.083	0.166	0.002	0.001	-0.001
	24:1.0D-1.0WL	-0.069	-0.023	0.006	0.073	0.000	0.000	0.001



Displacement

Displacement Check:

Antenna Pipe Mast (2 3/8" O.D. Pipe):
Max Displacement = 0.197 - 0.137 = 0.06"
(Node 48 with respect to Node 129)

Allowable Displacement = 1.5% cantilever = 0.21"

0.06" < 0.21" -- PASS

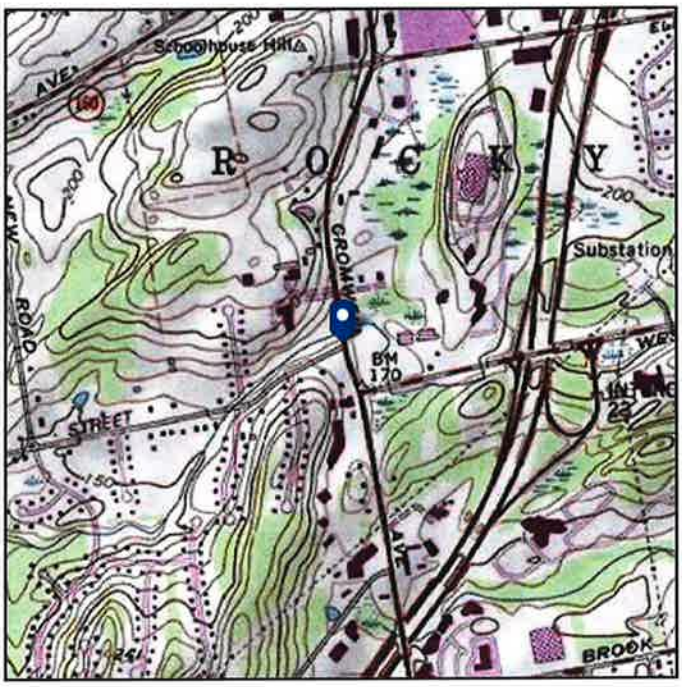
APPENDIX B

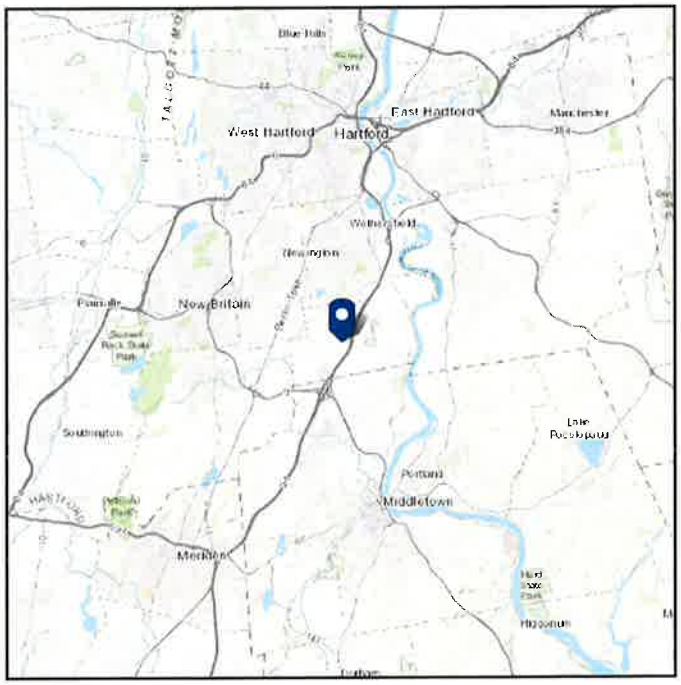
ASCE 7 Hazards Report

Address:
1 France St
Rocky Hill, Connecticut
06067

Standard: ASCE/SEI 7-10
Risk Category: II
Soil Class:

Elevation: 171.05 ft (NAVD 88)
Latitude: 41.650734
Longitude: -72.67939





Wind

Results:

Wind Speed:	124 Vmph
10-year MRI	77 Vmph
25-year MRI	87 Vmph
50-year MRI	93 Vmph
100-year MRI	101 Vmph
Data Source:	ASCE/SEI 7-10, Fig. 26.5-1A and Figs. CC-1–CC-4, incorporating errata of March 12, 2014
Date Accessed:	Mon May 13 2019

Value provided is 3-second gust wind speeds at 33 ft above ground for Exposure C Category, based on linear interpolation between contours. Wind speeds are interpolated in accordance with the 7-10 Standard. Wind speeds correspond to approximately a 7% probability of exceedance in 50 years (annual exceedance probability = 0.00143, MRI = 700 years).

Site is in a hurricane-prone region as defined in ASCE/SEI 7-10 Section 26.2. Glazed openings need not be protected against wind-borne debris.

Mountainous terrain, gorges, ocean promontories, and special wind regions should be examined for unusual wind conditions.

The ASCE 7 Hazard Tool is provided for your convenience, for informational purposes only, and is provided "as is" and without warranties of any kind. The location data included herein has been obtained from information developed, produced, and maintained by third party providers; or has been extrapolated from maps incorporated in the ASCE 7 standard. While ASCE has made every effort to use data obtained from reliable sources or methodologies, ASCE does not make any representations or warranties as to the accuracy, completeness, reliability, currency, or quality of any data provided herein. Any third-party links provided by this Tool should not be construed as an endorsement, affiliation, relationship, or sponsorship of such third-party content by or from ASCE.

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APPENDIX C

Site Name: **Rocky Hill CT**

Site Address: France Street, Lot 1
Rocky Hill, CT 06067

Rocky Hill CT –Site Photos

Dewberry

August 21, 2017

Mr. Dan Vadney
Crown Castle USA, Inc.
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Clifton Park, NY 12065
(518) 373-3510
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Tower Engineering Professionals
326 Tryon Road
Raleigh, NC 27603
(919) 661-6351 (office)
(919) 661-6350 (fax)
PMI@tepgroup.net

Subject: **Modification Inspection Report**

Crown Castle Designation:	**Crown Castle BU Number:**	806374
	Crown Castle Site Name:	HRT 081
	Crown Castle JDE Job Number:	400014
Engineering Firm Designation:	**TEP Project Number:**	25657.69403
Site Data:	**North Street** **Rocky Hill, Hartford County, CT 06067** **Latitude N *41° 38' 23.2"*, Longitude W *72° 42' 6.83"*** **140 Foot – Monopole Tower**	

Dear Mr. Vadney,

Tower Engineering Professionals is pleased to submit this **"Modification Inspection Report"** (MI Report) to Crown Castle for the modification/reinforcement to the subject structure. This Modification Inspection (MI) was performed in accordance with Crown Castle ENG-SOW-10007 Modification Inspection SOW, Contract Documents, and Crown Castle Purchase Order number 1025233. The purpose of this MI is to confirm that the modification installation configuration and workmanship are in accordance with the contract document(s) listed in Table 2. The MI is not a review of the adequacy or effectiveness of the modification/reinforcement solution.

Table 1 – General Information

	Company	Contact	Dates on Site
MI Inspector	Tower Engineering Professionals	Kevin R. Arnett, P.E., C.W.I.	N/A
MI Inspector Field Representative (if applicable)	Tower Engineering Professionals	Scott Coburn & James Harms	8/17/17
■ Independent	☐ EOR	☐ Turnkey	
Modification Design EOR	Paul J. Ford and Company	Justin T. Kline, P.E.	N/A
General Contractor	Advanced Network Services	Ron Brandon	Unknown
Sub to the General Contractor	N/A	N/A	N/A
Field CWI for the General Contractor	N/A	N/A	N/A
Field NDE for the General Contractor	N/A	N/A	N/A

Table 2 – Documents

Document(s)	Remarks	Source
Modification Drawings Date: March 1, 2017 EOR: Justin T. Kline, P.E. Job#: 37517-0519.002.7700	Creator of Drawings: Paul J. Ford and Company Job #: 37517-0519.002.7700 Date of Drawings: 3/1/17	CCIsites Drawing File: 6720736

Produced with a Trial Version of PDF Annotator - www.PDFAnnotator.com

Based on our inspection, Tower Engineering Professionals, Inc. determines this project:

_X_ PASSING MI

The configuration, materials and/or workmanship of the modifications are installed in accordance with the Contract Documents and no deficiencies were found.

__ PASS AS NOTED MI

The configuration, materials and/or workmanship of the modifications are accepted as noted (see detail below).

__ FAILED MI

The configuration, materials and/or workmanship of the modifications are NOT installed in accordance with the Contract Documents. The rejection is based on non-conformance in the following area(s):

__ Materials (see detail below)

__ Workmanship (see detail below)

__ Configuration (see detail below)

Discrepancies were noted between the As-Built field conditions and the structural modification drawings. The As-Built field conditions were approved by the Engineer of Record. All discrepancies are noted in the executive summary.

All observations were performed after the construction was complete. Tower Engineering Professionals, Inc. was not present during the construction phase. The onsite PMI was performed by Scott Coburn and James Harms of Tower Engineering Professionals, Inc., (919) 661-6351.

We at Tower Engineering Professionals, Inc. appreciate the opportunity of providing our continuing professional services to you and Crown Castle. If you have any questions or need further assistance on this or any other projects please give us a call.

Respectfully submitted,

STATE OF CONNECTICUT
KEVIN R. ARNETT
31057
LICENSED
PROFESSIONAL ENGINEER

8-21-17

Kevin R. Arnett, P.E., C.W.I.

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PRE-CONSTRUCTION

6.1.1 MI CHECKLIST DRAWING

MODIFICATION INSPECTION NOTES:

1. **GENERAL**
   1.1. THE MODIFICATION INSPECTION (MI) IS A VISUAL INSPECTION OF TOWER MODIFICATIONS AND A REVIEW OF CONSTRUCTION INSPECTIONS AND OTHER REPORTS TO ENSURE THE INSTALLATION WAS CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, NAMELY THE MODIFICATION DRAWINGS, AS DESIGNED BY THE EOR.
   1.2. THE MI IS TO CONFIRM INSTALLATION CONFIGURATION AND WORKMANSHIP ONLY AND IS NOT A REVIEW OF THE MODIFICATION DESIGN ITSELF, NOR DOES THE MI INSPECTOR TAKE OWNERSHIP OF THE MODIFICATION DESIGN OWNERSHIP OF THE STRUCTURAL MODIFICATION DESIGN EFFECTIVENESS AND INTEGRITY RESIDES WITH THE EOR AT ALL TIMES.
   1.3. ALL MI'S SHALL BE CONDUCTED BY A CROWN CASTLE ENGINEERING VENDOR (AEV) OR ENGINEERING SERVICE VENDOR (AESV) THAT IS APPROVED TO PERFORM ELEVATED WORK FOR CROWN CASTLE.
   1.4. TO ENSURE THAT THE REQUIREMENTS OF THE MI ARE MET, IT IS VITAL THAT THE GENERAL CONTRACTOR (GC) AND THE MI INSPECTOR BEGIN COMMUNICATING AND COORDINATING AS SOON AS A PO IS RECEIVED. IT IS EXPECTED THAT EACH PARTY WILL BE PROACTIVE IN REACHING OUT TO THE OTHER PARTY. IF CONTACT INFORMATION IS NOT KNOWN, CONTACT YOUR CROWN CASTLE POINT OF CONTACT (POC).
   1.5. REFER TO ENG-SOW-10007: MODIFICATION INSPECTION SOW FOR FURTHER DETAILS AND REQUIREMENTS.

2. **MI INSPECTOR**
   2.1. THE MI INSPECTOR IS REQUIRED TO CONTACT THE GC AS SOON AS RECEIVING A PO FOR THE MI TO, AT A MINIMUM:
      2.1.1. REVIEW THE REQUIREMENTS OF THE MI CHECKLIST.
      2.1.2. WORK WITH THE GC TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE INSPECTIONS, INCLUDING FOUNDATION INSPECTIONS.
      2.1.3. THE MI INSPECTOR IS RESPONSIBLE FOR COLLECTING ALL GC INSPECTION AND TEST REPORTS, REVIEWING THE DOCUMENTS FOR ADHERENCE TO THE CONTRACT DOCUMENTS, CONDUCTING THE IN-FIELD INSPECTIONS, AND SUBMITTING THE MI REPORT TO CROWN CASTLE.

3. **GENERAL CONTRACTOR**
   3.1. THE GC IS REQUIRED TO CONTACT THE MI INSPECTOR AS SOON AS RECEIVING A PO FOR THE MODIFICATION INSTALLATION OR TURNKEY PROJECT TO, AT A MINIMUM:
      3.1.1. REVIEW THE REQUIREMENTS OF THE MI CHECKLIST.
      3.1.2. WORK WITH THE MI INSPECTOR TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE INSPECTIONS, INCLUDING FOUNDATION INSPECTIONS.
      3.1.3. BETTER UNDERSTAND ALL INSPECTION AND TESTING REQUIREMENTS.
      3.1.4. THE GC SHALL PERFORM AND RECORD THE TEST AND INSPECTION RESULTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE MI CHECKLIST AND ENG-SOW-10007.

4. **RECOMMENDATIONS**
   4.1. THE FOLLOWING RECOMMENDATIONS AND SUGGESTIONS ARE OFFERED TO ENHANCE THE EFFICIENCY AND EFFECTIVENESS OF DELIVERING AN MI REPORT:
      4.1.1. IT IS SUGGESTED THAT THE GC PROVIDE A MINIMUM OF 5 BUSINESS DAYS NOTICE, PREFERABLE 10, TO THE MI INSPECTOR AS TO WHEN THE SITE WILL BE READY FOR THE MI TO BE CONDUCTED.
      4.1.2. THE GC AND MI INSPECTOR COORDINATE CLOSELY THROUGHOUT THE ENTIRE PROJECT.
      4.1.3. WHEN POSSIBLE, IT IS PREFERRED TO HAVE THE GC AND MI INSPECTOR ON-SITE SIMULTANEOUSLY FOR ANY GUY WIRE TENSIONING OR RE-TENSIONING OPERATIONS.
      4.1.4. IT MAY BE BENEFICIAL TO INSTALL ALL TOWER MODIFICATIONS PRIOR TO CONDUCTING THE FOUNDATION INSPECTIONS TO ALLOW FOUNDATION AND MI INSPECTION(S) TO COMMENCE WITH ONE SITE VISIT.
      4.1.5. WHEN POSSIBLE, IT IS PREFERRED TO HAVE THE GC AND MI INSPECTOR ON-SITE DURING THE MI TO HAVE ANY DEFICIENCIES CORRECTED DURING THE INITIAL MI. THEREFORE, THE GC MAY CHOOSE TO COORDINATE THE MI CAREFULLY TO ENSURE ALL CONSTRUCTION FACILITIES ARE AT THEIR DISPOSAL WHEN THE MI INSPECTOR IS ON SITE.

5. **CANCELLATION OR DELAYS IN SCHEDULED MI**
   5.1. IF THE GC AND MI INSPECTOR AGREE TO A DATE ON WHICH THE MI WILL BE CONDUCTED, AND EITHER PARTY CANCELS OR DELAYS, CROWN CASTLE SHALL NOT BE RESPONSIBLE FOR ANY COSTS, FEES, LOSS OF DEPOSITS AND/OR OTHER PENALTIES RELATED TO THE CANCELLATION OR DELAY INCURRED BY EITHER PARTY FOR ANY TIME (E.G. TRAVEL AND LODGING, COSTS OF KEEPING EQUIPMENT ON-SITE, ETC.). IF CROWN CASTLE CONTRACTS DIRECTLY FOR A THIRD PARTY MI, EXCEPTIONS MAY BE MADE IN THE EVENT THAT THE DELAY/CANCELLATION IS CAUSED BY WEATHER OR OTHER CONDITIONS THAT MAY COMPROMISE THE SAFETY OF THE PARTIES INVOLVED.

