

445 Hamilton Avenue, 14th Floor White Plains, New York 10601 T 914 761 1300 F 914 761 5372 cuddyfeder.com

Daniel Patrick <a href="mailto:dpatrick@cuddyfeder.com">dpatrick@cuddyfeder.com</a>

July 20, 2022

### VIA EMAIL & OVERNIGHT DELIVERY

Members of the Connecticut Siting Council Connecticut Siting Council 10 Franklin Square New Britain, Connecticut 06051

Re:

Tower Sharing Request by New Cingular Wireless PCS, LLC Premises: 186 Black Rock Turnpike, Redding, Connecticut

Dear Members of the Siting Council:

Pursuant to Connecticut General Statutes (C.G.S.) § 16-50aa, New Cingular Wireless PCS, LLC ("AT&T" or "the Applicant") hereby requests an order from the Connecticut Siting Council (the "Council") approving the proposed shared use of a communications tower and associated compound at the parcel owned by the Redding Ridge Fire District #1 and identified as 186 Black Rock Turnpike in the Town of Redding (the "Black Rock Turnpike Facility"). The owner of the facility, Message Center Management ("MCM"), and AT&T have agreed to share the use of the Black Rock Turnpike Facility as detailed below. Additionally, annexed here as **Attachment 1** is the Letter of Authorization between AT&T and MCM, owner of the tower equipment, authorizing AT&T to prepare and file an application for shared use of the tower.

### The Black Rock Turnpike Facility

On October 13, 2014, the Council granted approval to MCM and AT&T of its application in Docket No. 449 for a Certificate of Environmental Compatibility and Public Need to construct a 150' above ground level (AGL) monopole at 186 Black Rock Turnpike with the height to the top of AT&T's antennas not to exceed 154' AGL. Cellco Partnership ("Verizon") intervened in that proceeding and proposed collocation on the facility at the 140' AGL height. MCM thereafter began construction in June 2016. Construction was completed in 2017 with the installation of Verizon's equipment at the centerline height of 135' AGL. To date, AT&T's equipment has not been installed.

### **AT&T's Wireless Facility**

As depicted on the plans annexed hereto as **Attachment 2** prepared by Hudson Design Group LLC last revised April 27, 2022, including a site plan, compound plan, and tower elevation, AT&T now proposes the shared use of the Black Rock Turnpike Facility in accordance with its approvals



July 20, 2022 Page 2

in Docket No. 449 to provide FCC licensed services as well as FirstNet services.¹ AT&T will install 6 antennas and 9 remote radiohead units on a sector frame mount attached to the existing tower at the centerline height of approximately 145′ AGL, with the height to the top of AT&T's antennas at approximately 148′ AGL. As also depicted on the drawings, AT&T will install its unmanned equipment within a proposed 6′-8″ x 6′-8″ walk-in equipment cabinet on a concrete pad within AT&T's 20′ x 10′ leased area located within the existing fenced compound. AT&T will not install a separate emergency back-up generator but will instead tie into the existing propane generator.

Connecticut General Statutes § 16-50aa provides that, upon written request for shared use approval, an order approving such use shall be issued "if the Council finds that the proposed shared use of the facility is technically, legally, environmentally and economically feasible and meets public safety concerns." (C.G.S. § 16-50aa(c)(1)). Further, upon approval of such shared use, it is exclusive, and no local zoning or land use approvals are required. (C.G.S. § 16-50x). Shared use of the Black Rock Turnpike Facility satisfies the approval criteria set forth in C.G.S. § 16-50aa as follows:

- A. <u>Technical Feasibility:</u> As evidenced in the structural analysis prepared by Hudson Design Group LLC dated March 18, 2022, annexed hereto as **Attachment 3**, and the mount analysis prepared by Hudson Design Group LLC, dated December 3, 2021, annexed hereto as **Attachment 4**, AT&T confirmed that the Black Rock Turnpike Facility is capable of supporting AT&T's antennas and tower mounted equipment in addition to the Town's proposed loading. The proposed shared use of this tower is therefore technically feasible.
- B. <u>Legal Feasibility:</u> Pursuant to C.G.S. § 16-50aa, the Council is authorized to issue an order approving shared use of the existing Black Rock Turnpike Facility. (C.G.S. § 16-50aa(c)(1)). Under the authority vested in the Council by C.G.S. § 16-50aa, an order by the Council approving the shared use of a tower would permit the Applicant to obtain a building permit for the proposed installation. Notably, this tower was previously approved by the Council in contemplation of AT&T's proposed installation no higher than 154' AGL in Docket No. 449, with which AT&T's proposed installation complies.
- C. <u>Environmental Feasibility:</u> The proposed shared use would have a minimal environmental effect, for the following reasons:
  - 1. Given the height of the existing tower and the Council's review of visibility in Docket 449, AT&T's proposed installation would have a *de minimis* visual impact and would not cause any significant change or alteration in the physical or environmental characteristics of the facility;

<sup>&</sup>lt;sup>1</sup> FirstNet is a nationwide broadband public safety network dedicated to the needs of first responders. For more information, see <a href="https://about.att.com/newsroom/2019/fn">https://about.att.com/newsroom/2019/fn</a> purpose built cell sites.html and <a href="http://about.att.com/sites/first">http://about.att.com/sites/first</a> net powered by att.



July 20, 2022 Page 3

- 2. The installation by AT&T will not increase the height of the tower;
- 3. The proposed installation will not increase the noise levels at the site boundaries by six decibels or more;
- 4. Operation of AT&T's antennas at this site will not exceed the total radio frequency electromagnetic radiation power density level adopted by the FCC and Connecticut Department of Health. AT&T's proposed antenna installation along with the existing equipment is calculated to be within 18.86% of the FCC Standard for General Public/Uncontrolled Maximum Permissible Exposure ("MPE"). Please see the assessment of RF power density dated March 30, 2022, prepared by C Squared Systems, LLC, annexed hereto as **Attachment 5**; and
- 5. The proposed shared use of the Black Rock Turnpike Facility would not require any water or sanitary facilities or discharges into any waterbodies nor will there be any additional air emissions since the Applicant does not propose any new emergency back-up generator. Further, the installation will not generate any traffic other than for periodic maintenance visits.
- D. <u>Economic Feasibility</u>: The Applicant and the tower owner entered into a mutual agreement to share use of the Black Rock Turnpike Facility on terms agreeable to both parties. The proposed tower sharing is therefore economically feasible.
- E. <u>Public Safety</u>: As stated above and evidenced in attachments hereto the tower is structurally capable of supporting AT&T's installation and emissions are well within the maximum permitted by the FCC and the Connecticut Department of Health. Further, the addition of AT&T's telecommunications service and the provision of FirstNet service in the Redding area through shared use of the Black Rock Turnpike Facility will ensure that critical wireless services including emergency services are provided to the community. The installation of AT&T's equipment with shared use of the Black Rock Turnpike Facility will enhance the safety and welfare of local residents and travelers through the surrounding area resulting in an improvement to public safety in this area of the State.

### **Notice of Tower Share Filing**

Pursuant to R.C.S.A. Section 16-50j-88 and the August 2013 Tower Share Filing Guide, copies of AT&T's tower share filing request were sent to the tower owner, the property owner, as well as the Town of Redding chief elected official and Land Use Department. Copies of each notice and their respective FedEx labels are included in **Attachment 6**.



July 20, 2022 Page 4

#### **Conclusion**

As explained above, the proposed shared use of the Black Rock Turnpike Facility satisfies the criteria set forth in C.G.S. §16-50aa and advances the General Assembly's and the Siting Council's goal of preventing the proliferation of towers in the State of Connecticut. AT&T therefore requests the Siting Council issue an order approving the proposed shared use of the Black Rock Turnpike Facility.

Respectfully submitted,

Daniel Patrick

On behalf of AT&T

Attachments

cc: Redding Ridge Fire District #1, Property Owner

Message Center Management (MCM), Tower Owner Julia Pemberton, First Selectwoman, Town of Redding Aimee Pardee, Land Use Director, Town of Redding

AT&T

Lucia Chiocchio, Esq.

Riddar Nget

## **ATTACHMENT 1**

#### **Letter of Authorization**

RE: Building Permits and Land Use Approvals

MCM Holdings, LLC ("Owner") the Owner of a tower and wireless antenna equipment compound located at 186 Black Rock Turnpike, Redding, CT (tower and wireless antenna equipment compound hereinafter referred to as the ("Property") does hereby appoint New Cingular Wireless PCS, LLC ("AT&T") and its representatives, as Owner's agent for the purpose of completing and/or filing any application, form, map drawing, site plan or any document, useful or necessary in obtaining any zoning approval, special permit or other land use approval or building permit (collectively, the "Approvals"), required to provide AT&T with lawful access to, and the ability to use the Property for the purposes of installing, erecting or otherwise placing antennas, support structures and related equipment of the Property. Owner shall fully cooperate with AT&T and its agents in obtaining any required Approvals. AT&T shall be responsible for all costs, filing fees, or any other expense incurred in connection with securing any Approvals.

Property: Tower and wireless antenna compound located at 186 Black Rock Turnpike, Redding, CT.

By: Vugue Luce Print Name: Virginia King

Title: Project Manager

Authorized Signatory

STATE OF CONNECTICUT)

:ss. Hartford

COUNTY OF HARTFORD)

Personally appeared, Virginia Ling, duly authorized signer and sealer of the foregoing instrument and acknowledged the same to be her free act and deed of said MCM Holdings, LLC, before me

Notary Public

My Commission Expires: 00

HOLLY H. MARTINO
NOTARY PUBLIC
State of Connecticut
My Commission Expires
June 30, 2023

## **ATTACHMENT 2**

PROJECT INFORMATION

SCOPE OF WORK:

TELECOMMUNICATIONS FACILITY (NSB A EXISTING 150'-0" A.G.L. TALL MONOPOLE. PROPOSED WALK-IN CABINET WILL BE INSTALLED AT GRADE INSIDE A EXISTING FENCED-IN COMPOUND. PROPOSED SIX PANEL ANTENNAS AND ASSOCIATED EQUIPMENT

WILL BE INSTALLED AT A HEIGHT OF 145"-0" A.G.L.):

SITE ADDRESS:

186 BLACK ROCK TURNPIKE

REDDING, CT 06896

APPLICANT:

550 COCHITUATE ROAD

FRAMINGHAM, MA 01701

SITE OWNER: REDDING FIRE DISTRICT 1 P.O. BOX 45

REDDING, CT 06875

LATITUDE: 41.30994 N, 41° 18' 35.8" N

LONGITUDE: 73.34759 W, 73° 20' 51.3" W

TYPE OF SITE: MONOPOLE/ WALK-IN CABINET

TOWER HEIGHT: 150'-0"±

RAD CENTER: 145"-0"±



**SITE NUMBER: CT1338** 

SITE NAME: REDDING BLACK ROCK TURNPIKE

FA CODE:10128142

PACE ID: MRCTB057522

**PROJECT: NSB** 

	DRAWING INDEX					
SHEET NO.	DESCRIPTION	REV.				
T-1	TITLE SHEET	1				
GN-1	GENERAL NOTES	1				
SN-1	STRUCTURAL & SPECIAL INSPECTION NOTES	1				
C-1	PLOT PLAN	1				
A-1	COMPOUND & EQUIPMENT PLANS	1				
A-2	ANTENNA LAYOUT & ELEVATION	1				
A-3	DETAILS	1				
A-4	DETAILS	1				
E-1	ELECTRICAL NOTES & ONE-LINE DIAGRAM	1				
G-1	GROUNDING DETAILS	1				
RF-1	RF PLUMBING DIAGRAM	1				

DIRECTIONS TO SITE: DEPART AND HEAD NORTHEAST. TURN RIGHT, THEN IMMEDIATELY TURN LEFT ONTO LEGGATT MCCALL CONNECTOR RD. BEAR LEFT ONTO BURR ST. TURN LEFT ONTO MA-30 / COCHITUATE RD. TAKE THE RAMP ON THE RIGHT FOR I-90 EAST / I-90 WEST AND HEAD TOWARD BOSTON / SPRINGFIELD. AT EXIT 78, HEAD RIGHT ON THE RAMP FOR I-84 TOWARD HARTFORD / NEW YORK CITY. KEEP LEFT TO STAY ON 1-84 W. AT EXIT 10, HEAD RIGHT ON THE RAMP FOR US-6 WEST TOWARD NEWTOWN / SANDY HOOK. TURN RIGHT ONTO US-6 W / CT-34 / CHURCH HILL RD. TURN LEFT ONTO CT-25 / MAIN ST. TURN RIGHT ONTO CT-302 / SUGAR ST. TURN LEFT ONTO KEY ROCK RD. ROAD NAME CHANGES TO POVERTY HOLLOW RD. BEAR RIGHT ONTO CHURCH HILL RD. TURN LEFT ONTO CT-58 / BLACK ROCK TPKE. 186 BLACK ROCK TPKE, REDDING, CT1338 WILL BE ON THE LEFT.

> **PROJECT** SITE

VICINITY MAP

THIS DOCUMENT IS THE CREATION, DESIGN, PROPERTY AND COPYRIGHTED WORK OF AT&T. ANY DUPLICATION OR USE WITHOUT EXPRESS WRITTEN CONSENT IS STRICTLY PROHIBITED. DUPLICATION AND USE BY GOVERNMENT AGENCIES FOR THE PURPOSES OF CONDUCTING THEIR LAWFULLY AUTHORIZED REGULATORY AND ADMINISTRATIVE FUNCTIONS IS SPECIFICALLY ALLOWED.

**GENERAL NOTES** 

- THE FACILITY IS AN UNMANNED PRIVATE AND SECURED EQUIPMENT INSTALLATION. IT IS ONLY ACCESSED BY TRAINED TECHNICIANS FOR PERIODIC ROUTINE MAINTENANCE AND THEREFORE DOES NOT REQUIRE ANY WATER OR SANITARY SEWER SERVICE. THE FACILITY IS NOT GOVERNED BY REGULATIONS REQUIRING PUBLIC ACCESS PER ADA REQUIREMENTS.
- CONTRACTOR SHALL VERIFY ALL PLANS AND EXISTING DIMENSIONS AND CONDITIONS ON THE JOB SITE AND SHALL IMMEDIATELY NOTIFY THE AT&T MOBILITY REPRESENTATIVE IN WRITING OF DISCREPANCIES BEFORE PROCEEDING WITH THE WORK OR BE RESPONSIBLE FOR SAME.
- 4. CONSTRUCTION DRAWINGS ARE VALID FOR SIX MONTHS AFTER ENGINEER OF RECORD'S STAMPED AND SIGNED SUBMITTAL DATE LISTED HEREIN.

72 HOURS



BEFORE YOU DIG



CALL TOLL FREE 1 - 800 - 922 - 4455

OR CALL 811

#### UNDERGROUND SERVICE ALERT



NORTH ANDOVER, MA 01845

TEL: (978) 557-5553 FAX: (978) 336-5586

12 INDUSTRIAL WAY SALEM, NH 03079

**SITE NUMBER: CT1338** SITE NAME: REDDING BLACK ROCK TURNPIKE

> 186 BLACK ROCK TURNPIKE REDDING, CT 06896 FAIRFIELD COUNTY



FRAMINGHAM, MA 01701

	THE RESERVE OF THE PERSON NAMED IN	"IFE							
	04/27/22	ISSUED FOR	REVIEW				MJ	JC	DPH
	04/11/22	ISSUED FOR	REVIEW				ES	JC	DPH
	DATE			REVISION	ONS		BY	СНК	APP'D
A	LE: AS SH	HOWN	DESIGNE	D BY:	JC	DRAWN	N BY:	ES	

AT&T					
TITLE SHEET					
	(NSB)				
SITE NUMBER	DRAWING NUMBER	REV			
CT1338 T-1					

#### **GROUNDING NOTES**

- 1. THE SUBCONTRACTOR SHALL REVIEW AND INSPECT THE EXISTING FACILITY GROUNDING SYSTEM AND LIGHTNING PROTECTION SYSTEM (AS DESIGNED AND INSTALLED) FOR STRICT COMPLIANCE WITH THE NEC (AS ADOPTED BY THE AHJ), THE SITE—SPECIFIC (UL, LPI, OR NFPA) LIGHTING PROTECTION CODE, AND GENERAL COMPLIANCE WITH TELCORDIA AND TIA GROUNDING STANDARDS. THE SUBCONTRACTOR SHALL REPORT ANY VIOLATIONS OR ADVERSE FINDINGS TO THE CONTRACTOR FOR RESOLUTION.
- 2. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION, AND AC POWER GES'S) SHALL BE BONDED TOGETHER, AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
- 3. THE SUBCONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81 STANDARDS) FOR NEW GROUND ELECTRODE SYSTEMS. THE SUBCONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
- 4. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
- 5. EACH BTS CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 AWG STRANDED COPPER OR LARGER FOR INDOOR BTS AND #2 AWG STRANDED COPPER FOR OUTDOOR BTS.
- 6. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
- 7. APPROVED ANTIOXIDANT COATINGS (I.E., CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
- 8. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO GROUND BAR.
- 9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
- 10. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING. IN ACCORDANCE WITH THE NEC.
- 11. METAL CONDUIT SHALL BE MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 AWG COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
- 12. ALL NEW STRUCTURES WITH A FOUNDATION AND/OR FOOTING HAVING 20 FT. OR MORE OF 1/2 IN. OR GREATER ELECTRICALLY CONDUCTIVE REINFORCING STEEL MUST HAVE IT BONDED TO THE GROUND RING USING AN EXOTHERMIC WELD CONNECTION USING #2 AWG SOLID BARE TINNED COPPER GROUND WIRE, PER NEC 250.50

#### **GENERAL NOTES**

1. FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR — SAI SUBCONTRACTOR — GENERAL CONTRACTOR (CONSTRUCTION) OWNER — AT&T MOBILITY

- PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING SUBCONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CONTRACTOR.
- 3. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS, AND ORDINANCES. SUBCONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS, AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
- 4. DRAWINGS PROVIDED HERE ARE NOT TO BE SCALED AND ARE INTENDED TO SHOW OUTLINE ONLY.
- 5. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES, AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.
- "KITTING LIST" SUPPLIED WITH THE BID PACKAGE IDENTIFIES ITEMS THAT WILL BE SUPPLIED BY CONTRACTOR. ITEMS NOT INCLUDED IN THE BILL OF MATERIALS AND KITTING LIST SHALL BE SUPPLIED BY THE SUBCONTRACTOR.
- 7. THE SUBCONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
- 8. IF THE SPECIFIED EQUIPMENT CANNOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE SUBCONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION SPACE FOR APPROVAL BY THE CONTRACTOR.
- 9. SUBCONTRACTOR SHALL DETERMINE ACTUAL ROUTING OF CONDUIT, POWER AND T1 CABLES, GROUNDING CABLES AS SHOWN ON THE POWER, GROUNDING AND TELCO PLAN DRAWING. SUBCONTRACTOR SHALL UTILIZE EXISTING TRAYS AND/OR SHALL ADD NEW TRAYS AS NECESSARY. SUBCONTRACTOR SHALL CONFIRM THE ACTUAL ROUTING WITH THE CONTRACTOR.
- 10. THE SUBCONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT SUBCONTRACTOR'S EXPENSE TO THE SATISFACTION OF OWNER.
- 11. SUBCONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER'S DESIGNATED LOCATION.
- 12. SUBCONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION.
- 13. ALL CONCRETE REPAIR WORK SHALL BE DONE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301.

- 14. ANY NEW CONCRETE NEEDED FOR THE CONSTRUCTION SHALL BE AIR—ENTRAINED AND SHALL HAVE 4000 PSI STRENGTH AT 28 DAYS. ALL CONCRETE WORK SHALL BE DONE IN ACCORDANCE WITH ACI 318 CODE REQUIREMENTS.
- 15. ALL STRUCTURAL STEEL WORK SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC SPECIFICATIONS. ALL STRUCTURAL STEEL SHALL BE ASTM A36 (Fy = 36 ksi) UNLESS OTHERWISE NOTED. PIPES SHALL BE ASTM A53 TYPE E (Fy = 36 ksi). ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED. TOUCH UP ALL SCRATCHES AND OTHER MARKS IN THE FIELD AFTER STEEL IS ERECTED USING A COMPATIBLE ZINC RICH PAINT.
- 16. CONSTRUCTION SHALL COMPLY WITH SPECIFICATIONS AND "GENERAL CONSTRUCTION SERVICES FOR CONSTRUCTION OF AT&T SITES."
- 17. SUBCONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS MUST BE VERIFIED. SUBCONTRACTOR SHALL NOTIFY THE CONTRACTOR OF ANY DISCREPANCIES PRIOR TO ORDERING MATERIAL OR PROCEEDING WITH CONSTRUCTION.
- 18. THE EXISTING CELL SITE IS IN FULL COMMERCIAL OPERATION. ANY CONSTRUCTION WORK BY SUBCONTRACTOR SHALL NOT DISRUPT THE EXISTING NORMAL OPERATION. ANY WORK ON EXISTING EQUIPMENT MUST BE COORDINATED WITH CONTRACTOR. ALSO, WORK SHOULD BE SCHEDULED FOR AN APPROPRIATE MAINTENANCE WINDOW USUALLY IN LOW TRAFFIC PERIODS AFTER MIDNIGHT.
- 19. SINCE THE CELL SITE IS ACTIVE, ALL SAFETY PRECAUTIONS MUST BE TAKEN WHEN WORKING AROUND HIGH LEVELS OF ELECTROMAGNETIC RADIATION. EQUIPMENT SHOULD BE SHUTDOWN PRIOR TO PERFORMING ANY WORK THAT COULD EXPOSE THE WORKERS TO DANGER. PERSONAL RF EXPOSURE MONITORS ARE ADVISED TO BE WORN TO ALERT OF ANY DANGEROUS EXPOSURE LEVELS.

#### 20. APPLICABLE BUILDING CODES:

SUBCONTRACTOR'S WORK SHALL COMPLY WITH ALL APPLICABLE NATIONAL, STATE, AND LOCAL CODES AS ADOPTED BY THE LOCAL AUTHORITY HAVING JURISDICTION (AHJ) FOR THE LOCATION. THE EDITION OF THE AHJ ADOPTED CODES AND STANDARDS IN EFFECT ON THE DATE OF CONTRACT AWARD SHALL GOVERN THE DESIGN.

BUILDING CODE: IBC 2015 WITH 2018 CT STATE BUILDING CODE AMENDMENTS ELECTRICAL CODE: 2017 NATIONAL ELECTRICAL CODE (NFPA 70-2017)

SUBCONTRACTOR'S WORK SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING STANDARDS:

AMERICAN CONCRETE INSTITUTE (ACI) 318; BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE;

AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) MANUAL OF STEEL CONSTRUCTION, ASD, FOURTEENTH EDITION;

TELECOMMUNICATIONS INDUSTRY ASSOCIATION (TIA) 222-H, STRUCTURAL STANDARDS FOR STEEL

FOR ANY CONFLICTS BETWEEN SECTIONS OF LISTED CODES AND STANDARDS REGARDING MATERIAL, METHODS OF CONSTRUCTION, OR OTHER REQUIREMENTS, THE MOST RESTRICTIVE REQUIREMENT SHALL GOVERN. WHERE THERE IS CONFLICT BETWEEN A GENERAL REQUIREMENT AND A SPECIFIC REQUIREMENT, THE SPECIFIC REQUIREMENT SHALL GOVERN.

			ABBREVIATIONS		
AGL	ABOVE GRADE LEVEL	EQ	EQUAL	REQ	REQUIRED
AWG	AMERICAN WIRE GAUGE	GC	GENERAL CONTRACTOR	RF	RADIO FREQUENCY
BBU	BATTERY BACKUP UNIT	GRC	GALVANIZED RIGID CONDUIT	TBD	TO BE DETERMINED
втсм	BARE TINNED SOLID COPPER WIRE	MGB	MASTER GROUND BAR	TBR	TO BE REMOVED
BGR	BURIED GROUND RING	MIN	MINIMUM	TBRR	TO BE REMOVED AND REPLACED
BTS	BASE TRANSCEIVER STATION	Р	PROPOSED	TYP	TYPICAL
E	EXISTING	NTS	NOT TO SCALE	UG	UNDER GROUND
EGB	EQUIPMENT GROUND BAR	RAD	RADIATION CENTER LINE (ANTENNA)	VIF	VERIFY IN FIELD
EGR	EQUIPMENT GROUND RING	REF	REFERENCE		



TEL: (978) 557-5553 FAX: (978) 336-5586

45 BEECHWOOD DRIVE

NORTH ANDOVER, MA 01845



SITE NUMBER: CT1338
SITE NAME: REDDING BLACK ROCK TURNPIKE

186 BLACK ROCK TURNPIKE REDDING, CT 06896 FAIRFIELD COUNTY



FRAMINGHAM MA 01701

			LOIN		2011 1111	-111	011	00.42	, ,,,,,		11/
1	04/27/22	ISSUE	D FOR	REVIEW	7				MJ	JC	DPH
0	04/11/22	ISSUE	D FOR	REVIEW	<i></i>				ES	JC	DPH
NO.	DATE				REVISIO	ONS			BY	снк	APP'D
SCA	LE: AS SH	HOWN		DESIGN	IED BY:	JC		DRAWN	N BY:	ES	

AT&T

| GENERAL NOTES (NSB)
| SITE NUMBER | DRAWING NUMBER | REV | CT1338 | GN-1 | 1

#### STRUCTURAL NOTES:

- DESIGN REQUIREMENTS ARE PER STATE BUILDING CODE AND APPLICABLE SUPPLEMENTS, INTERNATIONAL BUILDING CODE, EIA/TIA-222-H STRUCTURAL STANDARDS FOR STEEL ANTENNA, TOWERS AND ANTENNA SUPPORTING STRUCTURES
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIFLD PRIOR TO FABRICATION AND ERECTION OF ANY MATERIAL. ANY UNUSUAL CONDITIONS SHALL BE REPORTED TO THE ATTENTION OF THE CONSTRUCTION MANAGER AND ENGINEER OF RECORD.
- DESIGN AND CONSTRUCTION OF STRUCTURAL STEEL SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- STRUCTURAL STEEL SHALL CONFORM TO ASTM A992 (Fv=50 ksi). MISCELLANEOUS STEEL SHALL CONFORM TO ASTM A36 UNLESS OTHERWISE
- STEEL PIPE SHALL CONFORM TO ASTM A500 "COLD-FORMED WELDED & SEAMLESS CARBON STEEL STRUCTURAL TUBING", GRADE B, OR ASTM A53 PIPE STEEL BLACK AND HOT-DIPPED ZINC-COATED WELDED AND SEAMLESS TYPE E OR S, GRADE B. PIPE SIZES INDICATED ARE NOMINAL. ACTUAL OUTSIDE DIAMETER IS LARGER.
- STRUCTURAL CONNECTION BOLTS SHALL BE HIGH STRENGTH BOLTS (BEARING TYPE) AND CONFORM TO ASTM A325 TYPE-X "HIGH STRENGTH BOLTS FOR STRUCTURAL JOINTS, INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS". ALL BOLTS SHALL BE 3/4" DIA UON.
- ALL STEEL MATERIALS SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123 "ZINC (HOT-DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS". UNLESS OTHERWISE NOTED.
- ALL BOLTS, ANCHORS AND MISCELLANEOUS HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153 "ZINC-COATING (HOT-DIP) ON IRON AND STEEL HARDWARE", UNLESS OTHERWISE NOTED.
- FIELD WELDS, DRILL HOLES, SAW CUTS AND ALL DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED WITH AN ORGANIC ZINC REPAIR PAINT COMPLYING WITH REQUIREMENTS OF ASTM A780. GALVANIZING REPAIR PAINT SHALL HAVE 65 PERCENT ZINC BY WEIGHT, ZIRP BY DUNCAN GALVANIZING, GALVA BRIGHT PREMIUM BY CROWN OR EQUAL. THICKNESS OF APPLIED GALVANIZING REPAIR PAINT SHALL BE NOT NOT LESS THAN 4 COATS (ALLOW TIME TO DRY BETWEEN COATS) WITH A RESULTING COATING THICKNESS REQUIRED BY ASTM A123 OR A153 AS APPLICABLE.
- 10. CONTRACTOR SHALL COMPLY WITH AWS CODE FOR PROCEDURES, APPEARANCE AND QUALITY OF WELDS, AND FOR METHODS USED IN CORRECTING WELDING. ALL WELDERS AND WELDING PROCESSES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES". ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND DI.I. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "STEEL CONSTRUCTION MANUAL". 14TH EDITION.
- INCORRECTLY FABRICATED, DAMAGED OR OTHERWISE MISFITTING OR NON-CONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE CONSTRUCTION MANAGER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE CONSTRUCTION MANAGER APPROVAL.
- 12. UNISTRUT SHALL BE FORMED STEEL CHANNEL STRUT FRAMING AS MANUFACTURED BY UNISTRUT CORP., WAYNE, MI OR EQUAL, STRUT MEMBERS SHALL BE 1 5/8"x1 5/8"x12GA, UNLESS OTHERWISE NOTED, AND SHALL BE HOT-DIP GALVÁNIZED ÁFTER FABRICATION.
- 13. EPOXY ANCHOR ASSEMBLY SHALL CONSIST OF STAINLESS STEEL ANCHOR ROD WITH NUTS & WASHERS. AN INTERNALLY THREADED INSERT, A SCREEN TUBE AND A EPOXY ADHESIVE. THE ANCHORING SYSTEM SHALL BE THE HILTI-HIT HY-270 AND OR HY-200 SYSTEMS (AS SPECIFIED IN DWG.) OR ENGINEERS
- 14. EXPANSION BOLTS SHALL CONFORM TO FEDERAL SPECIFICATION FF-S-325, GROUP II, TYPE 4, CLASS I, HILTI KWIK BOLT III OR APPROVED EQUAL. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 15. LUMBER SHALL COMPLY WITH THE REQUIREMENTS OF THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION AND THE NATIONAL FOREST PRODUCTS ASSOCIATION'S NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. ALL LUMBER SHALL BE PRESSURE TREATED AND SHALL BE STRUCTURAL GRADE NO. 2 OR BETTER.
- 16. WHERE ROOF PENETRATIONS ARE REQUIRED, THE CONTRACTOR SHALL CONTACT AND COORDINATE RELATED WORK WITH THE BUILDING OWNER AND THE EXISTING ROOF INSTALLER. WORK SHALL BE PERFORMED IN SUCH A MANNER AS TO NOT VOID THE EXISTING ROOF WARRANTY. ROOF SHALL BE WATERTIGHT.
- 17. ALL FIBERGLASS MEMBERS USED ARE AS MANUFACTURED BY STRONGWELI COMPANY OF BRISTOL, VA 24203. ALL DESIGN CRITERIA FOR THESE MEMBERS IS BASED ON INFORMATION PROVIDED IN THE DESIGN MANUAL. ALL REQUIREMENTS PUBLISHED IN SAID MANUAL MUST BE STRICTLY ADHERED TO.
- 18. NO MATERIALS TO BE ORDERED AND NO WORK TO BE COMPLETED UNTIL SHOP DRAWINGS HAVE BEEN REVIEWED AND APPROVED IN WRITING
- 19. SUBCONTRACTOR SHALL FIREPROOF ALL STEEL TO PRE-EXISTING CONDITIONS.

#### **SPECIAL INSPECTIONS (REFERENCE IBC CHAPTER 17):**

GENERAL: WHERE APPLICATION IS MADE FOR CONSTRUCTION, THE OWNER OR THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE INSPECTION CHECKLIST ABOVE.

THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND ENGINEERS OF RECORD INVOLVED IN THE DESIGN OF THE PROJECT ARE PERMITTED TO ACT AS THE APPROVED AGENCY AND THEIR PERSONNEL ARE PERMITTED TO ACT AS THE SPECIAL INSPECTOR FOR THE WORK DESIGNED BY THEM, PROVIDED THOSE PERSONNEL MEET THE

STATEMENT OF SPECIAL INSPECTIONS: THE APPLICANT SHALL SUBMIT A STATEMENT OF SPECIAL INSPECTIONS PREPARED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IN ACCORDANCE WITH SECTION 107.1 AS A CONDITION FOR ISSUANCE. THIS STATEMENT SHALL BE IN ACCORDANCE WITH SECTION 1705

REPORT REQUIREMENT: SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS SHALL BE SUBMITTED

#### NOTES:

- ALL CONNECTIONS TO BE SHOP WELDED & FIELD BOLTED USING 3/4"ø A325-X BOLTS, UNLESS OTHERWISE NOTIFIED
- SHOP DRAWING ENGINEER REVIEW & APPROVAL REQUIRED BEFORE ORDFRING MATERIAL.
- SHOP DRAWING ENGINEER REVIEW & APPROVAL REQUIRED PRIOR TO STEEL FARRICATION
- VERIFICATION OF EXISTING ROOF CONSTRUCTION IS REQUIRED PRIOR TO THE INSTALLATION OF THE ROOF PLATFORM. ENGINEER OF RECORD IS TO APPROVE EXISTING CONDITIONS IN ORDER TO MOVE FORWARD.
- CENTERLINE OF PROPOSED STEEL PLATFORM SUPPORT COLUMNS TO BE CENTRALLY LOCATED OVER THE EXISTING BUILDING COLUMNS.
- EXISTING BRICK MASONRY COLUMNS/BEARING TO BE REPAIRED/REPLACED AT ALL PROPOSED PLATFORM SUPPORT POINTS. ENGINEER OF RECORD TO REVIEW AND APPROVE.