6. **CORRECTION OF FAILING MI'S**
   6.1. IF THE MODIFICATION INSTALLATION WOULD FAIL THE MI ("FAILED MI"), THE GC SHALL WORK WITH CROWN CASTLE TO COORDINATE A REMEDIATION PLAN IN ONE OF TWO WAYS:
      6.1.1. CORRECT FAILING ISSUES TO COMPLY WITH THE SPECIFICATIONS CONTAINED IN THE ORIGINAL CONTRACT DOCUMENTS AND COORDINATE A SUPPLEMENT MI.
      6.1.2. OR, WITH CROWN CASTLE'S APPROVAL, THE GC MAY WORK WITH THE EOR TO RE-ANALYZE THE MODIFICATION/REINFORCEMENT USING THE AS-BUILT CONDITION.

7. **MI VERIFICATION INSPECTIONS**
   7.1. CROWN CASTLE RESERVES THE RIGHT TO CONDUCT A MI VERIFICATION INSPECTION TO VERIFY THE ACCURACY AND COMPLETENESS OF PREVIOUSLY COMPLETED MI INSPECTION(S) ON TOWER MODIFICATION PROJECTS.
   7.2. ALL VERIFICATION INSPECTIONS SHALL BE HELD TO THE SAME SPECIFICATIONS AND REQUIREMENTS IN THE CONTRACT DOCUMENTS AND IN ACCORDANCE WITH ENG-SOW-10007.
   7.3. VERIFICATION INSPECTION MAY BE CONDUCTED BY AN INDEPENDENT AEV/AESV FIRM AFTER A MODIFICATION PROJECT IS COMPLETED, AS MARKED BY THE DATE OF AN ACCEPTED "PASSING MI" OR "PASS AS NOTED MI" REPORT FOR THE ORIGINAL PROJECT.

8. **PHOTOGRAPHS**
   8.1. BETWEEN THE GC AND THE MI INSPECTOR THE FOLLOWING PHOTOGRAPHS, AT A MINIMUM, ARE TO BE TAKEN AND INCLUDED IN THE MI REPORT:
      8.1.1. PRECONSTRUCTION GENERAL SITE CONDITION
      8.1.2. PHOTOGRAPHS DURING THE REINFORCEMENT MODIFICATION CONSTRUCTION/ERECTION AND INSPECTION
      8.1.3. RAW MATERIALS
      8.1.4. PHOTOS OF ALL CRITICAL DETAILS
      8.1.5. FOUNDATION MODIFICATIONS
      8.1.6. WELD PREPARATION
      8.1.7. BOLT INSTALLATION AND TORQUE
      8.1.8. FINAL INSTALLED CONDITION
      8.1.9. SURFACE COATING REPAIR
      8.1.10. POST CONSTRUCTION PHOTOGRAPHS
      8.1.11. FINAL INFIELD CONDITION
      8.1.12. PHOTOS OF ELEVATED MODIFICATIONS TAKEN FROM THE GROUND SHALL BE CONSIDERED INADEQUATE.
      8.1.13. THIS IS NOT A COMPLETE LIST OF REQUIRED PHOTOS, PLEASE REFER TO ENG-SOW-10007.

9. **INSPECTION AND TESTING**
   9.1. ALL WORK SHALL BE SUBJECT TO REVIEW AND OBSERVATION BY CROWN CASTLE'S REPRESENTATIVE AND CROWN CASTLE'S AUTHORIZED INDEPENDENT INSPECTION AND TESTING AGENCY.
   9.2. INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS ARE STILL REQUIRED WHEN THE EOR PERFORMS SUPPORT SERVICES DURING CONSTRUCTION.
   9.3. OBSERVED DISCREPANCIES BETWEEN THE WORK AND THE CONTRACT DOCUMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT NO ADDITIONAL COST.
   9.4. AN INDEPENDENT QUALIFIED INSPECTION/TESTING AGENCY SHALL BE SELECTED AND RETAINED FOR THE SOLE PURPOSE OF INSPECTING, TESTING, DOCUMENTING, AND APPROVING ALL WELDING AND FIELD WORK PERFORMED BY THE CONTRACTOR.
      9.4.1. ACCESS TO ANY PLACE WHERE WORK IS BEING DONE SHALL BE PERMITTED AT ALL TIMES.
      9.4.2. THE INSPECTION AGENCY SHALL SO SCHEDULE THIS WORK AS TO CAUSE A MINIMUM OF INTERRUPTION TO, AND COORDINATE WITH, THE WORK IN PROGRESS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE WORK SCHEDULE WITH THE TESTING AGENCY. THE CONTRACTOR SHALL ALLOW FOR ADEQUATE TIME AND ACCESS FOR THE TESTING AGENCY TO PERFORM THEIR DUTIES.
   9.5. THE INSPECTION AND TESTING AGENCY SHALL BE RESPONSIBLE TO PERFORM THE FOLLOWING SERVICES AND INSPECT THE FOLLOWING ITEMS IN ACCORDANCE WITH THE CONSTRUCTION DRAWINGS. THE TESTING AGENCY SHALL INSPECT ITEMS ON THIS LIST AND OTHER ITEMS AS NECESSARY TO FULFILL THEIR RESPONSIBILITY. THE TESTING AGENCY SHALL UTILIZE EXPERIENCED, TRAINED INSPECTORS INCLUDING AWS CERTIFIED WELDING INSPECTORS (CWI). INSPECTORS SHALL HAVE THE TRAINING, CREDENTIALS, AND EXPERIENCE APPROPRIATE FOR AND COMMENSURATE WITH THE SCOPE AND TYPE OF INSPECTION WORK TO BE PERFORMED.
   9.6. GENERAL
      9.6.1. PERFORM PERIODIC ON-SITE OBSERVATION, INSPECTION, VERIFICATION, AND TESTING DURING THE TIME THE CONTRACTOR IS WORKING ON-SITE. AGENCY SHALL NOTIFY CROWN CASTLE AND THE EOR IMMEDIATELY WHEN FIELD PROBLEMS OR DISCREPANCIES OCCUR.
   9.7. FOUNDATIONS AND SOIL PREPARATION - (NOT REQUIRED)
   9.8. CONCRETE TESTING PER ACI - (NOT REQUIRED)
   9.9. STRUCTURAL STEEL
      9.9.1. CHECK STEEL ON THE JOB WITH THE PLANS.
      9.9.2. CHECK MILL CERTIFICATIONS. CALL FOR LABORATORY TEST REPORTS WHEN MILL CERTIFICATION IS IN QUESTION.
      9.9.3. CHECK GRADE OF STEEL MEMBERS, AND BOLTS FOR CONFORMANCE WITH DRAWINGS.
      9.9.4. INSPECT ALL STRUCTURAL BOLTS SHALL BE FIELD INSPECTED ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009.
      9.9.5. INSPECT STEEL MEMBERS FOR DISTORTION, EXCESSIVE RUST, FLAWS AND BURNED HOLES.
      9.9.6. CHECK STEEL MEMBERS FOR SIZES, SWEEP AND DIMENSIONAL TOLERANCES.
      9.9.7. CHECK FOR SURFACE FINISH SPECIFIED, GALVANIZED.
      9.9.8. CHECK THAT BOLTS HAVE BEEN TIGHTENED PROPERLY.
      9.9.9. PRIOR TO ANY FIELD CUTTING THE CONTRACTOR SHALL MARK THE CUTOUT LINES ON THE STEEL AND THE INSPECTION/TESTING AGENCY SHALL VERIFY PROPOSED LAYOUT, LOCATION, AND DIMENSIONS. THE INSPECTION/TESTING AGENCY SHALL CLOSELY AND CONTINUOUSLY MONITOR THIS ACTIVITY.
   9.10. WELDING:
      9.10.1. VERIFY FIELD WELDING PROCEDURES, WELDERS, AND WELDING OPERATORS, NOT DEEMED PREQUALIFIED, IN ACCORDANCE WITH AWS D1.1.
      9.10.2. INSPECT FIELD WELDED CONNECTIONS IN ACCORDANCE WITH THE REQUIREMENTS SPECIFIED AND WITH AWS D1.1.
      9.10.3. APPROVE FIELD WELDING SEQUENCE.
      9.10.4. A PROGRAM OF THE APPROVED SEQUENCES SHALL BE SUBMITTED TO CROWN CASTLE BEFORE WELDING BEGINS. NO CHANGE IN APPROVED SEQUENCES MAY BE MADE WITHOUT PERMISSION FROM CROWN CASTLE.
      9.10.5. INSPECT WELDED CONNECTIONS AS FOLLOWS AND IN ACCORDANCE WITH AWS D1.1:
         9.10.5.1. INSPECT WELDING EQUIPMENT FOR CAPACITY, MAINTENANCE, AND WORKING CONDITIONS.
         9.10.5.2. VERIFY SPECIFIED ELECTRODES AND HANDLING AND STORAGE OF ELECTRODES FOR CONFORMANCE TO SPECIFICATIONS.
         9.10.5.3. INSPECT PREHEATING AND INTERPASS TEMPERATURES FOR CONFORMANCE WITH AWS D1.1.
         9.10.5.4. VISUALLY INSPECT ALL WELDS AND VERIFY THAT QUALITY OF WELDS MEETS THE REQUIREMENTS OF AWS D1.1. OTHER TESTS MAY ALSO BE PERFORMED ON THE WELDS BY THE TESTING AGENCY IN ORDER FOR THEM TO PERFORM THEIR DUTIES FOR THIS PROJECT.
         9.10.5.5. SPOT TEST AT LEAST ONE FILLET WELD OF EACH MEMBER USING MAGNETIC PARTICLE.
         9.10.5.6. INSPECT FOR SIZE, SPACING, TYPE AND LOCATION AS PER APPROVED DRAWINGS.
         9.10.5.7. VERIFY THAT THE BASE METAL CONFORMS TO THE DRAWINGS.
         9.10.5.8. REVIEW THE REPORTS BY TESTING LABS.
         9.10.5.9. CHECK TO SEE THAT WELDS ARE CLEAN AND FREE FROM SLAG.
         9.10.5.10. INSPECT RUST PROTECTION OF WELDS AS PER SPECIFICATIONS.
         9.10.5.11. CHECK THAT DEFECTIVE WELDS ARE CLEARLY MARKED AND HAVE BEEN ADEQUATELY REPAIRED.
   9.11. REPORTS:
      9.11.1. COMPILE AND PERIODICALLY SUBMIT DAILY INSPECTION REPORTS TO CROWN CASTLE.
      9.11.2. THE INSPECTION PLAN OUTLINED HEREIN IS INTENDED AS A DESCRIPTION OF GENERAL AND SPECIFIC ITEMS OF CONCERN. IT IS NOT INTENDED TO BE ALL-INCLUSIVE. IT DOES NOT LIMIT THE TESTING AND INSPECTION AGENCY TO THE ITEMS LISTED. ADDITIONAL TESTING, INSPECTION, AND CHECKING MAY BE REQUIRED AND SHOULD BE ANTICIPATED. THE TESTING AGENCY SHALL USE THEIR PROFESSIONAL JUDGMENT AND KNOWLEDGE OF THE JOB SITE CONDITIONS AND THE CONTRACTOR'S PERFORMANCE TO DECIDE WHAT OTHER ITEMS REQUIRE ADDITIONAL ATTENTION. THE TESTING AGENCY'S JUDGMENT MUST PREVAIL ON ITEMS NOT SPECIFICALLY COVERED. ANY DISCREPANCIES OR PROBLEMS SHALL BE BROUGHT IMMEDIATELY TO CROWN CASTLE'S ATTENTION. RESOLUTIONS ARE NOT TO BE MADE WITHOUT CROWN CASTLE'S REVIEW AND SPECIFIC WRITTEN CONSENT. CROWN CASTLE RESERVES THE RIGHT TO DETERMINE WHETHER OR NOT A RESOLUTION IS ACCEPTABLE.
      9.11.3. AFTER EACH INSPECTION, THE TESTING AGENCY WILL PREPARE A WRITTEN ACCEPTANCE OR REJECTION WHICH WILL BE GIVEN TO THE CONTRACTOR AND FILED AS DAILY REPORTS TO CROWN CASTLE. THIS WRITTEN ACTION WILL GIVE THE CONTRACTOR A LIST OF ITEMS TO BE CORRECTED, PRIOR TO CONTINUING CONSTRUCTION, AND/OR LOADING OF STRUCTURAL ITEMS.
      9.11.4. THE TESTING AGENCY DOES NOT RELIEVE THE CONTRACTOR'S CONTRACTUAL OR STATUTORY OBLIGATIONS. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE TESTING AGENCY WILL NOT REPLACE THE CONTRACTOR'S QUALITY CONTROL PERSONNEL.

MI CHECKLIST

CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY EOR)	REPORT ITEM
	PRE-CONSTRUCTION
X	MI CHECKLIST DRAWINGS
X	EOR REVIEW
X	FABRICATION INSPECTION
NA	FABRICATOR CERTIFIED WELD INSPECTION
X	MATERIAL TEST REPORT (MTR)
NA	FABRICATOR NDE INSPECTION
NA	NDE REPORT OF MONOPOLE BASE PLATE (AS REQUIRED)
X	PACKING SLIPS
ADDITIONAL TESTING AND INSPECTIONS:	
	CONSTRUCTION
X	CONSTRUCTION INSPECTIONS
NA	FOUNDATION INSPECTIONS
NA	CONCRETE COMP. STRENGTH AND SLUMP TESTS
NA	POST INSTALLED ANCHOR ROD VERIFICATION
NA	BASE PLATE GROUT VERIFICATION
NA	CONTRACTOR'S CERTIFIED WELD INSPECTION
NA	EARTHWORK: PROVIDE PHOTO DOCUMENTATION OF EXCAVATION QUALITY AND COMPACTION
X	ON SITE COLD GALVANIZING VERIFICATION
NA	GUY WIRE TENSION REPORT
X	GC AS-BUILT DOCUMENTS
NA	MICROPILE/ROCK ANCHOR INSTALLER'S DRILLING AND INSTALLATION LOGS AND QA/QC DOCUMENTS
ADDITIONAL TESTING AND INSPECTIONS:	
	POST-CONSTRUCTION
X	MI INSPECTOR REDLINE OR RECORD DRAWING(S)
NA	POST INSTALLED ANCHOR ROD TARGET TENSION LOAD TESTING
NA	REFER TO MICROPILE/ROCK ANCHOR NOTES FOR SPECIAL INSPECTION AND TESTING REQUIREMENTS.
X	PHOTOGRAPHS
ADDITIONAL TESTING AND INSPECTIONS:	

NOTE: X DENOTES A DOCUMENT NEEDED FOR THE PMI REPORT
NA DENOTES A DOCUMENT THAT IS NOT REQUIRED FOR THE PMI REPORT

Built per Design
8/11/17
ANS
Ron Brandon

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PJF PAUL J. FORD & COMPANY
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Phone 614.221.6679 www.pauljford.com

CROWN CASTLE
8 PARKMEADOW DRIVE, PITTSFORD, NY 14534
PH: (585) 899-3445

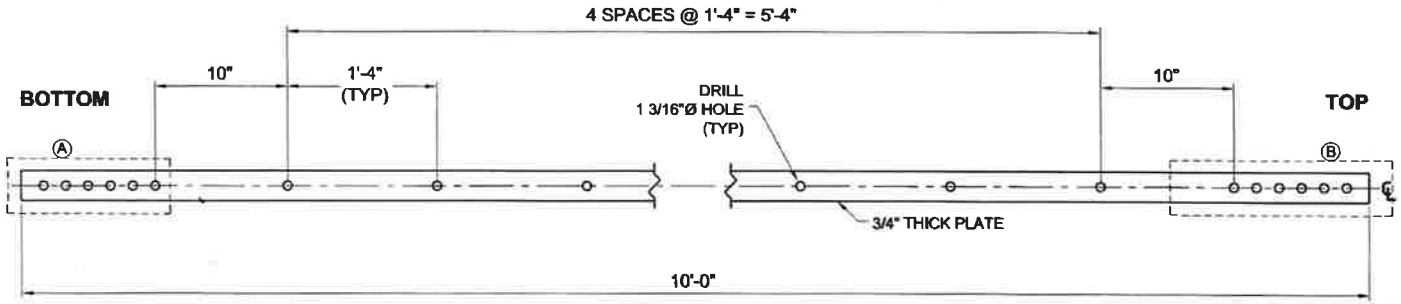
MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

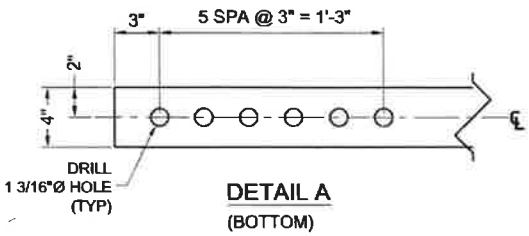
PROJECT No:	37517-0519.002.7706
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

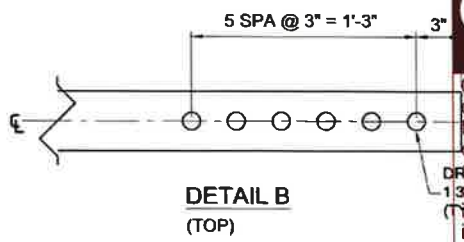
MI CHECKLIST

T-2

6.1.2 EOR REVIEW







PAUL J. FORD AND COMPANY
STRUCTURAL ENGINEERS
Columbus, Ohio · Orlando, Florida · Atlanta, Georgia

Corrections or comments made on this document do not relieve the contractor from compliance with the requirements of the drawings and specifications. This document was reviewed for general conformance to the design requirements in accordance with the information given in the contract documents. The contractor is responsible for confirming and correlating all quantities and dimensions, fabrication processes and techniques of construction, coordination of the work with other trades and the satisfactory performance of his work.

By: JWM
Date: 04/14/2017

- [x] Reviewed, No exceptions taken
- [ ] Reviewed, Make Noted Corrections
- [ ] Revise and Resubmit
- [ ] Rejected, See Remarks
- [ ] Reviewed Only For Loads Imposed on the Structure

NOTES:
1. ALL HOLES ARE TO BE DRILLED. DO NOT BURN OR PUNCH.
2. TOLERANCES: FRACTIONS ± 1/16"
   ANGLES ± 1/2 DEGREE
   DECIMALS ± .010"
3. THE 65 KSI MATERIAL SHALL CONFORM TO THE FOLLOWING:
   A. MATERIAL SHALL BE ASTM A572 HAVING A MINIMUM TENSILE STRENGTH (Fu) OF 80 KSI AND A MINIMUM YIELD STRENGTH (Fy) OF 65 KSI.
   B. THE FINISH SHALL BE HOT-DIP GALVANIZED PER ASTM A123

ALLFASTENERS USA
15401 COMMERCE PARK DRIVE - BROOKPARK, OHIO 44142
PH: (440) 232-6060 FX: (440) 232-6062
www.allfasteners.com

CUSTOMER:	HOLE QTY: 17
PROJECT: HRT 081 943236	BLACK WT (LBS): 102.0
LOCATION:	PART QTY: 3
DATE: 4/12/2017	PART #CCI-AFP-04007510

6.1.3 FABRICATION INSPECTION

ALLFASTENERS®

SERVICE | SOLUTIONS | SUPPLY FOR ALL YOUR FASTENING NEEDS

To: ANS Advanced Network Services
12 Elwood Road
Albany, NY 12204

Subject: PO: 1728251-01

April 18, 2017

Please accept this letter as certification that our work on the Steel Package for HRT 081 was performed in accordance with industry standards and the contractor documents.

Please contact me if you have any questions.

Thank you,

Mark Wootten

Fabrication Manager

Allfasteners USA LLC

New York
69 Orchard Street,
Ramsey, New Jersey 07446
Tel 1-800-577-3171
Fax +1 (201) 783-8840

Cleveland
15401 Commerce Park Drive,
Cleveland, Ohio 44142
Tel 1-888-859-6060
Fax +1 (440) 232-6062

Los Angeles
5450 W. 83rd Street,
Los Angeles, California 90045
Tel +1 (310) 410-5007
Fax +1 (310) 410-5004

Nebraska
102 8th Street,
Gothenburg, Nebraska 69138
Tel 1-855-330-2210
Fax +1 (308) 537-3500

sales@allfasteners.com | www.allfasteners.com

6.1.4 MATERIAL TEST REPORT (MTR)

ALLFASTENERS®

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Certification Report

April 2017

ANS Advanced Network Services

HRT 081

PO: 1728251-01

SO: 121600

NEXGEN2 BLIND BOLT ASSEMBLY NG2

Product Certificate of Conformance

AF Lot #: 7941 | Customer PO #: 1728251-01 | Part #: 2NG2036

Lot Number	Component Part Number	Description	Test Results: Proof Load (Mpa) Spec Min	Proof Load (Mpa) Sample	Hardness (HRC) Spec Min	Hardness (HRC) Spec Max	Hardness (HRC) Sample A	Hardness (HRC) Sample B	Wedge Tensile Test (psi) Spec Min	Wedge Tensile Test (psi) Sample A	Wedge Tensile Test (psi) Sample B
25202	2NG20095A490M	M20 x 95 NexGen2™ Structural Bolt			33	38	Pass	Pass	150000	Pass	Pass
780167	2HHNM2025G2HM	M20-2.50 Heavy Hex Nut Grade 2H	1165	Pass	24	36	Pass	Pass			
771228	14SLSS1116M	11/16" Shear Sleeve			24	33	Pass	Pass			
49075	2NG2SWH	M20 NexGen2™ Collapsible Split Washer			23	34	Pass	Pass			
778601	2NG2CWM	M20 Solid Collar Washer			23	34	Pass	Pass			

New York
69 Orchard Street
Ramsey, New Jersey 07446
Tel 800 577 3171
Fax 201 783 8840

Cleveland
959 Lake Road
Medina, Ohio 44256
Tel 888 859 6060
Fax 440 232 6062

Los Angeles
5450 W 83rd Street
Los Angeles, California 90045
Tel 310 410 5007
Fax 866 553 7702

Nebraska
806 8th Street
Gothenburg, Nebraska 69138
Tel 855 330 2210
Fax 308 537 3500

sales@allfasteners.com | www.allfasteners.com

ALLFASTENERS®

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Ship To: ALL FASTENERS - MEDINA
1021 INDUSTRIAL PARKWAY
MEDINA OH 44256
USA

METALLURGICAL TEST REPORT

We hereby certify that these Chemical and/or Test results are correct as contained in the records

Sold To: ALLFASTENERS USA, LLC
959 LAKE ROAD
MEDINA OH 44256
USA

By _____
Dick Peoples - Customer Service Metallurgist

Mercury source materials have not been used in the production of this material. No weld repair performed

Cert #:22314 Grade:A572 GR 65

Size:0.750 X 4.000 FL
Size(MM):19.05 X 101.60
Length:421.0000
Length(MM):10693.4

Item: 113280 Cust Item:

A572 GR 65,FL SE ,0.75 x 4 ,35' 1",35' 1"

Heat	Furnace	Melted Source	Cast	CE	DI	Sum	Shpd. Qty
W19685	EAF	USA	Bil			0.0000	3,221

Ladle Chemistry Analysis

C	Mn	P	S	Si	Cu	Ni	Cr	Mo	Sn	Al	V	Nb	B	Ti
0.15	1.15	0.011	0.020	0.26	0.22	0.11	0.11	0.05	0.010	0.004	0.073	0.004	0.0003	0.0007
N	**Pb**	**O**	**Ca**	**Bi**	**Se**	**Te**	**As**	**Fe**	**Zn**	**Zr**	**Mg**	**H2**	**CO**	**OT**
0.0166	0.00		0.000				0.006	0.003					0.0100	

Jominy (ASTM A225)

J1	J2	J3	J4	J5	J6	J7	J8	J9	J10	J11	J12	J13	J14	J15	J16	J18	J20	J24	J28	J32	Grain Size

Notes

Non-Metallic Inclusions (ASTM E45)

Method A				Method C		Magnetic Particle Inspection		ASTM E381			Reduction Ratio
A	B	C	D	Q	S	Frequency	Severity	R	C	S	8.8 : 1
				T							
				H							
				T							
				H							

Tensile Properties

Tensile	Yield 0.2%	Yield 0.01%	Yield 0.02%	Elong % 2"	Elong % 8"	ROA %	Decarb
93700	77000			42.00			
91600	71600			41.00			

Hardness

Type	RC	RB	RA	BHN	HV
Surface					
Mid					
Core					
Surface					
Mid					
Core					

Charpy

Temp	Size	Longitudinal	Transverse

New York
69 Orchard Street
Ramsey, New Jersey 07446
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Medina, Ohio 44256
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Fax 440 232 6062

Los Angeles
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Tel 310 410 5007
Fax 866 553 7702

Nebraska
806 8th Street
Gothenburg, Nebraska 69138
Tel 855 330 2210
Fax 308 537 3500

sales@allfasteners.com | www.allfasteners.com

6.1.5 PACKING SLIPS

Received per Ed 4/27 JV

ALLFASTENERS® USA

959 Lake Road
Medina, OH 442556
Ph: 888 859 6060
Fax: 440 232 6062
Email: sales@allfasteners.com

69 Orchard Street
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Fax: 201 783 8840
Email: sales@allfasteners.com

5450 W. 83rd Street
Los Angeles, CA 90045
Ph: 800 410 5511
Fax: 310 410 5004
Email: sales@allfasteners.com

INVOICE

ANS Advanced Network Services
12 Elmwood Road
Albany NY 12204-2410

Struct mod exempt

DELIVER

12 Elmwood Road
Albany
NY 12204

Invoice
E 92420
Acc No: 7388

Invoice Date	Customer PO No.	Order	Reference	Ordered By	Contact No.	Shipping Method	Terms
04.19.2017	1728251-01	121600	HRT 081	Ron	800 268 0937	Unishippers	NET 30 Days

Order Code	Description	Qty Ord	Back Ord	Supplied	Unit Price	UOM	Ext. Amount
14CCI-AFP-04007510	3/4 x 4 x 10ft Auxiliary Flat Plate Grade 65 Galvanized with 17 Holes	3		3		1	
2NG2036	M20 x 36 Grip Length NexGen2™ Structural Bolt Assembly Magni 24/Box	72		72		1	
12AF1000-1G	AF1000 Cold Galvanizing Coating 1 Gallon 2/Ctn	1		1		1	
13NG2IT	NexGen2 Bolt Installation Tool	1		1		1	
FREIGHTOUT	Shipping & Handling Charge	1		1		1	

RECEIVED APR 20 2017

Freight	
Subtotal	
Sales Tax	
TOTAL	

Mail checks to:
Allfasteners USA
69 Orchard Street
Ramsey, NJ 07446

Allfasteners USA LLC retains title of goods sold until the goods have been paid for in full. Discrepancies must be reported within 7 days.
This invoice is subject to terms and conditions posted on http://www.allfasteners.com/terms.php.

CONSTRUCTION

6.2.1 CONSTRUCTION INSPECTIONS

AdvancedNetworkServices

8/11/17

Dan Vadney
Project Manager Structural Upgrades
Crown Castle
3 Corporate Park Drive
Suite 101
Clifton Park, NY 12065

Dan,

This letter is to inform you and CCI that all construction/structural modifications on the Crown Castle BU# 806374 HRT 081 site are complete.
ANS would also like to inform you that all work was performed in accordance with industry standards and contract documents including modification drawings and specifications, state and local regulations, OSHA, and Engineering Standards.

If you have any questions or concerns please contact me.

Sincerely,

Ron Brandon
Advanced Network Services
[P] 607-220-4007
[F] 518-292-6511
[E] Rbrandon@anscorporate.com

6.2.2 ON SITE COLD GALVANIZING VERIFICATION

AdvancedNetworkServices

8/11/17

Dan Vadney
Project Manager Structural Upgrades
Crown Castle
3 Corporate Park Drive
Suite 101
Clifton Park, NY 12065

Dan,

This letter is to inform you and CCI that all construction/structural modifications on the Crown Castle BU# 806374 HRT081 site are complete.
ANS would like to inform you that the cold galvanizing product ZRC Galvilite or AF1000 was used and applied as necessary. This product contains 95% Zinc by DFT and is approved in the Crown Castle Tower Protective Coating Guidelines ENG-STD-10149.

If you have any questions or concerns please contact me.

Sincerely,

Ron Brandon
Advanced Network Services
[P] 607-220-4007
[F] 518-292-6511
[E] Rbrandon@anscorporate.com

CROWN CASTLE
CrownCastle.com
For Lease Information
877-486-9377
For 24 Hour Service Questions or Emergencies Call
800-788-7011
Crown Castle Business Unit
806374
North Street
Rocky Hill, CT
HRT 081 943236
ALLFASTENERS
AF1000
BRUSH-ON
GALVANIZING COMPOUND
COMPUESTO GALVANIZANTE EN FRIO
COMPOSÉ GALVANISATION À FRIOD
1 QUART (32 US FL. OZ.) / 7.58 lbs

6.2.3 GC AND TEP REDLINE DRAWINGS

MODIFICATION OF AN EXISTING 140' MONOPOLE
VALMONT #10604-86
BU #806374; HRT 081 943236

NORTH STREET
ROCKY HILL, CONNECTICUT 06067
HARTFORD COUNTY
LAT: 41° 38' 23.2"; LONG: -72° 42' 6.83"
APP: 363221 REV. 9; WO: 1365734

PROJECT CONTACTS

STRUCTURE OWNER:
CROWN CASTLE
MOD PM: DAN VADNEY AT DAN.VADNEY@CROWNCASTLE.COM
PH: (518) 373-3510
MOD CM: JASON D'AMICO AT JASON.D'AMICO@CROWNCASTLE.COM
PH: (860) 209-0104

ENGINEER OF RECORD:
PJFMOD@PJFWEB.COM

THIS PROJECT INCLUDES THE FOLLOWING ITEMS
SHAFT REINFORCING
PAINT MODIFICATIONS TO MATCH EXISTING POLE

SHEET INDEX

SHEET NUMBER	DESCRIPTION
T-1	TITLE SHEET
T-2	MI CHECKLIST
S-1	GENERAL NOTES
S-2A	FORGBOLT™ DETAILS
S-2B	NEXGEN2™ BOLT DETAIL
S-2C	AJAX ONESIDE™ BOLT DETAIL
S-3	MONOPOLE PROFILE

WIND DESIGN DATA

REFERENCE STANDARD	ANSI/TIA-222-G-2-2009
LOCAL CODE	2016 CONNECTICUT BUILDING CODE
ULTIMATE WIND SPEED (3-SECOND GUST)	125 MPH
CONVERTED NOMINAL WIND SPEED (3-SECOND GUST)	97 MPH
ICE THICKNESS	1.0 IN
ICE WIND SPEED	50 MPH
SERVICE WIND SPEED	60 MPH
RISK CATEGORY	II
EXPOSURE CATEGORY	C
Kzt	1.0

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
X Built As Drawn
__ Contractor Red Line Verified
__ Discrepancies Noted

Built per Design
8/11/17
ANS
Ron Brandon

QUALIFIED ENGINEERING SERVICES ARE AVAILABLE FROM PAUL J. FORD & COMPANY TO ASSIST CONTRACTORS IN CLASS IV RIGGING PLAN REVIEWS. FOR REQUESTED QUALIFIED ENGINEERING SERVICES, PLEASE CONTACT RIGGING@PJFWEB.COM.