#### NOTES:

- REQUIRED FOR ANY  ${\hbox{\scriptsize NEW}}$  SHOP FABRICATED FRP OR STEEL. PROVIDED BY MANUFACTURER,
- REQUIRED IF HIGH STRENGTH BOLTS OR STEEL.
- PROVIDED BY GENERAL CONTRACTOR; PROOF OF MATERIALS.
- HIGH WIND ZONE INSPECTION CATB 120MPH OR CAT C,D 110MPH INSPECT FRAMING OF WALLS. ANCHORING. FASTENING SCHEDULE.
- ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED CONCRET AND SEISMIC APPLICATIONS. DESIGN ADHESIVE BOND STRENGTH HAS BEEN BASED ON ACI 355.4 TEMPERATURE CATEGORY B WITH INSTALLATIONS INTO DRY HOLES DRILLED USING A CARBIDE BIT INTO CRACKED CONCRETE THAT HAS CURED FOR AT LEAST 21 DAYS. ADHESIVE ANCHORS REQUIRING CERTIFIED INSTALLATIONS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER PER ACI 318-11 D.9.2.2. INSTALLATIONS REQUIRING CERTIFIED INSTALLERS SHALL BE INSPECTED PER ACI 318-11 D.8.2.4. AS REQUIRED; FOR ANY FIELD CHANGES TO THE ITEMS IN THIS TABLE.

### REQUIRED N/A REQUIRED ADDITIONAL TESTING AND INSPECTIONS: **DURING CONSTRUCTION** CONSTRUCTION/INSTALLATION INSPECTIONS AND TESTING REQUIRED (COMPLETED BY ENGINEER OF RECORD) REQUIRED N/A N/A N/A N/A N/A N/A N/A

CONSTRUCTION / INSTALLATION

INSPECTIONS AND TESTING

REQUIRED (COMPLETED BY

REQUIRED

ENGINEER OF RECORD)

GUY WIRE TENSION REPORT ADDITIONAL TESTING AND INSPECTIONS AFTER CONSTRUCTION

SPECIAL INSPECTION CHECKLIST

BEFORE CONSTRUCTION

REPORT ITEM

SHOP DRAWINGS

PACKING SLIPS

REPORT ITEM

INSPECTIONS

VERIFICATION

VERIFICATION

STEEL INSPECTIONS

HIGH STRENGTH BOLT

HIGH WIND ZONE INSPECTIONS 4

FOUNDATION INSPECTIONS

POST INSTALLED ANCHOR

GROUT VERIFICATION

CONCRETE COMP. STRENGTH

CERTIFIED WELD INSPECTION

ON SITE COLD GALVANIZING

EARTHWORK: LIFT AND DENSITY

SLUMP TESTS AND PLACEMENT

MATERIAL\_ SPECIFICATIONS

FABRICATOR NDE INSPECTION

ENGINEER OF RECORD APPROVED

CONSTRUCTION /INSTALLATION INSPECTIONS AND TESTING REPORT ITEM REQUIRED (COMPLETED BY MODIFICATION INSPECTOR REDLINE

REQUIRED OR RECORD DRAWINGS POST INSTALLED ANCHOR N/A REQUIRED PHOTOGRAPHS

ADDITIONAL TESTING AND INSPECTIONS:

N/A

N/A

ENGINEER OF RECORD)

HUDSON **Design Group LLC** 

NORTH ANDOVER, MA 01845

TEL: (978) 557-5553 FAX: (978) 336-5586



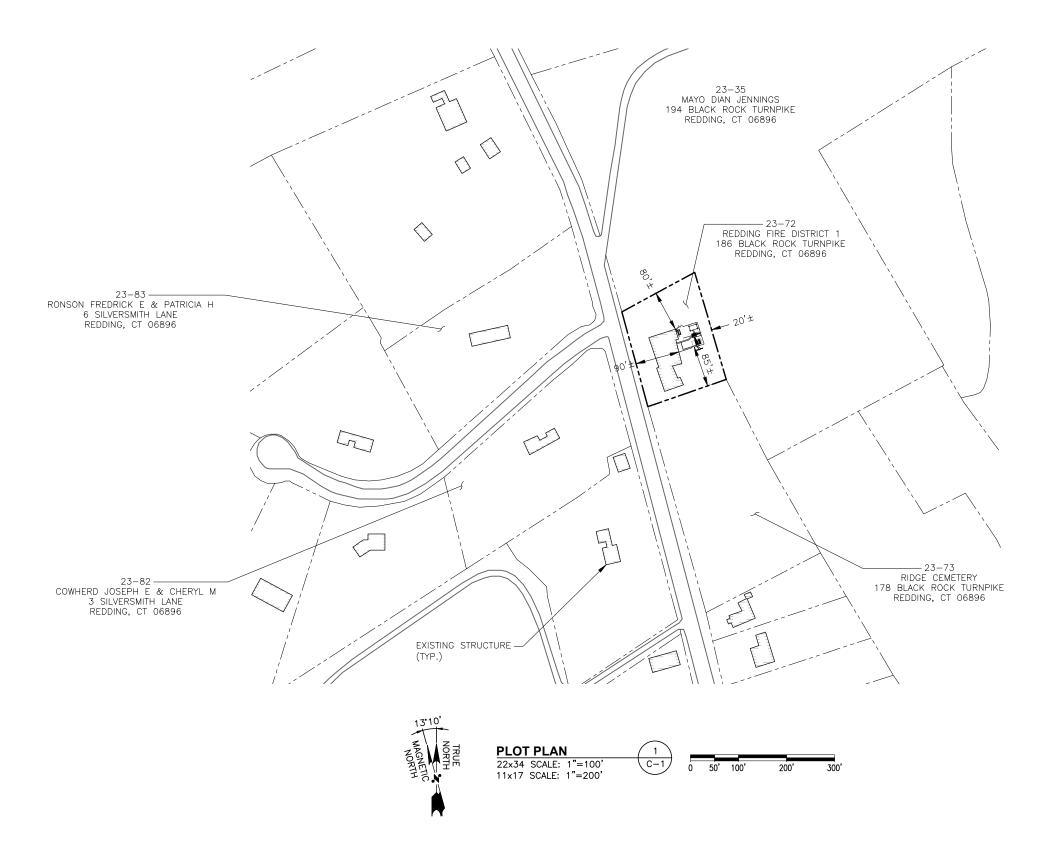
**SITE NUMBER: CT1338** SITE NAME: REDDING BLACK ROCK TURNPIKE

> 186 BLACK ROCK TURNPIKE REDDING, CT 06896 FAIRFIELD COUNTY



1	04/27/22	ISSUED FOR	R REVIEW			MJ	JC	DPH
0	04/11/22	ISSUED FOR	REVIEW			ES	JC	DPH
NO.	DATE		REVISIO	NS		BY	снк	APP'D
SCA	LE: AS SI	HOWN	DESIGNED BY:	JC	DRAWN	N BY:	ES	

	AT&T				
STRUCTURAL NOTES					
	(NSB)				
SITE NUMBER	DRAWING NUMBER	REV			
CT1338	SN-1	1			



INFORMATION TAKEN FROM PLANS BY REDDING CONNECTICUT GIS

DIMENSIONS REQUIREMENTS:	EXISTING	PROPOSED
ANTENNA SETBACKS		
FRONT YARD SETBACK:	90'±	NO CHANGE
SIDE YARD SETBACK:	80'±	NO CHANGE
SIDE YARD SETBACK:	85'±	NO CHANGE
REAR YARD SETBACK:	20'±	NO CHANGE

- 1. PROPERTY LINE INFORMATION (WHEN APPLICABLE) WAS PREPARED USING TAX MAPS, AND PLANS OF RECORD AND SHOULD NOT BE CONSTRUCTED AS A BOUNDARY
- 2. NO NOISE, SMOKE, DUST, OR ODOR WILL RESULT FROM THIS FACILITY.

  3. THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION (THERE IS NO HANDICAP ACCESS REQUIRED).
- 4. THE FACILITY IS UNMANNED AND DOES NOT REQUIRE POTABLE WATER OR SANITARY
- SERVICE.

  5. CONNECTION TO ELECTRICAL & TELEPHONE UTILITIES TO BE DETERMINED BY THE APPROPRIATE UTILITY COMPANY.
- 6. SUBCONTRACTOR TO VERIFY ANTENNA ELEVATION AND AZIMUTH WITH RF ENGINEER PRIOR TO INSTALLATION. SEE ANTENNA CONFIGURATION SHEETS FOR SITE SPECIFIC
- 7. SUBCONTRACTOR SHALL LOCATE ALL UTILITIES PRIOR TO EXCAVATING.
  8. SUBCONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND INSPECTIONS REQUIRED FOR CONSTRUCTION.





SITE NUMBER: CT1338 SITE NAME: REDDING BLACK ROCK TURNPIKE

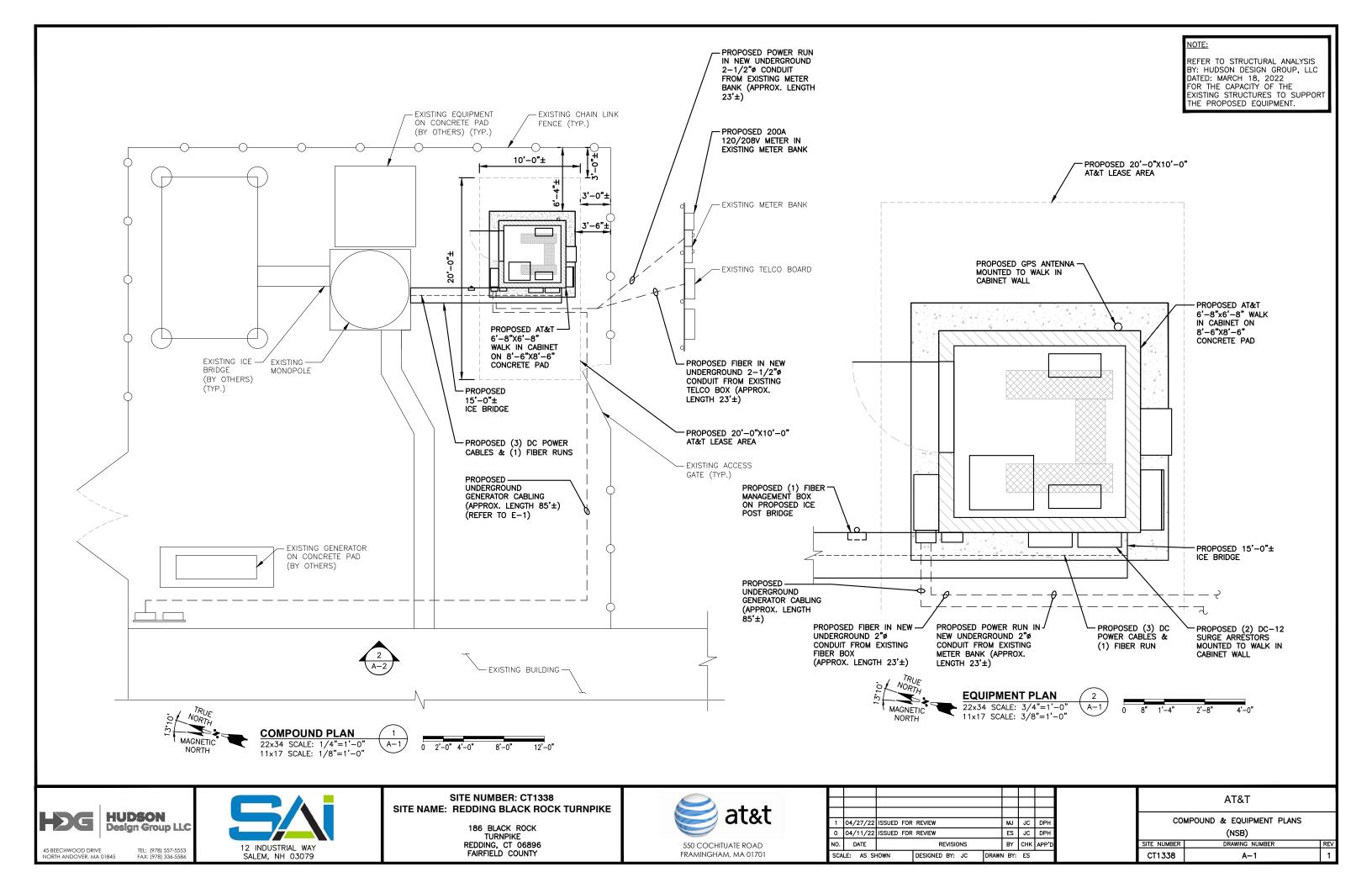
> 186 BLACK ROCK TURNPIKE REDDING, CT 06896 FAIRFIELD COUNTY

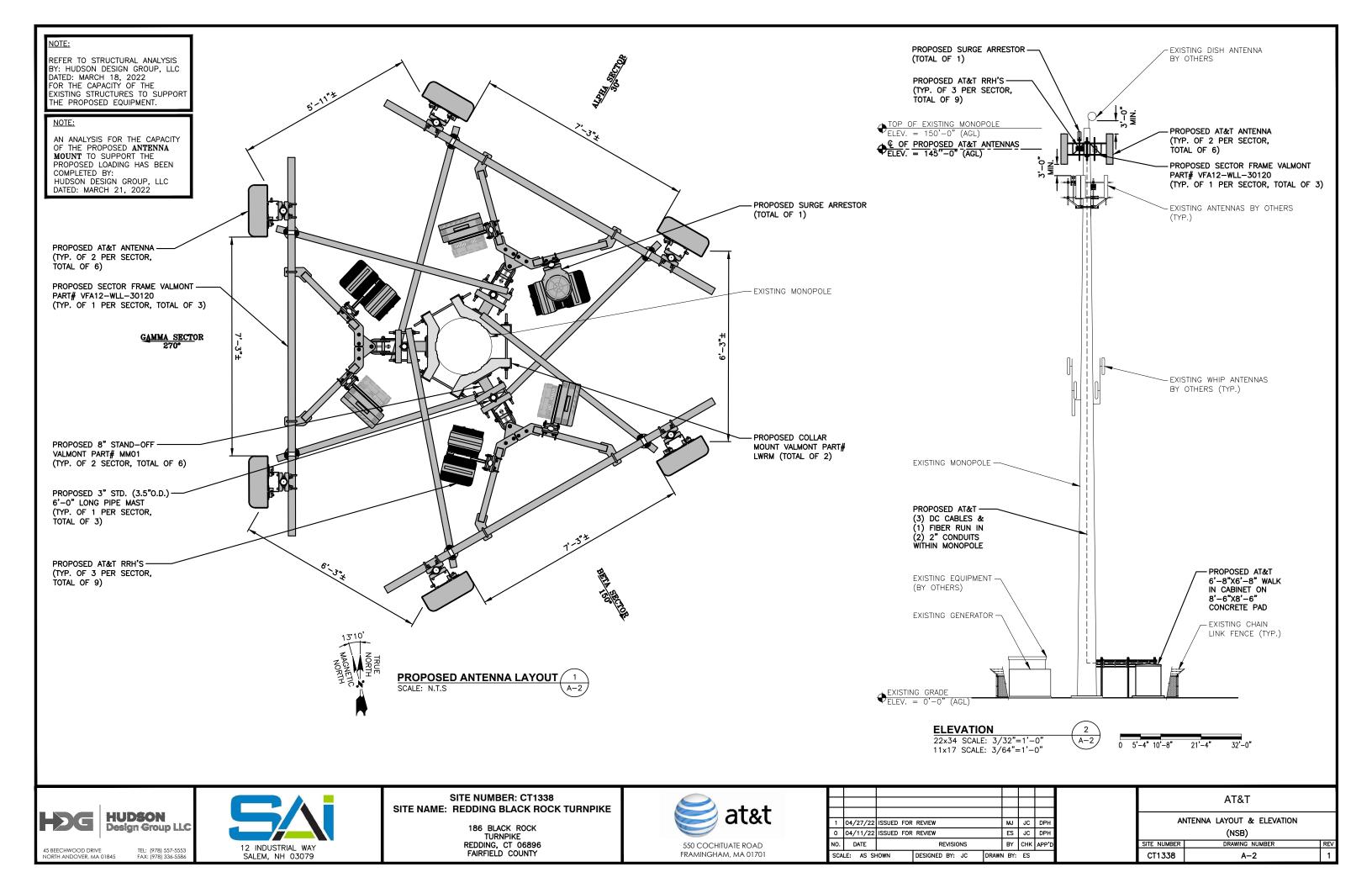


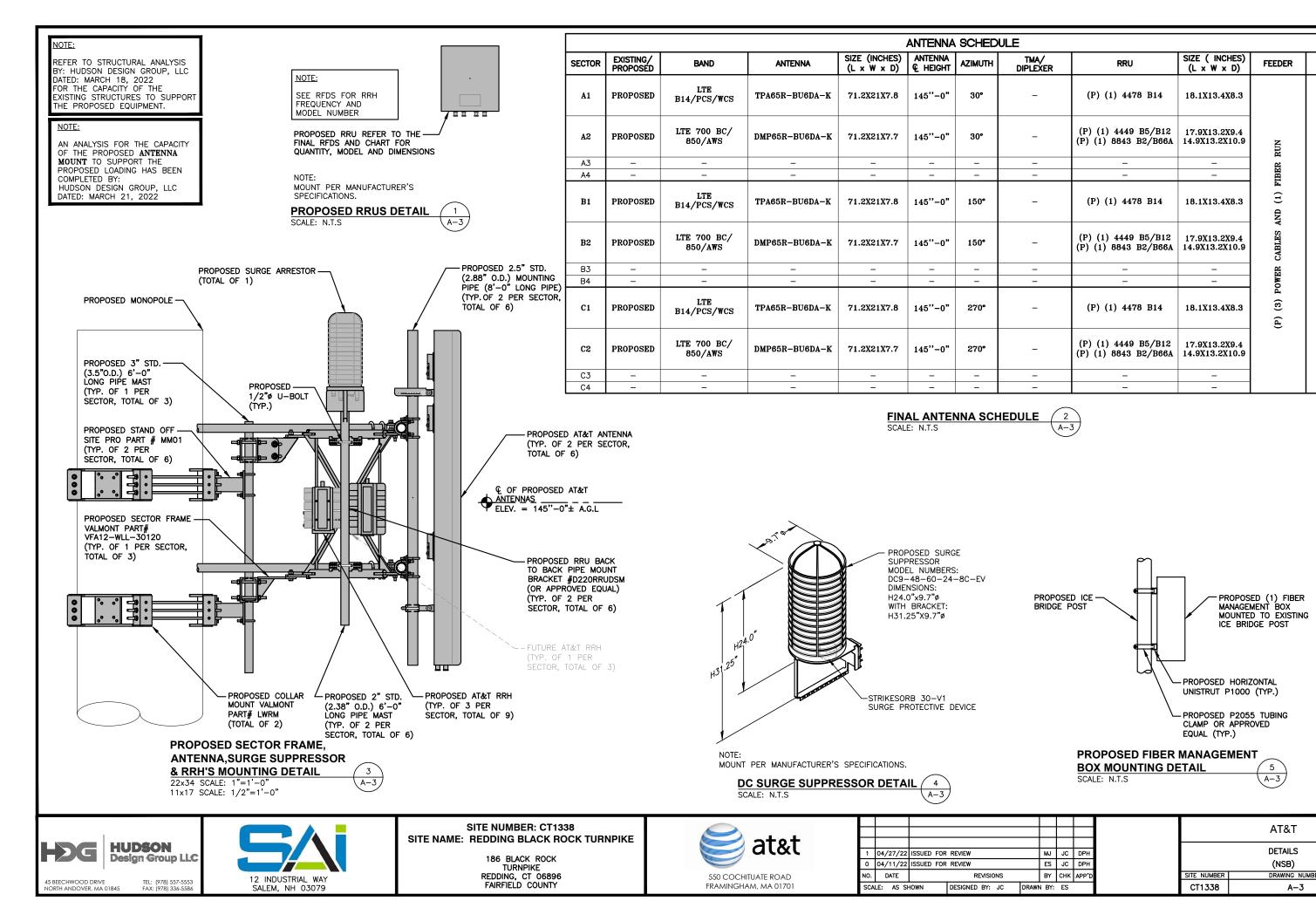
FRAMINGHAM, MA 01701

1	04/27/22	ISSUED FOR	REVIEW				MJ	JC	DPH
0	04/11/22	1/22 ISSUED FOR REVIEW					ES	JC	DPH
NO.	DATE	REVISIONS					BY	СНК	APP'D
SCA	LE: AS SI	HOWN	DESIGNED	BY:	JC	DRAWN	N BY:	ES	

AT&T PLOT PLAN (NSB) DRAWING NUMBE SITE NUMBER CT1338 C-1





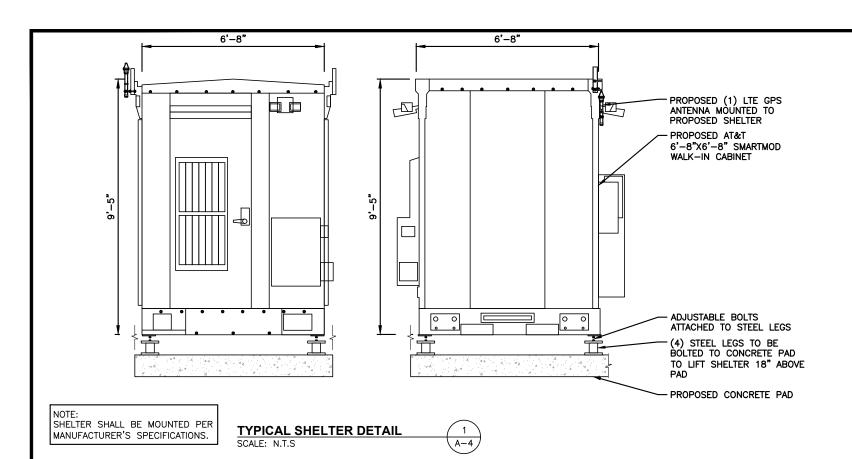


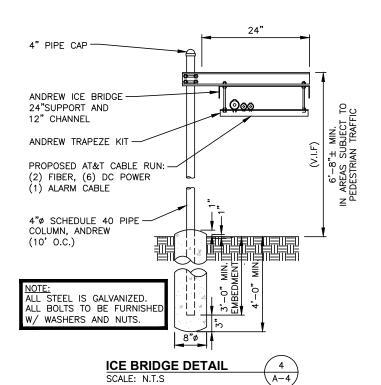
**RAYCAP** 

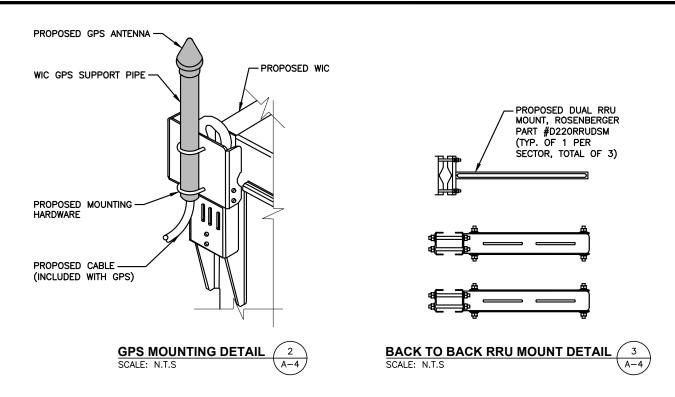
620

 $\Xi$ 

 $\overline{\mathbf{e}}$ 

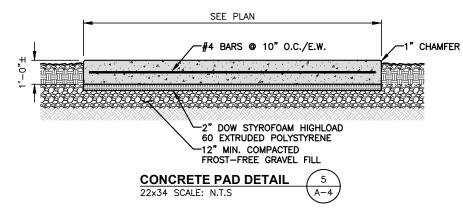






#### FOUNDATION NOTES & CONCRETE SPECIFICATIONS:

- 1. FOUNDATION AREA SHALL BE EXCAVATED TO THE DEPTH AND DIMENSIONS SHOWN ON THE PLANS. EXISTING LEDGE AND ALL OTHER EXISTING UNSUITABLE MATERIAL SHALL BE REMOVED AND LEGALLY DISPOSED OF OFF—SITE. THE SUBGRADE SHALL BE ROLLED WITH A 1-TON, VIBRATORY, WALK-BEHIND ROLLER AT A SPEED OF LESS THAN 2 FPS, 6 PASSES MINIMUM, TO PROVIDE UNYIELDING SURFACE.
- 2. UNDERCUT SOFT OR "WEAVING" AREAS A MINIMUM OF 12 INCHES DEEP. BACKFILL UNDERCUT AREA WITH FILL MEETING THE SPECIFICATIONS OF STRUCTURAL FILL.
- 3. CONCRETE TO HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH (f'c)=4000 psi. CONCRETE TO BE AIR ENTRAINED, DESIRED AIR CONTENT TO BE 6% (PLUS OR MINUS 2%)
- 4. REINFORCING BAR TO BE ASTM A615 GRADE 60.
- 5. WELDED WIRE FABRIC TO CONFORM TO THE REQUIREMENTS OF ASTM A185. WIRES FOR FABRIC TO CONFORM TO THE REQUIREMENTS OF ASTM A82.
- 6. COORDINATE WITH MANUFACTURER OF PREFABRICATED SHELTER FOR LOCATION OF ATTACHMENTS TO BASE SLAB.
- 7. ALL REINFORCING TO HAVE MINIMUM CONCRETE COVER PER ACI SPECIFICATIONS.
- 8. ALL CONCRETE MATERIALS AND WORKMANSHIP SHALL CONFORM TO LATEST EDITION OF ACI 318 AND APPLICABLE STATE BUILDING CODE.