THE ASSOCIATED FAILING SA WO NUMBER FOR THIS PROJECT IS 1355829

ATTENTION ALL CONTRACTORS, ANYTIME YOU ACCESS A CROWN SITE FOR ANY REASON YOU ARE TO CALL THE CROWN NOC UPON ARRIVAL AND DEPARTURE, DAILY AT (800) 788-7011.

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8 PARKMEADOW DRIVE, PITTSFORD, NY 14534
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MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

PROJECT No:	37517-0519.002.7700
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKR
DATE:	2-27-2017

TITLE SHEET

T-1

MODIFICATION INSPECTION NOTES:

1. **GENERAL**
   1.1. THE MODIFICATION INSPECTION (MI) IS A VISUAL INSPECTION OF TOWER MODIFICATIONS AND A REVIEW OF CONSTRUCTION INSPECTIONS AND OTHER REPORTS TO ENSURE THE INSTALLATION WAS CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, NAMELY THE MODIFICATION DRAWINGS, AS DESIGNED BY THE EOR.
   1.2. THE MI IS TO CONFIRM INSTALLATION CONFIGURATION AND WORKMANSHIP ONLY AND IS NOT A REVIEW OF THE MODIFICATION DESIGN ITSELF, NOR DOES THE MI INSPECTOR TAKE OWNERSHIP OF THE MODIFICATION DESIGN OWNERSHIP OF THE STRUCTURAL MODIFICATION DESIGN EFFECTIVENESS AND INTEGRITY RESIDES WITH THE EOR AT ALL TIMES.
   1.3. ALL MI'S SHALL BE CONDUCTED BY A CROWN CASTLE ENGINEERING VENDOR (AEV) OR ENGINEERING SERVICE VENDOR (AESV) THAT IS APPROVED TO PERFORM ELEVATED WORK FOR CROWN CASTLE.
   1.4. TO ENSURE THAT THE REQUIREMENTS OF THE MI ARE MET, IT IS VITAL THAT THE GENERAL CONTRACTOR (GC) AND THE MI INSPECTOR BEGIN COMMUNICATING AND COORDINATING AS SOON AS A PO IS RECEIVED. IT IS EXPECTED THAT EACH PARTY WILL BE PROACTIVE IN REACHING OUT TO THE OTHER PARTY. IF CONTACT INFORMATION IS NOT KNOWN, CONTACT YOUR CROWN CASTLE POINT OF CONTACT (POC).
   1.5. REFER TO ENG-SOW-10007: MODIFICATION INSPECTION SOW FOR FURTHER DETAILS AND REQUIREMENTS.

2. **MI INSPECTOR**
   2.1. THE MI INSPECTOR IS REQUIRED TO CONTACT THE GC AS SOON AS RECEIVING A PO FOR THE MI TO, AT A MINIMUM:
      2.1.1. REVIEW THE REQUIREMENTS OF THE MI CHECKLIST.
      2.1.2. WORK WITH THE GC TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE INSPECTIONS, INCLUDING FOUNDATION INSPECTIONS.
      2.1.3. THE MI INSPECTOR IS RESPONSIBLE FOR COLLECTING ALL GC INSPECTION AND TEST REPORTS, REVIEWING THE DOCUMENTS FOR ADHERENCE TO THE CONTRACT DOCUMENTS, CONDUCTING THE IN-FIELD INSPECTIONS, AND SUBMITTING THE MI REPORT TO CROWN CASTLE.

3. **GENERAL CONTRACTOR**
   3.1. THE GC IS REQUIRED TO CONTACT THE MI INSPECTOR AS SOON AS RECEIVING A PO FOR THE MODIFICATION INSTALLATION OR TURNKEY PROJECT TO, AT A MINIMUM:
      3.1.1. REVIEW THE REQUIREMENTS OF THE MI CHECKLIST.
      3.1.2. WORK WITH THE MI INSPECTOR TO DEVELOP A SCHEDULE TO CONDUCT ON-SITE INSPECTIONS, INCLUDING FOUNDATION INSPECTIONS.
      3.1.3. BETTER UNDERSTAND ALL INSPECTION AND TESTING REQUIREMENTS.
      3.1.4. THE GC SHALL PERFORM AND RECORD THE TEST AND INSPECTION RESULTS IN ACCORDANCE WITH THE REQUIREMENTS OF THE MI CHECKLIST AND ENG-SOW-10007.

4. **RECOMMENDATIONS**
   4.1. THE FOLLOWING RECOMMENDATIONS AND SUGGESTIONS ARE OFFERED TO ENHANCE THE EFFICIENCY AND EFFECTIVENESS OF DELIVERING AN MI REPORT:
      4.1.1. IT IS SUGGESTED THAT THE GC PROVIDE A MINIMUM OF 5 BUSINESS DAYS NOTICE, PREFERABLE 10, TO THE MI INSPECTOR AS TO WHEN THE SITE WILL BE READY FOR THE MI TO BE CONDUCTED.
      4.1.2. THE GC AND MI INSPECTOR COORDINATE CLOSELY THROUGHOUT THE ENTIRE PROJECT.
      4.1.3. WHEN POSSIBLE, IT IS PREFERRED TO HAVE THE GC AND MI INSPECTOR ON-SITE SIMULTANEOUSLY FOR ANY GUY WIRE TENSIONING OR RE-TENSIONING OPERATIONS.
      4.1.4. IT MAY BE BENEFICIAL TO INSTALL ALL TOWER MODIFICATIONS PRIOR TO CONDUCTING THE FOUNDATION INSPECTIONS TO ALLOW FOUNDATION AND MI INSPECTION(S) TO COMMENCE WITH ONE SITE VISIT.
      4.1.5. WHEN POSSIBLE, IT IS PREFERRED TO HAVE THE GC AND MI INSPECTOR ON-SITE DURING THE MI TO HAVE ANY DEFICIENCIES CORRECTED DURING THE INITIAL MI. THEREFORE, THE GC MAY CHOOSE TO COORDINATE THE MI CAREFULLY TO ENSURE ALL CONSTRUCTION FACILITIES ARE AT THEIR DISPOSAL WHEN THE MI INSPECTOR IS ON SITE.

5. **CANCELLATION OR DELAYS IN SCHEDULED MI**
   5.1. IF THE GC AND MI INSPECTOR AGREE TO A DATE ON WHICH THE MI WILL BE CONDUCTED, AND EITHER PARTY CANCELS OR DELAYS, CROWN CASTLE SHALL NOT BE RESPONSIBLE FOR ANY COSTS, FEES, LOSS OF DEPOSITS AND/OR OTHER PENALTIES RELATED TO THE CANCELLATION OR DELAY INCURRED BY EITHER PARTY FOR ANY TIME (E.G. TRAVEL AND LODGING, COSTS OF KEEPING EQUIPMENT ON-SITE, ETC.). IF CROWN CASTLE CONTRACTS DIRECTLY FOR A THIRD PARTY MI, EXCEPTIONS MAY BE MADE IN THE EVENT THAT THE DELAY/CANCELLATION IS CAUSED BY WEATHER OR OTHER CONDITIONS THAT MAY COMPROMISE THE SAFETY OF THE PARTIES INVOLVED.

6. **CORRECTION OF FAILING MI'S**
   6.1. IF THE MODIFICATION INSTALLATION WOULD FAIL THE MI ("FAILED MI"), THE GC SHALL WORK WITH CROWN CASTLE TO COORDINATE A REMEDIATION PLAN IN ONE OF TWO WAYS:
      6.1.1. CORRECT FAILING ISSUES TO COMPLY WITH THE SPECIFICATIONS CONTAINED IN THE ORIGINAL CONTRACT DOCUMENTS AND COORDINATE A SUPPLEMENT MI.
      6.1.2. OR, WITH CROWN CASTLE'S APPROVAL, THE GC MAY WORK WITH THE EOR TO RE-ANALYZE THE MODIFICATION/REINFORCEMENT USING THE AS-BUILT CONDITION.

7. **MI VERIFICATION INSPECTIONS**
   7.1. CROWN CASTLE RESERVES THE RIGHT TO CONDUCT A MI VERIFICATION INSPECTION TO VERIFY THE ACCURACY AND COMPLETENESS OF PREVIOUSLY COMPLETED MI INSPECTION(S) ON TOWER MODIFICATION PROJECTS.
   7.2. ALL VERIFICATION INSPECTIONS SHALL BE HELD TO THE SAME SPECIFICATIONS AND REQUIREMENTS IN THE CONTRACT DOCUMENTS AND IN ACCORDANCE WITH ENG-SOW-10007.
   7.3. VERIFICATION INSPECTION MAY BE CONDUCTED BY AN INDEPENDENT AEV/AESV FIRM AFTER A MODIFICATION PROJECT IS COMPLETED, AS MARKED BY THE DATE OF AN ACCEPTED "PASSING MI" OR "PASS AS NOTED MI" REPORT FOR THE ORIGINAL PROJECT.

8. **PHOTOGRAPHS**
   8.1. BETWEEN THE GC AND THE MI INSPECTOR THE FOLLOWING PHOTOGRAPHS, AT A MINIMUM, ARE TO BE TAKEN AND INCLUDED IN THE MI REPORT:
      8.1.1. PRECONSTRUCTION GENERAL SITE CONDITION
      8.1.2. PHOTOGRAPHS DURING THE REINFORCEMENT MODIFICATION CONSTRUCTION/ERECTION AND INSPECTION
      8.1.3. RAW MATERIALS
      8.1.4. PHOTOS OF ALL CRITICAL DETAILS
      8.1.5. FOUNDATION MODIFICATIONS
      8.1.6. WELD PREPARATION
      8.1.7. BOLT INSTALLATION AND TORQUE
      8.1.8. FINAL INSTALLED CONDITION
      8.1.9. SURFACE COATING REPAIR
      8.1.10. POST CONSTRUCTION PHOTOGRAPHS
      8.1.11. FINAL INFIELD CONDITION
      8.1.12. PHOTOS OF ELEVATED MODIFICATIONS TAKEN FROM THE GROUND SHALL BE CONSIDERED INADEQUATE.
      8.1.13. THIS IS NOT A COMPLETE LIST OF REQUIRED PHOTOS, PLEASE REFER TO ENG-SOW-10007.

9. **INSPECTION AND TESTING**
   9.1. ALL WORK SHALL BE SUBJECT TO REVIEW AND OBSERVATION BY CROWN CASTLE'S REPRESENTATIVE AND CROWN CASTLE'S AUTHORIZED INDEPENDENT INSPECTION AND TESTING AGENCY.
   9.2. INSPECTION SERVICES WHICH ARE FURNISHED BY OTHERS ARE STILL REQUIRED WHEN THE EOR PERFORMS SUPPORT SERVICES DURING CONSTRUCTION.
   9.3. OBSERVED DISCREPANCIES BETWEEN THE WORK AND THE CONTRACT DOCUMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT NO ADDITIONAL COST.
   9.4. AN INDEPENDENT QUALIFIED INSPECTION/TESTING AGENCY SHALL BE SELECTED AND RETAINED FOR THE SOLE PURPOSE OF INSPECTING, TESTING, DOCUMENTING, AND APPROVING ALL WELDING AND FIELD WORK PERFORMED BY THE CONTRACTOR.
      9.4.1. ACCESS TO ANY PLACE WHERE WORK IS BEING DONE SHALL BE PERMITTED AT ALL TIMES.
      9.4.2. THE INSPECTION AGENCY SHALL SO SCHEDULE THIS WORK AS TO CAUSE A MINIMUM OF INTERRUPTION TO, AND COORDINATE WITH, THE WORK IN PROGRESS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE WORK SCHEDULE WITH THE TESTING AGENCY. THE CONTRACTOR SHALL ALLOW FOR ADEQUATE TIME AND ACCESS FOR THE TESTING AGENCY TO PERFORM THEIR DUTIES.
   9.5. THE INSPECTION AND TESTING AGENCY SHALL BE RESPONSIBLE TO PERFORM THE FOLLOWING SERVICES AND INSPECT THE FOLLOWING ITEMS IN ACCORDANCE WITH THE CONSTRUCTION DRAWINGS. THE TESTING AGENCY SHALL INSPECT ITEMS ON THIS LIST AND OTHER ITEMS AS NECESSARY TO FULFILL THEIR RESPONSIBILITY. THE TESTING AGENCY SHALL UTILIZE EXPERIENCED, TRAINED INSPECTORS INCLUDING AWS CERTIFIED WELDING INSPECTORS (CWI). INSPECTORS SHALL HAVE THE TRAINING, CREDENTIALS, AND EXPERIENCE APPROPRIATE FOR AND COMMENSURATE WITH THE SCOPE AND TYPE OF INSPECTION WORK TO BE PERFORMED.
   9.6. GENERAL
      9.6.1. PERFORM PERIODIC ON-SITE OBSERVATION, INSPECTION, VERIFICATION, AND TESTING DURING THE TIME THE CONTRACTOR IS WORKING ON-SITE. AGENCY SHALL NOTIFY CROWN CASTLE AND THE EOR IMMEDIATELY WHEN FIELD PROBLEMS OR DISCREPANCIES OCCUR.
   9.7. FOUNDATIONS AND SOIL PREPARATION - (NOT REQUIRED)
   9.8. CONCRETE TESTING PER ACI - (NOT REQUIRED)
   9.9. STRUCTURAL STEEL
      9.9.1. CHECK STEEL ON THE JOB WITH THE PLANS.
      9.9.2. CHECK MILL CERTIFICATIONS. CALL FOR LABORATORY TEST REPORTS WHEN MILL CERTIFICATION IS IN QUESTION.
      9.9.3. CHECK GRADE OF STEEL, MEMBERS, AND BOLTS FOR CONFORMANCE WITH DRAWINGS.
      9.9.4. INSPECT ALL STRUCTURAL BOLTS SHALL BE FIELD INSPECTED ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009.
      9.9.5. INSPECT STEEL MEMBERS FOR DISTORTION, EXCESSIVE RUST, FLAWS AND BURNED HOLES.
      9.9.6. CHECK STEEL MEMBERS FOR SIZES, SWEEP AND DIMENSIONAL TOLERANCES.
      9.9.7. CHECK FOR SURFACE FINISH SPECIFIED, GALVANIZED.
      9.9.8. CHECK THAT BOLTS HAVE BEEN TIGHTENED PROPERLY.
      9.9.9. PRIOR TO ANY FIELD CUTTING THE CONTRACTOR SHALL MARK THE CUTOUT LINES ON THE STEEL AND THE INSPECTION/TESTING AGENCY SHALL VERIFY PROPOSED LAYOUT, LOCATION, AND DIMENSIONS. THE INSPECTION/TESTING AGENCY SHALL CLOSELY AND CONTINUOUSLY MONITOR THIS ACTIVITY.
   9.10. WELDING:
      9.10.1. VERIFY FIELD WELDING PROCEDURES, WELDERS, AND WELDING OPERATORS, NOT DEEMED PREQUALIFIED, IN ACCORDANCE WITH AWS D1.1.
      9.10.2. INSPECT FIELD WELDED CONNECTIONS IN ACCORDANCE WITH THE REQUIREMENTS SPECIFIED AND WITH AWS D1.1.
      9.10.3. APPROVE FIELD WELDING SEQUENCE.
      9.10.4. A PROGRAM OF THE APPROVED SEQUENCES SHALL BE SUBMITTED TO CROWN CASTLE BEFORE WELDING BEGINS. NO CHANGE IN APPROVED SEQUENCES MAY BE MADE WITHOUT PERMISSION FROM CROWN CASTLE.
      9.10.5. INSPECT WELDED CONNECTIONS AS FOLLOWS AND IN ACCORDANCE WITH AWS D1.1:
         9.10.5.1. INSPECT WELDING EQUIPMENT FOR CAPACITY, MAINTENANCE, AND WORKING CONDITIONS.
         9.10.5.2. VERIFY SPECIFIED ELECTRODES AND HANDLING AND STORAGE OF ELECTRODES FOR CONFORMANCE TO SPECIFICATIONS.
         9.10.5.3. INSPECT PREHEATING AND INTERPASS TEMPERATURES FOR CONFORMANCE WITH AWS D1.1.
         9.10.5.4. VISUALLY INSPECT ALL WELDS AND VERIFY THAT QUALITY OF WELDS MEETS THE REQUIREMENTS OF AWS D1.1. OTHER TESTS MAY ALSO BE PERFORMED ON THE WELDS BY THE TESTING AGENCY IN ORDER FOR THEM TO PERFORM THEIR DUTIES FOR THIS PROJECT.
         9.10.5.5. SPOT TEST AT LEAST ONE FILLET WELD OF EACH MEMBER USING MAGNETIC PARTICLE.
         9.10.5.6. INSPECT FOR SIZE, SPACING, TYPE AND LOCATION AS PER APPROVED DRAWINGS.
         9.10.5.7. VERIFY THAT THE BASE METAL CONFORMS TO THE DRAWINGS.
         9.10.5.8. REVIEW THE REPORTS BY TESTING LABS.
         9.10.5.9. CHECK TO SEE THAT WELDS ARE CLEAN AND FREE FROM SLAG.
         9.10.5.10. INSPECT RUST PROTECTION OF WELDS AS PER SPECIFICATIONS.
         9.10.5.11. CHECK THAT DEFECTIVE WELDS ARE CLEARLY MARKED AND HAVE BEEN ADEQUATELY REPAIRED.
   9.11. REPORTS:
      9.11.1. COMPILE AND PERIODICALLY SUBMIT DAILY INSPECTION REPORTS TO CROWN CASTLE.
      9.11.2. THE INSPECTION PLAN OUTLINED HEREIN IS INTENDED AS A DESCRIPTION OF GENERAL AND SPECIFIC ITEMS OF CONCERN. IT IS NOT INTENDED TO BE ALL-INCLUSIVE. IT DOES NOT LIMIT THE TESTING AND INSPECTION AGENCY TO THE ITEMS LISTED. ADDITIONAL TESTING, INSPECTION, AND CHECKING MAY BE REQUIRED AND SHOULD BE ANTICIPATED. THE TESTING AGENCY SHALL USE THEIR PROFESSIONAL JUDGMENT AND KNOWLEDGE OF THE JOB SITE CONDITIONS AND THE CONTRACTOR'S PERFORMANCE TO DECIDE WHAT OTHER ITEMS REQUIRE ADDITIONAL ATTENTION. THE TESTING AGENCY'S JUDGMENT MUST PREVAIL ON ITEMS NOT SPECIFICALLY COVERED. ANY DISCREPANCIES OR PROBLEMS SHALL BE BROUGHT IMMEDIATELY TO CROWN CASTLE'S ATTENTION. RESOLUTIONS ARE NOT TO BE MADE WITHOUT CROWN CASTLE'S REVIEW AND SPECIFIC WRITTEN CONSENT. CROWN CASTLE RESERVES THE RIGHT TO DETERMINE WHETHER OR NOT A RESOLUTION IS ACCEPTABLE.
      9.11.3. AFTER EACH INSPECTION, THE TESTING AGENCY WILL PREPARE A WRITTEN ACCEPTANCE OR REJECTION WHICH WILL BE GIVEN TO THE CONTRACTOR AND FILED AS DAILY REPORTS TO CROWN CASTLE. THIS WRITTEN ACTION WILL GIVE THE CONTRACTOR A LIST OF ITEMS TO BE CORRECTED, PRIOR TO CONTINUING CONSTRUCTION, AND/OR LOADING OF STRUCTURAL ITEMS.
      9.11.4. THE TESTING AGENCY DOES NOT RELIEVE THE CONTRACTOR'S CONTRACTUAL OR STATUTORY OBLIGATIONS. THE CONTRACTOR HAS THE SOLE RESPONSIBILITY FOR ANY DEVIATIONS FROM THE OFFICIAL CONTRACT DOCUMENTS. THE TESTING AGENCY WILL NOT REPLACE THE CONTRACTOR'S QUALITY CONTROL PERSONNEL.

MI CHECKLIST

CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY EOR)	REPORT ITEM
	PRE-CONSTRUCTION
X	MI CHECKLIST DRAWINGS
X	EOR REVIEW
X	FABRICATION INSPECTION
NA	FABRICATOR CERTIFIED WELD INSPECTION
X	MATERIAL TEST REPORT (MTR)
NA	FABRICATOR NDE INSPECTION
NA	NDE REPORT OF MONOPOLE BASE PLATE (AS REQUIRED)
X	PACKING SLIPS
ADDITIONAL TESTING AND INSPECTIONS:	
	CONSTRUCTION
X	CONSTRUCTION INSPECTIONS
NA	FOUNDATION INSPECTIONS
NA	CONCRETE COMP. STRENGTH AND SLUMP TESTS
NA	POST INSTALLED ANCHOR ROD VERIFICATION
NA	BASE PLATE GROUT VERIFICATION
NA	CONTRACTOR'S CERTIFIED WELD INSPECTION
NA	EARTHWORK: PROVIDE PHOTO DOCUMENTATION OF EXCAVATION QUALITY AND COMPACTION
X	ON SITE COLD GALVANIZING VERIFICATION
NA	GUY WIRE TENSION REPORT
X	GC AS-BUILT DOCUMENTS
NA	MICROPILE/ROCK ANCHOR INSTALLER'S DRILLING AND INSTALLATION LOGS AND QA/QC DOCUMENTS
ADDITIONAL TESTING AND INSPECTIONS:	
	POST-CONSTRUCTION
X	MI INSPECTOR REDLINE OR RECORD DRAWING(S)
NA	POST INSTALLED ANCHOR ROD TARGET TENSION LOAD TESTING
NA	REFER TO MICROPILE/ROCK ANCHOR NOTES FOR SPECIAL INSPECTION AND TESTING REQUIREMENTS.
X	PHOTOGRAPHS
ADDITIONAL TESTING AND INSPECTIONS:	

NOTE: X DENOTES A DOCUMENT NEEDED FOR THE PMI REPORT
NA DENOTES A DOCUMENT THAT IS NOT REQUIRED FOR THE PMI REPORT

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
X Built As Drawn
__ Contractor Red Line Verified
__ Discrepancies Noted

Built per Design
8/11/17
ANS
Ron Brandon

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CROWN CASTLE
6 PARKMEADOW DRIVE, PITTSFORD, NY 14534
PH: (585) 899-3445

MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

PROJECT No:	37517-0519.002.7700
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

MI CHECKLIST

T-2

MONOPOLE RETROFIT PROJECT MASTER NOTES DOCUMENT (REV. 3, 02/05/2015)

1. **GENERAL NOTES**

1.1. THE MONOPOLE STRUCTURE IN ITS EXISTING CONDITION DOES NOT HAVE THE STRUCTURAL CAPACITY TO CARRY ALL OF THE PROPOSED AND EXISTING LOADS FROM THE ATTACHED STRUCTURAL MODIFICATION REPORT AT THE REQUIRED MINIMUM WIND SPEEDS. DO NOT INSTALL ANY NEW LOADS UNTIL THE MONOPOLE REINFORCING SYSTEM IS COMPLETELY AND SUCCESSFULLY INSTALLED.

1.2. THESE DRAWINGS WERE PREPARED FROM INFORMATION PROVIDED BY CROWN CASTLE. THE INFORMATION PROVIDED HAS NOT BEEN FIELD VERIFIED BY THE ENGINEER OF RECORD (EOR) FOR ACCURACY AND THEREFORE DISCREPANCIES BETWEEN THESE DRAWINGS AND ACTUAL SITE CONDITIONS SHOULD BE ANTICIPATED. PRIOR TO FABRICATION AND INSTALLATION CONTRACTOR SHALL VERIFY ALL LENGTHS AND QUANTITIES GIVEN. LENGTH AND QUANTITIES PROVIDED ARE FOR QUOTING PURPOSES ONLY AND SHALL NOT BE USED FOR FABRICATION. THE CONTRACTOR SHALL COORDINATE WITH THE PROJECT DRAWINGS AND THEIR FIELD VERIFIED CONDITIONS AND DIMENSIONS BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL IMMEDIATELY REPORT ANY AND ALL DISCREPANCIES TO THE EOR AND CROWN CASTLE BEFORE PROCEEDING WITH THE WORK. ANY WORK PERFORMED WITHOUT A PREFABRICATION MAPPING IS DONE AT THE RISK OF THE GENERAL CONTRACTOR AND/OR THE FABRICATOR

1.3. IF MATERIALS, QUANTITIES, STRENGTHS OR SIZES INDICATED BY THE DRAWINGS OR SPECIFICATIONS ARE NOT IN AGREEMENT WITH THESE NOTES, THE BETTER QUALITY AND/OR GREATER QUANTITY, STRENGTH OR SIZE INDICATED, SPECIFIED OR NOTED SHALL BE PROVIDED.

1.4. THIS STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE INSTALLATION OF THE REINFORCING REPAIR SYSTEM HAS BEEN SUCCESSFULLY COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO ENSURE THE SAFETY AND STABILITY OF THE MONOPOLE AND ITS COMPONENT PARTS DURING FIELD MODIFICATIONS. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS OR TIE DOWNS THAT MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER THE COMPLETION OF THE PROJECT.

1.5. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN AND SHALL MEET ANSI/TIA-1019 (LATEST EDITION), OSHA AND GENERAL INDUSTRY STANDARDS. ALL RIGGING PLANS SHALL ADHERE TO ANSI/TIA-1019 (LATEST EDITION) INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION.

1.6. OBSERVATION VISITS TO THE SITE BY CROWN CASTLE AND/OR THE EOR SHALL NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES OR THE CONSTRUCTION PROCEDURES. ANY SUPPORT SERVICES PERFORMED BY THE EOR DURING CONSTRUCTION ARE SOLELY FOR THE PURPOSE OF ACHIEVING GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS. THEY DO NOT GUARANTEE THE CONTRACTOR'S PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

1.7. ALL MATERIALS AND EQUIPMENT FURNISHED SHALL BE NEW AND OF GOOD QUALITY, FREE FROM FAULTS AND DEFECTS AND IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. ANY AND ALL SUBSTITUTIONS MUST BE PROPERLY APPROVED AND AUTHORIZED IN WRITING BY CROWN CASTLE AND EOR PRIOR TO INSTALLATION. THE CONTRACTOR SHALL FURNISH SATISFACTORY EVIDENCE AS TO THE KIND AND QUALITY OF MATERIALS AND EQUIPMENT BEING SUBSTITUTED.

1.8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INITIATING, MAINTAINING, AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. THE CONTRACTOR IS RESPONSIBLE TO ENSURE THAT THIS PROJECT AND RELATED WORK COMPLIES WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL SAFETY CODES AND REGULATIONS GOVERNING THIS WORK AS WELL AS CROWN CASTLE SAFETY GUIDELINES.

1.9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL EXISTING AND NEW COAXIAL CABLES AND OTHER EQUIPMENT DURING CONSTRUCTION.

1.10. ANY EXISTING ATTACHMENTS AND/OR PROJECTIONS ON THE POLE THAT MAY INTERFERE WITH THE INSTALLATION OF THE REINFORCING SYSTEM WILL HAVE TO BE REMOVED AND RELOCATED, REPLACED, OR RE-INSTALLED AS REQUIRED AFTER THE REINFORCING IS SUCCESSFULLY COMPLETED. THE CONTRACTOR SHALL IDENTIFY AND COORDINATE THESE ITEMS PRIOR TO CONSTRUCTION WITH CROWN CASTLE, TESTING AGENCY, AND EOR.

1.11. ANY AND ALL EXISTING PLATFORMS THAT ARE LOCATED IN AREAS OF THE POLE SHAFT WHERE SHAFT REINFORCING MUST BE APPLIED SHALL BE TEMPORARILY REMOVED OR OTHERWISE SUPPORTED TO PERMIT NEW CONTINUOUS REINFORCEMENT TO BE ATTACHED. AFTER THE CONTRACTOR HAS SUCCESSFULLY INSTALLED THE MONOPOLE REINFORCEMENT SYSTEM, THE CONTRACTOR SHALL RE-INSTALL THE PLATFORMS.

1.12. THE CLIMBING FACILITIES, SAFETY CLIMB AND ALL PARTS THEREOF SHALL NOT BE IMPEDED, MODIFIED OR ALTERED WITHOUT THE EXPRESS APPROVAL OF THE EOR.

1.13. FOR STANDARD CROWN PARTS SEE THE MOST RECENT VERSION OF THE "CCI APPROVED REINFORCEMENT COMPONENTS" CATALOG.

1.14. ALL SOLUTIONS FOR THE REPLACEMENT, RELOCATION OR MODIFICATION OF THE SAFETY CLIMB AND/OR ANY OF THE MONOPOLE CLIMBING FACILITIES SHALL BE COORDINATED WITH TUF-TUG PRODUCTS. CONTACT DETAILS:
3434 ENCRETE LANE, MORAINE, OHIO 45439
PHONE: 937-299-1213 EMAIL: TUFTUG@AOL.COM

2. **STRUCTURAL STEEL**

2.1. STRUCTURAL STEEL MATERIALS, FABRICATION, DETAILING, AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING REFERENCE STANDARDS:

2.1.1. BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC):

2.1.1.1. "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS."

2.1.1.2. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM HIGH STRENGTH BOLTS," AS APPROVED BY THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS.

2.1.1.3. "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES"

2.1.2. BY THE AMERICAN WELDING SOCIETY (AWS):

2.1.2.1. "STRUCTURAL WELDING CODE – STEEL D1.1."

2.1.2.2. "STANDARD SYMBOLS FOR WELDING, BRAZING, AND NONDESTRUCTIVE EXAMINATION"

2.2. ALL STRUCTURAL BOLTS SHALL BE INSTALLED AND TIGHTENED TO THE PRETENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM HIGH STRENGTH BOLTS', DEC. 31, 2009.

2.3. ANY MATERIAL OR WORKMANSHIP WHICH IS OBSERVED TO BE DEFECTIVE OR INCONSISTENT WITH THE CONTRACT DOCUMENTS SHALL BE CORRECTED, MODIFIED, OR REPLACED AT THE CONTRACTOR'S EXPENSE.

2.4. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY, AWS D1.1. ALL WELD ELECTRODES SHALL BE E80XX UNLESS NOTED OTHERWISE ON THE DRAWINGS.