NORTH ANDOVER, MA 01845



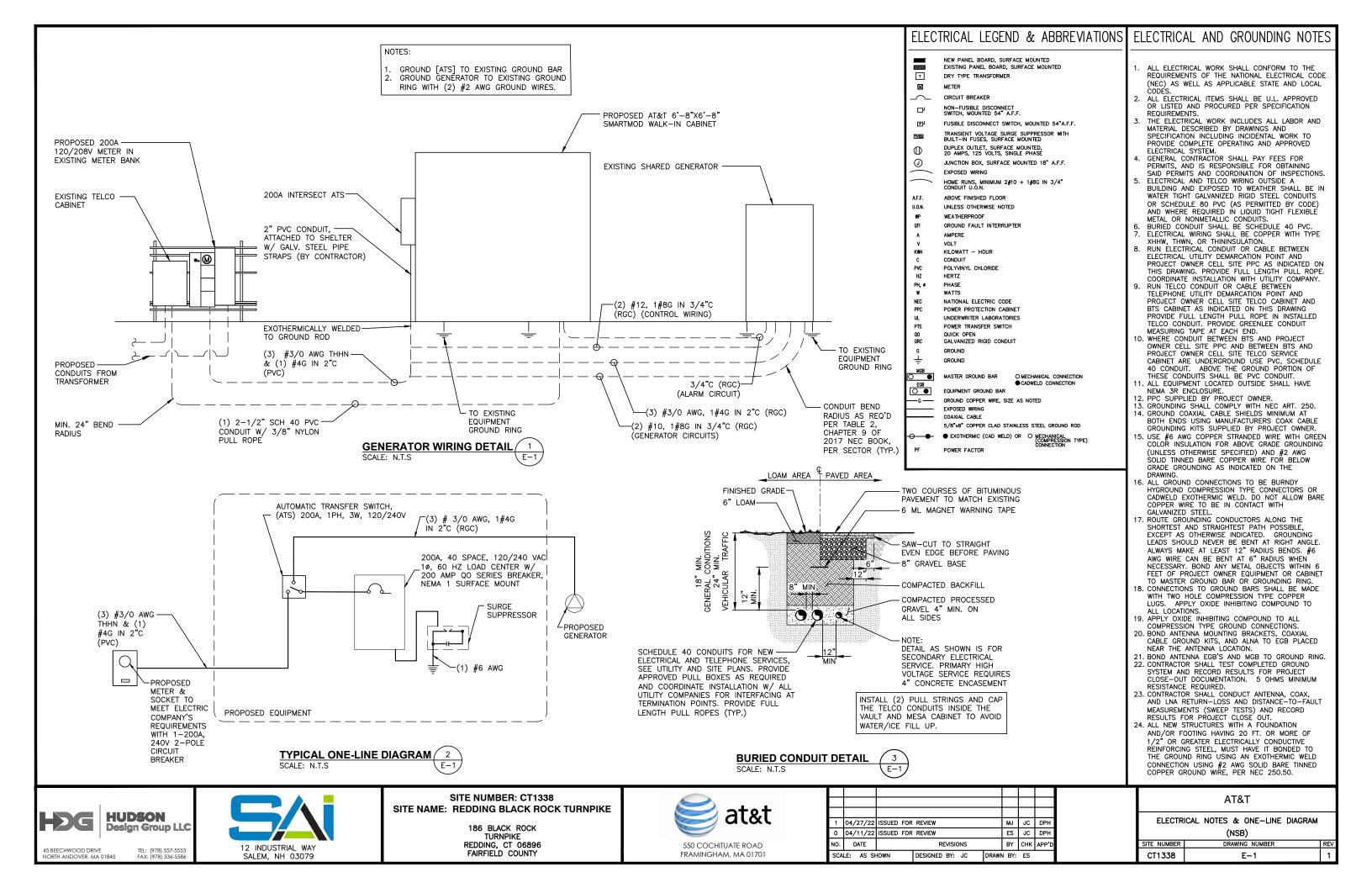
**SITE NUMBER: CT1338** SITE NAME: REDDING BLACK ROCK TURNPIKE

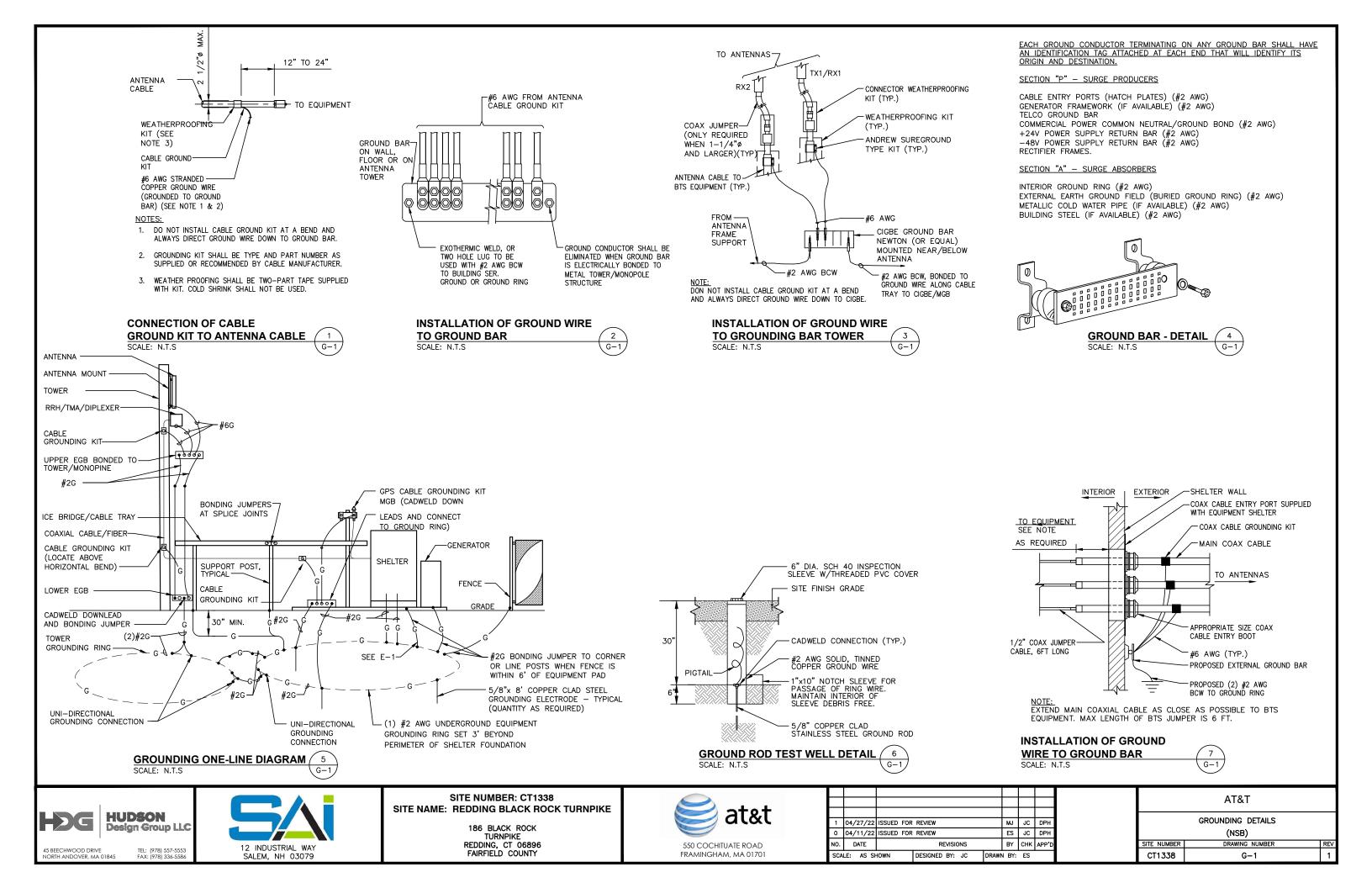
> 186 BLACK ROCK TURNPIKE REDDING, CT 06896 FAIRFIELD COUNTY

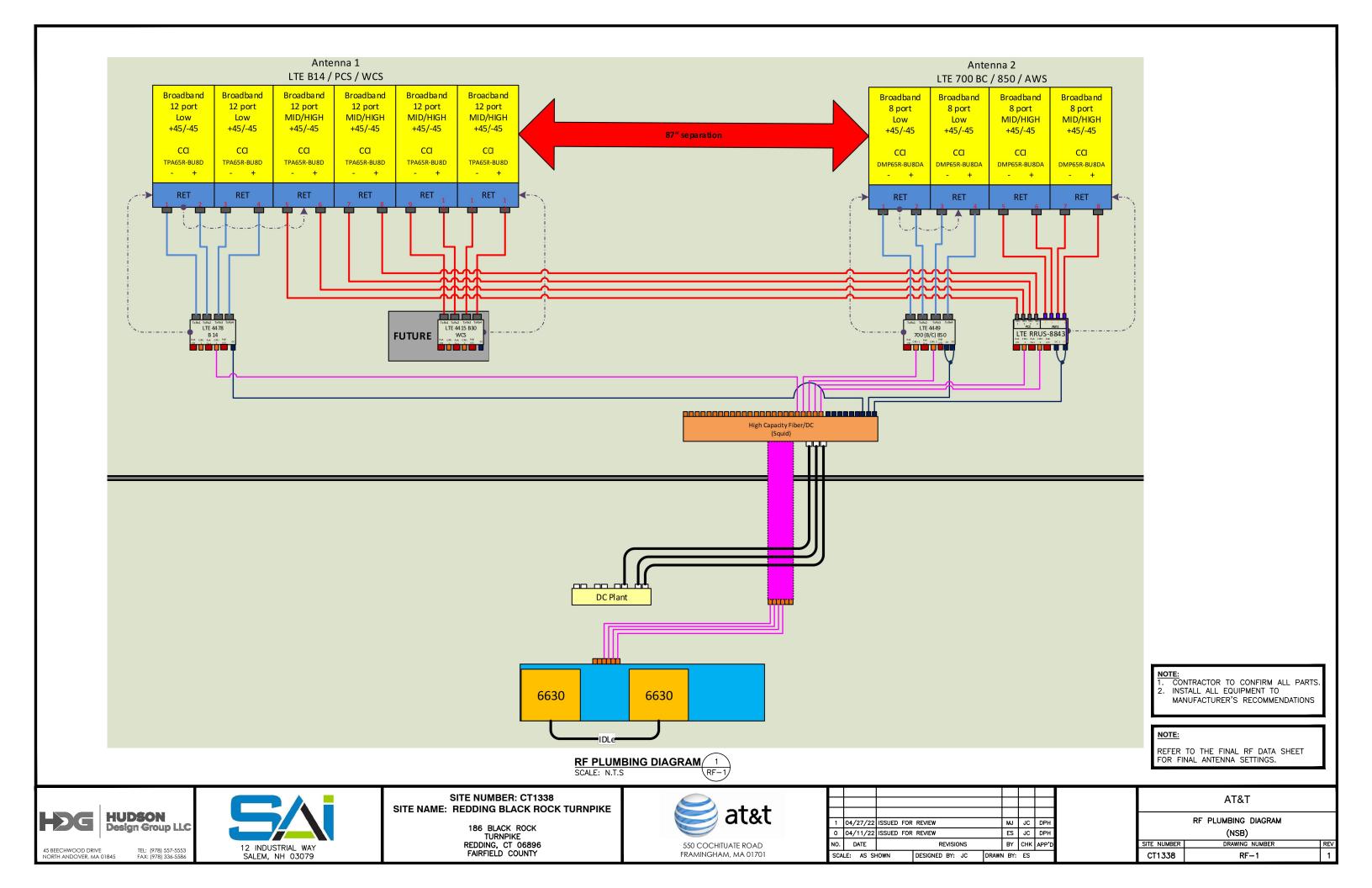


1	04/27/22	ISSUED FOR	REVIEW				MJ	JC	DPH
0	04/11/22	/11/22 ISSUED FOR REVIEW ES JC DPH					DPH		
Ю.	DATE	REVISIONS					BY	СНК	APP'D
SCA	LE: AS SI	HOWN	DESIGNED	BY:	JC	DRAW	N BY:	ES	

AT&T					
DETAILS					
	(NSB)				
SITE NUMBER	DRAWING NUMBER	REV			
CT1338 A-4 1					







## **ATTACHMENT 3**

## COMPREHENSIVE STRUCTURAL ANALYSIS REPORT

For

SITE NUMBER: CT1338
SITE NAME: REDDING BLACK ROCK ROAD

186 Black Rock Road Redding, CT 06896

## **Antennas Mounted on the Monopole**



Prepared for:





Dated: March 18, 2022

Prepared by:





45 Beechwood Drive North Andover, MA 01845 (P) 978.557.5553 (F) 978.336.5586 www.hudsondesigngroupllc.com



#### **SCOPE OF WORK:**

Hudson Design Group LLC (HDG) has been authorized by AT&T to conduct a structural evaluation of the 149' monopole supporting the proposed AT&T's antennas located at elevation 145' above the ground level.

This report represents this office's findings, conclusions and recommendations pertaining to the support of AT&T's existing and proposed antennas listed below.

The following documents were used for our reference:

- Geotechnical Report prepared by Terracon, dated November 15, 2013.
- Foundation Design Drawings prepared by Valmont dated November 13, 2015.
- Pole Design Drawings prepared by Valmont dated December 11, 2015.
- Tower Structural Analysis prepared by HDG dated July 21, 2021.

#### **CONCLUSION SUMMARY:**

Based on our evaluation, we have determined that the existing monopole <u>is in conformance</u> with the ANSI/TIA-222-H Standard for the loading considered under the criteria listed in this report. <u>The tower structure is rated at 43.4%</u> - (<u>Pole Section L2 from EL.106.42</u>' to EL.130.5' Controlling).

#### **FOUNDATION SUMMARY:**

Based on our evaluation, we have determined that the proposed foundation <u>is in</u> <u>conformance</u> with the ANSI/TIA-222-H Standard for the loading considered under the criteria listed in this report. The foundation is rated at **32.5%** - (Bearing (Controlling).



### APPURTENANCES CONFIGURATION:

Tenant	Appurtenances	Elev.	Mount
	(1) 2' Dish	153'	Side Mount Standoff
AT&T	(3) TPA65R-BU6DA-K Antennas	145'	VFA12—WLL-30120
AT&T	(3) DMP65R-BU6DA-K Antennas	145'	VFA12-WLL-30120
AT&T	(3) 4478 B14 RRH's	145'	VFA12-WLL-30120
AT&T	(3) 4449 B5/B12 RRH's	145'	VFA12-WLL-30120
AT&T	(3) 8843 B2/B66A RRH's	145'	VFA12-WLL-30120
AT&T	(2) DC9-48-60-24-8C-EV Surge	145'	VFA12-WLL-30120
	(3) SBNHH-1D65B Antennas	135'	Platform
	(6) JAHH-65B-R3B Antennas	135'	Platform
	(3) MT6407-77A Antennas	135'	Platform
	(3) B2-B66A RRH-BR049	135'	Platform
	(3) B5/B13 RRH-BR04C	135'	Platform
	(3) CBC78T-DS-43-2X	135'	Platform
	(2) OVP Boxes	135'	Platform
	(1) 10' Dipole Antenna	70.5'	Side Mount Standoff
	(1) 3" x 15' Omni Antenna	70.5	Side Mount Standoff

<sup>\*</sup>Proposed AT&T Appurtenances shown in Bold.

### AT&T EXISTING/PROPOSED COAX CABLES:

Tenant	Coax Cables	Elev.	Mount		
AT&T	(4) 7/8" Cables	149'	Tower Face		
AT&T	(4) DC Power Cables	145'	Tower Face		
AT&T	(1) Fiber Cable	145'	Tower Face		
AT&T	(2) 2" Conduits	145'	Tower Face		

<sup>\*</sup>Proposed AT&T Coax Cables shown in Bold.

### **ANALYSIS RESULTS SUMMARY:**

Component	Max. Stress Ratio			Comments
Pole Section L1	35.2 %	130.5 – 149.0	PASS	
Pole Section L2	43.4 %	106.42 – 130.5	PASS	Controlling
Pole Section L3	35.5 %	79.59 – 106.42	PASS	
Pole Section L4	35.6 %	46.08 – 79.59	PASS	
Pole Section L5	34.7 %	0 – 46.08	PASS	
Base plate & Anchor Rods	31.8 %	0 -	PASS	

### **FOUNDATION COMPARISON SUMMARY:**

	Stress Ratio	Pass/Fail	Comments
Bearing	32.5 %	PASS	Controlling
Overturning	27.3 <b>%</b>	PASS	
Shear	6.3 %	PASS	



#### **DESIGN CRITERIA:**

1. EIA/TIA-222-H Structural Standards for Steel Antenna Towers and Antenna Supporting Structures

County: Fairfield

Ultimate Wind Speed: 130 mph (3 second gust)

Structural Class: III
Exposure Category: B
Topographic Category: 1
Nominal Ice Thickness: 1 inch

2. Approximate height above grade to proposed antennas: 145'

\*Calculations and referenced documents are attached.

#### **ASSUMPTIONS:**

- 1. The appurtenances configuration is as stated in this report. All antennas, coax cables and waveguide cables are assumed to be properly installed and supported as per the manufacturer's requirements.
- 2. The monopole and foundation are properly constructed and maintained. All structural members and their connections are assumed to be in good condition and are free from defects with no deterioration to its member capacities.
- 3. The support mounts and platforms are not analyzed and are considered adequate to support the loading. The analysis is limited to the primary support structure itself.

#### SUPPORT RECOMMENDATIONS:

HDG recommends that the proposed antennas, RRH's, and surge arrestors be mounted on the proposed sector frame supported by the monopole.



31.42 24.08 18.50 18 18 18 18 18 18 18 18 18 18 18 18 18 1
---

#### **DESIGNED APPURTENANCE LOADING**

TYPE	ELEVATION	TYPE	ELEVATION	
Andrew 2' w/Radome	153	JAHH-65B-R3B Antenna w/ Mounting	135	
2' Side Mount Standoff (Dish Mount)	149.5	Pipe (Verizon)		
VFA12-WLL-30120 (ATI)	145	MT6407-77A Antenna w/ Mounting	135	
TPA65R-BU6DA-K Antenna w/	145	Pipe (Verizon)		
Mounting Pipe (ATI)		B2/B66A RRH-BR049 RRH (Verizon)	135	
DMP65R-BU6DA-K Antenna w/	145	B5/B13 RRH-BR04C RRH (Verizon)	135	
Mounting Pipe (ATI)		CBC78T-DS-43-2X (Verizon)	135	
B14 4478 RRH (ATI)	145	OVP Box (Verizon)	135	
4449 B5/B12 RRH w/Mount Pipe (ATI)	145	SBNHH-1D65B w/ Mount Pipe	135	
B2/B66A 8843 RRH (ATI)	145	(Verizon)		
DC9-48-60-24-8C-EV Surge Arrestor (ATI)	145	JAHH-65B-R3B Antenna w/ Mounting Pipe (Verizon)	135	
VFA12-WLL-30120 (ATI)	145	JAHH-65B-R3B Antenna w/ Mounting	135	
TPA65R-BU6DA-K Antenna w/	145	Pipe (Verizon)		
Mounting Pipe (ATI)		MT6407-77A Antenna w/ Mounting	135	
DMP65R-BU6DA-K Antenna w/	145	Pipe (Verizon)		
Mounting Pipe (ATI)		B2/B66A RRH-BR049 RRH (Verizon)	135	
B14 4478 RRH (ATI)	145	B5/B13 RRH-BR04C RRH (Verizon)	135	
4449 B5/B12 RRH w/Mount Pipe (ATI)	145	CBC78T-DS-43-2X (Verizon)	135	
B2/B66A 8843 RRH (ATI)	145	SBNHH-1D65B w/ Mount Pipe	135	
DC9-48-60-24-8C-EV Surge Arrestor	145	(Verizon)	405	
(ATI)		JAHH-65B-R3B Antenna w/ Mounting Pipe (Verizon)	135	
VFA12-WLL-30120 (ATI)	145	JAHH-65B-R3B Antenna w/ Mounting	135	
TPA65R-BU6DA-K Antenna w/ Mounting Pipe (ATI)	145	Pipe (Verizon)		
DMP65R-BU6DA-K Antenna w/ Mounting Pipe (ATI)	145	MT6407-77A Antenna w/ Mounting Pipe (Verizon)	135	
B14 4478 RRH (ATI)	145	B2/B66A RRH-BR049 RRH (Verizon)	135	
4449 B5/B12 RRH w/Mount Pipe (ATI)	145	B5/B13 RRH-BR04C RRH (Verizon)	135	
B2/B66A 8843 RRH (ATI)	145	CBC78T-DS-43-2X (Verizon)	135	
SitePro1 LWRM Ring Mount	145	Omni 3"x15' (OTHER)	70.5	
PiROD 13' Low Profile Platform	135	8' Stand off Arm (OTHER)	70.5	
(Verizon)	133	10' Dipole (OTHER)	70.5	
SBNHH-1D65B w/ Mount Pipe	135	8' Stand off Arm (OTHER)	70.5	
(Verizon)	100	SitePro1 UQB4 Ring Mount (OTHER)	70.5	
JAHH-65B-R3B Antenna w/ Mounting Pipe (Verizon)	135	,	1	

#### **MATERIAL STRENGTH**

GRADE	Fy	Fu	GRADE	Fy	Fu
Δ572-65	65 kei	80 kei			

#### **TOWER DESIGN NOTES**

- gned for Exposure B to the TIA-222-H Standard. gned for a 130.0 mph basic wind in accordance with the TIA-222-H Standard. so designed for a 50.0 mph basic wind with 1.00 in ice. Ice is considered to thickness with height.
- are based upon a 60.0 mph wind.
- Category III.

Category 1 with Crest Height of 0.00 ft her tower sections have flange connections. In the constant of the control of the cont 2 and AISC Specifications.

nbers are "hot dipped" galvanized in accordance with ASTM A123 and ASTM

fabricated with ER-70S-6 electrodes.

#### HUDSON Design Group LLC

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553

FAX: (978)336-5586

Job:		
Project: CT1338		
Client: SAI / AT&T	Drawn by: AM	App'd:
Code: TIA-222-H	Date: 03/17/22	Scale: NTS
Path:	SIS SOETIMADE Try Toward Try Projecte AT 8 T/CT/CT (*	Dwg No. E-

tnxTower	Job	<b>Page</b> 1 of 19	
HUDSON Design Group LLC 45 Beachwood Drive	Project	CT1338	Date 14:32:40 03/17/22
North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586	Client	SAI / AT&T	Designed by AM

### **Tower Input Data**

The tower is a monopole.

This tower is designed using the TIA-222-H standard.

The following design criteria apply:

Tower base elevation above sea level: 0.00 ft.

Basic wind speed of 130.0 mph.

Risk Category III.

Exposure Category B.

Simplified Topographic Factor Procedure for wind speed-up calculations is used.

Topographic Category: 1. Crest Height: 0.00 ft.

Nominal ice thickness of 1.0000 in.

Ice thickness is considered to increase with height.

Ice density of 56.0 pcf.

A wind speed of 50.0 mph is used in combination with ice.

Temperature drop of 50.0 °F.

Deflections calculated using a wind speed of 60.0 mph.

Weld together tower sections have flange connections..

Connections use galvanized A325 bolts, nuts and locking devices. Installation per TIA/EIA-222 and AISC Specifications..

Tower members are "hot dipped" galvanized in accordance with ASTM A123 and ASTM A153 Standards...

Welds are fabricated with ER-70S-6 electrodes..

A non-linear (P-delta) analysis was used.

Pressures are calculated at each section.

Stress ratio used in pole design is 1.

Local bending stresses due to climbing loads, feed line supports, and appurtenance mounts are not considered.

## **Tapered Pole Section Geometry**

Section	Elevation	Section Length	Splice Length	Number of	Top Diameter	Bottom Diameter	Wall Thickness	Bend Radius	Pole Grade
	ft	ft	ft	Sides	in	in	in	in	
L1	149.00-130.50	18.50	0.00	18	19.7500	25.0400	0.1875	0.7500	A572-65
									(65 ksi)
L2	130.50-106.42	24.08	4.58	18	25.0400	31.9300	0.2500	1.0000	A572-65
									(65 ksi)
L3	106.42-79.58	31.42	5.42	18	30.1195	39.1100	0.3750	1.5000	A572-65
									(65 ksi)
L4	79.58-46.08	38.92	6.42	18	36.8091	47.9400	0.4375	1.7500	A572-65
									(65 ksi)
L5	46.08-0.00	52.50		18	45.2289	60.2500	0.5000	2.0000	A572-65
									(65 ksi)

### **Tapered Pole Properties**

### **HUDSON Design Group LLC**

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		2 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Section	Tip Dia.	Area	I	r	C	I/C	J	It/Q	w	w/t
	in	$in^2$	$in^4$	in	in	$in^3$	in <sup>4</sup>	$in^{\bar{2}}$	in	
L1	20.0258	11.6421	562.8837	6.9447	10.0330	56.1032	1126.5082	5.8222	3.1460	16.779
	25.3974	14.7903	1154.1331	8.8226	12.7203	90.7315	2309.7848	7.3966	4.0770	21.744
L2	25.3877	19.6709	1527.2635	8.8004	12.7203	120.0649	3056.5365	9.8373	3.9670	15.868
	32.3840	25.1381	3187.4245	11.2464	16.2204	196.5067	6379.0430	12.5714	5.1797	20.719
L3	31.8571	35.4034	3957.2806	10.5593	15.3007	258.6336	7919.7682	17.7051	4.6410	12.376
	39.6555	46.1043	8739.4947	13.7509	19.8679	439.8806	17490.4890	23.0565	6.2234	16.596
L4	38.8835	50.5066	8441.3263	12.9119	18.6990	451.4311	16893.7599	25.2581	5.7084	13.048
	48.6121	65.9632	18804.9347	16.8634	24.3535	772.1650	37634.6134	32.9878	7.6674	17.526
L5	47.7147	70.9848	17942.3631	15.8788	22.9763	780.9077	35908.3353	35.4991	7.0803	14.161
	61.1023	94.8233	42768.9025	21.2113	30.6070	1397.3569	85594.0814	47.4206	9.7240	19.448

Tower Elevation	Gusset Area (per face)	Gusset Thickness	Gusset Grade	Adjust. Factor $A_f$	Adjust. Factor A <sub>r</sub>	Weight Mult.	Double Angle Stitch Bolt Spacing Diagonals	Double Angle Stitch Bolt Spacing Horizontals	Double Angle Stitch Bolt Spacing Redundants
ft	$ft^2$	in					in	in	in
L1				1	1	1			
149.00-130.50									
L2				1	1	1			
130.50-106.42									
L3				1	1	1			
106.42-79.58									
L4 79.58-46.08				1	1	1			
L5 46.08-0.00				1	1	1			

## **Monopole Base Plate Data**

Base Plate Da	ıta
Base plate is square	
Base plate is grouted	
Anchor bolt grade	A615-75
Anchor bolt size	2.2500 in
Number of bolts	24
Embedment length	54.0000 in
$\mathbf{f_c}$	4.0 ksi
Grout space	2.0000 in
Base plate grade	A572-50
Base plate thickness	3.5000 in
Bolt circle diameter	67.6800 in
Outer diameter	74.8200 in
Inner diameter	45.0000 in
Base plate type	Plain Plate

## Feed Line/Linear Appurtenances - Entered As Round Or Flat

Description		Allow		Component	Placement	Total	Number			Perimeter	Weight
	or	Shield	From	Туре		Number	Per Row	Spacing	Diameter		
	Leg		Torque		ft			in	in	in	plf
			Calculation								
***											

## Feed Line/Linear Appurtenances - Entered As Area

tnxTower	Job		Page 3 of 19
HUDSON Design Group LLC 45 Beachwood Drive	Project	CT1338	Date 14:32:40 03/17/22
North Andover, MA 01845  Phone: (978)557-5553  EAY: (978)336-5586	Client	SAI / AT&T	Designed by AM

Description	Face or	Allow Shield	Exclude From	Component Type	Placement	Total Number		$C_A A_A$	Weight
	Leg	Sincia	Torque	Type	ft	1 vumoer		ft²/ft	plf
			Calculation		<i>J</i> -			<i>J</i> • • <i>J</i> •	P U
7/8"	Α	No	Yes	Inside Pole	149.00 - 8.00	2	No Ice	0.00	0.54
							1/2" Ice	0.00	0.54
							1" Ice	0.00	0.54
7/8"	Α	No	Yes	Inside Pole	71.00 - 8.00	2	No Ice	0.00	0.54
							1/2" Ice	0.00	0.54
							1" Ice	0.00	0.54
1-5/8	В	No	Yes	Inside Pole	135.00 - 8.00	6	No Ice	0.00	1.04
(Verizon)							1/2" Ice	0.00	1.04
· · · · · · · · · · · · · · · · · · ·							1" Ice	0.00	1.04
-5/8 Fiber Cable	В	No	Yes	Inside Pole	135.00 - 8.00	2	No Ice	0.00	1.04
							1/2" Ice	0.00	1.04
							1" Ice	0.00	1.04
***									
DC Cable	A	No	Yes	Inside Pole	145.00 - 8.00	4	No Ice	0.00	0.88
							1/2" Ice	0.00	0.88
							1" Ice	0.00	0.88
Fiber	A	No	Yes	Inside Pole	145.00 - 8.00	1	No Ice	0.00	0.48
							1/2" Ice	0.00	0.48
							1" Ice	0.00	0.48
2" Conduit	Α	No	Yes	Inside Pole	145.00 - 8.00	2	No Ice	0.00	1.00
							1/2" Ice	0.00	1.00
***							1" Ice	0.00	1.00

FAX: (978)336-5586

## Feed Line/Linear Appurtenances Section Areas

Tower	Tower	Face	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation				In Face	Out Face	Ü
	ft		$ft^2$	ft²	$ft^2$	$ft^2$	lb
L1	149.00-130.50	A	0.000	0.000	0.000	0.000	106.86
		В	0.000	0.000	0.000	0.000	37.44
		C	0.000	0.000	0.000	0.000	0.00
L2	130.50-106.42	Α	0.000	0.000	0.000	0.000	170.29
		В	0.000	0.000	0.000	0.000	200.35
		C	0.000	0.000	0.000	0.000	0.00
L3	106.42-79.58	Α	0.000	0.000	0.000	0.000	189.81
		В	0.000	0.000	0.000	0.000	223.31
		C	0.000	0.000	0.000	0.000	0.00
L4	79.58-46.08	A	0.000	0.000	0.000	0.000	263.83
		В	0.000	0.000	0.000	0.000	278.72
		C	0.000	0.000	0.000	0.000	0.00
L5	46.08-0.00	A	0.000	0.000	0.000	0.000	310.43
		В	0.000	0.000	0.000	0.000	316.83
		C	0.000	0.000	0.000	0.000	0.00

## Feed Line/Linear Appurtenances Section Areas - With Ice

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_A A_A$	$C_A A_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	$ft^2$	ft <sup>2</sup>	ft²	$ft^2$	lb

# tnxTower Job

HUDSON Design Group LLC 45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		4 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Tower	Tower	Face	Ice	$A_R$	$A_F$	$C_AA_A$	$C_AA_A$	Weight
Section	Elevation	or	Thickness			In Face	Out Face	
	ft	Leg	in	$ft^2$	$ft^2$	ft²	ft²	lb
L1	149.00-130.50	A	1.328	0.000	0.000	0.000	0.000	106.86
		В		0.000	0.000	0.000	0.000	37.44
		C		0.000	0.000	0.000	0.000	0.00
L2	130.50-106.42	A	1.306	0.000	0.000	0.000	0.000	170.29
		В		0.000	0.000	0.000	0.000	200.35
		C		0.000	0.000	0.000	0.000	0.00
L3	106.42-79.58	A	1.275	0.000	0.000	0.000	0.000	189.81
		В		0.000	0.000	0.000	0.000	223.31
		C		0.000	0.000	0.000	0.000	0.00
L4	79.58-46.08	A	1.226	0.000	0.000	0.000	0.000	263.83
		В		0.000	0.000	0.000	0.000	278.72
		C		0.000	0.000	0.000	0.000	0.00
L5	46.08-0.00	A	1.106	0.000	0.000	0.000	0.000	310.43
		В		0.000	0.000	0.000	0.000	316.83
		C		0.000	0.000	0.000	0.000	0.00

<b>—</b> :	_	
Discrete	IOWAR	Dade
		Luaus

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weight
	Leg	JF -	Lateral Vert	najustment			170111	Sitte	
			ft	0	ft		$ft^2$	$ft^2$	lb
			ft ft						
2' Side Mount Standoff	A	From Face	1.00	0.0000	149.50	No Ice	1.00	1.00	30.00
(Dish Mount)			0.00			1/2" Ice	1.50	1.50	50.00
			0.00			1" Ice	2.00	2.00	70.00
SitePro1 LWRM Ring Mount	Α	None		0.0000	145.00	No Ice	0.00	1.40	265.00
_						1/2" Ice	0.00	0.00	344.50
						1" Ice	0.00	0.00	424.00
*SECTOR A*									
VFA12-WLL-30120	A	From Face	3.00	0.0000	145.00	No Ice	13.20	9.20	658.00
(AT&T)			0.00			1/2" Ice	19.50	14.60	804.00
			0.00			1" Ice	25.80	19.50	1015.00
TPA65R-BU6DA-K Antenna	A	From Face	4.00	0.0000	145.00	No Ice	12.73	7.04	90.90
w/ Mounting Pipe			6.00			1/2" Ice	13.23	7.99	180.74
(AT&T)			0.00			1" Ice	13.73	8.82	278.91
DMP65R-BU6DA-K	Α	From Face	4.00	0.0000	145.00	No Ice	12.73	7.04	101.90
Antenna w/ Mounting Pipe			-6.00			1/2" Ice	13.23	7.99	191.74
(AT&T)			0.00			1" Ice	13.73	8.82	289.91
B14 4478 RRH	Α	From Face	2.50	0.0000	145.00	No Ice	2.02	1.25	60.00
(AT&T)			-2.00			1/2" Ice	2.20	1.40	77.66
,			-1.00			1" Ice	2.39	1.56	98.08
4449 B5/B12 RRH w/Mount	Α	From Face	2.50	0.0000	145.00	No Ice	2.51	2.27	21.80
Pipe			2.00			1/2" Ice	2.84	2.67	50.95
(AT&T)			-1.00			1" Ice	3.19	3.09	84.20
B2/B66A 8843 RRH	Α	From Face	2.50	0.0000	145.00	No Ice	1.64	1.35	72.00
(AT&T)			2.00			1/2" Ice	1.80	1.50	89.60
,			-1.00			1" Ice	1.97	1.65	109.91
DC9-48-60-24-8C-EV Surge	Α	From Face	2.50	0.0000	145.00	No Ice	1.14	1.14	29.00
Arrestor			2.00			1/2" Ice	1.79	1.79	49.30
(AT&T) *SECTOR B*			1.00			1" Ice	2.00	2.00	72.38
VFA12-WLL-30120	В	From Face	3.00	0.0000	145.00	No Ice	13.20	9.20	658.00
(AT&T)			0.00			1/2" Ice	19.50	14.60	804.00
(11101)			0.00			1" Ice	25.80	19.50	1015.00

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		5 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weight
	Leg		Lateral Vert						
			ft	0	ft		$ft^2$	$ft^2$	lb
			ft ft		J		v	3	
TPA65R-BU6DA-K Antenna	В	From Face	4.00	0.0000	145.00	No Ice	12.73	7.04	90.90
w/ Mounting Pipe			6.00			1/2" Ice	13.23	7.99	180.74
(AT&T)			0.00	0.0000	1.15.00	1" Ice	13.73	8.82	278.91
DMP65R-BU6DA-K	В	From Face	4.00	0.0000	145.00	No Ice	12.73	7.04	101.90
Antenna w/ Mounting Pipe (AT&T)			-6.00 0.00			1/2" Ice 1" Ice	13.23 13.73	7.99 8.82	191.74
B14 4478 RRH	В	From Face	2.50	0.0000	145.00	No Ice	2.02	1.25	289.91 60.00
(AT&T)	ь	110III 1 acc	-2.00	0.0000	143.00	1/2" Ice	2.02	1.40	77.66
(11141)			-1.00			1" Ice	2.39	1.56	98.08
4449 B5/B12 RRH w/Mount	В	From Face	2.50	0.0000	145.00	No Ice	2.51	2.27	21.80
Pipe			2.00			1/2" Ice	2.84	2.67	50.95
(AT&T)			-1.00			1" Ice	3.19	3.09	84.20
B2/B66A 8843 RRH	В	From Face	2.50	0.0000	145.00	No Ice	1.64	1.35	72.00
(AT&T)			2.00			1/2" Ice	1.80	1.50	89.60
			-1.00			1" Ice	1.97	1.65	109.91
DC9-48-60-24-8C-EV Surge	В	From Face	2.50	0.0000	145.00	No Ice	1.14	1.14	29.00
Arrestor			2.00			1/2" Ice	1.79	1.79	49.30
(AT&T) *SECTOR C*			1.00			1" Ice	2.00	2.00	72.38
VFA12-WLL-30120	C	From Face	3.00	0.0000	145.00	No Ice	13.20	9.20	658.00
(AT&T)			0.00			1/2" Ice	19.50	14.60	804.00
			0.00			1" Ice	25.80	19.50	1015.00
TPA65R-BU6DA-K Antenna	C	From Face	4.00	0.0000	145.00	No Ice	12.73	7.04	90.90
w/ Mounting Pipe			6.00			1/2" Ice	13.23	7.99	180.74
(AT&T)	C	г г	0.00	0.0000	1.45.00	1" Ice	13.73	8.82	278.91
DMP65R-BU6DA-K	С	From Face	4.00	0.0000	145.00	No Ice	12.73	7.04	101.90
Antenna w/ Mounting Pipe			-6.00 0.00			1/2" Ice 1" Ice	13.23 13.73	7.99 8.82	191.74 289.91
(AT&T) B14 4478 RRH	С	From Face	2.50	0.0000	145.00	No Ice	2.02	1.25	60.00
(AT&T)	C	rioin race	-2.00	0.0000	143.00	1/2" Ice	2.02	1.40	77.66
(AI&I)			-1.00			1" Ice	2.39	1.56	98.08
4449 B5/B12 RRH w/Mount	C	From Face	2.50	0.0000	145.00	No Ice	2.51	2.27	21.80
Pipe		110111111100	2.00	0.0000	1.0.00	1/2" Ice	2.84	2.67	50.95
(AT&T)			-1.00			1" Ice	3.19	3.09	84.20
B2/B66A 8843 RRH	C	From Face	2.50	0.0000	145.00	No Ice	1.64	1.35	72.00
(AT&T)			2.00			1/2" Ice	1.80	1.50	89.60
***			-1.00			1" Ice	1.97	1.65	109.91
SitePro1 UQB4 Ring Mount	C	None		0.0000	70.50	No Ice	0.00	1.20	352.00
(OTHER)						1/2" Ice	0.00	0.00	457.60
,						1" Ice	0.00	0.00	563.20
8' Stand off Arm	A	From Face	4.00	0.0000	70.50	No Ice	4.80	0.53	275.00
(OTHER)			0.00			1/2" Ice	5.25	0.63	317.29
			0.00			1" Ice	5.70	0.73	365.48
10' Dipole	A	From Face	8.00	0.0000	70.50	No Ice	6.67	6.67	25.00
(OTHER)			0.00			1/2" Ice	7.82	7.82	60.81
			5.00			1" Ice	8.65	8.65	105.11
8' Stand off Arm	В	From Face	4.00	0.0000	70.50	No Ice	4.80	0.53	275.00
(OTHER)			0.00			1/2" Ice	5.25	0.63	317.29
O: 211 151	D	E E	0.00	0.0000	70.50	1" Ice	5.70	0.73	365.48
Omni 3"x15"	В	From Face	8.00	0.0000	70.50	No Ice	4.50	4.50	50.00
(OTHER)			0.00 10.50			1/2" Ice 1" Ice	6.03 7.58	6.03 7.58	82.48 124.58
*SECTOR A*									
PiROD 13' Low Profile	A	None		0.0000	135.00	No Ice	15.70	15.70	1300.00
Platform						1/2" Ice	20.10	20.10	1765.00
(Verizon)						1" Ice	24.50	24.50	2230.00