2.5. ALL WELDED CONNECTIONS SHALL BE MADE BY WELDERS CERTIFIED BY AWS. CONTRACTOR SHALL SUBMIT WELDERS' CERTIFICATION AND QUALIFICATION DOCUMENTATION TO CROWN CASTLE'S TESTING AGENCY FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

2.6. STRUCTURAL STEEL PLATES SHALL CONFORM TO ASTM A572 GRADE 65(FY = 65 KSI MIN) UNLESS NOTED OTHERWISE ON THE DRAWINGS.

2.7. SURFACES OF EXISTING STEEL SHALL BE PREPARED AS REQUIRED FOR FIELD WELDING PER AWS. SEE SECTION I NOTES REGARDING TOUCH UP OF GALVANIZED SURFACES DAMAGED DURING TRANSPORTATION OR ERECTION AND ASSEMBLY AS WELL AS FIELD WELDING.

2.8. NO WELDING SHALL BE DONE TO THE EXISTING STRUCTURE WITHOUT THE PRIOR APPROVAL AND SUPERVISION OF THE TESTING AGENCY.

2.9. FIELD CUTTING OF STEEL:

2.9.1. IMPORTANT CUTTING AND WELDING SAFETY GUIDELINES: THE CONTRACTOR SHALL FOLLOW ALL CROWN CASTLE CUTTING, WELDING, FIRE PREVENTION AND SAFETY GUIDELINES. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL OBTAIN A COPY OF THE CURRENT CROWN CASTLE GUIDELINES. PER THE 12-01-2005 CROWN CASTLE DIRECTIVE: "ALL CUTTING AND WELDING ACTIVITIES SHALL BE CONDUCTED IN ACCORDANCE WITH CROWN CASTLE POLICY 'CUTTING AND WELDING SAFETY PLAN' (DOC # ENG-PLN-10015) ON AN ONGOING BASIS THROUGHOUT THE ENTIRE LIFE OF THE PROJECT". ANY DAMAGE TO THE COAX CABLES, AND/OR OTHER EQUIPMENT AND/OR THE STRUCTURE, RESULTING FROM THE CONTRACTOR'S ACTIVITIES SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE. THE INSPECTION/TESTING AGENCY SHALL CLOSELY AND CONTINUOUSLY MONITOR THIS ACTIVITY.

2.9.2. ALL REQUIRED CUTS SHALL BE CUT WITHIN THE DIMENSIONS SHOWN ON THE DRAWINGS. NO CUTS SHALL EXTEND BEYOND THE OUTLINE OF THE DIMENSIONS SHOWN ON THE DRAWINGS. ALL CUT EDGES SHALL BE GROUND SMOOTH AND DE-BURRED. CUT EDGES THAT ARE TO BE FIELD WELDED SHALL BE PREPARED FOR FIELD WELDING PER AWS D1.1 AND AS SHOWN ON THE DRAWINGS. CONTRACTOR TO AVOID 90 DEGREE CORNERS. IT MAY BE NECESSARY TO DRILL STARTER HOLES AS REQUIRED TO MAKE THE CUTS.

3. **BASE PLATE GROUT - (NOT REQUIRED)**

4. **FOUNDATION WORK - (NOT REQUIRED)**

5. **CAST-IN-PLACE CONCRETE - (NOT REQUIRED)**

6. **EPOXY GROUTED REINFORCING ANCHOR RODS - (NOT REQUIRED)**

7. **TOUCH UP OF GALVANIZING**

7.1. THE CONTRACTOR SHALL TOUCH UP ANY AND ALL AREAS OF GALVANIZING ON THE EXISTING STRUCTURE OR NEW COMPONENTS THAT ARE DAMAGED OR ABRADED DURING CONSTRUCTION. GALVANIZED SURFACES DAMAGED DURING TRANSPORTATION OR ERECTION AND ASSEMBLY AS WELL AS ANY AND ALL ABRASIONS, CUTS, FIELD DRILLING, AND ALL FIELD WELDING SHALL BE TOUCHED UP WITH TWO (2) COATS OF ZRC COLD GALVANIZING COMPOUND. FILM THICKNESS PER COAT SHALL BE: WET 3.0 MILS; DRY 1.5 MILS. APPLY PER ZRC (MANUFACTURER) RECOMMENDED PROCEDURES. CONTACT ZRC AT 1-800-831-3275 FOR PRODUCT INFORMATION.

7.2. CONTRACTOR SHALL CLEAN AND PREPARE ALL FIELD WELDS ON GALVANIZED AND PRIME PAINTED SURFACES FOR TOUCH-UP COATING IN ACCORDANCE WITH AWS D1.1. CROWN CASTLE'S TESTING AGENCY SHALL VERIFY THE PREPARED SURFACE PRIOR TO APPLICATION OF THE TOUCH-UP COATING.

7.3. CROWN CASTLE'S TESTING AGENCY SHALL TEST AND VERIFY THE COATING THICKNESS AFTER THE CONTRACTOR HAS APPLIED THE ZRC COLD GALVANIZING COMPOUND AND IT HAS SUFFICIENTLY DRIED. AREAS FOUND TO BE ADEQUATELY COATED, SHALL BE RE-COATED BY THE CONTRACTOR AND RE-TESTED BY THE TESTING AGENCY.

8. **HOT-DIP GALVANIZING**

8.1. HOT-DIP GALVANIZE ALL STRUCTURAL STEEL MEMBERS AND ALL STEEL ACCESSORIES, BOLTS, WASHERS, ETC. PER ASTM A123 OR PER ASTM A153, AS APPROPRIATE.

8.2. PROPERLY PREPARE STEEL ITEMS FOR GALVANIZING. DRILL OR PUNCH WEEP AND/OR DRAINAGE HOLES WITH EOR APPROVAL OF LOCATIONS.

8.3. ALL GALVANIZING SHALL BE DONE AFTER FABRICATION IS COMPLETED AND PRIOR TO FIELD INSTALLATION.

9. **PERPETUAL INSPECTION AND MAINTENANCE BY THE OWNER**

9.1. AFTER THE CONTRACTOR HAS SUCCESSFULLY COMPLETED THE INSTALLATION OF THE MONOPOLE REINFORCING SYSTEM AND THE WORK HAS BEEN ACCEPTED BY CROWN CASTLE, CROWN CASTLE WILL BE RESPONSIBLE FOR THE LONG TERM AND PERPETUAL INSPECTION AND MAINTENANCE OF THE POLE AND REINFORCING SYSTEM.

9.2. ANY FIELD WELDED CONNECTIONS ARE SUBJECT TO CORROSION DAMAGE AND DETERIORATION IF THEY ARE NOT PROPERLY MAINTAINED AND COVERED WITH CORROSION PREVENTIVE COATING SUCH AS THE ZRC GALVANIZING COMPOUND SPECIFIED PREVIOUSLY. THE STRUCTURAL LOAD CARRYING CAPACITY OF THE REINFORCED POLE SYSTEM IS DEPENDENT UPON THE INSTALLED SIZE AND QUALITY, MAINTAINED SOUND CONDITION AND STRENGTH OF THESE FIELD WELDED CONNECTIONS. ANY CORROSION OF, DAMAGE TO, FATIGUE, FRACTURE, AND/OR DETERIORATION OF THESE WELDS AND/OR THE EXISTING GALVANIZED STEEL POLE STRUCTURE AND THE WELDED COMPONENTS WILL RESULT IN THE LOSS OF STRUCTURAL LOAD CARRYING CAPACITY AND MAY LEAD TO FAILURE OF THE STRUCTURAL SYSTEM. THEREFORE, IT IS IMPERATIVE THAT CROWN CASTLE REGULARLY INSPECTS, MAINTAINS, AND REPAIRS AS NECESSARY, ALL OF THESE WELDS, CONNECTIONS, AND COMPONENTS FOR THE LIFE OF THE STRUCTURE.

9.3. CROWN CASTLE SHALL REFER TO ANSI/TIA-222-G-2-2009, SECTION 14 AND ANNEX J FOR RECOMMENDATIONS FOR MAINTENANCE AND INSPECTION. THE FREQUENCY OF THE INSPECTION AND MAINTENANCE INTERVALS IS TO BE DETERMINED BY CROWN CASTLE BASED UPON ACTUAL SITE AND ENVIRONMENTAL CONDITIONS. THE EOR RECOMMENDS THAT A COMPLETE AND THOROUGH INSPECTION OF THE ENTIRE REINFORCED MONOPOLE STRUCTURAL SYSTEM BE PERFORMED YEARLY AND/OR AS FREQUENTLY AS CONDITIONS WARRANT. ACCORDING TO ANSI/TIA-222-G-2-2009 SECTION 14.2: "IT IS RECOMMENDED THAT THE STRUCTURE BE INSPECTED AFTER SEVERE WIND AND/OR ICE STORMS OR OTHER EXTREME LOADING CONDITIONS".

10. **FIELD NDE MINIMUM REQUIREMENTS**

10.1. ALL NDE SHALL BE IN ACCORDANCE WITH AWS D1.1.

10.2. FOR NEW BASE STIFFENERS (INCLUSIVE OF TRANSITION STIFFENERS) AND ANCHOR ROD BRACKETS, COMPLETE JOINT PENETRATION WELDS SHALL BE 100% INSPECTED BY UT. ALL PARTIAL JOINT PENETRATION AND FILLET WELDS SHALL BE 100% INSPECTED BY MT.

10.3. FOR NEW FLAT PLATE REINFORCEMENT AT THE BASE OF THE TOWER, COMPLETE JOINT PENETRATION WELDS SHALL BE 100% INSPECTED BY UT. ALL PARTIAL JOINT PENETRATION AND FILLET WELDS SHALL BE 100% INSPECTED BY MT, BUT MAY BE LIMITED TO A HEIGHT OF 10'-0".

10.4. FOR NDE OF THE EXISTING BASE PLATE CIRCUMFERENTIAL WELD, GC SHALL REFERENCE THE MI CHECKLIST FOR APPLICABILITY. PLEASE SEE ENG-SOW-10033: 'TOWER BASE PLATE NDE, AND ENG-BUL-10051: 'NDE REQUIREMENTS FOR MONOPOLE BASE PLATE TO PREVENT CONNECTION FAILURE'. NOTIFY THE EOR AND CROWN ENGINEERING IMMEDIATELY IF ANY CRACKS ARE SUSPECTED OR HAVE BEEN IDENTIFIED. THE NDE SHALL INCLUDE ALL EXISTING MODIFICATIONS THAT HAVE BEEN WELDED TO THE BASE PLATE.

10.5. ALL TESTING LIMITATIONS SHALL BE DETAILED IN THE NDE REPORT.

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
X Built As Drawn
__ Contractor Red Line Verified
__ Discrepancies Noted

Built per Design
8/11/17
ANS
Ron Brandon

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CROWN CASTLE
6 PARKMEADOW DRIVE, PITTSFORD, NY 14534
PH: (585) 899-3445

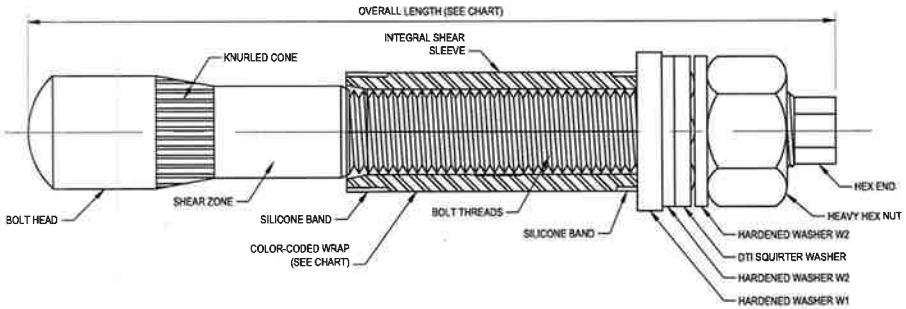
MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

PROJECT No:	37517-0519.002.7700
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

GENERAL NOTES

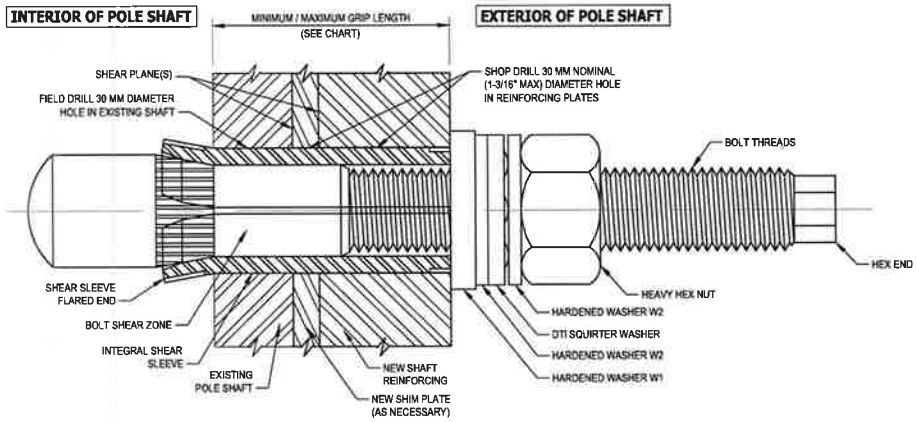
S-1

FORGBolt™ NOTE SHEET: A325/PC8.8 PORTRAIT VERSION DATE 04/24/2015



PRE-INSTALLED FORGBolt™ ASSEMBLY DETAIL 1 / S-2A

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
X Built As Drawn
__ Contractor Red Line Verified
__ Discrepancies Noted



INSTALLED FORGBolt™ ASSEMBLY DETAIL 2 / S-2A

FORGBolt®		AISC Group A Material: ASTM A325 and PC8.8 (Tensile Stress, Fu = 120 ksi minimum)					
GROUP A		FORGBolt® Size (mm)	Overall Length (inches)	Estimated Weight Each (lbs)	Grip Range (inch)	Comment	Color Code
FORGBolt® A325 - PC8.8	1	135	5.31	1.3	3/8" to 1"	--	RED
	2	160	6.30	1.6	3/4" to 1-1/2"	--	GREEN
	3	195	7.68	1.9	1-1/4" to 2-1/4"	--	BLUE
	4	260	10.24	2.6	2" to 3-1/2"	Splice Bolt	YELLOW
	5	365	14.37	3.6	3-1/2" to 5-1/2"	Flange Jump Bolt	ORANGE
	6	440	17.32	4.3	5-1/2" to 8-1/2"	Flange Jump Bolt	BLACK
DTI Note		Each Group A (A325/PC8.8) FORGBolt® assembly shall have a 'Squirter' DTI that is compatible with a M20-PC8.8 bolt.					

FOLLOW ALL MANUFACTURER / DISTRIBUTOR RECOMMENDATIONS FOR INSTALLATION, TIGHTENING, AND INSPECTION

INSTALLATION NOTES:
1. FIELD DRILL HOLES TO 30 MM DIAMETER.
2. SELECT CORRECT BOLT SIZE FOR INSTALLATION GRIP (REFER TO PLANS).
3. INSERT BOLT ASSEMBLY THROUGH HOLES IN SHAFT REINFORCING PLATES AND SEAT THE HARDENED WASHER W1 FLUSH AGAINST OUTSIDE OF PLATE.
4. HAND TIGHTEN NUT TO FINGER TIGHT.
5. TIGHTEN NUT TO PRETENSIONED CONDITION AND UNTIL DTI SHOWS PROPER INDICATION.
6. PROPERLY DOCUMENT AND INSPECT BOLT TIGHTENING PER PLAN REQUIREMENTS.

BOLT HOLE NOTES:
1. ALL SHOP-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM SHOP-DRILLED HOLE DIAMETER PERMITTED IS 1-3/16".
2. ALL FIELD-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM FIELD-DRILLED HOLE DIAMETER PERMITTED IS 30 MM.

BOLT TIGHTENING AND INSPECTION NOTES:
1. ALL STRUCTURAL BOLTS SHALL BE INSTALLED AND TIGHTENED TO THE PRETENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009.
2. ALL STRUCTURAL BOLTS SHALL BE INSPECTED ACCORDING TO THE REQUIREMENTS OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009.

AISC GROUP A MATERIAL: ASTM A325 AND PC8.8
(Fu = 120 KSI MIN TENSILE STRESS)

CONTAINS PROPRIETARY INFORMATION PATENT PENDING

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DISTRIBUTOR CONTACT:
PRECISION TOWER PRODUCTS
PHONE: 888-926-4857
EMAIL: info@precisiontowerproducts.com
WEB: www.precisiontowerproducts.com

Built per Design
8/11/17
ANS
Ron Brandon

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CROWN CASTLE
8 PARKMEADOW DRIVE, PITTSFORD, NY 14534
PH: (585) 899-3445

MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

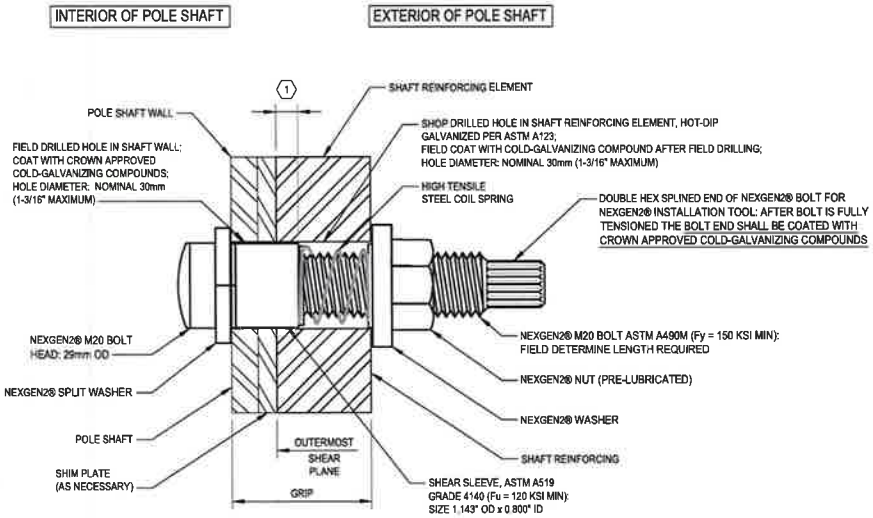
PROJECT No:	37517-0519.002.7700
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

FORGBOLT™ DETAILS

S-2A

NEXGEN2® BOLT NOTE SHEET: REV. 1.01, 04-15-2015

(1) NOTE: SHEAR SLEEVE LENGTH: THE SHEAR SLEEVE SHALL PROJECT A MINIMUM OF 3/8" BEYOND THE OUTERMOST SHEAR PLANE. THE CONTRACTOR SHALL SUBMIT FABRICATION DRAWINGS SHOWING NEXGEN2® BOLT LENGTHS AND SHEAR SLEEVE LENGTHS TO THE EOR FOR REVIEW AND APPROVAL.



TYPICAL NEXGEN2™ BOLT DETAIL (1) S-2B

FOLLOW ALL MANUFACTURER / DISTRIBUTOR RECOMMENDATIONS FOR INSTALLATION, TIGHTENING, AND INSPECTION

BOLT HOLE NOTES:

1. ALL SHOP-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM SHOP-DRILLED HOLE DIAMETER PERMITTED IS 1-3/16".
2. ALL FIELD-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM FIELD-DRILLED HOLE DIAMETER PERMITTED IS 30 MM.

BOLT TIGHTENING AND INSPECTION NOTES:

1. ALL NEXGEN2® BOLT ASSEMBLIES SHALL BE INSTALLED AND TIGHTENED TO THE PRETENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF SECTION 8.2.3 OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009. PER SECTION 8.2.3: ALL FASTENER ASSEMBLIES SHALL BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS IN AISC SECTION 8.1 WITHOUT SEVERING THE SPLINED END AND WITH WASHERS POSITIONED AS REQUIRED IN AISC SECTION 6.2. PER REQUIREMENTS IN SECTION 8.1: PRIOR TO BOLT PRETENSIONING, THE JOINT SHALL FIRST BE COMPACTED TO THE SNUG-TIGHT CONDITION. SNUG TIGHT IS THE CONDITION THAT EXISTS WHEN ALL OF THE PLIES IN THE CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS AND THE BOLTS HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH. ONCE THE SNUG TIGHT CONDITION IS ACHIEVED, THEN THE BOLT ASSEMBLY CAN BE TIGHTENED TO THE PRETENSIONED CONDITION.
2. ALL NEXGEN2® BOLT ASSEMBLIES SHALL BE INSPECTED ACCORDING TO THE REQUIREMENTS OF SECTION 9.2.3 OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009. NOTE THAT COMPLETE INSPECTION OF ALL NEXGEN2® BOLT ASSEMBLIES IS REQUIRED IN ADDITION TO ROUTINE OBSERVATION.
3. ALL NEXGEN2® BOLTS SHALL BE INSPECTED BY A QUALIFIED BOLT INSPECTOR PER NOTES 1 AND 2, ABOVE. DURING INSTALLATION, THE BOLT INSPECTOR SHALL VERIFY AND DOCUMENT: THE SHOP-DRILLED AND FIELD-DRILLED HOLE SIZES; THE INSTALLATION OF THE NEXGEN2® BOLT ASSEMBLY, INCLUDING THE SHEAR SLEEVE PLACEMENT AND NUT LUBRICATION; AND THE CONTRACTOR'S TENSIONING PROCEDURE. THE BOLT INSPECTOR SHALL PROVIDE COMPLETE DOCUMENTATION OF ALL BOLTS AFTER TIGHTENING CLEARLY SHOWING THAT THE DOUBLE HEX SPLINED END OF THE BOLTS HAVE BEEN TWISTED OFF AND COATED WITH CROWN APPROVED COLD-GALVANIZING COMPOUND..

PART NUMBER	BOLT LENGTH	SLEEVE LENGTH	MIN GRIP RANGE	MAX GRIP RANGE
M20x36	M20x95	11/16"	15/16"	1 7/16"
M20x48	M20x95	1 3/16"	1 7/16"	1 7/8"
M20x57	M20x95	1 5/8"	1 7/8"	2 1/4"
M20x68	M20x135	2"	2 1/4"	2 11/16"
M20x96	M20x135	2 7/16"	2 11/16"	3 3/4"
M20x127	M20x165	3"	3 3/4"	5"
M20x212	M20x250	4"	5"	8 5/16"

NOTE: NEXGEN2® BOLT ASSEMBLY SHALL BE MAGNI 565 COATED PER ASTM F2833 AND MANUFACTURER SPECIFICATIONS.

NOTE: INSTALL NEXGEN2® BOLT ASSEMBLY PER MANUFACTURER'S INSTRUCTIONS.

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
X Built As Drawn
__ Contractor Red Line Verified
__ Discrepancies Noted

DISTRIBUTOR CONTACT DETAILS:
ALLFASTENERS
15401 COMMERCE PARK DR.
BROOKPARK, OHIO 44142
PHONE: 440-232-6060
E-MAIL: SALES@ALLFASTENERS.COM

Built per Design
8/11/17
ANS
Ron Brandon

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CROWN CASTLE
8 PARKMEADOW DRIVE, PITTSFORD, NY 14534
PH: (585) 899-3445

MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

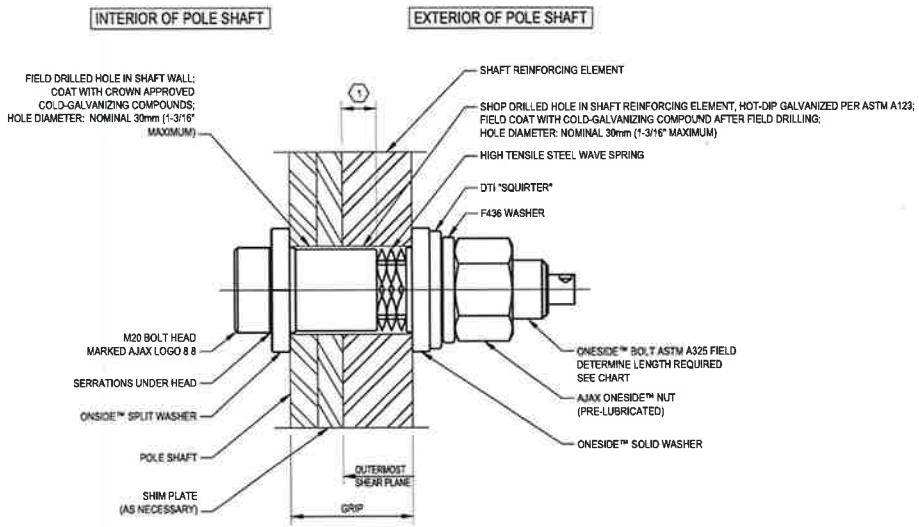
PROJECT No:	37517-0519.002.7700
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

NEXGEN2™ BOLT DETAIL

S-2B

AJAX ONESIDE™ BOLT NOTE SHEET: REV. 1.01, 06-08-2016

(1) NOTE: SHEAR SLEEVE LENGTH: THE SHEAR SLEEVE SHALL PROJECT A MINIMUM OF 3/8" BEYOND THE OUTERMOST SHEAR PLANE. THE CONTRACTOR SHALL SUBMIT FABRICATION DRAWINGS SHOWING AJAX ONESIDE™ BOLT LENGTHS AND SHEAR SLEEVE LENGTHS TO THE EOR FOR REVIEW AND APPROVAL.



TYPICAL AJAX ONESIDE™ BOLT DETAIL (1) S-2C

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
X Built As Drawn
__ Contractor Red Line Verified
__ Discrepancies Noted

FOLLOW ALL MANUFACTURER / DISTRIBUTOR RECOMMENDATIONS FOR INSTALLATION, TIGHTENING, AND INSPECTION

BOLT HOLE NOTES:

1. ALL SHOP-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM SHOP-DRILLED HOLE DIAMETER PERMITTED IS 1-3/16".
2. ALL FIELD-DRILLED HOLES SHALL BE NOMINAL 30 MM DIAMETER. THE MAXIMUM FIELD-DRILLED HOLE DIAMETER PERMITTED IS 30 MM.

BOLT TIGHTENING AND INSPECTION NOTES:

1. ALL AJAX ONESIDE™ BOLT ASSEMBLIES SHALL BE INSTALLED AND TIGHTENED TO THE PRETENSIONED CONDITION ACCORDING TO THE REQUIREMENTS OF SECTION 8.2.4 OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009. PER SECTION 8.2.4: ALL FASTENER ASSEMBLIES SHALL BE INSTALLED IN ACCORDANCE WITH THE REQUIREMENTS IN AISC SECTION 8.1 WITH WASHERS POSITIONED AS REQUIRED IN AISC SECTION 6.2. PER REQUIREMENTS IN SECTION 8.1: PRIOR TO BOLT PRETENSIONING, THE JOINT SHALL FIRST BE COMPACTED TO THE SNUG-TIGHT CONDITION. SNUG TIGHT IS THE CONDITION THAT EXISTS WHEN ALL OF THE PLIES IN THE CONNECTION HAVE BEEN PULLED INTO FIRM CONTACT BY THE BOLTS AND THE BOLTS HAVE BEEN TIGHTENED SUFFICIENTLY TO PREVENT THE REMOVAL OF THE NUTS WITHOUT THE USE OF A WRENCH. ONCE THE SNUG TIGHT CONDITION IS ACHIEVED, THEN THE BOLT ASSEMBLY CAN BE TIGHTENED TO THE PRETENSIONED CONDITION.
2. ALL AJAX ONESIDE™ BOLT ASSEMBLIES SHALL BE INSPECTED ACCORDING TO THE REQUIREMENTS OF SECTION 9.2.4 OF THE AISC 'SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS', DEC. 31, 2009. NOTE THAT COMPLETE INSPECTION OF ALL AJAX ONESIDE™ BOLT ASSEMBLIES IS REQUIRED IN ADDITION TO ROUTINE OBSERVATION.
3. ALL AJAX ONESIDE™ BOLTS SHALL BE INSPECTED BY A QUALIFIED BOLT INSPECTOR PER NOTES 1 AND 2, ABOVE. DURING INSTALLATION, THE BOLT INSPECTOR SHALL VERIFY AND DOCUMENT: THE SHOP-DRILLED AND FIELD-DRILLED HOLE SIZES; THE INSTALLATION OF THE AJAX ONESIDE™ BOLT ASSEMBLY, INCLUDING THE SHEAR SLEEVE PLACEMENT AND NUT LUBRICATION; AND THE CONTRACTOR'S TENSIONING PROCEDURE. THE BOLT INSPECTOR SHALL PROVIDE COMPLETE DOCUMENTATION OF ALL BOLTS AFTER TIGHTENING CLEARLY SHOWING THAT THE DIRECT TENSION INDICATOR WASHERS SHOW THAT THE PROPER BOLT TENSION HAS BEEN REACHED.
4. A MINIMUM OF 4 OUT OF 5 SQUIRTER® DTI PROTRUSIONS SHALL BE ENGAGED IN ANY AJAX ONESIDE™/DTI BOLT ASSEMBLY IN THE REINFORCING MEMBERS. A FEELER GAGE MAY BE USED TO VERIFY PROTRUSION COMPRESSION.
5. INSPECTIONS SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS REQUIREMENTS AND CROWN DOCUMENT ENG-SOW-10007: *MODIFICATION INSPECTION SOW*.