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		6 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Description	Face or	Offset Type	Offsets: Horz	Azimuth Adjustment	Placement		$C_AA_A$ Front	$C_AA_A$ Side	Weight
	Leg	JF -	Lateral Vert	. <b>.</b>					
			ft	0	ft		$ft^2$	$ft^2$	lb
			ft ft		J.		J.	Ji	
SBNHH-1D65B w/ Mount	A	From Face	3.00	0.0000	135.00	No Ice	8.42	7.09	66.55
Pipe			-6.00			1/2" Ice	8.98	8.27	135.68
(Verizon) JAHH-65B-R3B Antenna w/	A	From Face	0.00 3.00	0.0000	135.00	1" Ice No Ice	9.50 9.11	9.17 7.71	212.84 103.74
Mounting Pipe	А	Fiolii Face	2.50	0.0000	133.00	1/2" Ice	9.58	8.53	180.55
(Verizon)			0.00			1" Ice	10.05	9.37	266.12
JAHH-65B-R3B Antenna w/	A	From Face	3.00	0.0000	135.00	No Ice	9.11	7.71	103.74
Mounting Pipe			3.75			1/2" Ice	9.58	8.53	180.55
(Verizon)			0.00			1" Ice	10.05	9.37	266.12
MT6407-77A Antenna w/	A	From Face	3.00	0.0000	135.00	No Ice	5.43	3.27	109.00
Mounting Pipe			6.00			1/2" Ice	5.97	3.99	154.17
(Verizon)			0.00	0.0000	125.00	1" Ice	6.46	4.59	204.90
B2/B66A RRH-BR049 RRH	A	From Face	2.00	0.0000	135.00	No Ice	1.88	1.25	98.00
(Verizon)			-3.00 0.00			1/2" Ice 1" Ice	2.05 2.22	1.39 1.54	116.34 137.47
B5/B13 RRH-BR04C RRH	A	From Face	2.00	0.0000	135.00	No Ice	1.88	1.01	82.00
(Verizon)	Α	110m racc	3.00	0.0000	133.00	1/2" Ice	2.05	1.14	98.43
(venzen)			0.00			1" Ice	2.22	1.28	117.53
CBC78T-DS-43-2X	A	From Leg	2.50	0.0000	135.00	No Ice	0.37	0.51	22.00
(Verizon)			6.00			1/2" Ice	0.00	0.00	0.00
			0.00			1" Ice	0.00	0.00	0.00
OVP Box	A	From Leg	2.50	0.0000	135.00	No Ice	1.14	1.14	29.00
(Verizon)			5.50			1/2" Ice	1.79	1.79	49.30
*SECTOR B*			0.00			1" Ice	2.00	2.00	72.38
SBNHH-1D65B w/ Mount	В	From Face	3.00	0.0000	135.00	No Ice	8.42	7.09	66.55
Pipe			-6.00			1/2" Ice	8.98	8.27	135.68
(Verizon)			0.00			1" Ice	9.50	9.17	212.84
JAHH-65B-R3B Antenna w/	В	From Face	3.00	0.0000	135.00	No Ice	9.11	7.71	103.74
Mounting Pipe			2.50			1/2" Ice	9.58	8.53	180.55
(Verizon)	_		0.00			1" Ice	10.05	9.37	266.12
JAHH-65B-R3B Antenna w/	В	From Face	3.00	0.0000	135.00	No Ice	9.11	7.71	103.74
Mounting Pipe			3.75			1/2" Ice	9.58	8.53	180.55
(Verizon) MT6407-77A Antenna w/	В	From Face	0.00 3.00	0.0000	135.00	1" Ice No Ice	10.05 5.43	9.37 3.27	266.12 109.00
Mounting Pipe	ь	Fiolii Face	6.00	0.0000	133.00	1/2" Ice	5.43	3.27	154.17
(Verizon)			0.00			1" Ice	6.46	4.59	204.90
B2/B66A RRH-BR049 RRH	В	From Face	2.00	0.0000	135.00	No Ice	1.88	1.25	98.00
(Verizon)			-3.00			1/2" Ice	2.05	1.39	116.34
,			0.00			1" Ice	2.22	1.54	137.47
B5/B13 RRH-BR04C RRH	В	From Face	2.00	0.0000	135.00	No Ice	1.88	1.01	82.00
(Verizon)			3.00			1/2" Ice	2.05	1.14	98.43
			0.00			1" Ice	2.22	1.28	117.53
CBC78T-DS-43-2X	A	From Leg	2.50	0.0000	135.00	No Ice	0.37	0.51	22.00
(Verizon)			6.00 0.00			1/2" Ice 1" Ice	$0.00 \\ 0.00$	0.00	$0.00 \\ 0.00$
*SECTOR C*			0.00			1 100	0.00	0.00	0.00
SBNHH-1D65B w/ Mount	C	From Face	3.00	0.0000	135.00	No Ice	8.42	7.09	66.55
Pipe	_	1101111 400	-6.00	0.000	122.00	1/2" Ice	8.98	8.27	135.68
(Verizon)			0.00			1" Ice	9.50	9.17	212.84
JAHH-65B-R3B Antenna w/	C	From Face	3.00	0.0000	135.00	No Ice	9.11	7.71	103.74
Mounting Pipe			2.50			1/2" Ice	9.58	8.53	180.55
(Verizon)			0.00			1" Ice	10.05	9.37	266.12
JAHH-65B-R3B Antenna w/	C	From Face	3.00	0.0000	135.00	No Ice	9.11	7.71	103.74
Mounting Pipe			3.75			1/2" Ice	9.58	8.53	180.55
(Verizon)	C	ь ь	0.00	0.0000	125.00	1" Ice	10.05	9.37	266.12
MT6407-77A Antenna w/	C	From Face	3.00	0.0000	135.00	No Ice	5.43	3.27	109.00

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		7 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client		Designed by
	SAI / AT&T	AM

Description	Face or Leg	Offset Type	Offsets: Horz Lateral Vert	Azimuth Adjustment	Placement		$C_AA_A$ Front	C₄A₄ Side	Weight
			ft ft ft	0	ft		ft²	ft²	lb
Mounting Pipe			6.00			1/2" Ice	5.97	3.99	154.17
(Verizon)			0.00			1" Ice	6.46	4.59	204.90
B2/B66A RRH-BR049 RRH	C	From Face	2.00	0.0000	135.00	No Ice	1.88	1.25	98.00
(Verizon)			-3.00			1/2" Ice	2.05	1.39	116.34
			0.00			1" Ice	2.22	1.54	137.47
B5/B13 RRH-BR04C RRH	C	From Face	2.00	0.0000	135.00	No Ice	1.88	1.01	82.00
(Verizon)			3.00			1/2" Ice	2.05	1.14	98.43
` ,			0.00			1" Ice	2.22	1.28	117.53
CBC78T-DS-43-2X	C	From Leg	2.50	0.0000	135.00	No Ice	0.37	0.51	22.00
(Verizon)		J	6.00			1/2" Ice	0.00	0.00	0.00
• • •			0.00			1" Ice	0.00	0.00	0.00

					Dis	shes					
Description	Face or Leg	Dish Type	Offset Type	Offsets: Horz Lateral	Azimuth Adjustment	3 dB Beam Width	Elevation	Outside Diameter		Aperture Area	Weight
				Vert ft	٥	0	ft	ft		ft <sup>2</sup>	lb
Andrew 2' w/Radome	A	Paraboloid w/Radome	From Face	2.00 0.00	0.0000		153.00	2.00	No Ice 1/2" Ice	3.14 3.41	70.00 282.00
***				0.00					1" Ice	3.68	494.00

## **Tower Pressures - No Ice**

 $G_H = 1.100$ 

Section	Z	$K_Z$	$q_z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_AA_A$	$C_AA_A$
Elevation					а				%	In	Out
					С					Face	Face
ft	ft		psf	$ft^2$	е	$ft^2$	$ft^2$	$ft^2$		$ft^2$	$ft^2$
L1	139.39	1.087	44.7	35.014	Α	0.000	35.014	35.014	100.00	0.000	0.000
149.00-130.50					В	0.000	35.014		100.00	0.000	0.000
					C	0.000	35.014		100.00	0.000	0.000
L2	117.97	1.036	42.6	57.964	Α	0.000	57.964	57.964	100.00	0.000	0.000
130.50-106.42					В	0.000	57.964		100.00	0.000	0.000
					C	0.000	57.964		100.00	0.000	0.000
L3	92.51	0.967	39.7	79.975	Α	0.000	79.975	79.975	100.00	0.000	0.000
106.42-79.58					В	0.000	79.975		100.00	0.000	0.000
					C	0.000	79.975		100.00	0.000	0.000
L4 79.58-46.08	62.53	0.864	35.4	122.129	Α	0.000	122.129	122.129	100.00	0.000	0.000
					В	0.000	122.129		100.00	0.000	0.000
					C	0.000	122.129		100.00	0.000	0.000
L5 46.08-0.00	22.32	0.7	29.3	208.929	Α	0.000	208.929	208.929	100.00	0.000	0.000
					В	0.000	208.929		100.00	0.000	0.000
					C	0.000	208.929		100.00	0.000	0.000

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		8 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client		Designed by
	SAI / AT&T	AM

## **Tower Pressure - With Ice**

 $G_H = 1.100$ 

Section	Z	$K_Z$	$q_z$	$t_Z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_AA_A$
Elevation						a				%	In	Out
						С					Face	Face
ft	ft		psf	in	ft <sup>2</sup>	е	$ft^2$	$ft^2$	$ft^2$		$ft^2$	$ft^2$
L1	139.39	1.087	6.6	1.3282	39.109	Α	0.000	39.109	39.109	100.00	0.000	0.000
149.00-130.50						В	0.000	39.109		100.00	0.000	0.000
						C	0.000	39.109		100.00	0.000	0.000
L2	117.97	1.036	6.3	1.3062	63.207	Α	0.000	63.207	63.207	100.00	0.000	0.000
130.50-106.42						В	0.000	63.207		100.00	0.000	0.000
						C	0.000	63.207		100.00	0.000	0.000
L3 106.42-79.58	92.51	0.967	5.9	1.2749	85.818	A	0.000	85.818	85.818	100.00	0.000	0.000
						В	0.000	85.818		100.00	0.000	0.000
						C	0.000	85.818		100.00	0.000	0.000
L4 79.58-46.08	62.53	0.864	5.2	1.2259	129.247	A	0.000	129.247	129.247	100.00	0.000	0.000
						В	0.000	129.247		100.00	0.000	0.000
						C	0.000	129.247		100.00	0.000	0.000
L5 46.08-0.00	22.32	0.7	4.3	1.1059	218.344	Α	0.000	218.344	218.344	100.00	0.000	0.000
						В	0.000	218.344		100.00	0.000	0.000
						C	0.000	218.344		100.00	0.000	0.000

## **Tower Pressure - Service**

 $G_H = 1.100$ 

Section	z	$K_Z$	$q_z$	$A_G$	F	$A_F$	$A_R$	$A_{leg}$	Leg	$C_A A_A$	$C_A A_A$
Elevation					a			_	%	In	Out
					c					Face	Face
ft	ft		psf	$ft^2$	е	ft²	$ft^2$	$ft^2$		ft <sup>2</sup>	$ft^2$
L1	139.39	1.087	8.5	35.014	Α	0.000	35.014	35.014	100.00	0.000	0.000
149.00-130.50					В	0.000	35.014		100.00	0.000	0.000
					C	0.000	35.014		100.00	0.000	0.000
L2	117.97	1.036	8.1	57.964	Α	0.000	57.964	57.964	100.00	0.000	0.000
130.50-106.42					В	0.000	57.964		100.00	0.000	0.000
					C	0.000	57.964		100.00	0.000	0.000
L3	92.51	0.967	7.6	79.975	Α	0.000	79.975	79.975	100.00	0.000	0.000
106.42-79.58					В	0.000	79.975		100.00	0.000	0.000
					C	0.000	79.975		100.00	0.000	0.000
L4 79.58-46.08	62.53	0.864	6.7	122.129	Α	0.000	122.129	122.129	100.00	0.000	0.000
					В	0.000	122.129		100.00	0.000	0.000
					C	0.000	122.129		100.00	0.000	0.000
L5 46.08-0.00	22.32	0.7	5.6	208.929	Α	0.000	208.929	208.929	100.00	0.000	0.000
					В	0.000	208.929		100.00	0.000	0.000
					C	0.000	208.929		100.00	0.000	0.000

## **Tower Forces - No Ice - Wind Normal To Face**

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		9 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	W	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf			a2			
ft	lb	lb	е						ft <sup>2</sup>	lb	plf	
L1	144.30	831.98	Α	1	0.73	44.7	1	1	35.014	1255.65	67.87	C
149.00-130.50			В	1	0.73		1	1	35.014			
			C	1	0.73		1	1	35.014			
L2	370.64	1835.81	Α	1	0.73	42.6	1	1	57.964	1981.96	82.31	C
130.50-106.42			В	1	0.73		1	1	57.964			
			C	1	0.73		1	1	57.964			
L3	413.12	4357.23	Α	1	0.73	39.7	1	1	79.975	2551.06	95.05	С
106.42-79.58			В	1	0.73		1	1	79.975			
			C	1	0.73		1	1	79.975			
L4	542.55	7712.43	Α	1	0.73	35.4	1	1	122.129	3471.74	103.63	С
79.58-46.08			В	1	0.73		1	1	122.129			
			C	1	0.73		1	1	122.129			
L5 46.08-0.00	627.25	14810.52	Α	1	0.73	29.3	1	1	208.929	4917.34	106.71	С
			В	1	0.73		1	1	208.929			
			С	1	0.73		1	1	208.929			
Sum Weight:	2097.86	29547.97						OTM	971688.79	14177.75		
	.,,,,,,								lb-ft	2,,,,,		

## **Tower Forces - No Ice - Wind 60 To Face**

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	e						$ft^2$	lb	plf	
L1	144.30	831.98	Α	1	0.73	44.7	1	1	35.014	1255.65	67.87	C
149.00-130.50			В	1	0.73		1	1	35.014			
			C	1	0.73		1	1	35.014			
L2	370.64	1835.81	Α	1	0.73	42.6	1	1	57.964	1981.96	82.31	C
130.50-106.42			В	1	0.73		1	1	57.964			
			C	1	0.73		1	1	57.964			
L3	413.12	4357.23	Α	1	0.73	39.7	1	1	79.975	2551.06	95.05	С
106.42-79.58			В	1	0.73		1	1	79.975			
			C	1	0.73		1	1	79.975			
L4	542.55	7712.43	Α	1	0.73	35.4	1	1	122.129	3471.74	103.63	C
79.58-46.08			В	1	0.73		1	1	122.129			
			C	1	0.73		1	1	122.129			
L5 46.08-0.00	627.25	14810.52	Α	1	0.73	29.3	1	1	208.929	4917.34	106.71	С
			В	1	0.73		1	1	208.929			
			C	1	0.73		1	1	208.929			
Sum Weight:	2097.86	29547.97						OTM	971688.79	14177.75		
									lb-ft			

## **Tower Forces - No Ice - Wind 90 To Face**

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	e						$ft^2$	lb	plf	
L1	144.30	831.98	Α	1	0.73	44.7	1	1	35.014	1255.65	67.87	C

### HUDSON Design Group LLC

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		<b>Page</b> 10 of 19
		10 01 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf			_			
ft	lb	lb	е						$ft^2$	lb	plf	
149.00-130.50			В	1	0.73		1	1	35.014			
			C	1	0.73		1	1	35.014			
L2	370.64	1835.81	A	1	0.73	42.6	1	1	57.964	1981.96	82.31	C
130.50-106.42			В	1	0.73		1	1	57.964			
			C	1	0.73		1	1	57.964			
L3	413.12	4357.23	Α	1	0.73	39.7	1	1	79.975	2551.06	95.05	C
106.42-79.58			В	1	0.73		1	1	79.975			
			C	1	0.73		1	1	79.975			
L4	542.55	7712.43	Α	1	0.73	35.4	1	1	122.129	3471.74	103.63	C
79.58-46.08			В	1	0.73		1	1	122.129			
			C	1	0.73		1	1	122.129			
L5 46.08-0.00	627.25	14810.52	Α	1	0.73	29.3	1	1	208.929	4917.34	106.71	C
			В	1	0.73		1	1	208.929			
			C	1	0.73		1	1	208.929			
Sum Weight:	2097.86	29547.97						OTM	971688.79	14177.75		
									lb-ft			

## **Tower Forces - With Ice - Wind Normal To Face**

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf						
ft	lb	lb	e						$ft^2$	lb	plf	
L1	144.30	1551.51	Α	1	1.2	6.6	1	1	39.109	341.05	18.44	C
149.00-130.50			В	1	1.2		1	1	39.109			
			C	1	1.2		1	1	39.109			
L2	370.64	2992.46	Α	1	1.2	6.3	1	1	63.207	525.55	21.82	C
130.50-106.42			В	1	1.2		1	1	63.207			
			C	1	1.2		1	1	63.207			
L3	413.12	5900.74	Α	1	1.2	5.9	1	1	85.678	664.58	24.76	C
106.42-79.58			В	1	1.2		1	1	85.678			
			C	1	1.2		1	1	85.678			
L4	542.55	9961.88	Α	1	1.2	5.2	1	1	128.974	891.54	26.61	C
79.58-46.08			В	1	1.2		1	1	128.974			
			C	1	1.2		1	1	128.974			
L5 46.08-0.00	627.25	18255.24	Α	1	1.2	4.3	1	1	217.422	1244.36	27.00	C
			В	1	1.2		1	1	217.422			
			C	1	1.2		1	1	217.422			
Sum Weight:	2097.86	38661.82						OTM	254542.72	3667.08		
									lb-ft			

## **Tower Forces - With Ice - Wind 60 To Face**

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	e						$ft^2$	lb	plf	
L1	144.30	1551.51	Α	1	1.2	6.6	1	1	39.109	341.05	18.44	C
149.00-130.50			В	1	1.2		1	1	39.109			

	Job		Page
tnxTower			11 of 19
HUDSON Design Group LLC	Project		Date
45 Beachwood Drive		CT1338	14:32:40 03/17/22
North Andover, MA 01845 Phone: (978)557-5553	Client	SAI / AT&T	Designed by

AM

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			c			psf			_			
ft	lb	lb	е						$ft^2$	lb	plf	
			C	1	1.2		1	1	39.109			
L2	370.64	2992.46	Α	1	1.2	6.3	1	1	63.207	525.55	21.82	C
130.50-106.42			В	1	1.2		1	1	63.207			
			C	1	1.2		1	1	63.207			
L3	413.12	5900.74	Α	1	1.2	5.9	1	1	85.678	664.58	24.76	C
106.42-79.58			В	1	1.2		1	1	85.678			
			C	1	1.2		1	1	85.678			
L4	542.55	9961.88	Α	1	1.2	5.2	1	1	128.974	891.54	26.61	С
79.58-46.08			В	1	1.2		1	1	128.974			
			C	1	1.2		1	1	128.974			
L5 46.08-0.00	627.25	18255.24	Α	1	1.2	4.3	1	1	217.422	1244.36	27.00	С
			В	1	1.2		1	1	217.422			
			C	1	1.2		1	1	217.422			
Sum Weight:	2097.86	38661.82						OTM	254542.72	3667.08		
S									lb-ft			

FAX: (978)336-5586

## **Tower Forces - With Ice - Wind 90 To Face**

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	a									Face
			С			psf						
ft	lb	lb	е						$ft^2$	lb	plf	
L1	144.30	1551.51	Α	1	1.2	6.6	1	1	39.109	341.05	18.44	C
149.00-130.50			В	1	1.2		1	1	39.109			
			C	1	1.2		1	1	39.109			
L2	370.64	2992.46	Α	1	1.2	6.3	1	1	63.207	525.55	21.82	C
130.50-106.42			В	1	1.2		1	1	63.207			
			C	1	1.2		1	1	63.207			
L3	413.12	5900.74	Α	1	1.2	5.9	1	1	85.678	664.58	24.76	C
106.42-79.58			В	1	1.2		1	1	85.678			
			C	1	1.2		1	1	85.678			
L4	542.55	9961.88	Α	1	1.2	5.2	1	1	128.974	891.54	26.61	C
79.58-46.08			В	1	1.2		1	1	128.974			
			C	1	1.2		1	1	128.974			
L5 46.08-0.00	627.25	18255.24	Α	1	1.2	4.3	1	1	217.422	1244.36	27.00	C
			В	1	1.2		1	1	217.422			
			C	1	1.2		1	1	217.422			
Sum Weight:	2097.86	38661.82						OTM	254542.72	3667.08		
									lb-ft			

## **Tower Forces - Service - Wind Normal To Face**

Section Elevation	Add Weight	Self Weight	F a	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl. Face
	"		с			psf			c.?	11	1.0	
ft	lb	lb	е						$ft^2$	lb	plf	
L1	144.30	831.98	Α	1	0.73	8.5	1	1	35.014	239.32	12.94	C
149.00-130.50			В	1	0.73		1	1	35.014			
			С	1	0.73		1	1	35.014			

tnxTower	Job		<b>Page</b> 12 of 19
HUDSON Design Group LLC 45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586	Project	CT1338	Date 14:32:40 03/17/22
	Client	SAI / AT&T	Designed by AM

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а			_						Face
			c			psf						
ft	lb	lb	е						$ft^2$	lb	plf	
L2	370.64	1835.81	Α	1	0.73	8.1	1	1	57.964	377.75	15.69	C
130.50-106.42			В	1	0.73		1	1	57.964			
			C	1	0.73		1	1	57.964			
L3	413.12	4357.23	Α	1	0.73	7.6	1	1	79.975	486.22	18.12	C
106.42-79.58			В	1	0.73		1	1	79.975			
			C	1	0.73		1	1	79.975			
L4	542.55	7712.43	Α	1	0.73	6.7	1	1	122.129	661.69	19.75	C
79.58-46.08			В	1	0.73		1	1	122.129			
			C	1	0.73		1	1	122.129			
L5 46.08-0.00	627.25	14810.52	Α	1	0.73	5.6	1	1	208.929	937.22	20.34	C
			В	1	0.73		1	1	208.929			
			C	1	0.73		1	1	208.929			
Sum Weight:	2097.86	29547.97						OTM	185198.86	2702.21		
									lb-ft			

# Tower Forces - Service - Wind 60 To Face

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	e						$ft^2$	lb	plf	
L1	144.30	831.98	Α	1	0.73	8.5	1	1	35.014	239.32	12.94	C
149.00-130.50			В	1	0.73		1	1	35.014			
			C	1	0.73		1	1	35.014			
L2	370.64	1835.81	Α	1	0.73	8.1	1	1	57.964	377.75	15.69	C
130.50-106.42			В	1	0.73		1	1	57.964			
			C	1	0.73		1	1	57.964			
L3	413.12	4357.23	Α	1	0.73	7.6	1	1	79.975	486.22	18.12	C
106.42-79.58			В	1	0.73		1	1	79.975			
			C	1	0.73		1	1	79.975			
L4	542.55	7712.43	Α	1	0.73	6.7	1	1	122.129	661.69	19.75	C
79.58-46.08			В	1	0.73		1	1	122.129			
			C	1	0.73		1	1	122.129			
L5 46.08-0.00	627.25	14810.52	Α	1	0.73	5.6	1	1	208.929	937.22	20.34	C
			В	1	0.73		1	1	208.929			
			C	1	0.73		1	1	208.929			
Sum Weight:	2097.86	29547.97						OTM	185198.86	2702.21		
									lb-ft			

# **Tower Forces - Service - Wind 90 To Face**

Section	Add	Self	F	е	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	e						$ft^2$	lb	plf	
L1	144.30	831.98	Α	1	0.73	8.5	1	1	35.014	239.32	12.94	С
149.00-130.50			В	1	0.73		1	1	35.014			
			C	1	0.73		1	1	35.014			
L2	370.64	1835.81	Α	1	0.73	8.1	1	1	57.964	377.75	15.69	C

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		13 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

Section	Add	Self	F	e	$C_F$	$q_z$	$D_F$	$D_R$	$A_E$	F	w	Ctrl.
Elevation	Weight	Weight	а									Face
			С			psf						
ft	lb	lb	е						$ft^2$	lb	plf	
130.50-106.42			В	1	0.73		1	1	57.964			
			C	1	0.73		1	1	57.964			
L3	413.12	4357.23	Α	1	0.73	7.6	1	1	79.975	486.22	18.12	C
106.42-79.58			В	1	0.73		1	1	79.975			
			C	1	0.73		1	1	79.975			
L4	542.55	7712.43	Α	1	0.73	6.7	1	1	122.129	661.69	19.75	C
79.58-46.08			В	1	0.73		1	1	122.129			
			C	1	0.73		1	1	122.129			
L5 46.08-0.00	627.25	14810.52	Α	1	0.73	5.6	1	1	208.929	937.22	20.34	C
			В	1	0.73		1	1	208.929			
			C	1	0.73		1	1	208.929			
Sum Weight:	2097.86	29547.97						OTM	185198.86	2702.21		
									lb-ft			

# **Force Totals**

Load	Vertical	Sum of	Sum of	Sum of	Sum of	Sum of Torques
Case	Forces	Forces	Forces	Overturning	Overturning	
		X	Z	Moments, $M_x$	Moments, $M_z$	
	lb	lb	lb	lb-ft	lb-ft	lb-ft
Leg Weight	29547.97					
Bracing Weight	0.00					
Total Member Self-Weight	29547.97			-2487.85	-340.67	
Total Weight	39143.72			-2487.85	-340.67	
Wind 0 deg - No Ice		2.08	-26092.58	-2601937.77	-711.73	-497.86
Wind 30 deg - No Ice		13155.16	-22588.02	-2252354.50	-1309559.86	-1998.28
Wind 60 deg - No Ice		22798.00	-13048.88	-1302654.87	-2269850.01	-2993.81
Wind 90 deg - No Ice		26351.62	7.01	-1467.68	-2625006.37	-3179.38
Wind 120 deg - No Ice		22822.44	13086.88	1303401.24	-2273536.29	-2478.07
Wind 150 deg - No Ice		13181.88	22662.36	2258699.87	-1313557.01	-1112.76
Member Ice	9113.85					
Total Weight Ice	57246.80			-5133.03	663.76	
Wind 0 deg - Ice		0.89	-6199.97	-607239.02	527.18	29.61
Wind 30 deg - Ice		3118.94	-5368.03	-526372.62	-302019.43	-318.51
Wind 60 deg - Ice		5403.88	-3100.90	-306325.74	-523861.05	-586.72
Wind 90 deg - Ice		6244.31	0.72	-5022.66	-605685.54	-696.34
Wind 120 deg - Ice		5407.70	3106.74	296952.90	-524444.68	-613.16
Wind 150 deg - Ice		3122.78	5380.70	518045.42	-302606.47	-365.69
Total Weight	39143.72			-2487.85	-340.67	
Wind 0 deg - Service		0.40	-4977.38	-498552.88	-411.40	-90.62
Wind 30 deg - Service		2509.44	-4308.86	-431840.61	-250182.74	-380.76
Wind 60 deg - Service		4348.88	-2489.18	-250604.62	-433437.22	-574.70
Wind 90 deg - Service		5026.75	1.34	-2293.41	-501211.69	-613.17
Wind 120 deg - Service		4353.54	2496.43	246719.51	-434139.81	-480.67
Wind 150 deg - Service		2514.53	4323.03	429022.64	-250944.57	-219.38

# **Load Combinations**

Comb.	Description	
No.		

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		14 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client		Designed by
	SAI / AT&T	AM

Comb.	Description
No.	·
1	Dead Only
2	1.2 Dead+1.0 Wind 0 deg - No Ice
3	0.9 Dead+1.0 Wind 0 deg - No Ice
4	1.2 Dead+1.0 Wind 30 deg - No Ice
5	0.9 Dead+1.0 Wind 30 deg - No Ice
6	1.2 Dead+1.0 Wind 60 deg - No Ice
7	0.9 Dead+1.0 Wind 60 deg - No Ice
8	1.2 Dead+1.0 Wind 90 deg - No Ice
9	0.9 Dead+1.0 Wind 90 deg - No Ice
10	1.2 Dead+1.0 Wind 120 deg - No Ice
11	0.9 Dead+1.0 Wind 120 deg - No Ice
12	1.2 Dead+1.0 Wind 150 deg - No Ice
13	0.9 Dead+1.0 Wind 150 deg - No Ice
14	1.2 Dead+1.0 Ice+1.0 Temp
15	1.2 Dead+1.0 Wind 0 deg+1.0 Ice+1.0 Temp
16	1.2 Dead+1.0 Wind 30 deg+1.0 Ice+1.0 Temp
17	1.2 Dead+1.0 Wind 60 deg+1.0 Ice+1.0 Temp
18	1.2 Dead+1.0 Wind 90 deg+1.0 Ice+1.0 Temp
19	1.2 Dead+1.0 Wind 120 deg+1.0 Ice+1.0 Temp
20	1.2 Dead+1.0 Wind 150 deg+1.0 Ice+1.0 Temp
21	Dead+Wind 0 deg - Service
22	Dead+Wind 30 deg - Service
23	Dead+Wind 60 deg - Service
24	Dead+Wind 90 deg - Service
25	Dead+Wind 120 deg - Service
26	Dead+Wind 150 deg - Service

## **Maximum Member Forces**

Section	Elevation	Component	Condition	Gov.	Axial	Major Axis	Minor Axis
No.	ft	Туре		Load		Moment	Moment
				Comb.	lb	lb-ft	lb-ft
L1	149 - 130.5	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-17994.44	824.38	1644.34
			Max. Mx	8	-8288.43	-125901.91	540.62
			Max. My	2	-8298.55	-249.12	124610.07
			Max. Vy	8	13090.96	-125901.91	540.62
			Max. Vx	2	-13000.88	-249.12	124610.07
			Max. Torque	8			803.65
L2	130.5 - 106.42	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-21033.83	837.89	1690.91
			Max. Mx	8	-10469.61	-395972.85	440.50
			Max. My	2	-10477.95	-294.29	392916.17
			Max. Vy	8	14653.71	-395972.85	440.50
			Max. Vx	2	-14563.46	-294.29	392916.17
			Max. Torque	8			803.51
L3	106.42 - 79.58	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-27918.29	841.17	1722.46
			Max. Mx	8	-15745.07	-808237.55	300.54
			Max. My	2	-15751.54	-350.70	802815.36
			Max. Vy	8	17085.45	-808237.55	300.54
			Max. Vx	2	-16994.77	-350.70	802815.36
			Max. Torque	8			802.61
L4	79.58 - 46.08	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-41481.11	615.44	5702.43
			Max. Mx	8	-26256.61	-1436044.0	2377.55
						9	
			Max. My	2	-26264.12	-691.07	1426611.43
			Max. Vy	8	21205.90	-1436044.0	2377.55

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		15 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client		Designed by
	SAI / AT&T	AM

Section No.	Elevation ft	Component Type	Condition	Gov. Load	Axial	Major Axis Moment	Minor Axis Moment
				Comb.	lb	lb-ft	lb-ft
						9	
			Max. Vx	2	-20942.34	-691.07	1426611.43
			Max. Torque	8			3222.69
L5	46.08 - 0	Pole	Max Tension	1	0.00	0.00	0.00
			Max. Compression	14	-66182.80	590.94	5622.42
			Max. Mx	8	-46962.39	-2684428.9	2027.34
						9	
			Max. My	2	-46962.54	-805.21	2661201.00
			Max. Vy	8	26368.74	-2684428.9	2027.34
						9	
			Max. Vx	2	-26108.28	-805.21	2661201.00
			Max. Torque	8			3221.47

## **Maximum Reactions**

Location	Condition	Gov.	Vertical	Horizontal, $X$	Horizontal, 2
		Load	lb	lb	lb
		Comb.			
Pole	Max. Vert	14	66182.80	-0.44	-1.44
	Max. H <sub>x</sub>	1	39143.72	0.00	-0.01
	Max. H <sub>z</sub>	3	35229.31	-2.08	26091.12
	$Max. M_x$	2	2661201.00	-2.08	26090.52
	Max. M <sub>z</sub>	8	2684428.99	-26350.82	-7.01
	Max. Torsion	8	3220.59	-26350.82	-7.01
	Min. Vert	9	35229.31	-26350.14	-7.01
	Min. H <sub>x</sub>	8	46972.44	-26350.82	-7.01
	Min. H <sub>z</sub>	13	35229.35	-13181.85	-22662.30
	$Min. M_x$	12	-2309406.70	-13181.83	-22662.27
	Min. M <sub>z</sub>	14	-590.94	-0.44	-1.44
	Min. Torsion	15	-7.22	-0.89	6199.63

# **Tower Mast Reaction Summary**

Load	Vertical	$Shear_x$	$Shear_z$	Overturning	Overturning	Torque
Combination				Moment, $M_x$	Moment, $M_z$	
	lb	lb	lb	lb-ft	lb-ft	lb-ft
Dead Only	39143.72	-0.00	0.01	-2487.55	-340.65	-0.00
1.2 Dead+1.0 Wind 0 deg - No	46972.41	2.08	-26090.52	-2661201.00	-805.26	-513.19
Ice						
0.9 Dead+1.0 Wind 0 deg - No	35229.31	2.08	-26091.12	-2645184.19	-693.82	-508.93
Ice						
1.2 Dead+1.0 Wind 30 deg - No	46972.47	13155.10	-22587.93	-2303932.28	-1339279.07	-2029.16
Ice						
0.9 Dead+1.0 Wind 30 deg - No	35229.35	13155.12	-22587.96	-2289901.28	-1331473.09	-2019.32
Ice						
1.2 Dead+1.0 Wind 60 deg - No	46972.47	22797.91	-13048.83	-1332710.83	-2321307.65	-3034.79
Ice						
0.9 Dead+1.0 Wind 60 deg - No	35229.35	22797.94	-13048.84	-1324268.64	-2307853.74	-3021.96
Ice						
1.2 Dead+1.0 Wind 90 deg - No	46972.44	26350.82	7.01	-2026.97	-2684428.99	-3220.59
Ice						
0.9 Dead+1.0 Wind 90 deg - No	35229.31	26350.14	7.01	-1247.49	-2668794.25	-3208.20
Ice						

### HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		16 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client		Designed by
	SAI / AT&T	AM

Load Combination	Vertical	$Shear_x$	$Shear_z$	Overturning Moment, $M_x$	Overturning Moment, Mz	Torque
	lb	lb	lb	lb-ft	lb-ft	lb-ft
1.2 Dead+1.0 Wind 120 deg -	46972.47	22822.35	13086.83	1332438.21	-2325088.64	-2506.15
No Ice						
0.9 Dead+1.0 Wind 120 deg -	35229.35	22822.38	13086.84	1325529.00	-2311608.69	-2497.54
No Ice						
1.2 Dead+1.0 Wind 150 deg -	46972.47	13181.83	22662.27	2309406.70	-1343378.18	-1120.40
No Ice						
0.9 Dead+1.0 Wind 150 deg -	35229.35	13181.85	22662.30	2296868.22	-1335544.09	-1117.88
No Ice						
1.2 Dead+1.0 Ice+1.0 Temp	66182.80	0.44	1.44	-5622.42	590.94	-0.31
1.2 Dead+1.0 Wind 0 deg+1.0	66182.80	0.89	-6199.63	-632035.87	523.40	7.22
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 30 deg+1.0	66182.80	3118.78	-5367.73	-547949.32	-314048.44	-353.20
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 60 deg+1.0	66182.80	5403.59	-3100.73	-319149.76	-544711.55	-624.67
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 90 deg+1.0	66182.80	6243.98	0.73	-5853.95	-629796.72	-727.52
Ice+1.0 Temp						
1.2 Dead+1.0 Wind 120	66182.80	5407.41	3106.57	308148.57	-545324.82	-629.13
deg+1.0 Ice+1.0 Temp						
1.2 Dead+1.0 Wind 150	66182.80	3122.61	5380.41	538046.94	-314665.30	-362.26
deg+1.0 Ice+1.0 Temp						
Dead+Wind 0 deg - Service	39143.72	0.39	-4976.55	-507997.90	-423.11	-93.25
Dead+Wind 30 deg - Service	39143.72	2509.03	-4308.14	-440023.20	-254910.25	-386.75
Dead+Wind 60 deg - Service	39143.72	4348.16	-2488.77	-255362.10	-441626.41	-582.58
Dead+Wind 90 deg - Service	39143.72	5025.92	1.34	-2354.69	-510684.24	-620.87
Dead+Wind 120 deg - Service	39143.72	4352.82	2496.02	251370.95	-442345.27	-486.04
Dead+Wind 150 deg - Service	39143.72	2514.12	4322.32	437124.37	-255689.61	-220.99

# **Solution Summary**

	Sui	m of Applied Forces	S		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	PY	PZ	% Error
Comb.	lb	lb	lb	lb	lb	lb	
1	0.00	-39143.72	0.00	0.00	39143.72	-0.01	0.000%
2	2.08	-46972.47	-26092.58	-2.08	46972.41	26090.52	0.004%
3	2.08	-35229.35	-26092.58	-2.08	35229.31	26091.12	0.003%
4	13155.16	-46972.47	-22588.02	-13155.10	46972.47	22587.93	0.000%
5	13155.16	-35229.35	-22588.02	-13155.12	35229.35	22587.96	0.000%
6	22798.00	-46972.47	-13048.88	-22797.91	46972.47	13048.83	0.000%
7	22798.00	-35229.35	-13048.88	-22797.94	35229.35	13048.84	0.000%
8	26351.62	-46972.47	7.01	-26350.82	46972.44	-7.01	0.001%
9	26351.62	-35229.35	7.01	-26350.14	35229.31	-7.01	0.003%
10	22822.44	-46972.47	13086.88	-22822.35	46972.47	-13086.83	0.000%
11	22822.44	-35229.35	13086.88	-22822.38	35229.35	-13086.84	0.000%
12	13181.88	-46972.47	22662.36	-13181.83	46972.47	-22662.27	0.000%
13	13181.88	-35229.35	22662.36	-13181.85	35229.35	-22662.30	0.000%
14	0.00	-66182.80	0.00	-0.44	66182.80	-1.44	0.002%
15	0.89	-66182.80	-6199.97	-0.89	66182.80	6199.63	0.001%
16	3118.94	-66182.80	-5368.03	-3118.78	66182.80	5367.73	0.001%
17	5403.88	-66182.80	-3100.90	-5403.59	66182.80	3100.73	0.001%
18	6244.31	-66182.80	0.72	-6243.98	66182.80	-0.73	0.001%
19	5407.70	-66182.80	3106.74	-5407.41	66182.80	-3106.57	0.001%
20	3122.78	-66182.80	5380.70	-3122.61	66182.80	-5380.41	0.001%
21	0.40	-39143.72	-4977.38	-0.39	39143.72	4976.55	0.002%
22	2509.44	-39143.72	-4308.86	-2509.03	39143.72	4308.14	0.002%
23	4348.88	-39143.72	-2489.18	-4348.16	39143.72	2488.77	0.002%
24	5026.75	-39143.72	1.34	-5025.92	39143.72	-1.34	0.002%
25	4353.54	-39143.72	2496.43	-4352.82	39143.72	-2496.02	0.002%

## HUDSON Design Group LLC

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		17 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client	SAI / AT&T	Designed by AM

	Sur	n of Applied Forces	,		Sum of Reaction	ıs	
Load	PX	PY	PZ	PX	$\dot{P}Y$	PZ	% Error
Comb.	lb	lb	lb	lb	lb	lb	
26	2514.53	-39143.72	4323.03	-2514.12	39143.72	-4322.32	0.002%

## **Maximum Tower Deflections - Service Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	٥	0
L1	149 - 130.5	9.7335	24	0.6405	0.0000
L2	130.5 - 106.42	7.2975	24	0.5975	0.0001
L3	111 - 79.58	5.0934	24	0.4719	0.0000
L4	85 - 46.08	2.8690	24	0.3355	0.0000
L5	52.5 - 0	1.0566	24	0.1887	0.0000

## **Maximum Tower Deflections - Design Wind**

Section	Elevation	Horz.	Gov.	Tilt	Twist
No.		Deflection	Load		
	ft	in	Comb.	0	٥
L1	149 - 130.5	51.1632	8	3.3684	0.0000
L2	130.5 - 106.42	38.3619	8	3.1408	0.0005
L3	111 - 79.58	26.7782	8	2.4816	0.0001
L4	85 - 46.08	15.0839	8	1.7642	0.0000
L5	52.5 - 0	5.5549	8	0.9922	0.0000

## **Base Plate Design Data**

Plate	Number	Anchor Bolt	Actual	Actual	Actual	Actual	Controlling	Ratio
Thickness	of Anchor	Size	Allowable	Allowable	Allowable	Allowable	Condition	
	Bolts		Ratio	Ratio	Ratio	Ratio		
			Bolt	Bolt	Plate	Stiffener		
			Tension	Compression	Stress	Stress		
in		in	lb	lb	ksi	ksi		
3.5000	24	2.2500	77397.62	81283.89	12.502		Bolt T	0.32
			243576.14	404336.40	45.000			~
			0.32	0.20	0.28			•

# **Compression Checks**

Pole Design Data	Po	le [	Des	ian	Data
------------------	----	------	-----	-----	------

Section	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio
No.								' "	$P_u$
	ft		ft	ft		$in^2$	lb	lb	$\Phi P_n$
I.1	149 - 130.5 (1)	TP25.04x19.75x0.1875	18.50	149.00	202.7	14.7903	-8288.43	81354.20	0.102

## HUDSON Design Group LLC 45 Beachwood Drive

45 Beachwood Drive North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586

Job		Page
		18 of 19
Project		Date
	CT1338	14:32:40 03/17/22
Client		Designed by
	SAI / AT&T	АМ

Section No.	Elevation	Size	L	$L_u$	Kl/r	A	$P_u$	$\phi P_n$	Ratio $P_u$
	ft		ft	ft		$in^2$	lb	lb	$\phi P_n$
L2	130.5 - 106.42 (2)	TP31.93x25.04x0.25	24.08	149.00	165.8	24.0982	-10469.60	197935.00	0.053
L3	106.42 - 79.58	TP39.11x30.1195x0.375	31.42	149.00	135.5	44.2584	-15745.10	544970.00	0.029
L4	79.58 - 46.08 (4)	TP47.94x36.8091x0.4375	38.92	149.00	110.3	63.4135	-26256.60	1177710.00	0.022
L5	46.08 - 0 (5)	TP60.25x45.2289x0.5	52.50	149.00	84.3	94.8232	-46962.40	2823220.00	0.017

# Pole Bending Design Data

Section No.	Elevation	Size	$M_{ux}$	$\phi M_{nx}$	Ratio M <sub>ux</sub>	$M_{uy}$	$\phi M_{ny}$	$Ratio$ $M_{uy}$
	ft		lb-ft	lb-ft	$\phi M_{nx}$	lb-ft	lb-ft	$\phi M_{ny}$
L1	149 - 130.5 (1)	TP25.04x19.75x0.1875	125903.33	508939.17	0.247	0.00	508939.17	0.000
L2	130.5 - 106.42 (2)	TP31.93x25.04x0.25	395973.33	1043633.33	0.379	0.00	1043633.33	0.000
L3	106.42 - 79.58	TP39.11x30.1195x0.375	808237.50	2482925.00	0.326	0.00	2482925.00	0.000
L4	79.58 - 46.08 (4)	TP47.94x36.8091x0.4375	1436050.00	4313416.67	0.333	0.00	4313416.67	0.000
L5	46.08 - 0 (5)	TP60.25x45.2289x0.5	2684433.33	8121224.67	0.331	0.00	8121224.67	0.000

# Pole Shear Design Data

Section No.	Elevation	Size	Actual $V_u$	$\phi V_n$	$Ratio$ $V_u$	Actual T <sub>u</sub>	$\phi T_n$	Ratio $T_u$
	ft		lb	lb	$\phi V_n$	lb-ft	lb-ft	$\phi T_n$
L1	149 - 130.5 (1)	TP25.04x19.75x0.1875	13091.00	259571.00	0.050	803.54	564944.17	0.001
L2	130.5 - 106.42	TP31.93x25.04x0.25	14653.70	422924.00	0.035	802.77	1124808.33	0.001
	(2)							
L3	106.42 - 79.58	TP39.11x30.1195x0.375	17085.40	776735.00	0.022	801.64	2529358.33	0.000
	(3)							
L4	79.58 - 46.08	TP47.94x36.8091x0.4375	21205.90	1112910.00	0.019	3221.71	4450783.33	0.001
	(4)							
L5	46.08 - 0 (5)	TP60.25x45.2289x0.5	26368.70	1664150.00	0.016	3220.58	8707833.33	0.000

# Pole Interaction Design Data

Section	Elevation	Ratio	Ratio	Ratio	Ratio	Ratio	Comb.	Allow.	Criteria
No.		$P_u$	$M_{ux}$	$M_{uy}$	$V_u$	$T_u$	Stress	Stress	
	ft	$\phi P_n$	$\phi M_{nx}$	$\phi M_{ny}$	$\phi V_n$	$\phi T_n$	Ratio	Ratio	
L1	149 - 130.5 (1)	0.102	0.247	0.000	0.050	0.001	0.352	1.000	4.8.2
L2	130.5 - 106.42 (2)	0.053	0.379	0.000	0.035	0.001	0.434	1.000	4.8.2

tnxTower	Job		<b>Page</b> 19 of 19
HUDSON Design Group LLC 45 Beachwood Drive	Project	CT1338	Date 14:32:40 03/17/22
North Andover, MA 01845 Phone: (978)557-5553 FAX: (978)336-5586	Client	SAI / AT&T	Designed by AM

Section No.	Elevation	Ratio $P_u$	Ratio $M_{ux}$	Ratio $M_{uy}$	$Ratio\ V_u$	Ratio $T_u$	Comb. Stress	Allow. Stress	Criteria
	ft	$\phi P_n$	$\phi M_{nx}$	$\phi M_{ny}$	$\phi V_n$	$\phi T_n$	Ratio	Ratio	
L3	106.42 - 79.58 (3)	0.029	0.326	0.000	0.022	0.000	0.355	1.000	4.8.2
L4	79.58 - 46.08 (4)	0.022	0.333	0.000	0.019	0.001	0.356	1.000	4.8.2
L5	46.08 - 0 (5)	0.017	0.331	0.000	0.016	0.000	0.347	1.000	4.8.2

# **Section Capacity Table**

Section	Elevation	Component	Size	Critical	P	$ olimits P_{allow} $	%	Pass
No.	ft	Туре		Element	lb	lb	Capacity	Fail
L1	149 - 130.5	Pole	TP25.04x19.75x0.1875	1	-8288.43	81354.20	35.2	Pass
L2	130.5 - 106.42	Pole	TP31.93x25.04x0.25	2	-10469.60	197935.00	43.4	Pass
L3	106.42 - 79.58	Pole	TP39.11x30.1195x0.375	3	-15745.10	544970.00	35.5	Pass
L4	79.58 - 46.08	Pole	TP47.94x36.8091x0.4375	4	-26256.60	1177710.00	35.6	Pass
L5	46.08 - 0	Pole	TP60.25x45.2289x0.5	5	-46962.40	2823220.00	34.7	Pass
							Summary	
						Pole (L2)	43.4	Pass
						Base Plate	31.8	Pass
						RATING =	43.4	Pass

 $Program\ Version\ 8.1.1.0\ -\ 6/3/2021\ File: W:/STRUCTURAL\ DEPARTMENT/ANALYSIS\ SOFTWARE/TnxTower/Tnx\ Projects/AT\&T/CT/CT1338/CT1338.erii$ 

# **Monopole Pier and Pad Foundation**

BU#:

Site Name: CT1338

App. Number:

TIA-222 Revision: H

TIN Y ELE TYOTICION							
Design Reactions							
Shear, <b>S</b> :	26.351	kips					
Moment, M:	2684.43	ft-kips					
Tower Height, H:	149	ft					
Tower Weight, Wt:	46.972	kips					
Base Diameter, <b>BD</b> :	5.02	ft					

Foundation Dimensions						
Depth, <b>D</b> :	10	ft				
Pad Width, <b>W</b> :	25	ft				
Neglected Depth, N:	0	ft				
Thickness, <b>T</b> :	3.00	ft				
Pier Diameter, <b>Pd</b> :	8.00	ft				
Ext. Above Grade, <b>E</b> :	0.50	ft				
BP Dist. Above Pier:	3	in.				
Clear Cover, <b>Cc</b> :	3.0	in				

Soil Properties							
Soil Unit Weight, γ:	0.125	kcf					
Ult. Bearing Capacity, Bc:	10.0	ksf					
Angle of Friction, <b>Φ</b> :	34	deg					
Cohesion, Co:	0.000	ksf					
Passive Pressure, <b>Pp</b> :	0.003	ksf					
Base Friction, <b>μ</b> :	0.50						

Material Properties		
Rebar Yield Strength, Fy:	60000	psi
Concrete Strength, F'c:	4000	psi
Concrete Unit Weight, δc:	0.150	kcf
Seismic Zone, z:	1	

Rebar Properties		
Pier Rebar Size, <b>Sp</b> :	11	
Pier Rebar Quanity, <b>mp</b> :	54	24
Pad Rebar Size, <b>Spad</b> :	9	
Pad Rebar Quanity, mpad:	132	10
Pier Tie Size, <b>St</b> :	5	4
Tie Quanity, <b>mt</b> :	12	7

Design Checks			
	Capacity/ Availability	Demand/ Limits	Check
Req'd Pier Diam.(ft)	8	7.02	OK
Overturning (ft-kips)	9843.53	2684.43	27.3%
Shear Capacity (kips)	420.26	26.35	6.3%
Bearing (ksf)	7.50	2.44	32.5%
Pad Shear - 1-way (kips)	923.14	386.34	41.9%
Pad Shear - 2-way (kips)	2483.22	126.14	5.1%
Pad Moment Capacity (k-ft)	16960.87	862.02	5.1%
Pier Moment Capacity (k-ft)	14446.76	2882.06	19.9%

Effective Date: 9/9/2010

# **ATTACHMENT 4**



December 3, 2021



SAI Communications 12 Industrial Way Salem NH, 03079

RE: Site Number: CT1338 (NSB) FA Number: 10128142

PACE Number: MRCTB057522 PT Number: 2051A12J24

Site Name: REDDING BLACK ROCK ROAD

Site Address: 186 Black Rock Road Redding, CT 06896

### To Whom It May Concern:

Hudson Design Group LLC (HDG) has been authorized by SAI Communications to perform a mount analysis on the proposed AT&T antenna/RRH mounts to determine their capability of supporting the following additional loading:

- (3) TPA65R-BU6DA-K Antennas (71.2"x20.7"x7.7" Wt. = 68 lbs. /each)
- (3) DMP65R-BU6DA-K Antennas (71.2"x20.7"x7.7" Wt. = 80 lbs. /each)
- (3) B14 4478 RRH's (18.1"x13.4"x8.3" Wt. = 60 lbs. /each)
- (3) 4415 B30 RRH's (16.5"x13.4"x5.9" Wt. = 46 lbs. /each)
- (3) B5/B12 4449 RRH's (17.9"x13.2"x9.4" Wt. = 73 lbs. /each)
- (3) B2/B66A 8843 RRH's (14.9"x13.2"x10.9" Wt. = 72 lbs. /each)
- (2) Squid Surge Arrestors (24.0"x9.7" Ø Wt. = 33 lbs.)

Mount fabrication drawings prepared by SitePro1, P/N VFA12-WLL-30120 dated May 3, 2018, P/N MM01 dated May 10, 2010, and P/N LWRM dated August 24, 2012, were used to perform this analysis.

<sup>\*</sup>Proposed equipment shown in bold.

### Mount Analysis Methods:

- This analysis was conducted in accordance with EIA/TIA-222-H, Structural Standards for Steel Antenna Towers and Antenna Supporting Structures, the International Building Code 2015 with 2018 Connecticut State Building Code, and AT&T Mount Technical Directive R16.
- HDG considers this mount to be asymmetrical and has applied wind loads in 30-degree increments
  all around the mount. Per TIA-222-H and Appendix N of the Connecticut State Building Code, the
  max basic wind speed for this site is equal to 130 mph with a max basic wind speed with ice of 50
  mph and a max ice thickness of 1.0 in. An escalated ice thickness of 1.45 in was used for this
  analysis.
- HDG considers this site to be exposure category B; tower is located in an urban/suburban or wooded area with numerous closely spaced obstructions.
- HDG considers this site to be topographic category 1; tower is located on flat terrain or the bottom
  of a hill or ridge.
- HDG considers this site to have a spectral response acceleration parameter at short periods, S<sub>S</sub>, of 0.220 and a spectral response acceleration parameter at a period of 1 second, S<sub>1</sub>, of 0.067.
- The mount has been analyzed with load combinations consisting of 500 lbs live load using a service wind speed of 30 mph wind on the worst-case antenna. Analysis performed on each antenna pipe to determine worst case location; worst case location was antenna position 2.
- The mount has been analyzed with load combinations consisting of a 250 lbs live load in a worst-case location on the mount.
- The proposed mounts are secured to the existing monopole with HSS members secured to ring mounts. The ring mounts are secured around the monopole using threaded rods. HDG considers the threaded rods to be the governing connection member.

Based on our evaluation, we have determined that the (3) <u>Proposed SitePro1 VFA12-WLL-30120</u> mounts, (6) <u>Proposed SitePro1 MM01</u> standoffs, and (2) <u>Proposed SitePro1 LWRM</u> collar mounts <u>ARE CAPABLE</u> of supporting the proposed installation.

	Component	Controlling Load Case	Stress Ratio	Pass/Fail
Proposed Mount Rating	9	LC23	78%	PASS

### **Reference Documents:**

- Fabrication drawings prepared by SitePro1 P/N VFA12-M3-WLL, dated May 3, 2018.
- Fabrication drawings prepared by SitePro1 P/N MM01, dated May 10, 2010.
- Fabrication drawings prepared by SitePro1 P/N LWRM, dated August 24, 2012.

### This determination was based on the following limitations and assumptions:

- 1. HDG is not responsible for any modifications completed prior to and hereafter which HDG was not directly involved.
- 2. All structural members and their connections are assumed to be in good condition and are free from defects with no deterioration to its member capacities.
- 3. All antennas, coax cables and waveguide cables are assumed to be properly installed and supported as per the manufacturer's requirements.
- 4. The proposed mount will be adequately secured to the tower structure per the mount manufacturer's specifications.
- 5. All components pertaining to AT&T's mounts must be tightened and re-plumbed prior to the installation of new appurtenances.
- 6. HDG performed a localized analysis on the mount itself and not on the supporting tower structure.

Please feel free to contact our office should you have any questions.

Respectfully Submitted, Hudson Design Group LLC

Michael Cabral Vice President Daniel P. Hamm, PE Principal



Wind & Ice Calculations

Project Name: REDDING BLACK ROCK ROAD

Project No.: CT1338

Designed By: ID Checked By: MSC



### 2.6.5.2 Velocity Pressure Coeff:



 $Kzmin \le Kz \le 2.01$ 

#### Table 2-4

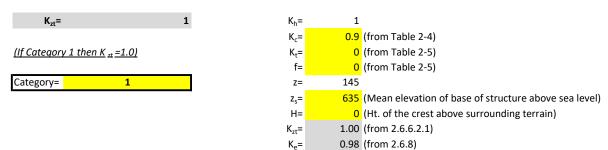
Exposure	Z <sub>g</sub>	α	$K_{zmin}$	K <sub>c</sub>
В	1200 ft	7.0	0.70	0.9
С	900 ft	9.5	0.85	1.0
D	700 ft	11.5	1.03	1.1

#### 2.6.6.2 Topographic Factor:

#### Table 2-5

Topo. Category	K <sub>t</sub>	f
2	0.43	1.25
3	0.53	2.0
4	0.72	1.5

$$K_{zt} = \left[ 1 + (K_c \; K_t / K_h) \right]^2 \qquad \qquad K_h = \, e^{\, \left( f^* z / H \right)} \label{eq:Kzt}$$



### 2.6.10 Design Ice Thickness

Max Ice Thickness =	t <sub>i</sub> =	1.00 in
Importance Factor =	l=	1.25 (from Table 2-3)
	K <sub>iz</sub> =	1.16 (from Sec. 2.6.10)
$t_{iz} = t_i * I * K_{iz} * (K_{zt})^{0.35}$	t <sub>iz</sub> =	1.45 in

Project Name: REDDING BLACK ROCK ROAD

Project No.: CT1338

Designed By: ID Checked By: MSC



### 2.6.9 Gust Effect Factor

### 2.6.9.1 Self Supporting Lattice Structures

G<sub>h</sub> = 1.0 Latticed Structures > 600 ft

G<sub>h</sub> = 0.85 Latticed Structures 450 ft or less

 $G_h = 0.85 + 0.15 [h/150 - 3.0]$ 

h= ht. of structure



G<sub>h</sub>= 0.85

2.6.9.2 Guyed Masts

G<sub>h</sub>= 0.85

2.6.9.3 Pole Structures

G<sub>h</sub>= 1.1

2.6.9 Appurtenances

G<sub>h</sub>= 1.0

### 2.6.9.4 Structures Supported on Other Structures

(Cantilivered tubular or latticed spines, pole, structures on buildings (ht.: width ratio > 5)

 $G_{h}$ = 1.35  $G_{h}$ = 1.00

### 2.6.11.2 Design Wind Force on Appurtenances

 $F = q_z * G_h * (EPA)_A$ 

 $q_z = 0.00256*K_z*K_{zt}*K_s*K_e*K_d*V_{max}^2$ 

K<sub>z</sub>= 1.099 (from 2.6.5.2) K<sub>zt</sub>= 1.0 (from 2.6.6.2.1)

K<sub>s</sub>= 1.0 (from 2.6.7) K<sub>e</sub>= 0.98 (from 2.6.8)

K<sub>d</sub>= 0.95 (from Table 2-2) V<sub>max</sub>= 130 mph (Ultimate Wind Speed)

 $V_{\text{max (ice)}} = \frac{50}{\text{mph}}$   $V_{30} = \frac{30}{\text{mph}}$ 

 $q_z$ = 44.14  $q_{z (ice)}$ = 6.53  $q_{z (30)}$ = 2.35

### Table 2-2

Structure Type	Wind Direction Probability Factor, Kd
Latticed structures with triangular, square or rectangular cross sections	0.85
Tubular pole structures, latticed structures with other cross sections, appurtenances	0.95
Tubular pole structures supporting antennas enclosed within a cylindrical shroud	1.00

Project Name: REDDING BLACK ROCK ROAD

Project No.: CT1338

Designed By: ID Checked By: MSC



### Determine Ca:

Table 2-9

	Force Coefficients (Ca) for Appurtenances											
	Mambar Tyna	Aspect Ratio ≤ 2.5	Aspect Ratio = 7	Aspect Ratio ≥ 25								
	Member Type	Ca	Ca	Ca								
	Flat	1.2	1.4	2.0								
Squ	uare/Rectangular HSS	$1.2 - 2.8(r_s) \ge 0.85$	$1.4 - 4.0(r_s) \ge 0.90$	$2.0 - 6.0(r_s) \ge 1.25$								
Round	C < 39	0.7	0.8	1.2								
	(Subcritical)	0.7	0.8	1.2								
	39 ≤ C ≤ 78	4.14/(C <sup>0.485</sup> )	3.66/(C <sup>0.415</sup> )	46.8/(C <sup>.1.0</sup> )								
	(Transitional)	4.14/(C )	3.66/(C )	46.8/(C )								
	C > 78	0.5	0.6	0.6								
	(Supercritical)	0.5	0.6	0.6								

Aspect Ratio is the overall length/width ratio in the plane normal to the wind direction.

(Aspect ratio is independent of the spacing between support points of a linear appurtenance,

Note: Linear interpolation may be used for aspect ratios other than those shown.

Ice Thickness =	1.45	in	Angle =	0 (deg)	[	Equival	ent Angle =	180 (deg)	
<u>Appurtenances</u>	Height	<u>Width</u>	<u>Depth</u>	Flat Area	Aspect Ratio	<u>Ca</u>	Force (lbs)	Force (lbs) (w/ lce)	Force (lbs) (30 mph)
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.44	1.24	561	98	30
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.44	1.24	561	98	30
B14 4478 RRH	18.1	8.3	13.4	1.04	2.18	1.20	55	13	3
4415 B30 RRH	16.5	5.9	13.4	0.68	2.80	1.21	36	9	2
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.90	1.20	62	14	3
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.20	60	13	3
Surge Arrestor	24.0	9.7	9.7	1.62	2.47	0.70	50	11	3
2" Pipe	2.4	12.0		0.20	0.20	0.70	6		
2-1/2" Pipe	2.9	12.0		0.24	0.24	0.70	7		
3" Pipe	3.5	12.0		0.29	0.29	0.70	9		
5/8" Round Bar	0.6	12.0		0.05	0.05	0.70	2		
3/4" Round Bar	0.8	12.0		0.06	0.06	0.70	2		
PL 3-1/2x5/8 Plate	0.6	12.0		0.05	0.05	1.20	3		
PL 11-1/4x5/8 Plate	0.6	12.0		0.05	0.05	1.20	3		
HSS 4x4	4.0	12.0		0.33	0.33	1.20	18		



				W	IND LOADS							
Angle = 30	(deg)		Ice Thickr	ness =	1.45	in.		I		Equivalent Angle =		(deg)
WIND LOADS WITH NO ICE:	<u>Height</u>	Width	<u>Depth</u>	Flat Area		Aspect	Aspect	<u>Ca (normal)</u>	<u>Ca</u>	Force (III)	Force	Force
				(normal)	(side)	Ratio	Ratio		(side)	(lbs)	(lbs)	(lbs)
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	483
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	483
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	55	89	64
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	36	81	47
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	62	87	68
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	60	72	63
WIND LOADS WITH ICE:												
TPA65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	86
DMP65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	86
B14 4478 RRH	21.0	11.2	16.3	1.63	2.38	1.88	1.29	1.20	1.20	13	19	14
4415 B30 RRH	19.4	8.8	16.3	1.19	2.20	2.20	1.19	1.20	1.20	9	17	11
B5/B12 4449 RRH	20.8	12.3	16.1	1.78	2.33	1.69	1.29	1.20	1.20	14	18	15
B2/B66A 8843 RRH	17.8	13.8	16.1	1.71	1.99	1.29	1.11	1.20	1.20	13	16	14
WIND LOADS AT 30 MPH:												
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	26
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	26
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	3	5	3
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	2	4	3
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	3	5	4
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	3	4	3



				W	IND LOADS							
Angle = 60	(deg)		Ice Thick	ness =	1.45	in.		[	Equivale	ent Angle =	240	(deg)
WIND LOADS WITH NO ICE:  Appurtenances	<u>Height</u>	Width	<u>Depth</u>	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	<u>Ca</u> (normal)	<u>Ca</u> (side)	Force (lbs)	Force (lbs)	Force (lbs)
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	326
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	326
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	55	89	81
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	36	81	70
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	62	87	81
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	60	72	69
WIND LOADS WITH ICE:												
TPA65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	62
DMP65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	62
B14 4478 RRH	21.0	11.2	16.3	1.63	2.38	1.88	1.29	1.20	1.20	13	19	17
4415 B30 RRH	19.4	8.8	16.3	1.19	2.20	2.20	1.19	1.20	1.20	9	17	15
B5/B12 4449 RRH	20.8	12.3	16.1	1.78	2.33	1.69	1.29	1.20	1.20	14	18	17
B2/B66A 8843 RRH	17.8	13.8	16.1	1.71	1.99	1.29	1.11	1.20	1.20	13	16	15
WIND LOADS AT 30 MPH:												
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	17
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	17
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	3	5	4
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	2	4	4
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	3	5	4
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	3	4	4



Angle = 90	(deg)		Ice Thick	ness =	1.45	in.		[	Equivale	ent Angle =	270	(deg)
WIND LOADS WITH NO ICE:												
<u>Appurtenances</u>	<u>Height</u>	Width	<u>Depth</u>	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	<u>Ca</u> (normal)	<u>Ca</u> (side)	Force (lbs)	Force (lbs)	Force (lbs)
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	248
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	248
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	55	89	89
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	36	81	81
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	62	87	87
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	60	72	72
WIND LOADS WITH ICE:												
TPA65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	50
DMP65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	50
B14 4478 RRH	21.0	11.2	16.3	1.63	2.38	1.88	1.29	1.20	1.20	13	19	19
4415 B30 RRH	19.4	8.8	16.3	1.19	2.20	2.20	1.19	1.20	1.20	9	17	17
B5/B12 4449 RRH	20.8	12.3	16.1	1.78	2.33	1.69	1.29	1.20	1.20	14	18	18
B2/B66A 8843 RRH	17.8	13.8	16.1	1.71	1.99	1.29	1.11	1.20	1.20	13	16	16
WIND LOADS AT 30 MPH:												
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	13
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	13
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	3	5	5
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	2	4	4
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	3	5	5
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	3	4	4

Project Name: REDDING BLACK ROCK ROAD
Project No.: CT1338



				WI	ND LOADS							
Angle = 120	(deg)		Ice Thick	ness =	1.45	in.		[	Equivale	ent Angle =	300	(deg)
WIND LOADS WITH NO ICE:  Appurtenances	<u>Height</u>	<u>Width</u>	<u>Depth</u>	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	<u>Ca</u> (normal)	<u>Ca</u> (side)	Force (lbs)	Force (lbs)	Force (lbs)
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	326
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	326
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	55	89	81
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	36	81	70
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	62	87	81
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	60	72	69
WIND LOADS WITH ICE: TPA65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	62
DMP65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	62
B14 4478 RRH	21.0	11.2	16.3	1.63	2.38	1.88	1.29	1.20	1.20	13	19	17
4415 B30 RRH	19.4	8.8	16.3	1.19	2.20	2.20	1.19	1.20	1.20	9	17	15
B5/B12 4449 RRH	20.8	12.3	16.1	1.78	2.33	1.69	1.29	1.20	1.20	14	18	17
B2/B66A 8843 RRH	17.8	13.8	16.1	1.71	1.99	1.29	1.11	1.20	1.20	13	16	15
WIND LOADS AT 30 MPH:												
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	17
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	17
814 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	2	5	4
4415 B30 RRH B5/B12 4449 RRH	16.5 17.9	5.9 9.4	13.4	0.68	1.54	1.90	1.23	1.21	1.20	3	5	4
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	3	4	4



Angle = 150	(deg)		Ice Thick	ness =	1.45	in.			Equivale	ent Angle =	330	(deg)
WIND LOADS WITH NO ICE:												
Appurtenances	<u>Height</u>	Width	<u>Depth</u>	Flat Area (normal)	Flat Area (side)	Ratio (normal)	Ratio (side)	<u>Ca</u> (normal)	<u>Ca</u> (side)	Force (lbs)	Force (lbs)	Force (lbs)
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	483
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	561	248	483
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	55	89	64
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	36	81	47
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	62	87	68
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	60	72	63
WIND LOADS WITH ICE:												
TPA65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	86
DMP65R-BU6DA-K Antenna	74.1	23.6	10.6	12.14	5.45	3.14	6.99	1.23	1.40	97	50	86
B14 4478 RRH	21.0	11.2	16.3	1.63	2.38	1.88	1.29	1.20	1.20	13	19	14
4415 B30 RRH	19.4	8.8	16.3	1.19	2.20	2.20	1.19	1.20	1.20	9	17	11
B5/B12 4449 RRH	20.8	12.3	16.1	1.78	2.33	1.69	1.29	1.20	1.20	14	18	15
B2/B66A 8843 RRH	17.8	13.8	16.1	1.71	1.99	1.29	1.11	1.20	1.20	13	16	14
WIND LOADS AT 30 MPH:												
TPA65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	26
DMP65R-BU6DA-K Antenna	71.2	20.7	7.7	10.24	3.81	3.44	9.25	1.24	1.47	30	13	26
B14 4478 RRH	18.1	8.3	13.4	1.04	1.68	2.18	1.35	1.20	1.20	3	5	3
4415 B30 RRH	16.5	5.9	13.4	0.68	1.54	2.80	1.23	1.21	1.20	2	4	3
B5/B12 4449 RRH	17.9	9.4	13.2	1.17	1.64	1.90	1.36	1.20	1.20	3	5	4
B2/B66A 8843 RRH	14.9	10.9	13.2	1.13	1.37	1.37	1.13	1.20	1.20	3	4	3

Project Name: REDDING BLACK ROCK ROAD

Project No.: CT1338

Designed By: ID Checked By: MSC



#### ICE WEIGHT CALCULATIONS

Thickness of ice: 1.45 in.