BOLT ASSEMBLY AND INSTALLATION:

1. BOLT MUST BE PURCHASED PRE-ASSEMBLED.
2. FOLLOW BOLT AND DTI MANUFACTURERS INSTRUCTIONS FOR INSTALLATION.

AJAX ONESIDE™ BOLT DETAIL

CODE	SIZE	COLOR	SLEEVE LENGTH	GRIP	GRIP IMP
OSBA20.65-6	M20 x 65	ORANGE	6.0 (0.236")	12.5 / 20.0	0.500" / 0.787"
OSBA20.95-14	M20 x 95	BLACK	14.0 (0.551")	20.0 / 32.0	0.787" / 1.259"
OSBA20.95-22	M20 x 95	GREEN	22.0 (0.866")	30.0 / 50.0	1.181" / 1.968"
OSBA20.95-30	M20 x 95	YELLOW	30.0 (1.181")	40.5 / 50.0	1.595" / 1.968"
OSBA20.135-39	M20 x 135	BLUE	39.0 (1.535")	49.0 / 77.0	1.929" / 3.031"
OSBA20.135-48	M20 x 135	BROWN	48.0 (1.889")	60.5 / 77.0	2.375" / 3.031"
OSBA20.135-57	M20 x 135	PURPLE	57.0 (2.244")	67.0 / 90.0	2.637" / 3.543"
OSBA20.165-76	M20 x 165	RED	76.0 (3.000")	87.0 / 120.0	3.425" / 4.724"
OSBA20.250	M20 x 250	SILVER	MTO	121.0 / 211.0	4.724" / 8.310"

DISTRIBUTOR
IRA SVENSGAARD AND ASSOCIATES
PETER SVENDSGAARD - PETERS@IRASVENS.COM
JOHN KILLAM - JOHN@IRASVENS.COM
PHONE: (530) 647-8225
FAX: (530) 647-8229

MANUFACTURER
AJAX FASTENERS
SALES + TECH: ONESIDE@AJAXFAST.COM.AU

Built per Design
8/11/17
ANS
Ron Brandon

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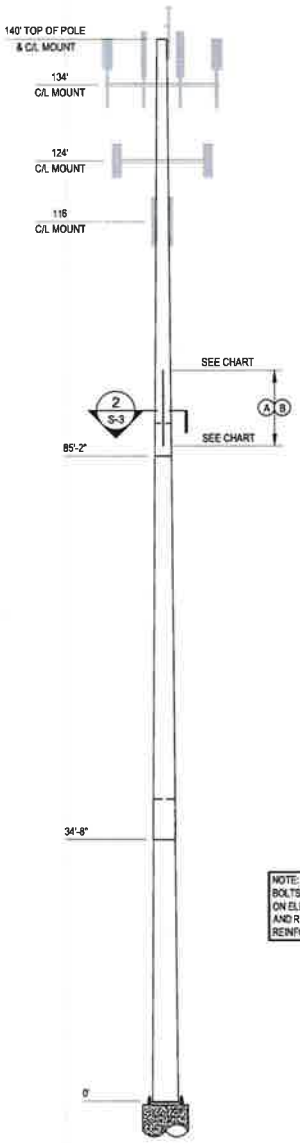
CROWN CASTLE
8 PARKMEADOW DRIVE, PITTSFORD, NY 14534
PH: (585) 889-3445

MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

PROJECT No:	37517-0519.002.7700
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

AJAX ONESIDE™ BOLT DETAIL

S-2C



POLE ELEVATION 1 / S-3

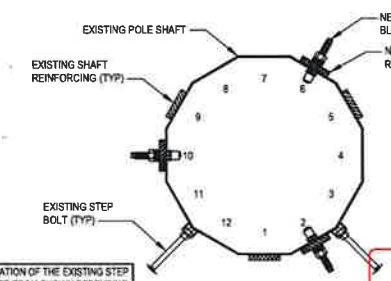
NEW CCI FLAT PLATE (65 KSI) REINFORCING SCHEDULE											
BOTTOM ELEVATION	TOP ELEVATION	FLAT #/ DEGREE SEPARATION	ELEMENT	ELEMENT LENGTH	ELEMENT QUANTITY	APPROXIMATE BOLTS PER ELEMENT	APPROXIMATE TOTAL BOLT QUANTITY	TERMINATION BOLTS (BOTTOM)	TERMINATION BOLTS (TOP)	MAXIMUM INTERMEDIATE BOLT SPACING	ESTIMATED TOTAL STEEL WEIGHT
86'-6"	96'-6"	F2, F6 & F10	CCI-AFP-04007510	10'-0"	3	17	51	6	6	16"	306 LBS
							51				306 LBS

NOTES:

1) ALL STEEL SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123. ALTERNATIVELY, ALL NEW STIFFENER PLATE STEEL REINFORCING MAY BE COLD GALVANIZED AS FOLLOWS: APPLY A MINIMUM OF TWO COATS OF ZRC-BRAND ZINC-RICH COLD GALVANIZING COMPOUND. FILM THICKNESS PER COAT SHALL BE: WET 3.0 MILS; DRY 1.5 MILS. APPLY PER ZRC (MANUFACTURER) RECOMMENDED PROCEDURES. CONTACT ZRC AT 1-800-831-3275 FOR PRODUCT INFORMATION.
2) ALL REINFORCING SHALL BE ASTM A572 GR. 65.
3) WELDS SHALL BE E80XX OR GREATER. TERMINATION WELDS SHALL BE 3/8" FILLET WELDS.
4) HOLES FOR BOLTS ARE 30mm UNLESS NOTED OTHERWISE.
5) ALL SHIMS SHALL BE ASTM A36.
6) ALL HOLES ARE TO BE DRILLED, DO NOT BURN OR PUNCH.
7) FOR PLATES STARTING AT 6', THE BOTTOM OF THE FLAT PLATE SHALL BEGIN AT 6" ± 1". FOR SINGLE PLATES OR MULTIPLE PLATES SPLICED TOGETHER, THE BOTTOM OF THE FLAT PLATE SHALL BEGIN AT THE PROPOSED ELEVATION ± 3". FOR MULTIPLE PLATES SPLICED TOGETHER, THE TOP OF THE FLAT PLATE IS TO BE PLACED SUCH THAT THERE IS NO MORE THAN 3" DIFFERENCE BETWEEN THE ACTUAL OVERALL LENGTH OF THE SPAN AND THE PROPOSED OVERALL LENGTH OF THE SPAN, FROM THE BOTTOM OF THE PLATE TO THE TOP OF THE PLATE.

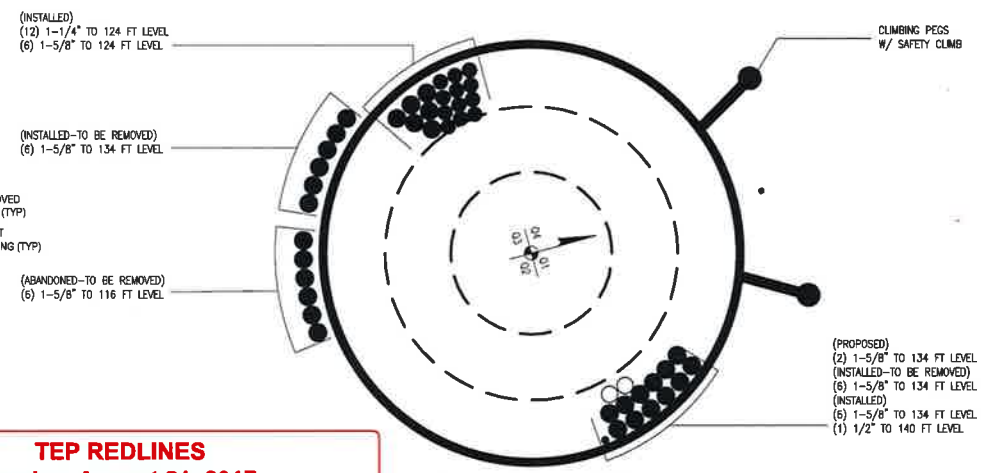
SHAFT SECTION DATA							
SHAFT SECTION	SECTION LENGTH (FT)	PLATE THICKNESS (IN)	LAP SPLICE (IN)	DIAMETER ACROSS FLATS (IN) @ TOP	DIAMETER ACROSS FLATS (IN) @ BOTTOM	POLE GRADE (ksi)	POLE SHAPE
1	54.83	0.1875	52.00	16.000	26.280	65	12-SIDED
2	54.83	0.3125	64.00	25.093	35.380	65	12-SIDED
3	40.00	0.3750		33.755	41.250	65	12-SIDED
NOTE: DIMENSIONS SHOWN DO NOT INCLUDE GALVANIZING TOLERANCES							

TOWER MODIFICATION SCHEDULE			
	ELEVATION	TOWER MODIFICATION DESCRIPTION	REFERENCE SHEETS
A	86'-6" TO 96'-6"	INSTALL NEW SHAFT REINFORCING	S-3
B	86'-6" TO 96'-6"	PAINT MODIFICATIONS TO MATCH EXISTING POLE	S-3



NOTE: FLAT LOCATION OF THE EXISTING STEP BOLTS MAY DIFFER FROM SHOWN DEPENDING ON ELEVATION. CONTRACTOR SHALL REMOVE AND REPLACE STEP BOLTS AS REQUIRED FOR REINFORCING INSTALLATION

SECTION 2 / S-3
EL. 90'



BASE LEVEL DRAWING

TEP REDLINES
Monday, August 21, 2017
Scott Coburn
__ Built As Drawn
__ Contractor Red Line Verified
X Discrepancies Noted.
The second tower splice was verified at 86'-1" where 85*-'2" is showin in the modification drawings. This was approved by the engineer of record. Please see Section 6.3.4 for correspondence.

Built per Design
8/11/17
ANS
Ron Brandon

CROWN CASTLE US PATENT NOS 8,046,972; 8,156,712; 7,849,659; 8,424,269 AND PATENT PENDING

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Ph: (585) 899-3445

MODIFICATION OF AN EXISTING 140' MONOPOLE
BU #806374; HRT 081 943236
ROCKY HILL, CONNECTICUT

PROJECT No:	37517-0519.002.7706
DRAWN BY:	B.M.S.
DESIGNED BY:	J.W.M.
CHECKED BY:	BKK
DATE:	2-27-2017

MONOPOLE PROFILE

S-3

POST-CONSTRUCTION

6.3.1 EXECUTIVE SUMMARY

Photograph	Observations and Recommendations
	<u>Item 1</u> **Observation:** New reinforcement plates were installed to the tower from an elevation of 86.5-ft to 96.5-ft. This was completed in accordance with page S-3 of the modification drawings provided by Paul J. Ford and Company dated March 1, 2017 except as noted. <u>Note:</u> The tower splice 2 was field verified at 86'-1" where the drawings state 85'-2". This was approved by the engineer of record. Please see Section 6.3.4 for correspondence.

6.3.2 PHOTOGRAPHS

CROWN CASTLE
877-486-9377
800-788-7011
806374
PRIVATE PROPERTY
NO TRESPASSING
VIOLATORS WILL BE PROSECUTED
17.06.2017 16:26

17.06.2017 16:31

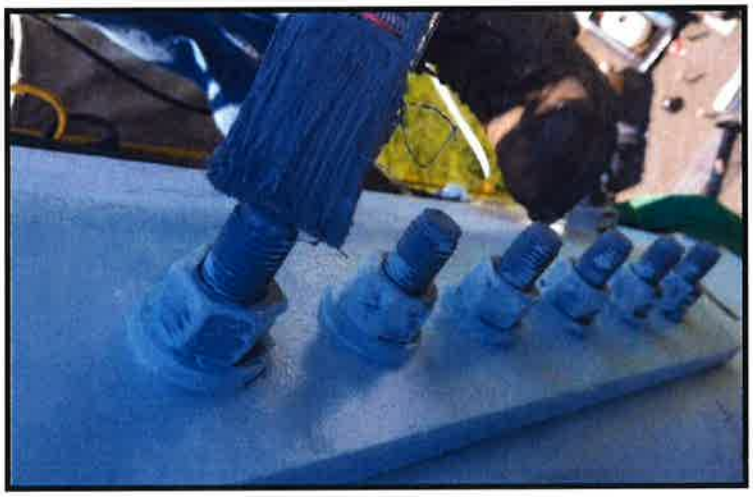
17.06.2017 16:43

17.06.2017 16:43

F216,10
86'-6"
17.06.2017 16:38

17.06.2017 16:42

6.3.3 CONTRACTOR PHOTOGRAPHS



6.3.4 ENGINEER OF RECORD EMAIL

Punchlist BU # 806374 - HRT 081 | **Status:** Complete

Project Information

Project Contacts		Punchlist Issuance #	Date	Visit
MI Vendor	Tower Engineering Professionals	1	15-Aug	OnSite
MI On-site Inspector	James Harms, Scott Coburn			
MI WO #	1391964			
General Contractor	ANS			
Crown POC	Dan Vadney			
EOR	Please Select			
BU	806374			
Site Name	HRT 081			

Structural Impact To Capacity
No
# of Punchlist Items
1

1 Puchlist Item(s)

NonConformance Impact to Capacity (Shall Be Provided by EOR)	New Overall Structure Capacity Rating :	-

MI Checklist Documents

Documentation Complete / Documentation Missing

MI Checklist Documents	Required	Date Submitted	Requirement Waived	Date Compliance Verified	Status
PRE-CONSTRUCTION					
EOR Approved Shop Drawings	Required	8/11/2017	N/A	8/21/2017	Complete
Foundations Inspection	N/A	N/A	N/A	-	N/A
Fabricator Certified Weld Inspection	N/A	N/A	N/A	-	N/A
Material Test Report (MTR)	Required	8/11/2017	N/A	8/21/2017	Complete
Fabricator NDE Report	N/A	N/A	N/A	-	N/A
NDE Insp. Report of Monopole Base Plate	N/A	N/A	N/A	-	N/A
Packing Slips	Required	8/11/2017	N/A	8/21/2017	Complete
Additional Pre-Construction Inspections	N/A	N/A	N/A	-	N/A
CONSTRUCTION					
Foundation Inspections	N/A	N/A	N/A	-	N/A
Post-Installed Anchor Rod Verification	N/A	N/A	N/A	-	N/A
Base Plate Grout Verification	N/A	N/A	N/A	-	N/A
Contractor's Certified Weld Inspection	N/A	N/A	N/A	-	N/A
Hot Work Permit	N/A	N/A	N/A	-	N/A
On-Site Cold Galvanizing Verification	Required	8/11/2017	N/A	8/21/2017	Complete
Guy Wire Tension Report	N/A	N/A	N/A	-	N/A
GC As-Built Drawings	Required	8/11/2017	N/A	8/21/2017	Complete
Additional Construction Inspections	N/A	N/A	N/A	-	N/A
POST-CONSTRUCTION					
Construction Compliance Verification	Required	8/11/2017	N/A	8/21/2017	Complete
Post-Installed Anchor Rod Pull-Out Testing	N/A	N/A	N/A	-	N/A
Additional Post-Construction Inspections	N/A	N/A	N/A	-	N/A
MI Checklist Comments					

Nonconformance List

Add Punchlist Item | Create PDF

Nonconformance # 1

MI Vendor

Nonconformance Issue Description

The tower splice shown as 85'-2" was field verified at 86'-1"

Section/Height/Elev	Panel	Leg	Face/Flat
86'-1"			

Crown

EOR Feedback Requested

EOR

EOR Approved - No Capacity Impact

Capacity Impact As Is

No Capacity Impact

EOR Feedback By

Name of Engineer Providing Feedback

Final Crown Approval

Crown Approval By

MI Vendor

Status of Correction	Type of Verification
Conformance	*EOR/Crown Approved*

Correction Photo (additional photos uploaded to WO)

MI Comments:	*Enter Comments*
EOR Comments:	*Enter Comments*
Crown Comments:	*EOR to confirm issue ok or recommend fix in field.*

Dewberry

Dewberry Engineers Inc.
600 Parsippany Road, Suite 301
Parsippany, NJ 07054

973.739.9400
973.739.9710 fax
www.dewberry.com

May 28, 2019

State Of Connecticut
Connecticut Siting Council
10 Franklin Square, New Britain, CT 06051

Re: Letter of Completion
Site Name: Rocky Hill CT
Dewberry Site No. 50091678
France Street, Lot 1
Rocky Hill, CT 06067

To Whom It May Concern:

Verizon Wireless has contracted Dewberry Engineers Inc. (Dewberry) to verify that the tower related construction activities at the above referenced project were completed in general conformance with the requirements of the construction and structural contract documents.

Dewberry referenced the following:

- Construction Drawings Rev. 0. (Dated September 5, 2017) by Dewberry Engineers signed and sealed by Jiang Yu, PE.
- Structural Modification Analysis Report (Dated May 17, 2019) by Dewberry Engineers signed and sealed by Benjamin B. Revette, PE.
- Photos of completed construction activities provided by Verizon Wireless.

Based on the information and photos provided, the mount modification construction on the tower appears to have been completed in general accordance with the Structural Modification Analysis Report.

Please contact me at 973.739.9400 or brevette@dewberry.com should you have any questions on this matter.

Sincerely,
Dewberry Engineers Inc.

STATE OF CONNECTICUT
BENJAMIN B. REVETTE
28971
LICENSED
PROFESSIONAL ENGINEER

Benjamin Revette, PE
Senior Associate
Connecticut License No. 28971