Density of ice: 56 pcf

#### TPA65R-BU6DA-K Antenna

Weight of ice based on total radial SF area:

Height (in): 71.2
Width (in): 20.7
Depth (in): 7.7

Total weight of ice on object: 247 lbs

Weight of object: 68.0 lbs

Combined weight of ice and object: 315 lbs

#### **B14 4478 RRH**

Weight of ice based on total radial SF area:

Height (in): 18.1
Width (in): 13.4
Depth (in): 8.3

Total weight of ice on object: 46 lbs

Weight of object: 60.0 lbs

Combined weight of ice and object: 106 lbs

#### B5/B12 4449 RRH

Weight of ice based on total radial SF area:

Height (in): 17.9
Width (in): 13.2
Depth (in): 9.4

Total weight of ice on object: 47 lbs

Weight of object: 73.0 lbs

Combined weight of ice and object: 120 lbs

### **Squid Surge Arrestor**

Weight of ice based on total radial SF area:

Depth (in): 24.0
Diameter(in): 9.7

Total weight of ice on object: 40 lbs

Weight of object: 33 lbs

Combined weight of ice and object: 73 lbs

#### 2-1/2" Pipe

Per foot weight of ice:

diameter (in):

Per foot weight of ice on object:

8 plf

### 5/8" Round Bar

Per foot weight of ice:

diameter (in): 0.625

Per foot weight of ice on object: 4 plf

### PL 3-1/2x5/8 Plate

Weight of ice based on total radial SF area:

Height (in): 3.5
Width (in): 0.625

Per foot weight of ice on object: 9 plf

#### DMP65R-BU6DA-K Antenna

Weight of ice based on total radial SF area:

Height (in): 71.2
Width (in): 20.7
Depth (in): 7.7

Total weight of ice on object: 247 lbs

Weight of object: 80.0 lbs

Combined weight of ice and object: 327 lbs

#### 4415 B30 RRH

Weight of ice based on total radial SF area:

 Height (in):
 16.5

 Width (in):
 13.4

 Depth (in):
 5.9

 Total weight of ice on object:
 39 lbs

 Weight of object:
 46.0 lbs

85 lbs

13 plf

#### B2/B66A 8843 RRH

Weight of ice based on total radial SF area:

Combined weight of ice and object:

Height (in): 14.9
Width (in): 13.2
Depth (in): 10.9

Total weight of ice on object: 41 lbs

Weight of object: 72.0 lbs

Combined weight of ice and object: 113 lbs

### 2" Pipe

Per foot weight of ice:

diameter (in):

Per foot weight of ice on object:

7 plf

### 3" Pipe

Per foot weight of ice:

diameter (in):

Per foot weight of ice on object:

9 plf

#### 3/4" Round Bar

Per foot weight of ice:

diameter (in): 0.75

Per foot weight of ice on object: 4 plf

#### PL 11-1/4x5/8 Plate

Weight of ice based on total radial SF area:

Height (in): 11.25
Width (in): 0.625

Per foot weight of ice on object: 23 plf

### HSS 4x4

Weight of ice based on total radial SF area:

Height (in):

Width (in):

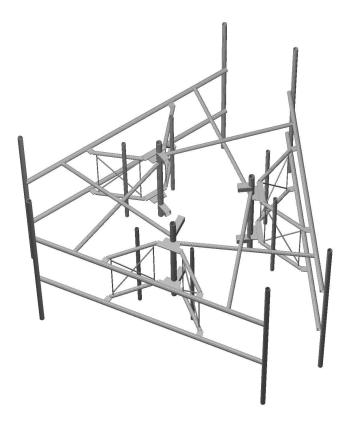
Per foot weight of ice on object:



Mount Calculations (Proposed Conditions)



Bentley
Current Date: 11/30/2021 4:49 PM
Units system: English
File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT1338\CT1338.retx

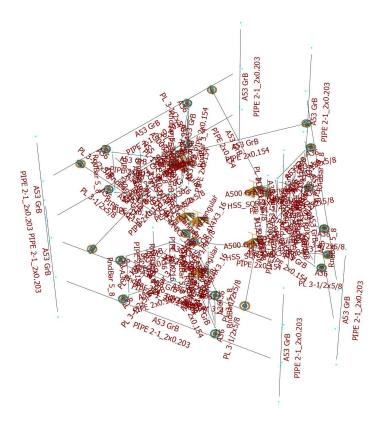






Bentley

Current Date: 11/30/2021 4:49 PM
Units system: English
File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT1338\CT1338.retx







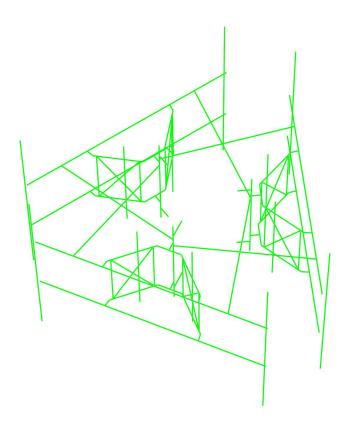
Bentley

Current Date: 11/30/2021 4:50 PM

Units system: English

File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT1338\CT1338.retx



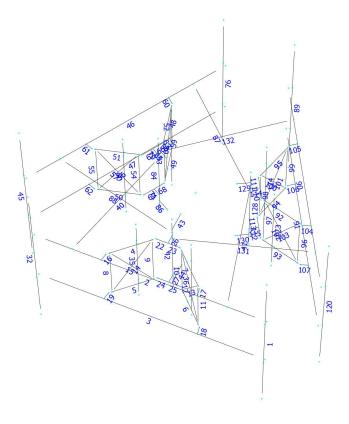






Bentley

Current Date: 11/30/2021 4:50 PM
Units system: English
File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT1338\CT1338.retx







Current Date: 12/2/2021 9:35 AM

Units system: English
File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT1338\CT1338.retx

## **Load data**

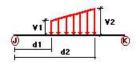
GLOSSARY

Comb : Indicates if load condition is a load combination

### **Load Conditions**

Condition	Description	Comb.	Category
 DL	Dead Load	 No	DL
W0	Wind Load 0/60/120 deg	No	WIND
W30	Wind Load 30/90/150 deg	No	WIND
Di	Ice Load	No	LL
Wi0	Ice Wind Load 0/60/120 deg	No	WIND
Wi30	Ice Wind Load 30/90/150 deg	No	WIND
WL0	WL 30 mph 0/60/120 deg	No	WIND
WL30	WL 30 mph 30/90/150 deg	No	WIND
LL1	250 lb Live Load Center of Mount	No	LL
LL2	250 lb Live Load End of Mount	No	LL
LLa1	500 lb Live Load Antenna 1	No	LL
LLa2	500 lb Live Load Antenna 2	No	LL
LLa3	500 lb Live Load Antenna 3	No	LL
LLa4	500 lb Live Load Antenna 4	No	LL

### Distributed force on members



Condition	Member	Dir1	<b>Val1</b> [Kip/ft]	<b>Val2</b> [Kip/ft]	Dist1 [ft]	%	Dist2 [ft]	%
W0	2	z	-0.007	0.00	0.00	No	0.00	No
	3	Z	-0.007	0.00	0.00	No	0.00	No
	4	Z	-0.006	0.00	0.00	No	0.00	No
	5	Z	-0.006	0.00	0.00	No	0.00	No
	6	Z	-0.006	0.00	0.00	No	0.00	No
	7	Z	-0.006	0.00	0.00	No	0.00	No
	8	Z	-0.002	0.00	0.00	No	0.00	No
	9	Z	-0.002	0.00	0.00	No	0.00	No
	10	Z	-0.002	0.00	0.00	No	0.00	No
	11	Z	-0.002	0.00	0.00	No	0.00	No
	12	Z	-0.002	0.00	0.00	No	0.00	No
	13	Z	-0.002	0.00	0.00	No	0.00	No
	14	Z	-0.002	0.00	0.00	No	0.00	No
	15	Z	-0.002	0.00	0.00	No	0.00	No
	16	Z	-0.003	0.00	0.00	No	0.00	No
	17	Z	-0.003	0.00	0.00	No	0.00	No
	18	Z	-0.003	0.00	0.00	No	0.00	No
	19	Z	-0.003	0.00	0.00	No	0.00	No
	22	Z	-0.003	0.00	0.00	No	0.00	No
	23	Z	-0.003	0.00	0.00	No	0.00	No
	24	Z	-0.003	0.00	0.00	No	0.00	No
	25	Z	-0.003	0.00	0.00	No	0.00	No

26	Z	-0.003	0.00	0.00	No	0.00	No
27	z	-0.003	0.00	0.00	No	0.00	No
35	Z	-0.006	0.00	0.00	No	0.00	No
36	Z	-0.006	0.00	0.00	No	0.00	No
40	Z	-0.006	0.00	0.00	No	0.00	No
41	Z	-0.006	0.00	0.00	No	0.00	No
42	Z	-0.009	0.00	0.00	No	0.00	No
43	z	-0.018	0.00	0.00	No	0.00	No
44	Z	-0.018	0.00	0.00	No	0.00	No
45	Z	-0.007	0.00	0.00	No	0.00	No
46	Z	-0.007	0.00	0.00	No	0.00	No
47	Z	-0.007	0.00	0.00	No	0.00	No
48	Z	-0.006	0.00	0.00	No	0.00	No
49	z	-0.006	0.00	0.00	No	0.00	No
50		-0.006	0.00	0.00	No	0.00	No
	Z						
51	Z	-0.006	0.00	0.00	No	0.00	No
52	Z	-0.002	0.00	0.00	No	0.00	No
53	Z	-0.002	0.00	0.00	No	0.00	No
54	Z	-0.002	0.00	0.00	No	0.00	No
55	Z	-0.002	0.00	0.00	No	0.00	No
56		-0.002	0.00	0.00	No	0.00	No
	Z						
57	Z	-0.002	0.00	0.00	No	0.00	No
58	Z	-0.002	0.00	0.00	No	0.00	No
59	Z	-0.002	0.00	0.00	No	0.00	No
60	Z	-0.003	0.00	0.00	No	0.00	No
61	Z	-0.003	0.00	0.00	No	0.00	No
62	Z	-0.003	0.00	0.00	No	0.00	No
63	Z	-0.003	0.00	0.00	No	0.00	No
66	Z	-0.003	0.00	0.00	No	0.00	No
67	Z	-0.003	0.00	0.00	No	0.00	No
68	Z	-0.003	0.00	0.00	No	0.00	No
69	Z	-0.003	0.00	0.00	No	0.00	No
70	Z	-0.003	0.00	0.00	No	0.00	No
71	Z	-0.003	0.00	0.00	No	0.00	No
76	Z	-0.007	0.00	0.00	No	0.00	No
79	Z	-0.006	0.00	0.00	No	0.00	No
80	Z	-0.006	0.00	0.00	No	0.00	No
84	Z	-0.009	0.00	0.00	No	0.00	No
85	Z	-0.018	0.00	0.00	No	0.00	No
86	z	-0.018	0.00	0.00	No	0.00	No
87	Z	-0.006	0.00	0.00	No	0.00	No
88	Z	-0.006	0.00	0.00	No	0.00	No
89	Z	-0.007	0.00	0.00	No	0.00	No
90	Z	-0.007	0.00	0.00	No	0.00	No
91	Z	-0.007	0.00	0.00	No	0.00	No
92	Z	-0.006	0.00	0.00	No	0.00	No
93	z	-0.006	0.00	0.00	No	0.00	No
94							
	Z	-0.006	0.00	0.00	No	0.00	No
95	Z	-0.006	0.00	0.00	No	0.00	No
96	Z	-0.002	0.00	0.00	No	0.00	No
97	Z	-0.002	0.00	0.00	No	0.00	No
98	Z	-0.002	0.00	0.00	No	0.00	No
99	Z	-0.002	0.00	0.00	No	0.00	No
100		-0.002	0.00	0.00	No	0.00	No
	Z						
101	Z	-0.002	0.00	0.00	No	0.00	No
102	Z	-0.002	0.00	0.00	No	0.00	No
103	Z	-0.002	0.00	0.00	No	0.00	No
104	Z	-0.003	0.00	0.00	No	0.00	No
105	Z	-0.003	0.00	0.00	No	0.00	No
106	Z	-0.003	0.00	0.00	No	0.00	No
107	Z	-0.003	0.00	0.00	No	0.00	No
110	Z	-0.003	0.00	0.00	No	0.00	No
111	Z	-0.003	0.00	0.00	No	0.00	No
112	Z	-0.003	0.00	0.00	No	0.00	No
113	Z	-0.003	0.00	0.00	No	0.00	No
114	z	-0.003	0.00	0.00	No	0.00	No
	-	0.000	0.00	0.00		3.00	. 10

	115	Z	-0.003	0.00	0.00	No	0.00	No
	120	Z	-0.007	0.00	0.00	No	0.00	No
	123	Z	-0.006	0.00	0.00	No	0.00	No
	124	Z	-0.006	0.00	0.00	No	0.00	No
	128	Z	-0.009	0.00	0.00	No	0.00	No
	129	Z	-0.018	0.00	0.00	No	0.00	No
	130	Z	-0.018	0.00	0.00	No	0.00	No
	131	Z	-0.006	0.00	0.00	No	0.00	No
	132	Z	-0.006	0.00	0.00	No	0.00	No
W30	1	X	-0.007	0.00	0.00	No	0.00	No
	2	X	-0.007	0.00	0.00	No	0.00	No
	2							
	3	X	-0.007	0.00	0.00	No	0.00	No
	4	X	-0.006	0.00	0.00	No	0.00	No
	5	X	-0.006	0.00	0.00	No	0.00	No
	5							
	6	X	-0.006	0.00	0.00	No	0.00	No
	7	X	-0.006	0.00	0.00	No	0.00	No
	8	X	-0.002	0.00	0.00	No	0.00	No
	9	X	-0.002	0.00	0.00	No	0.00	No
	10	X	-0.002	0.00	0.00	No	0.00	No
	11	X	-0.002	0.00	0.00	No	0.00	No
	12	X	-0.002	0.00	0.00	No	0.00	No
	13		-0.002	0.00	0.00	No	0.00	No
		X						
	14	X	-0.002	0.00	0.00	No	0.00	No
	15	X	-0.002	0.00	0.00	No	0.00	No
	16	X	-0.003	0.00	0.00	No	0.00	No
	17	X	-0.003	0.00	0.00	No	0.00	No
	18	x	-0.003	0.00	0.00	No	0.00	No
	19	X	-0.003	0.00	0.00	No	0.00	No
	22	X	-0.003	0.00	0.00	No	0.00	No
	23	X	-0.003	0.00	0.00	No	0.00	No
	24	X	-0.003	0.00	0.00	No	0.00	No
	25	X	-0.003	0.00	0.00	No	0.00	No
	26	X	-0.003	0.00	0.00	No	0.00	No
	27	X	-0.003	0.00	0.00	No	0.00	No
	32	X	-0.007	0.00	0.00	No	0.00	No
	35	X	-0.006	0.00	0.00	No	0.00	No
	36	X	-0.006	0.00	0.00	No	0.00	No
	40	X	-0.006	0.00	0.00	No	0.00	No
	41	X	-0.006	0.00	0.00	No	0.00	No
	42				0.00	No	0.00	No
		X	-0.009	0.00				
	43	X	-0.018	0.00	0.00	No	0.00	No
	44	X	-0.018	0.00	0.00	No	0.00	No
	45		-0.007	0.00	0.00	No	0.00	No
		X						
	46	X	-0.007	0.00	0.00	No	0.00	No
	47	X	-0.007	0.00	0.00	No	0.00	No
	48	X	-0.006	0.00	0.00	No	0.00	No
	49	X	-0.006	0.00	0.00	No	0.00	No
	50	X	-0.006	0.00	0.00	No	0.00	No
	51	X	-0.006	0.00	0.00	No	0.00	No
	52	X	-0.002	0.00	0.00	No	0.00	No
	53	X	-0.002	0.00	0.00	No	0.00	No
	54	X	-0.002	0.00	0.00	No	0.00	No
	55	X	-0.002	0.00	0.00	No	0.00	No
	56	X	-0.002	0.00	0.00	No	0.00	No
	57	X	-0.002	0.00	0.00	No	0.00	No
	58	X	-0.002	0.00	0.00	No	0.00	No
	59	x	-0.002	0.00	0.00	No	0.00	No
	60	X	-0.003	0.00	0.00	No	0.00	No
	61	X	-0.003	0.00	0.00	No	0.00	No
	62	X	-0.003	0.00	0.00	No	0.00	No
	63	X	-0.003	0.00	0.00	No	0.00	No
	66	X	-0.003	0.00	0.00	No	0.00	No
	67	X	-0.003	0.00	0.00	No	0.00	No
	68		-0.003	0.00	0.00	No	0.00	No
		X						
	69	X	-0.003	0.00	0.00	No	0.00	No
	70	X	-0.003	0.00	0.00	No	0.00	No
	-	-		****				

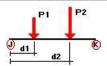
71	X	-0.003	0.00	0.00	No	0.00	No
76	Х	-0.007	0.00	0.00	No	0.00	No
79	X	-0.006	0.00	0.00	No	0.00	No
80	Χ	-0.006	0.00	0.00	No	0.00	No
84	Χ	-0.009	0.00	0.00	No	0.00	No
85	Х	-0.018	0.00	0.00	No	0.00	No
86	X	-0.018	0.00	0.00	No	0.00	No
87	Х	-0.006	0.00	0.00	No	0.00	No
88	Х	-0.006	0.00	0.00	No	0.00	No
89	Х	-0.007	0.00	0.00	No	0.00	No
90	Х	-0.007	0.00	0.00	No	0.00	No
91	Х	-0.007	0.00	0.00	No	0.00	No
92	Х	-0.006	0.00	0.00	No	0.00	No
93	X	-0.006	0.00	0.00	No	0.00	No
94	Х	-0.006	0.00	0.00	No	0.00	No
95	Х	-0.006	0.00	0.00	No	0.00	No
96	Х	-0.002	0.00	0.00	No	0.00	No
97	х	-0.002	0.00	0.00	No	0.00	No
98	Х	-0.002	0.00	0.00	No	0.00	No
99	Х	-0.002	0.00	0.00	No	0.00	No
100	X	-0.002	0.00	0.00	No	0.00	No
101	X	-0.002	0.00	0.00	No	0.00	No
102	Х	-0.002	0.00	0.00	No	0.00	No
103	Х	-0.002	0.00	0.00	No	0.00	No
104	Х	-0.003	0.00	0.00	No	0.00	No
105	X	-0.003	0.00	0.00	No	0.00	No
106	Χ	-0.003	0.00	0.00	No	0.00	No
107	Х	-0.003	0.00	0.00	No	0.00	No
110	Х	-0.003	0.00	0.00	No	0.00	No
111		-0.003	0.00	0.00	No	0.00	
	Х						No
112	Х	-0.003	0.00	0.00	No	0.00	No
113	Х	-0.003	0.00	0.00	No	0.00	No
114	х	-0.003	0.00	0.00	No	0.00	No
115	Х	-0.003	0.00	0.00	No	0.00	No
120	Х	-0.007	0.00	0.00	No	0.00	No
123	X	-0.006	0.00	0.00	No	0.00	No
124	X	-0.006	0.00	0.00	No	0.00	No
128	Х	-0.009	0.00	0.00	No	0.00	No
129	Х	-0.018	0.00	0.00	No	0.00	No
130	Х	-0.018	0.00	0.00	No	0.00	No
131	X	-0.006	0.00	0.00	No	0.00	No
132	Χ	-0.006	0.00	0.00	No	0.00	No
1	У	-0.008	0.00	0.00	No	0.00	No
		-0.008	0.00	0.00	No	0.00	No
2	y y	-0.008	0.00	0.00	No	0.00	No
	У						
4	у	-0.007	0.00	0.00	No	0.00	No
5	У	-0.007	0.00	0.00	No	0.00	No
6	у	-0.007	0.00	0.00	No	0.00	No
7	,	-0.007	0.00	0.00	No	0.00	No
,	у						
8	у	-0.004	0.00	0.00	No	0.00	No
9	У	-0.004	0.00	0.00	No	0.00	No
10	y	-0.004	0.00	0.00	No	0.00	No
11		-0.004	0.00	0.00	No	0.00	No
	У						
12	У	-0.004	0.00	0.00	No	0.00	No
13	у	-0.004	0.00	0.00	No	0.00	No
14	y	-0.004	0.00	0.00	No	0.00	No
15	y V	-0.004	0.00	0.00	No	0.00	No
	У						
16	у	-0.009	0.00	0.00	No	0.00	No
17	у	-0.009	0.00	0.00	No	0.00	No
18	ý	-0.009	0.00	0.00	No	0.00	No
						0.00	
19	у	-0.009	0.00	0.00	No		No
22	У	-0.009	0.00	0.00	No	0.00	No
23	у	-0.009	0.00	0.00	No	0.00	No
24		-0.009	0.00	0.00	No	0.00	No
	У						
25	у	-0.009	0.00	0.00	No	0.00	No
26	У	-0.023	0.00	0.00	No	0.00	No

Di

27	у	-0.023	0.00	0.00	No	0.00	No
32	ý	-0.008	0.00	0.00	No	0.00	No
35	y	-0.007	0.00	0.00	No	0.00	No
36	у	-0.007	0.00	0.00	No	0.00	No
40	У	-0.007	0.00	0.00	No	0.00	No
41	У	-0.007	0.00	0.00	No	0.00	No
42	У	-0.009	0.00	0.00	No	0.00	No
43	у	-0.013	0.00	0.00	No	0.00	No
44	y	-0.013	0.00	0.00	No	0.00	No
45	У	-0.008	0.00	0.00	No	0.00	No
	у						
46	У	-0.008	0.00	0.00	No	0.00	No
47	У	-0.008	0.00	0.00	No	0.00	No
48	У	-0.007	0.00	0.00	No	0.00	No
49	у	-0.007	0.00	0.00	No	0.00	No
50	ý	-0.007	0.00	0.00	No	0.00	No
51	y	-0.007	0.00	0.00	No	0.00	No
52			0.00	0.00	No	0.00	No
	У	-0.004					
53	У	-0.004	0.00	0.00	No	0.00	No
54	У	-0.004	0.00	0.00	No	0.00	No
55	У	-0.004	0.00	0.00	No	0.00	No
56	у	-0.004	0.00	0.00	No	0.00	No
57	ý	-0.004	0.00	0.00	No	0.00	No
58		-0.004	0.00	0.00	No	0.00	No
	У						
59	у	-0.004	0.00	0.00	No	0.00	No
60	У	-0.009	0.00	0.00	No	0.00	No
61	У	-0.009	0.00	0.00	No	0.00	No
62	У	-0.009	0.00	0.00	No	0.00	No
63	у	-0.009	0.00	0.00	No	0.00	No
66	y	-0.009	0.00	0.00	No	0.00	No
67	y	-0.009	0.00	0.00	No	0.00	No
68	у	-0.009	0.00	0.00	No	0.00	No
69	У	-0.009	0.00	0.00	No	0.00	No
70	У	-0.023	0.00	0.00	No	0.00	No
71	У	-0.023	0.00	0.00	No	0.00	No
76	у	-0.008	0.00	0.00	No	0.00	No
79	ý	-0.007	0.00	0.00	No	0.00	No
80	y	-0.007	0.00	0.00	No	0.00	No
84			0.00	0.00	No	0.00	No
	У	-0.009					
85	У	-0.013	0.00	0.00	No	0.00	No
86	У	-0.013	0.00	0.00	No	0.00	No
87	У	-0.007	0.00	0.00	No	0.00	No
88	У	-0.007	0.00	0.00	No	0.00	No
89	у	-0.008	0.00	0.00	No	0.00	No
90	ý	-0.008	0.00	0.00	No	0.00	No
91	y	-0.008	0.00	0.00	No	0.00	No
92		-0.007	0.00	0.00	No	0.00	No
	У						
93	у	-0.007	0.00	0.00	No	0.00	No
94	У	-0.007	0.00	0.00	No	0.00	No
95	У	-0.007	0.00	0.00	No	0.00	No
96	У	-0.004	0.00	0.00	No	0.00	No
97	у	-0.004	0.00	0.00	No	0.00	No
98	y	-0.004	0.00	0.00	No	0.00	No
99		-0.004	0.00	0.00	No	0.00	No
	У						
100	У	-0.004	0.00	0.00	No	0.00	No
101	У	-0.004	0.00	0.00	No	0.00	No
102	У	-0.004	0.00	0.00	No	0.00	No
103	У	-0.004	0.00	0.00	No	0.00	No
104	у	-0.009	0.00	0.00	No	0.00	No
105	y	-0.009	0.00	0.00	No	0.00	No
106	y	-0.009	0.00	0.00	No	0.00	No
107	У	-0.009	0.00	0.00	No	0.00	No
110	У	-0.009	0.00	0.00	No	0.00	No
111	У	-0.009	0.00	0.00	No	0.00	No
112	У	-0.009	0.00	0.00	No	0.00	No
113	y	-0.009	0.00	0.00	No	0.00	No
114	y	-0.023	0.00	0.00	No	0.00	No
	,		3.00				

115 120 123 124 128 129 130	y y y y y y	-0.023 -0.008 -0.007 -0.007 -0.009 -0.013 -0.007	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	No No No No No No No No	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	No No No No No No No
	y y y						

## **Concentrated forces on members**



Condition	Member	Dir1	Value1 [Kip]	Dist1 [ft]	%
DL	 1	 у	-0.034	 1.50	 No
DL	•	y	-0.034	8.50	No
	32	y	-0.04	1.50	No
		y	-0.04	8.50	No
	35	y	-0.073	50.00	Yes
		y	-0.072	50.00	Yes
		y	-0.033	25.00	Yes
	36	У	-0.06	50.00	Yes
		У	-0.046	50.00	Yes
	45	У	-0.034	1.50	No
		у	-0.034	8.50	No
	76	У	-0.04	1.50	No
		У	-0.04	8.50	No
	79	У	-0.073	50.00	Yes
		У	-0.072	50.00	Yes
		У	-0.033	25.00	Yes
	80	У	-0.06	50.00	Yes
		У	-0.046	50.00	Yes
	89	У	-0.034	1.50	No
		У	-0.034	8.50	No
	120	У	-0.04	1.50	No
		У	-0.04	8.50	No
	123	У	-0.073	50.00	Yes
		У	-0.072	50.00	Yes
		У	-0.033	25.00	Yes
	124	У	-0.06	50.00	Yes
		У	-0.046	50.00	Yes
W0	1	Z	-0.281	2.50	No
		Z	-0.281	7.50	No
	32	Z	-0.281	2.50	No
		Z	-0.281	7.50	No
	35	Z	-0.055	50.00	Yes
		Z	-0.036	50.00	Yes
		Z	-0.05	25.00	Yes
	36	Z	-0.062	50.00	Yes
		Z	-0.06	50.00	Yes
	45	Z	-0.164	2.50	No
		Z	-0.164	7.50	No
	76	Z	-0.164	2.50	No
	70	Z	-0.164	7.50	No
	79	Z	-0.081	50.00	Yes
		Z	-0.05	25.00	Yes
	80	Z	-0.081	50.00	Yes

	00	_	0.404	0.50	NI-
	89	Z	-0.164 -0.164	2.50 7.50	No No
	120	z z	-0.164 -0.164	2.50	No
	120	Z	-0.164	7.50	No
	123	Z	-0.081	50.00	Yes
	120	z	-0.05	25.00	Yes
	124	z	-0.081	50.00	Yes
W30	1	x	-0.124	2.50	No
		X	-0.124	7.50	No
	32	x	-0.124	2.50	No
		X	-0.124	7.50	No
	35	Х	-0.089	50.00	Yes
		X	-0.05	25.00	Yes
	36	X	-0.087	50.00	Yes
	45	X	-0.242	2.50	No
		X	-0.242	7.50	No
	76	Х	-0.242	2.50	No
		Х	-0.242	7.50	No
	79	Х	-0.064	50.00	Yes
		X	-0.05	25.00	Yes
	80	X	-0.068	50.00	Yes
	89	X	-0.242	2.50	No
	400	X	-0.242	7.50	No
	120	X	-0.242	2.50	No
	400	X	-0.242	7.50	No
	123	X	-0.064	50.00	Yes
	124	X	-0.05 -0.068	25.00	Yes
Di	124 1	X	-0.006 -0.124	50.00 1.50	Yes No
DI	,	у	-0.124 -0.124	8.50	No
	32	У	-0.124	1.50	No
	32	y y	-0.124	8.50	No
	35	y	-0.046	50.00	Yes
	00	y	-0.039	50.00	Yes
		y	-0.04	25.00	Yes
	36	y	-0.047	50.00	Yes
		y	-0.041	50.00	Yes
	45	ý	-0.124	1.50	No
		ý	-0.124	8.50	No
	76	ý	-0.124	1.50	No
		y	-0.124	8.50	No
	79	У	-0.046	50.00	Yes
		У	-0.039	50.00	Yes
		У	-0.04	25.00	Yes
	80	У	-0.047	50.00	Yes
		У	-0.041	50.00	Yes
	89	У	-0.124	1.50	No
	400	У	-0.124	8.50	No
	120	У	-0.124	1.50	No
	400	У	-0.124	8.50	No
	123	У	-0.046	50.00	Yes
		У	-0.039	50.00	Yes
	124	У	-0.04 -0.047	25.00	Yes Yes
	124	У	-0.047	50.00 50.00	Yes
Wi0	1	y z	-0.04 i -0.05	2.50	No
VVIO	'	Z	-0.05	7.50	No
	32	Z	-0.05	2.50	No
	<b>~</b> _	z	-0.05	7.50	No
	35	z	-0.013	50.00	Yes
		z	-0.009	50.00	Yes
	36	z	-0.014	50.00	Yes
		Z	-0.013	50.00	Yes
		Z	-0.011	25.00	Yes
	45	z	-0.031	2.50	No
		z	-0.031	7.50	No

	76	z	-0.031	2.50	No
	, ,	z	-0.031	7.50	No
	79	Z	-0.017	50.00	Yes
	80	Z	-0.017	50.00	Yes
	00	Z			
	00		-0.011	25.00	Yes
	89	Z	-0.031	2.50	No
	400	Z	-0.031	7.50	No
	120	Z	-0.031	2.50	No
		Z	-0.031	7.50	No
	123	Z	-0.017	50.00	Yes
	124	Z	-0.017	50.00	Yes
		Z	-0.011	25.00	Yes
Wi30	1	X	-0.025	2.50	No
		X	-0.025	7.50	No
	32	X	-0.025	2.50	No
		Х	-0.025	7.50	No
	35	X	-0.019	50.00	Yes
		x	-0.011	25.00	Yes
	36	x	-0.018	50.00	Yes
	45	X	-0.043	2.50	No
	10	X	-0.043	7.50	No
	76	X	-0.043	2.50	No
		x	-0.043	7.50	No
	70				Yes
	79	X	-0.014	50.00	
	00	X	-0.011	25.00	Yes
WL0	80	X	-0.015	50.00	Yes
	89	X	-0.043	2.50	No
	120	X	-0.043	7.50	No
		X	-0.043	2.50	No
		X	-0.043	7.50	No
	123	X	-0.014	50.00	Yes
		X	-0.011	25.00	Yes
	124	X	-0.015	50.00	Yes
	1	Z	-0.015	2.50	No
		z	-0.015	7.50	No
	32	z	-0.015	2.50	No
		z	-0.015	7.50	No
	35	z	-0.003	50.00	Yes
	00	z	-0.002	50.00	Yes
		Z	-0.002	25.00	Yes
	36	Z	-0.003	50.00	Yes
	30	Z	-0.003	50.00	Yes
	15				
	45	Z	-0.009	2.50	No
	70	Z	-0.009	7.50	No
	76	Z	-0.009	2.50	No
		Z	-0.009	7.50	No
	79	Z	-0.004	50.00	Yes
		Z	-0.003	25.00	Yes
	80	Z	-0.004	50.00	Yes
WL30	89	Z	-0.009	2.50	No
		Z	-0.009	7.50	No
	120	Z	-0.009	2.50	No
		Z	-0.009	7.50	No
	123	Z	-0.004	50.00	Yes
		Z	-0.003	25.00	Yes
	124	Z	-0.004	50.00	Yes
	1	x	-0.007	2.50	No
		X	-0.007	7.50	No
	32	X	-0.007	2.50	No
	02	X	-0.007	7.50	No
	35			50.00	Yes
	55	X	-0.005 0.003		Yes
	26	X	-0.003	25.00	
	36	X	-0.005	50.00	Yes
	45	X	-0.013	2.50	No
		X	-0.013	7.50	No
	76	X	-0.013	2.50	No

		x	-0.013	7.50	No
	79	X	-0.003	50.00	Yes
		X	-0.003	25.00	Yes
	80	x	-0.003	50.00	Yes
	89	X	-0.013	2.50	No
		x	-0.013	7.50	No
	120	X	-0.013	2.50	No
		X	-0.013	7.50	No
	123	x	-0.003	50.00	Yes
		X	-0.003	25.00	Yes
	124	X	-0.003	50.00	Yes
LL1	2	У	-0.25	50.00	Yes
LL2	2	У	-0.25	100.00	Yes
LLa1	1	У	-0.50	50.00	Yes
LLa2	32	У	-0.50	50.00	Yes

# Self weight multipliers for load conditions

			Self weight multiplier				
Condition	Description	Comb.	MultX	MultY	MultZ		
			0.00	4.00			
DL	Dead Load	No	0.00	-1.00	0.00		
W0	Wind Load 0/60/120 deg	No	0.00	0.00	0.00		
W30	Wind Load 30/90/150 deg	No	0.00	0.00	0.00		
Di	Ice Load	No	0.00	0.00	0.00		
Wi0	Ice Wind Load 0/60/120 deg	No	0.00	0.00	0.00		
Wi30	Ice Wind Load 30/90/150 deg	No	0.00	0.00	0.00		
WL0	WL 30 mph 0/60/120 deg	No	0.00	0.00	0.00		
WL30	WL 30 mph 30/90/150 deg	No	0.00	0.00	0.00		
LL1	250 lb Live Load Center of Mount	No	0.00	0.00	0.00		
LL2	250 lb Live Load End of Mount	No	0.00	0.00	0.00		
LLa1	500 lb Live Load Antenna 1	No	0.00	0.00	0.00		
LLa2	500 lb Live Load Antenna 2	No	0.00	0.00	0.00		
LLa3	500 lb Live Load Antenna 3	No	0.00	0.00	0.00		
LLa4	500 lb Live Load Antenna 4	No	0.00	0.00	0.00		

# Earthquake (Dynamic analysis only)

Condition	a/g	Ang. [Deg]	Damp. [%]
DL	0.00	0.00	0.00
W0	0.00	0.00	0.00
W30	0.00	0.00	0.00
Di	0.00	0.00	0.00
Wi0	0.00	0.00	0.00
Wi30	0.00	0.00	0.00
WL0	0.00	0.00	0.00
WL30	0.00	0.00	0.00
LL1	0.00	0.00	0.00
LL2	0.00	0.00	0.00
LLa1	0.00	0.00	0.00
LLa2	0.00	0.00	0.00
LLa3	0.00	0.00	0.00
LLa4 	0.00	0.00	0.00



Current Date: 12/2/2021 9:35 AM

Units system: English

File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT\338\CT1338.retx

# **Steel Code Check**

Report: Summary - Group by member

#### Load conditions to be included in design:

LC1=1.2DL+W0

LC2=1.2DL+W30 LC3=1.2DL-W0

LC4=1.2DL-W30

LC5=0.9DL+W0

LC6=0.9DL+W30

LC7=0.9DL-W0

LC8=0.9DL-W30

LC9=1.2DL+Di+Wi0

LC10=1.2DL+Di+Wi30

LC11=1.2DL+Di-Wi0

LC12=1.2DL+Di-Wi30

LC13=1.4DL

LC14=1.2DL+1.6LL1

LC15=1.2DL+1.6LL2

LC16=1.2DL+WL0+1.6LLa1

LC17=1.2DL+WL30+1.6LLa1

LC18=1.2DL-WL0+1.6LLa1

LC19=1.2DL-WL30+1.6LLa1

LC20=1.2DL+WL0+1.6LLa2

LC21=1.2DL+WL30+1.6LLa2

LC22=1.2DL-WL0+1.6LLa2

LC23=1.2DL-WL30+1.6LLa2

LC24=1.2DL+WL0+1.6LLa3

LC25=1.2DL+WL30+1.6LLa3

LC26=1.2DL-WL0+1.6LLa3

LC27=1.2DL-WL30+1.6LLa3 LC28=1.2DL+WL0+1.6LLa4

LC29=1.2DL+WL30+1.6LLa4

LC30=1.2DL-WL0+1.6LLa4

LC31=1.2DL-WL30+1.6LLa4

Description	Section	Member	Ctrl Eq.	Ratio	Status	Reference
	HSS_SQR 4X4X3_16	43	LC2 at 100.00%	0.20	 OK	Eq. H1-1b
		44	LC12 at 0.00%	0.17	OK	Eq. H1-1b
		85	LC1 at 100.00%	0.13	OK	Eq. H1-1b
		86	LC11 at 0.00%	0.12	OK	Eq. H1-1b
		129	LC3 at 100.00%	0.14	OK	Eq. H1-1b
		130	LC9 at 0.00%	0.12	OK	Eq. H1-1b
	PIPE 2-1_2x0.203	1	LC19 at 33.33%	0.47	 OK	Eq. H1-1b
	_	2	LC21 at 28.75%	0.46	OK	Eq. H1-1b
		3	LC16 at 70.83%	0.60	OK	Eq. H1-1b
		32	LC21 at 33.33%	0.48	OK	Eq. H1-1b
		45	LC4 at 64.58%	0.20	OK	Eq. H1-1b
		46	LC2 at 16.25%	0.33	OK	Eq. H1-1b
		47	LC4 at 70.83%	0.46	OK	Eq. H1-1b
		76	LC9 at 33.33%	0.27	OK	Eq. H1-1b
		89	LC9 at 33.33%	0.26	OK	Eq. H1-1b
		90	LC4 at 83.75%	0.35	OK	Eq. H1-1b
		91	LC2 at 29.17%	0.46	OK	Eq. H1-1b
		120	LC11 at 33.33%	0.21	OK	Eq. H1-1b
	PIPE 2x0.154	4	LC21 at 93.75%	0.42	 OK	Eq. H1-1b
		5	LC20 at 93.75%	0.44	OK	Eq. H1-1b
		6	LC16 at 93.75%	0.41	OK	Eq. H1-1b
		7	LC16 at 93.75%	0.41	OK	Eq. H1-1b

	35 36 40 41 48 49 50 51 79 80 87 88 92 93 94 95 123 124 131	LC21 at 22.92% LC19 at 22.92% LC22 at 0.00% LC18 at 0.00% LC12 at 93.75% LC3 at 93.75% LC4 at 93.75% LC9 at 22.92% LC11 at 22.92% LC8 at 100.00% LC3 at 50.00% LC11 at 93.75% LC2 at 93.75% LC2 at 93.75% LC3 at 93.75% LC10 at 93.75% LC10 at 93.75% LC10 at 93.75% LC10 at 22.92% LC9 at 22.92% LC9 at 22.92% LC9 at 22.92% LC3 at 50.00% LC6 at 0.00%	0.22 0.19 0.09 0.08 0.25 0.31 0.32 0.24 0.11 0.12 0.08 0.07 0.25 0.35 0.28 0.24 0.15 0.09	OK OK OK OK OK OK OK OK OK OK OK OK OK O	Eq. H1-1b Eq. H1-1b Eq. H3-1 Eq. H3-1 Eq. H3-1 Eq. H1-1b
PIPE 3x0.216	42 84 128	LC12 at 85.71% LC1 at 26.79% LC10 at 85.71%	<b>0.33</b> 0.30 0.29	OK OK OK	Eq. H1-1b Eq. H1-1b Eq. H1-1b Eq. H1-1b
PL 11-1/4x5/8	26 27 70 71 114 115	LC9 at 100.00% LC12 at 100.00% LC12 at 100.00% LC11 at 100.00% LC10 at 100.00% LC9 at 100.00%	0.27 0.21 0.27 0.22 <b>0.22</b> <b>0.22</b>	ОК ОК ОК ОК ОК	Eq. H1-1b Eq. H1-1b Eq. H1-1b Eq. H1-1b Eq. H1-1b Eq. H1-1b
PL 3-1/2x5/8	16 17 18 19 22 23 24 25 60 61 62 63 66 67 68 69 104 105 106 107 110 111 112	LC20 at 100.00% LC16 at 100.00% LC16 at 100.00% LC20 at 100.00% LC21 at 100.00% LC16 at 0.00% LC17 at 0.00% LC10 at 100.00% LC11 at 100.00% LC19 at 100.00% LC11 at 100.00% LC10 at 100.00% LC10 at 100.00% LC10 at 100.00%	0.47 0.45 0.47 0.48 <b>0.66</b> 0.61 0.62 0.23 0.22 0.21 0.22 0.46 0.41 0.44 0.39 0.23 0.22 0.21 0.24 0.44 0.39	OK OK OK OK OK OK OK OK OK OK OK OK OK O	Eq. H1-1b
RndBar 3_4	12 13 14 15 56 57 58 59 100 101 102 103	LC16 at 96.88% LC16 at 0.00% LC22 at 0.00% LC23 at 100.00% LC10 at 100.00% LC12 at 0.00% LC9 at 0.00% LC11 at 100.00% LC9 at 100.00% LC11 at 0.00% LC11 at 0.00%	0.32 0.25 0.35 0.28 0.15 0.15 0.18 0.19 0.15 0.15 0.15	OK OK OK OK OK OK OK OK OK OK	Eq. H1-1a Eq. H1-1b Eq. H1-1a Eq. H1-1b
RndBar 5_8	8	LC22 at 87.50%	0.74	OK	Eq. H1-1a

9	LC23 at 87.50%	0.78	OK	Eq. H1-1a
10	LC17 at 87.50%	0.74	OK	Eq. H1-1a
11	LC16 at 87.50%	0.73	OK	Eq. H1-1a
52	LC10 at 87.50%	0.45	OK	Eq. H1-1a
53	LC11 at 87.50%	0.45	OK	Eq. H1-1a
54	LC9 at 87.50%	0.39	OK	Eq. H1-1a
55	LC12 at 87.50%	0.41	OK	Eq. H1-1a
96	LC9 at 87.50%	0.46	OK	Eq. H1-1a
97	LC10 at 87.50%	0.45	OK	Eq. H1-1a
98	LC11 at 87.50%	0.39	OK	Eq. H1-1a
99	LC11 at 87.50%	0.40	OK	Eq. H1-1a



Current Date: 11/30/2021 4:50 PM

Units system: English

File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT\38\CT1338\crtx

# **Geometry data**

**GLOSSARY** 

Cb22, Cb33 : Moment gradient coefficients

: Coefficients applied to bending term in interaction formula Cm22, Cm33 : Tapered member section depth at J end of member d0 : Rigid end offset distance measured from J node in axis X DJX DJY : Rigid end offset distance measured from J node in axis Y DJZ : Rigid end offset distance measured from J node in axis Z : Rigid end offset distance measured from K node in axis X DKX DKY : Rigid end offset distance measured from K node in axis Y : Rigid end offset distance measured from K node in axis Z DKZ dL : Tapered member section depth at K end of member

Ig factor : Inertia reduction factor (Effective Inertia/Gross Inertia) for reinforced concrete members

K22 : Effective length factor about axis 2 K33 : Effective length factor about axis 3

L22 : Member length for calculation of axial capacity
L33 : Member length for calculation of axial capacity

LB pos : Lateral unbraced length of the compression flange in the positive side of local axis 2
LB neg : Lateral unbraced length of the compression flange in the negative side of local axis 2

RX : Rotation about X
RY : Rotation about Y
RZ : Rotation about Z

TO : 1 = Tension only member 0 = Normal member

TX : Translation in X
TY : Translation in Y
TZ : Translation in Z

#### **Nodes**

Node	<b>X</b> [ft]	<b>Y</b> [ft]	<b>Z</b> [ft]	Rigid Floor
1	0.00	0.00	1.50	0
2	-0.6362	0.00	1.9783	0
3	0.00	-3.3333	1.50	0
4	-0.6362	-3.3333	1.9783	0
5	0.6362	-3.3333	1.9783	0
6	0.6362	0.00	1.9783	0
7	6.00	-6.6667	4.33	0
8	6.00	3.3333	4.33	0
9	-6.00	0.00	4.13	0
10	6.00	0.00	4.13	0
11	-6.00	-3.3333	4.13	0
12	6.00	-3.3333	4.13	0
13	-2.4126	0.00	3.7374	0
14	-2.4126	-3.3333	3.7374	0
15	2.4126	-3.3333	3.7374	0
16	2.4126	0.00	3.7374	0
17	-2.2835	0.00	3.6096	0
18	-2.2835	-3.3333	3.6096	0
19	-0.7653	0.00	2.1062	0
20	-0.7653	-3.3333	2.1062	0
21	0.7653	0.00	2.1062	0
22	0.7653	-3.3333	2.1062	0
23	2.2835	0.00	3.6096	0
24	2.2835	-3.3333	3.6096	0
25	-2.4792	0.00	4.13	0
26	2.4792	0.00	4.13	0
27	2.4792	-3.3333	4.13	0
28	-2.4792	-3.3333	4.13	0

20	6.00	2 225 06	4 22	0
29		3.33E-06	4.33	0
30	6.00	-3.3333	4.33	0
31	0.00	0.00	1.9783	0
32	0.00	-3.3333	1.9783	0
33	-1.2713	0.00	2.6073	0
34	-1.2713	-3.3333	2.6073	0
35	-1.4213	0.00	2.4573	0
36	-1.4213	-3.3333	2.4573	0
37	1.2713	0.00	2.6073	0
38	1.2713	-3.3333	2.6073	0
39	1.4213	0.00	2.4573	0
40	1.4213	-3.3333	2.4573	0
41	-6.00	-6.6667	4.33	0
42	-6.00	3.3333	4.33	0
43	-6.00	3.33E-06	4.33	0
44	-6.00	-3.3333	4.33	0
	-1.4213			
45		-4.6667	2.4573	0
46	1.4213	-4.6667	2.4573	0
47	-1.4213	1.3333	2.4573	0
48	1.4213	1.3333	2.4573	0
55	-4.00	0.00	4.13	0
56	-2.601	-0.25	-2.5952	0
57	4.00	0.00	4.13	0
58	2.3055	0.25	-2.5431	0
59	0.00	1.335	1.50	0
60	0.00	-4.665	1.50	0
61	0.00	0.50	1.50	0
62	0.00	-3.8333	1.50	0
63	0.00	0.50	0.50	0
64	0.00	-3.8333	0.50	0
65	0.00	0.00	-1.25	0
66	-2.601	0.00	-2.5952	0
	-2.6971			
67		0.00	-3.3853	0
68	-2.601	-3.3333	-2.5952	0
69	-2.6971	-3.3333	-3.3853	0
70	-3.3333	-3.3333	-2.2835	0
71	-3.3333	0.00	-2.2835	0
72	-8.0518	-6.6667	1.1859	0
73	-8.0518	3.3333	1.1859	0
74	-1.8786	0.00	-9.1064	0
75	-7.8786	0.00	1.2859	0
76	-1.8786	-3.3333	-9.1064	0
77	-7.8786	-3.3333	1.2859	0
78	-3.3323	0.00	-5.8033	0
79	-3.3323	-3.3333	-5.8033	0
		-3.3333		0
80	-5.7449		-1.6246	
81	-5.7449	0.00	-1.6246	0
82	-3.2862	0.00	-5.6276	0
83	-3.2862	-3.3333	-5.6276	0
84	-2.7433	0.00	-3.5611	0
85	-2.7433	-3.3333	-3.5611	0
86	-3.5086	0.00	-2.2356	0
87	-3.5086	-3.3333	-2.2356	0
88	-5.5697	0.00	-1.6725	0
89	-5.5697	-3.3333	-1.6725	0
90	-3.639	0.00	-6.0573	
				0
91	-6.1182	0.00	-1.7632	0
92	-6.1182	-3.3333	-1.7632	0
93	-3.639	-3.3333		0
			-6.0573	
94	-8.0518	3.33E-06	1.1859	0
95	-8.0518	-3.3333	1.1859	0
96	-3.0152	0.00	-2.8344	0
97	-3.0152	-3.3333	-2.8344	0
98	-2.9243	0.00	-4.2499	0
99	-2.9243	-3.3333	-4.2499	0
100	-2.7194	0.00	-4.3048	0

101	-2.7194	-3.3333	-4.3048	0
102	-4.1956	0.00	-2.0479	0
103	-4.1956	-3.3333	-2.0479	0
104	-4.1407	0.00	-1.843	0
105	-4.1407	-3.3333	-1.843	0
106	-2.0518	-6.6667	-9.2064	0
107	-2.0518	3.3333	-9.2064	0
108	-2.0518	3.33E-06	-9.2064	0
109	-2.0518	-3.3333	-9.2064	0
110	-2.7194	-4.6667	-4.3048	0
111	-4.1407	-4.6667	-1.843	0
112	-2.7194	1.3333	-4.3048	0
113	-4.1407	1.3333	-1.843	0
120	-2.8786	0.00	-7.3743	0
121	-6.8786	0.00	-0.4461	0
122	-2.601	1.335	-2.5952	0
123	-2.601	-4.665	-2.5952	0
124	-2.601	0.50	-2.5952	0
125	-2.601	-3.8333	-2.5952	0
126	-2.0207	0.50	-2.2602	0
127	-2.0207	-3.8333	-2.2602	0
128	2.3055	-0.25	-2.5431	0
129	0.00	0.25	1.50	0
130	2.3055	0.00	-2.5431	0
131	3.0379	0.00	-2.2314	0
132	2.3055	-3.3333	-2.5431	0
133	3.0379	-3.3333	-2.2314	0
134	2.4017	-3.3333	-3.3332	0
135	2.4017	0.00	-3.3332	0
136	1.7564	-6.6667	-9.1543	0
137	1.7564	3.3333	-9.1543	0
138	7.5832	0.00	1.338	0
139	1.5832	0.00	-9.0543	0
140	7.5832	-3.3333	1.338	0
141	1.5832	-3.3333	-9.0543	0
142	5.4495	0.00	-1.5725	0
143	5.4495	-3.3333	-1.5725	0
144	3.0369	-3.3333	-5.7512	0
145	3.0369	0.00	-5.7512	0
146	5.2742	0.00	-1.6204	0
147	5.2742	-3.3333	-1.6204	0
148	3.2131	0.00	-2.1835	0
149	3.2131	-3.3333	-2.1835	0
150	2.4479	0.00	-3.509	0
151	2.4479	-3.3333	-3.509	0
152	2.9907	0.00	-5.5755	0
			1 1 1 1	_
153	2.9907	-3.3333	-5.5755	0
154	5.8228	0.00	-1.7111	0
155	3.3436	0.00	-6.0052	0
156	3.3436	-3.3333	-6.0052	0
157	5.8228	-3.3333	-1.7111	0
158	1.7564	3.33E-06	-9.1543	0
159	1.7564	-3.3333	-9.1543	0
160	2.7198	0.00	-2.7823	0
161	2.7198	-3.3333	-2.7823	0
162	3.9002	0.00	-1.9958	0
163	3.9002	-3.3333	-1.9958	0
164	3.8453	0.00	-1.7909	0
165	3.8453	-3.3333	-1.7909	0
166	2.6288	0.00	-4.1978	0
167	2.6288	-3.3333	-4.1978	0
168	2.4239	0.00	-4.2527	0
169	2.4239	-3.3333	-4.2527	0
170	7.7564	-6.6667	1.238	0
171	7.7564	3.3333	1.238	0
172	7.7564	3.33E-06	1.238	0

173	7.7564	-3.3333	1.238	0
174	3.8453	-4.6667	-1.7909	0
175	2.4239	-4.6667	-4.2527	0
176	3.8453	1.3333	-1.7909	0
177	2.4239	1.3333	-4.2527	0
184	6.5832	0.00	-0.394	0
185	2.5832	0.00	<b>-</b> 7.3222	0
186	2.3055	1.335	-2.5431	0
187	2.3055	-4.665	-2.5431	0
188	2.3055	0.50	-2.5431	0
189	2.3055	-3.8333	-2.5431	0
190	1.7253	0.50	-2.2081	0
191	1.7253	-3.8333	-2.2081	0
192	0.00	-0.25	1.50	0
193	-2.601	0.25	-2.5952	0

### Restraints

Node	TX	TY	TZ	RX	RY	RZ
63	 1	 1	 1	0	 1	0
64	1	1	1	0	1	0
126	1	1	1	0	1	0
127	1	1	1	0	1	0
190	1	1	1	0	1	0
191	1	1	1	0	1	0

#### Members

Member	NJ	NK	Description	Section	Material	<b>d0</b> [in]	<b>dL</b> [in]	lg factor
 1	 8	 7		PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
2	9	10		PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
3	11	12		PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
4	13	2		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
5	14	4		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
6	15	5		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
7	16	6		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
8	17	18		RndBar 5_8	A36	0.00	0.00	0.00
9	19	20		RndBar 5 8	A36	0.00	0.00	0.00
10	21	22		RndBar 5_8	A36	0.00	0.00	0.00
11	23	24		RndBar 5 8	A36	0.00	0.00	0.00
12	21	24		RndBar 3_4	A36	0.00	0.00	0.00
13	22	23		RndBar 3 4	A36	0.00	0.00	0.00
14	18	19		RndBar 3 4	A36	0.00	0.00	0.00
15	17	20		RndBar 3 4	A36	0.00	0.00	0.00
16	13	25		PL 3-1/2x5/8	A36	0.00	0.00	0.00
17	16	26		PL 3-1/2x5/8	A36	0.00	0.00	0.00
18	15	27		PL 3-1/2x5/8	A36	0.00	0.00	0.00
19	14	28		PL 3-1/2x5/8	A36	0.00	0.00	0.00
22	2	31		PL 3-1/2x5/8	A36	0.00	0.00	0.00
23	31	6		PL 3-1/2x5/8	A36	0.00	0.00	0.00
24	4	32		PL 3-1/2x5/8	A36	0.00	0.00	0.00
25	32	5		PL 3-1/2x5/8	A36	0.00	0.00	0.00
26	31	1		PL 11-1/4x5/8	A36	11.25	9.25	0.00
27	32	3		PL 11-1/4x5/8	A36	11.25	9.25	0.00
32	42	41		PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
35	47	45		PIPE 2x0.154	A53 GrB	0.00	0.00	0.00

36	48	46	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
40	55	56	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
41	58	57	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
42	59	60	PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
43	61	63	HSS SQR 4X4X3 16	A500 GrB rectangular	0.00	0.00	0.00
44	62	64	HSS SQR 4X4X3 16	A500 GrB rectangular		0.00	0.00
45	73	72	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
46	74	75	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
47	76	77	PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
48	78	67	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
49	79	69	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
50	80	70	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
51	81	71	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
52	82	83	RndBar 5_8	A36	0.00	0.00	0.00
53	84	85	RndBar 5 8	A36	0.00	0.00	0.00
54	86	87	RndBar 5_8	A36	0.00	0.00	0.00
55	88	89	RndBar 5_8	A36	0.00	0.00	0.00
56	86	89	RndBar 3_4	A36	0.00	0.00	0.00
57	87	88	RndBar 3_4	A36	0.00	0.00	0.00
58	83	84	RndBar 3_4	A36	0.00	0.00	0.00
59	82	85	RndBar 3_4	A36	0.00	0.00	0.00
60	78	90	PL 3-1/2x5/8	A36	0.00	0.00	0.00
61	81	91	PL 3-1/2x5/8	A36	0.00	0.00	0.00
62	80	92	PL 3-1/2x5/8	A36	0.00	0.00	0.00
63	79	93	PL 3-1/2x5/8	A36	0.00	0.00	0.00
66	67	96	PL 3-1/2x5/8	A36	0.00	0.00	0.00
67	96	71	PL 3-1/2x5/8	A36	0.00	0.00	0.00
68	69	97		A36	0.00	0.00	0.00
			PL 3-1/2x5/8				
69	97	70	PL 3-1/2x5/8	A36	0.00	0.00	0.00
70	96	66	PL 11-1/4x5/8	A36	11.25	9.25	0.00
71	97	68	PL 11-1/4x5/8	A36	11.25	9.25	0.00
76	107	106	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
79	112	110	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
80	113	111	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
84	122	123	PIPE 3x0.216	A53 GrB	0.00	0.00	0.00
85	124	126	HSS SQR 4X4X3 16	A500 GrB rectangular	0.00	0.00	0.00
86	125	127		A500 GrB rectangular		0.00	0.00
			HSS_SQR 4X4X3_16	· ·			
87	120	128	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
88	129	121	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
89	137	136	PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
90	138	139	PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
91	140	141	PIPE 2-1_2x0.203	A53 GrB	0.00	0.00	0.00
92	142	131	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
93	143	133	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
94	144	134	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
95	145	135	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00
96	146	147	RndBar 5_8	A36	0.00	0.00	0.00
97	148	149	RndBar 5_8	A36	0.00	0.00	0.00
98	150	151	RndBar 5 8	A36	0.00	0.00	0.00
99	152	153	RndBar 5_8	A36	0.00	0.00	0.00
100	150	153	RndBar 3_4	A36	0.00	0.00	0.00
			District O. 4				
101	151	152	RndBar 3_4	A36	0.00	0.00	0.00
102	147	148	RndBar 3_4	A36	0.00	0.00	0.00
103	146	149	RndBar 3 4	A36	0.00	0.00	0.00
104	142	154	PL 3-1/2x5/8	A36	0.00	0.00	0.00
105	145	155	PL 3-1/2x5/8		0.00	0.00	0.00
				A36			
106	144	156	PL 3-1/2x5/8	A36	0.00	0.00	0.00
107	143	157	PL 3-1/2x5/8	A36	0.00	0.00	0.00
110	131	160	PL 3-1/2x5/8	A36	0.00	0.00	0.00
111	160	135	PL 3-1/2x5/8	A36	0.00	0.00	0.00
112	133	161	PL 3-1/2x5/8	A36	0.00	0.00	0.00
113	161	134	PL 3-1/2x5/8	A36	0.00	0.00	0.00
114	160	130	PL 11-1/4x5/8	A36	11.25	9.25	0.00
115	161	132	PL 11-1/4x5/8		11.25	9.25	0.00
120	171	170	PIPE 2-1 2x0.203	A53 GrB	0.00	0.00	0.00
123	176	174	PIPE 2x0.154	A53 GrB	0.00	0.00	0.00

124	177	175	PIPE 2x0.154 A53 GrB	0.00	0.00	0.00
128	186	187	PIPE 3x0.216 A53 GrB	0.00	0.00	0.00
129	188	190	HSS_SQR 4X4X3_16 A500 GrB rectangular	0.00	0.00	0.00
130	189	191	HSS_SQR 4X4X3_16 A500 GrB rectangular	0.00	0.00	0.00
131	184	192	PIPE 2x0.154 A53 GrB	0.00	0.00	0.00
132	193	185	PIPE 2x0.154 A53 GrB	0.00	0.00	0.00

### Orientation of local axes

Member	Rotation [Deg]	Axes23	NX	NY	NZ	
1	315.00	0	0.00	0.00	0.00	
8	0.00	2	0.00	0.00	1.00	
9	0.00	2	0.00	0.00	1.00	
10	0.00	2	0.00	0.00	1.00	
11	0.00	2	0.00	0.00	1.00	
16	90.00	0	0.00	0.00	0.00	
17	90.00	0	0.00	0.00	0.00	
18	90.00	0	0.00	0.00	0.00	
19	90.00	0	0.00	0.00	0.00	
22	90.00	0	0.00	0.00	0.00	
23 24	90.00	0 0	0.00 0.00	0.00	0.00 0.00	
25 25	90.00 90.00	0	0.00	0.00 0.00	0.00	
26	90.00	0	0.00	0.00	0.00	
27	90.00	0	0.00	0.00	0.00	
32	315.00	0	0.00	0.00	0.00	
35	315.00	Ö	0.00	0.00	0.00	
36	315.00	0	0.00	0.00	0.00	
45	0.00	2	0.8192	0.00	-0.5736	
52	0.00	2	-0.1736	0.00	0.9848	
53	0.00	2	-0.1736	0.00	0.9848	
54	0.00	2	-0.1736	0.00	0.9848	
55	0.00	2	-0.1736	0.00	0.9848	
60	90.00	0	0.00	0.00	0.00	
61	90.00	0	0.00	0.00	0.00	
62	90.00	0	0.00	0.00	0.00	
63	90.00	0	0.00	0.00	0.00	
66	90.00	0	0.00	0.00	0.00	
67	90.00	0	0.00	0.00	0.00	
68	90.00	0	0.00	0.00	0.00	
69	90.00	0	0.00	0.00	0.00	
70	90.00	0	0.00	0.00	0.00	
71	90.00	0	0.00	0.00	0.00	
76 70	0.00	2	0.8192	0.00	-0.5736	
79	0.00	2 2	0.8192	0.00	-0.5736	
80 84	0.00 0.00	2	0.8192 0.9848	0.00 0.00	-0.5736 0.1736	
89	0.00	2	0.5736	0.00	-0.8192	
96	0.00	2	0.3736	0.00	0.9848	
97	0.00	2	0.1736	0.00	0.9848	
98	0.00	2	0.1736	0.00	0.9848	
99	0.00	2	0.1736	0.00	0.9848	
104	90.00	0	0.00	0.00	0.00	
105	90.00	0	0.00	0.00	0.00	
106	90.00	0	0.00	0.00	0.00	
107	90.00	0	0.00	0.00	0.00	
110	90.00	0	0.00	0.00	0.00	
111	90.00	0	0.00	0.00	0.00	
112	90.00	0	0.00	0.00	0.00	
113	90.00	0	0.00	0.00	0.00	
114	90.00	0	0.00	0.00	0.00	

115	90.00	0	0.00	0.00	0.00	
120	0.00	2	0.5736	0.00	-0.8192	
123	0.00	2	0.5736	0.00	-0.8192	
124	0.00	2	0.5736	0.00	-0.8192	
128	0.00	2	0.9848	0.00	-0.1736	

# Rigid end offsets

Member	DJX	DJY	DJZ	DKX	DKY	DKZ	
	[in]	[in]	[in] 	[in] 	[in] 	[in] 	
12	0.00	-3.50	0.00	0.00	3.50	0.00	
13	0.00	3.50	0.00	0.00	-3.50	0.00	
14	0.00	3.50	0.00	0.00	-3.50	0.00	
15	0.00	-3.50	0.00	0.00	3.50	0.00	
26	0.00	-0.625	0.00	0.00	-0.625	0.00	
27	0.00	-0.625	0.00	0.00	-0.625	0.00	
56	0.00	-3.50	0.00	0.00	3.50	0.00	
57	0.00	3.50	0.00	0.00	-3.50	0.00	
58	0.00	3.50	0.00	0.00	-3.50	0.00	
59	0.00	-3.50	0.00	0.00	3.50	0.00	
70	0.00	-0.625	0.00	0.00	-0.625	0.00	
71	0.00	-0.625	0.00	0.00	-0.625	0.00	
100	0.00	-3.50	0.00	0.00	3.50	0.00	
101	0.00	3.50	0.00	0.00	-3.50	0.00	
102	0.00	3.50	0.00	0.00	-3.50	0.00	
103	0.00	-3.50	0.00	0.00	3.50	0.00	
114	0.00	-0.625	0.00	0.00	-0.625	0.00	
115	0.00	-0.625	0.00	0.00	-0.625	0.00	

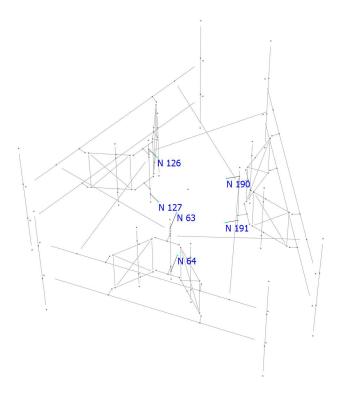
# Hinges

Node-J						Node	2-K				
Member	M33	M22	V3	V2	M33	M22	V3	V2	TOR	AXL	Axial rigidity
13	0	0	0	0	0	0	0	 0	 0	0	Tension only
15	0	0	0	0	0	0	0	0	0	0	Tension only
16	1	1	0	0	0	0	0	0	0	0	Full
17	1	1	0	0	0	0	0	0	0	0	Full
18	1	1	0	0	0	0	0	0	0	0	Full
19	1	1	0	0	0	0	0	0	0	0	Full
40	1	1	0	0	1	1	0	0	0	0	Full
41	1	1	0	0	1	1	0	0	0	0	Full
57	0	0	0	0	0	0	0	0	0	0	Tension only
59	0	0	0	0	0	0	0	0	0	0	Tension only
60	1	1	0	0	0	0	0	0	0	0	Full
61	1	1	0	0	0	0	0	0	0	0	Full
62	1	1	0	0	0	0	0	0	0	0	Full
63	1	1	0	0	0	0	0	0	0	0	Full
87	1	1	0	0	1	1	0	0	0	0	Full
88	1	1	0	0	1	1	0	0	0	0	Full
101	0	0	0	0	0	0	0	0	0	0	Tension only
103	0	0	0	0	0	0	0	0	0	0	Tension only
104	1	1	0	0	0	0	0	0	0	0	Full
105	1	1	0	0	0	0	0	0	0	0	Full
106	1	1	0	0	0	0	0	0	0	0	Full
107	1	1	0	0	0	0	0	0	0	0	Full
131	1	1	0	0	1	1	0	0	0	0	Full
132	1	1	0	0	1	1	0	0	0	0	Full

\_\_\_\_\_



Bentley
Current Date: 12/3/2021 3:29 PM
Units system: English
File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT1338\CT1338.retx







Current Date: 12/3/2021 3:30 PM

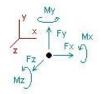
Units system: English

File name: W:\STRUCTURAL DEPARTMENT\ANALYSIS SOFTWARE\RAM Elements\RAM Projects\AT&T\CT\CT\338\CT1338.retx

# **Analysis result**

#### **Envelope for nodal reactions**

Note.-Ic is the controlling load condition



Direction of positive forces and moments

Envelope of nodal reactions for

LC1=1.2DL+W0

LC2=1.2DL+W30

LC3=1.2DL-W0

LC4=1.2DL-W30

LC5=0.9DL+W0

LC6=0.9DL+W30

LC7=0.9DL-W0

LC8=0.9DL-W30

LC9=1.2DL+Di+Wi0

LC10=1.2DL+Di+Wi30

LC11=1.2DL+Di-Wi0

LC12=1.2DL+Di-Wi30

LC13=1.4DL

LC14=1.2DL+1.6LL1

LC15=1.2DL+1.6LL2

LC16=1.2DL+WL0+1.6LLa1

LC17=1.2DL+WL30+1.6LLa1

LC18=1.2DL-WL0+1.6LLa1 LC19=1.2DL-WL30+1.6LLa1

LC20=1.2DL+WL0+1.6LLa2

LC21=1.2DL+WL30+1.6LLa2

LC22=1.2DL-WL0+1.6LLa2

LC23=1.2DL-WL30+1.6LLa2

LC24=1.2DL+WL0+1.6LLa3

LC25=1.2DL+WL30+1.6LLa3 LC26=1.2DL-WL0+1.6LLa3

LC27=1.2DL-WL30+1.6LLa3

LC28=1.2DL+WL0+1.6LLa4

LC29=1.2DL+WL30+1.6LLa4 LC30=1.2DL-WL0+1.6LLa4

LC31=1.2DL-WL30+1.6LLa4

	Forces							Mome	ents				
Node		<b>Fx</b> [Kip]	lc	<b>Fy</b> [Kip]	lc	<b>Fz</b> [Kip]	lc	<b>Mx</b> [Kip*ft]	lc	<b>My</b> [Kip*ft]	lc	<b>Mz</b> [Kip*ft]	lc
63	Max	1.644	LC2	1.084	LC10	-0.198	LC7	0.00000	LC1	1.62169	LC2	0.00000	LC1
	Min	-1.527	LC8	0.320	LC5	-1.315	LC9	0.00000	LC1	-1.53042	LC8	0.00000	LC1
64	Max	1.051	LC17	1.246	LC12	1.688	LC9	0.00000	LC1	1.13002	LC17	0.00000	LC1
	Min	-1.172	LC23	0.365	LC7	-0.190	LC7	0.00000	LC1	-1.31679	LC23	0.00000	LC1
126	Max	1.063	LC3	1.037	LC9	1.586	LC1	0.00000	LC1	1.08432	LC1	0.00000	LC1
	Min	-0.377	LC5	0.250	LC7	-1.048	LC7	0.00000	LC1	-1.01779	LC7	0.00000	LC1
127	Max	0.343	LC6	1.306	LC11	0.495	LC5	0.00000	LC1	0.80995	LC5	0.00000	LC1
	Min	-1.279	LC12	0.334	LC5	-1.134	LC3	0.00000	LC1	-0.90559	LC3	0.00000	LC1

190	Max	0.348	LC5	1.028	LC12	1.576	LC1	0.00000	LC1	1.18111	LC3	0.00000	LC1
	Min	-1.150	LC3	0.254	LC6	-1.242	LC7	0.00000	LC1	-1.12414	LC5	0.00000	LC1
191	Max	1.372	LC2	1.295	LC11	0.494	LC5	0.00000	LC1	0.80009	LC7	0.00000	LC1
	Min	-0.357	LC8	0.337	LC8	-0.967	LC3	0.00000	LC1	-0.90110	LC1	0.00000	LC1



**Connection Check** 

**Date:** 12/3/2021

Project Name: REDDING BLACK ROCK ROAD

Project No.: CT1338

Designed By: ID Checked By: MSC



### **CHECK CONNECTION CAPACITY (Worst Case)**

**Reference:** AISC Steel Construction Manual 14th Edition (ASD)

**Bolt Type =** A325 5/8" (Threaded Rod)

Allowable Tensile Load =

 $\mathbf{F}_{\mathsf{Tall}} = 13806 \; \mathsf{lbs}.$ 

Allowable Shear Load =

F<sub>Vall</sub>= 8283 lbs.

**TENSILE FORCES** 

**Reaction F** = 1315 lbs. (See Bentley Output)

**SHEAR FORCES** 

Reactions in X direction: 1644 lbs. (See Bentley Output)
Reactions in Y direction: 1084 lbs. (See Bentley Output)

Resultant: 1969 lbs.

No. of Supports = 1 No. of Bolts / Support = 3

Tension Design Load /Bolts =

 $f_t$ = 438.33 lbs. < 13806 lbs. Therefore, OK!

Shear Design Load / Bolts=

 $f_v$ = 656.40 lbs. **<** 8283 lbs. **Therefore, OK!** 

**CHECK COMBINED TENSION AND SHEAR** 

 $f_t/F_T$  +  $f_v/F_V$   $\leq$  1.0

0.032 + 0.079 = 0.111 < 1.0 Therefore, OK!

# **ATTACHMENT 5**



# Calculated Radio Frequency Exposure



CT1338

186 Black Rock Turnpike, Redding, CT

# Table of Contents

1. Introduction	l
2. FCC Guidelines for Evaluating RF Radiation Exposure Limits	
3. RF Exposure Calculation Methods	2
4. Calculation Results	3
5. Conclusion	
6. Statement of Certification	
Attachment A: References	
Attachment B: FCC Limits for Maximum Permissible Exposure (MPE)	6
Attachment C: AT&T Antenna Data Sheets and Electrical Patterns	
List of Tables  Table 1: Carrier Information	3
Table 2: FCC Limits for Maximum Permissible Exposure (MPE)	6
List of Figures	
Figure 1: Graph of FCC Limits for Maximum Permissible Exposure (MPE)	7



#### 1. Introduction

The purpose of this report is to investigate compliance with applicable FCC regulations for the proposed installation of AT&T antenna arrays on an existing tower located at 186 Black Rock Turnpike in Redding, CT. The coordinates of the existing tower are 41° 18' 35.77"N, 73° 20' 51.35"W.

AT&T is proposing the following:

1) Install six (6) multi-band antennas (two (2) per sector) to support its commercial LTE network and the FirstNet National Public Safety Broadband Network ("NPSBN").

This report considers the planned antenna configuration for AT&T<sup>1</sup> to derive the resulting % Maximum Permissible Exposure of its proposed installation.

#### 2. FCC Guidelines for Evaluating RF Radiation Exposure Limits

In 1985, the FCC established rules to regulate radio frequency (RF) exposure from FCC licensed antenna facilities. In 1996, the FCC updated these rules, which were further amended in August 1997 by OET Bulletin 65 Edition 97-01. These new rules include Maximum Permissible Exposure (MPE) limits for transmitters operating between 300 kHz and 100 GHz. The FCC MPE limits are based upon those recommended by the National Council on Radiation Protection and Measurements (NCRP), developed by the Institute of Electrical and Electronics Engineers, Inc., (IEEE) and adopted by the American National Standards Institute (ANSI).

The FCC general population/uncontrolled limits set the maximum exposure to which most people may be subjected. General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure.

Public exposure to radio frequencies is regulated and enforced in units of milliwatts per square centimeter (mW/cm²). The general population exposure limits for the various frequency ranges are defined in the attached "FCC Limits for Maximum Permissible Exposure (MPE)" in Attachment B of this report.

Higher exposure limits are permitted under the occupational/controlled exposure category, but only for persons who are exposed as a consequence of their employment and who have been made fully aware of the potential for exposure, and they must be able to exercise control over their exposure. General population/uncontrolled limits are five times more stringent than the levels that are acceptable for occupational, or radio frequency trained individuals. Attachment B contains excerpts from OET Bulletin 65 and defines the Maximum Exposure Limit.

Finally, it should be noted that the MPE limits adopted by the FCC for both general population/uncontrolled exposure and for occupational/controlled exposure incorporate a substantial margin of safety and have been established to be well below levels generally accepted as having the potential to cause adverse health effects.

<sup>&</sup>lt;sup>1</sup> As referenced to AT&T's Radio Frequency Design Sheet dated 10/6/2021.



#### 3. RF Exposure Calculation Methods

The power density calculation results were generated using the following formula as outlined in FCC bulletin OET 65, and Connecticut Siting Council recommendations:

Power Density = 
$$\left(\frac{1.6^2 \times 1.64 \times ERP}{4\pi \times R^2}\right)$$
 X Off Beam Loss

Where:

ERP = Effective Radiated Power

R = Radial Distance = 
$$\sqrt{(H^2 + V^2)}$$

H = Horizontal Distance from antenna

V = Vertical Distance from radiation center of antenna

Ground reflection factor of 1.6

Off Beam Loss is determined by the selected antenna pattern

These calculations assume that the antennas are operating at 100 percent capacity and power, and that all antenna channels are transmitting simultaneously. Obstructions (trees, buildings, etc.) that would normally attenuate the signal are not taken into account. The calculations assume even terrain in the area of study and do not consider actual terrain elevations which could attenuate the signal. As a result, the predicted signal levels reported below are much higher than the actual signal levels will be from the final installations.



#### 4. Calculation Results

Table 1 below outlines the cumulative power density information for the AT&T installation on the existing tower at the site. The proposed antennas are directional in nature; therefore, the majority of the RF power is focused out towards the horizon. As a result, there will be less RF power directed below the antennas relative to the horizon, and consequently lower power density levels around the base of the tower. Please refer to Attachment C for the vertical pattern of the proposed AT&T antennas. The calculated results for AT&T in Table 1 include a nominal 10 dB off-beam pattern loss to account for the lower relative gain below the antennas.

Carrier	Antenna Height (Feet)	Operating Frequency (MHz)	Number of Trans.	ERP Per Transmitter (Watts)	Power Density (mw/cm²)	Limit	% MPE
Redding FD WNWN646	85	33	3	95	0.0164	0.2000	0.82%
Redding FD WNWN646	60	33	1	95	0.0117	0.2000	0.59%
Redding FD WQKB457	100	150	2	17	0.0013	0.2000	0.07%
Verizon	140	751	4	591	0.0473	0.5007	0.95%
Verizon	140	874	4	677	0.0542	0.5827	0.93%
Verizon	140	1980	4	1008	0.0808	1.0000	0.81%
Verizon	140	2120	4	987	0.0791	1.0000	0.79%
Verizon	140	3730	4	6531	0.5232	1.0000	5.23%
AT&T	145	739	1	2450	0.0046	0.4927	0.93%
AT&T	145	763	1	2749	0.0051	0.5087	1.01%
AT&T	145	885	1	2813	0.0052	0.5900	0.89%
AT&T	145	1900	2	6297	0.0234	1.0000	2.34%
AT&T	145	2100	2	9445	0.0352	1.0000	3.52%
						AT&T Total	18.86%

Table 1: Carrier Information<sup>2</sup>

\_

<sup>&</sup>lt;sup>2</sup> The existing record in the CSC Power Density Table for AT&T should be removed and replaced with the updated AT&T technologies and values provided in Table 1. The power density information for Redding FD and Verizon was taken directly from the CSC database dated 01/21/2022. Please note that % MPE values listed are rounded to two decimal points and the total % MPE listed is a summation of each unrounded contribution. Therefore, summing each rounded value may not identically match the total value reflected in the table.



#### 5. Conclusion

The above analysis concludes that RF exposure at ground level from the proposed site will be below the maximum power density levels as outlined by the FCC in the OET Bulletin 65 Ed. 97-01. Using conservative calculation methods, the highest expected percent of Maximum Permissible Exposure at ground level is **18.86% of the FCC General Population/Uncontrolled limit**.

As noted previously, the calculated % MPE levels are more conservative (higher) than the actual signal levels will be from the finished modifications.

#### 6. Statement of Certification

I certify to the best of my knowledge that the statements in this report are true and accurate. The calculations follow guidelines set forth in FCC OET Bulletin 65 Edition 97-01, ANSI/IEEE Std. C95.1 and ANSI/IEEE Std. C95.3.

March 30, 2022 Date

Reviewed/Approved By: Martin J. Lavin

Senior RF Engineer C Squared Systems, LLC

Mark f Law



#### **Attachment A: References**

OET Bulletin 65 - Edition 97-01 - August 1997 Federal Communications Commission Office of Engineering & Technology

<u>IEEE C95.1-2005, IEEE Standard Safety Levels With Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz</u> IEEE-SA Standards Board

<u>IEEE C95.3-2002 (R2008), IEEE Recommended Practice for Measurements and Computations of Radio Frequency Electromagnetic Fields With Respect to Human Exposure to Such Fields, 100 kHz-300 GHz IEEE-SA Standards Board Computations of Radio Frequency Electromagnetic Fields With Respect to Human Exposure to Such Fields, 100 kHz-300 GHz IEEE-SA Standards Board</u>

CT1338 5 March 30, 2022



# Attachment B: FCC Limits for Maximum Permissible Exposure (MPE)

# (A) Limits for Occupational/Controlled Exposure<sup>3</sup>

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm <sup>2</sup> )	Averaging Time $ E ^2$ , $ H ^2$ or S (minutes)
0.3-3.0	614	1.63	(100)*	6
3.0-30	1842/f	4.89/f	$(900/f^2)*$	6
30-300	61.4	0.163	1.0	6
300-1500	-	-	f/300	6
1500-100,000	-	-	5	6

# (B) Limits for General Population/Uncontrolled Exposure<sup>4</sup>

Frequency Range (MHz)	Electric Field Strength (E) (V/m)	Magnetic Field Strength (E) (A/m)	Power Density (S) (mW/cm <sup>2</sup> )	Averaging Time $ E ^2$ , $ H ^2$ or S (minutes)
0.3-1.34	614	1.63	(100)*	30
1.34-30	824/f	2.19/f	$(180/f^2)*$	30
30-300	27.5	0.073	0.2	30
300-1500	-	-	f/1500	30
1500-100,000	-	-	1.0	30

f = frequency in MHz \* Plane-wave equivalent power density

**Table 2: FCC Limits for Maximum Permissible Exposure (MPE)** 

CT1338 6 March 30, 2022

<sup>&</sup>lt;sup>3</sup> Occupational/controlled limits apply in situations in which persons are exposed as a consequence of their employment provided those persons are fully aware of the potential for exposure and can exercise control over their exposure. Limits for occupational/controlled exposure also apply in situations when an individual is transient through a location where occupational/controlled limits apply provided he or she is made aware of the potential for exposure

<sup>&</sup>lt;sup>4</sup> General population/uncontrolled exposures apply in situations in which the general public may be exposed, or in which persons that are exposed as a consequence of their employment may not be fully aware of the potential for exposure or cannot exercise control over their exposure



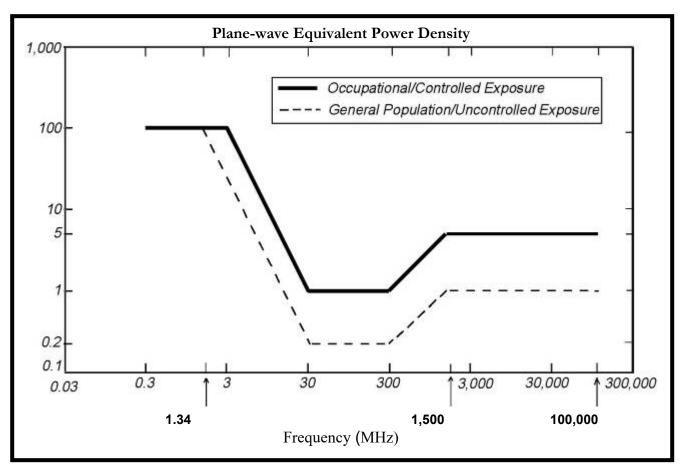


Figure 1: Graph of FCC Limits for Maximum Permissible Exposure (MPE)



#### Attachment C: AT&T Antenna Data Sheets and Electrical Patterns

# 739 MHz

Manufacturer: CCI Products

Model #: DMP65R-BU6D

Frequency Band: 698-798 MHz

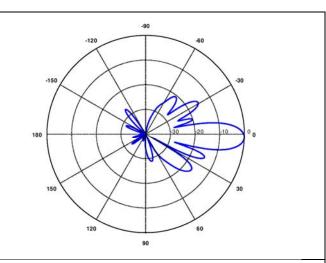
Gain: 14.0 dBi

Vertical Beamwidth: 13.0°

Horizontal Beamwidth: 74°

Polarization: Dual Linear 45°

Size L x W x D:  $71.2" \times 20.7" \times 7.7"$ 



#### **763 MHz**

Manufacturer: CCI Products

Model #: TPA65R-BU6D

Frequency Band: 698 - 806MHz

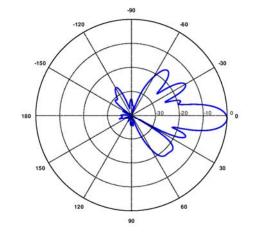
Gain: 14.5 dBi

Vertical Beamwidth: 12.8°

Horizontal Beamwidth: 73°

Polarization: Dual Linear 45°

Size L x W x D:  $71.2" \times 20.7" \times 7.7"$ 



#### 885 MHz

Manufacturer: CCI Products

Model #: DMP65R-BU6D

Frequency Band: 824 - 896 MHz

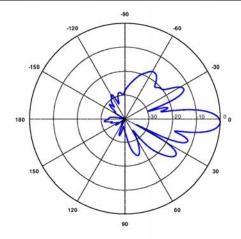
Gain: 14.6 dBi

Vertical Beamwidth: 11.1°

Horizontal Beamwidth: 63°

Polarization: Dual Linear 45°

Size L x W x D: 71.2" × 20.7" × 7.7"





# 1900 MHz

Manufacturer: CCI Products

Model #: TPA65R-BU6D

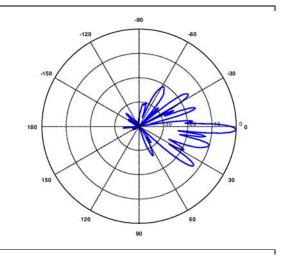
Frequency Band: 1920-1990 MHz

Gain: 18.1 dBi

Vertical Beamwidth: 5.2° Horizontal Beamwidth: 66°

Polarization: Dual Linear 45°

Size L x W x D: 71.2" × 20.7" × 7.7"



#### 2100 MHz

Manufacturer: CCI Products

Model #: DMP65R-BU6D

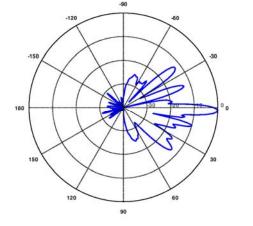
Frequency Band: 1920-2180 MHz

Gain: 18.1 dBi

Vertical Beamwidth: 4.8° Horizontal Beamwidth: 68°

Polarization: Dual Linear 45°

Size L x W x D: 71.2" × 20.7" × 7.7"



# **ATTACHMENT 6**



445 Hamilton Avenue, 14th Floor White Plains, New York 10601 T 914 761 1300 F 914 761 5372 cuddyfeder.com

Daniel Patrick dpatrick@cuddyfeder.com

July 20, 2022

#### FEDERAL EXPRESS

Julia Pemberton, First Selectwoman Town of Redding 100 Hill Road P.O. Box 1028 Redding, CT 06875

Re:

Tower Sharing Request by New Cingular Wireless PCS, LLC

Premises: 186 Black Rock Turnpike, Redding, Connecticut

Dear First Selectwoman Pemberton:

We are writing to you on behalf of our client New Cingular Wireless PCS, LLC ("AT&T") with respect to the above referenced request to the Connecticut Siting Council ("Council") for shared use approval to allow AT&T to install its wireless communications equipment on the existing communications tower and at the associated compound at 186 Black Rock Turnpike in the Town of Redding. AT&T proposes to install 6 antennas, 9 remote radiohead units on a sector frame mount attached to the existing tower at the centerline height of approximately 145' above ground level, with the height to the top of AT&T's antennas at approximately 148' above ground level.

Enclosed herein is a copy of the submission made to the Council requesting approval of the tower share which includes information regarding the technical, legal, environmental, and economic feasibility of AT&T's proposed installation.

Should you have any questions please feel free to contact me at the address above or the Council at 860.827.2935.

Very truly yours,

Daniel Patrick Enclosure



#### Shipment Receipt

#### **Address Information**

Ship to:

Ship from:

Julia Pemberton

**Daniel Patrick** 

First Selectwoman

Cuddy & Feder LLP

Town of Redding

445 Hamilton Avenue

100 Hill Road

**Suite 1400** 

REDDING CENTER, CT

White Plains, NY

06875

10601

US

US

914 761 1300

9147611300

# **Shipment Information:**

Tracking no.: 777437715114

Ship date: 07/20/2022

Estimated shipping charges: 27.23 USD

### **Package Information**

Pricing option: FedEx Standard Rate Service type: Priority Overnight

Package type: FedEx Pak Number of packages: 1 Total weight: 1 LBS

Declared Value: 0.00 USD

Special Services: No signature required

Pickup/Drop-off: Use an already scheduled pickup at my location

# **Billing Information:**

Bill transportation to: CuddyFeder-963

Your reference: 1844-3768

P.O. no.: Invoice no.: Department no.:

#### Thank you for shipping online with FedEx ShipManager at fedex.com.

#### Please Note

FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1000, e.g., jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits; Consult the applicable FedEx Service Guide for details.

The estimated shipping charge may be different than the actual charges for your shipment. Differences may occur based on actual weight, dimensions, and other factors. Consult the applicable FedEx Service Guide or the FedEx Rate Sheets for details on how shipping charges are calculated.



Daniel Patrick dpatrick@cuddyfeder.com

July 20, 2022

#### FEDERAL EXPRESS

Aimee Pardee, Land Use Director Old Town House 23 Cross Highway P.O. Box 1028 Redding, CT 06875

Re:

Tower Sharing Request by New Cingular Wireless PCS, LLC

Premises: 186 Black Rock Turnpike, Redding, Connecticut

Dear Ms. Pardee:

We are writing to you on behalf of our client New Cingular Wireless PCS, LLC ("AT&T") with respect to the above referenced request to the Connecticut Siting Council ("Council") for shared use approval to allow AT&T to install its wireless communications equipment on the existing communications tower and at the associated compound at 186 Black Rock Turnpike in the Town of Redding. AT&T proposes to install 6 antennas, 9 remote radiohead units on a sector frame mount attached to the existing tower at the centerline height of approximately 145' above ground level, with the height to the top of AT&T's antennas at approximately 148' above ground level.

Enclosed herein is a copy of the submission made to the Council requesting approval of the tower share which includes information regarding the technical, legal, environmental, and economic feasibility of AT&T's proposed installation.

Should you have any questions please feel free to contact me at the address above or the Council at 860.827.2935.

Very truly yours,

Daniel Patrick Enclosure



# Shipment Receipt

#### **Address Information**

Ship to:

Ship from:

Aimee Pardee

Daniel Patrick

Land Use Director

Cuddy & Feder LLP

Old Town House

445 Hamilton Avenue

23 Cross Highway

**Suite 1400** 

REDDING CENTER, CT

White Plains, NY

06875

10601

US

US

914 761-1300

9147611300

# **Shipment Information:**

Tracking no.: 777437816812

Ship date: 07/20/2022

Estimated shipping charges: 21.52 USD

### **Package Information**

Pricing option: FedEx Standard Rate Service type: Priority Overnight Package type: FedEx Envelope

Number of packages: 1 Total weight: 1 LBS

Declared Value: 0.00 USD

Special Services: No signature required

Pickup/Drop-off: Use an already scheduled pickup at my location

# **Billing Information:**

Bill transportation to: CuddyFeder-963

Your reference: 1844-3768

P.O. no.: Invoice no.: Department no.:

#### Thank you for shipping online with FedEx ShipManager at fedex.com.

FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1000, e.g., jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits; Consult the applicable FedEx Service Guide for details.

The estimated shipping charge may be different than the actual charges for your shipment. Differences may occur based on actual weight, dimensions, and other factors. Consult the applicable FedEx Service Guide or the FedEx Rate Sheets for details on how shipping charges are calculated.



Daniel Patrick dpatrick@cuddyfeder.com

July 20, 2022

#### FEDERAL EXPRESS

Redding Fire District 1 P.O. Box 45 Redding, CT 06875

Re:

Tower Sharing Request by New Cingular Wireless PCS, LLC

Premises: 186 Black Rock Turnpike, Redding, Connecticut

#### Dear Sir or Madam:

We are writing to you on behalf of our client New Cingular Wireless PCS, LLC ("AT&T") with respect to the above referenced request to the Connecticut Siting Council ("Council") for shared use approval to allow AT&T to install its wireless communications equipment on the existing communications tower and at the associated compound at 186 Black Rock Turnpike in the Town of Redding. AT&T proposes to install 6 antennas, 9 remote radiohead units on a sector frame mount attached to the existing tower at the centerline height of approximately 145' above ground level, with the height to the top of AT&T's antennas at approximately 148' above ground level.

Enclosed herein is a copy of the submission made to the Council requesting approval of the tower share which includes information regarding the technical, legal, environmental, and economic feasibility of AT&T's proposed installation.

Should you have any questions please feel free to contact me at the address above or the Council at 860.827.2935.

Very truly yours,

Daniel Patrick Enclosure



# Shipment Receipt

#### **Address Information**

Ship to:

Ship from:

Redding Fire District 1

**Daniel Patrick** Cuddy & Feder LLP

445 Hamilton Avenue

186 Black Road Turnpike

**Suite 1400** 

REDDING CENTER, CT

White Plains, NY

06875

10601

US

US

914 761 1300

9147611300

# **Shipment Information:**

Tracking no.: 777437946531

Ship date: 07/20/2022

Estimated shipping charges: 27.23 USD

# **Package Information**

Pricing option: FedEx Standard Rate Service type: Priority Overnight

Package type: FedEx Pak Number of packages: 1 Total weight: 1 LBS

Declared Value: 0.00 USD

Special Services: No signature required

Pickup/Drop-off: Use an already scheduled pickup at my location

# **Billing Information:**

Bill transportation to: CuddyFeder-963

Your reference: 1844-3768

P.O. no.: Invoice no.: Department no.:

Thank you for shipping online with FedEx ShipManager at fedex.com.

#### Please Note

FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including pay an additional challet, occurrently out actual tokes and line a unterly data. Eliminatoris joint in the control in the cont



Daniel Patrick dpatrick@cuddyfeder.com

July 20, 2022

#### FEDERAL EXPRESS

Message Center Management 40 Woodland Street Hartford, CT 06105

Re:

Tower Sharing Request by New Cingular Wireless PCS, LLC

Premises: 186 Black Rock Turnpike, Redding, Connecticut

Dear Sir or Madam:

We are writing to you on behalf of our client New Cingular Wireless PCS, LLC ("AT&T") with respect to the above referenced request to the Connecticut Siting Council ("Council") for shared use approval to allow AT&T to install its wireless communications equipment on the existing communications tower and at the associated compound at 186 Black Rock Turnpike in the Town of Redding. AT&T proposes to install 6 antennas, 9 remote radiohead units on a sector frame mount attached to the existing tower at the centerline height of approximately 145' above ground level, with the height to the top of AT&T's antennas at approximately 148' above ground level.

Enclosed herein is a copy of the submission made to the Council requesting approval of the tower share which includes information regarding the technical, legal, environmental, and economic feasibility of AT&T's proposed installation.

Should you have any questions please feel free to contact me at the address above or the Council at 860.827.2935.

Very truly yours,

Daniel Patrick Enclosure



# Shipment Receipt

#### **Address Information**

Ship to: Ship from:

Message Center Management Daniel Patrick

Cuddy & Feder LLP

40 Woodland Street

445 Hamilton Avenue

**Suite 1400** 

HARTFORD, CT

White Plains, NY

06105

10601

US

US

914 761 1300

9147611300

#### **Shipment Information:**

Tracking no.: 777437975906

Ship date: 07/20/2022

Estimated shipping charges: 22.97 USD

# **Package Information**

Pricing option: FedEx Standard Rate Service type: Priority Overnight

Package type: FedEx Pak Number of packages: 1 Total weight: 1 LBS

Declared Value: 0.00 USD

Special Services: No signature required

Pickup/Drop-off: Use an already scheduled pickup at my location

# **Billing Information:**

Bill transportation to: CuddyFeder-963

Your reference: 1844-3768

P.O. no.: Invoice no.: Department no.:

Thank you for shipping online with FedEx ShipManager at fedex.com.

#### **Please Note**

FedEx will not be responsible for any claim in excess of \$100 per package, whether the result of loss, damage, delay, non-delivery, misdelivery, or misinformation, unless you declare a higher value, pay an additional charge, document your actual loss and file a timely claim. Limitations found in the current FedEx Service Guide apply. Your right to recover from FedEx for any loss, including intrinsic value of the package, loss of sales, income interest, profit, attorney's fees, costs, and other forms of damage whether direct, incidental, consequential, or special is limited to the greater of \$100 or the authorized declared value. Recovery cannot exceed actual documented loss. Maximum for items of extraordinary value is \$1000, e.g., jewelry, precious metals, negotiable instruments and other items listed in our Service Guide. Written claims must be filed within strict time limits; Consult the applicable FedEx Service Guide for details.

The estimated shipping charge may be different than the actual charges for your shipment. Differences may occur based on actual weight, dimensions, and other factors. Consult the applicable FedEx Service Guide or the FedEx Rate Sheets for details on how shipping charges are calculated.