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October 18, 2018

BY EMAIL & OVERNIGHT DELIVERY

Hon. Robin Stein, Chairman
and Members of the Connecticut Siting Council
10 Franklin Square
New Britain, CT 06051

Re: Development and Management Plan ("D&M Plan")
Connecticut Siting Council Docket No. 478
Certificate of Environmental Compatibility and Public Need for the
Construction, Maintenance and Operation ("Certificate") of a
Telecommunications Facility in Glastonbury, Connecticut

Dear Chairman Stein and Members of the Council:

On behalf of the certificate holder Eco-Site and in furtherance of the captioned Certificate, please accept for review this Development Management Plan ("D&M Plan") filing for the captioned Facility as approved in Docket No. 478.

Tower, Compound & Other Equipment

Enclosed are an original and fifteen (15) sets of 11" x 17" drawings prepared by Infinigy dated August 30, 2018 and last revised October 12, 2018 ("D&M Plan Drawings") being filed in accordance with the Siting Council ("Council") Decision and Order ("Decision and Order"). Two full-sized sets of the D&M Plan drawings are also included for the Council's files.

By letter dated June 7, 2018 and as per the Council's Decision and Order Eco-Site consulted with the Town of Glastonbury regarding the needs associated with a colocation of town emergency antennas on the tower and the Town's interest in a monopine design. On June 26, 2018 Attorney Christopher B. Fisher appeared before the Town of Glastonbury Town Council to review the prospective monopole and monopine designs including the equipment identified by the Town of Glastonbury as its desired installation (3 omnidirectional antennas and a dish antenna at the top of the tower).

As documented in the letter to the Council by Town of Glastonbury Manager Richard Johnson dated June 29, 2018, the Town Board voted to confirm its desire for a monopine design. As reflected in the included D&M Plan Drawings, the monopine design at the approved height will not fully conceal the Town's antennas which was a concern the Council specifically identified in its Opinion in this Docket. As such the Certificate Holder respectfully requests the Council provide affirmative direction as to which design, monopole or monopine, should be implemented.



October 18, 2018

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We include herein a geotechnical report of the facility site but the tower and foundation structural design will be finalized once the Council provides direction as to the final design in Accordance with Order 1 of the Decision and Order. Incorporated in the D&M Drawings are specifications regarding the antennas, remote radiohead units (RRUs) and backup power equipment. The D&M Plan Drawings also include site preparation and erosion and sedimentation control measures consistent with the 2002 Connecticut Guidelines for Soil Erosion and Sediment Control as amended.

Please note that hours of construction are anticipated to be between 8:00 a.m. and 6:00 p.m. or otherwise in accordance with the Town of Glastonbury regulations and direction.

Required Notifications

In accordance with RCSA Section 16-50j-61(d) a copy of this filing is being provided to property owner as well as to the Town of Glastonbury as intervenor.

In accordance with the provisions of RCSA Section 16-50j-77, the certificate holder hereby notifies the Council of its intention to begin site work after Council approval of the final D&M Plan. Construction of the tower and other site improvements will commence after issuance of a local building permit. T-Mobile intends to install its antennas as soon as the tower is erected. Eco-Site does not have a timeline regarding the Town's installation. The supervisor for all construction related matters on this project is Charles Moore who can be reached by telephone at (518) 368-2545.

We respectfully request that this matter be included on the Council's earliest available agenda for review and consideration of the design alternatives in the interest of finalizing the design

Thank you for your consideration of the enclosed.

Very truly yours,

A handwritten signature in blue ink, appearing to read 'D. Laub', with a long horizontal flourish extending to the right.

Daniel M. Laub

Attachments

cc: Steve Russo, Eco-Site
Mark Richards, T-Mobile
Paul Cavanna, Property Owner
Project Team



ENGINEERING INNOVATION

FDH Velocitel, 6521 Meridien Drive, Raleigh, NC 27616, Ph: 919.755.1012, Fax: 919.755.1031

Geotechnical Report

New Monopole Tower

Report Prepared for
Infinigy Engineering

Site Name: Glastonbury
Site ID: CT-0007

63-80 Woodland Street – Glastonbury, CT 06073
Lat: 41.660806
Lon: -72.574111

FDH Velocitel Project Number 18PGJC1600

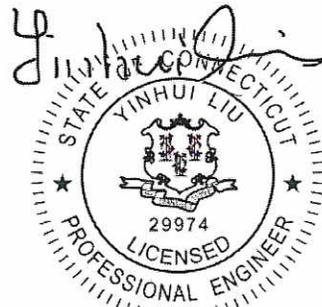
Prepared by:

Steven Allen
Project Manager

Yinhui "Cindy" Liu, PhD, P.E.
Senior Geotechnical Engineer

FDH Velocitel
6521 Meridien Drive
Raleigh, NC 27616
(919) 755-1012
geotech@FDHVelocitel.com

04/10/2018



04-10-18

INTRODUCTION

FDH Velocitel is pleased to present this geotechnical report for a new 150-ft monopole telecommunication tower. The design foundation loads at the base of the tower were not provided to us. The purpose of this study was to determine the general subsurface conditions in the vicinity of the proposed tower site and provide foundation recommendations. The results of the boring and laboratory testing are included, in addition to recommendations for designing and constructing the new tower's foundation.

SITE CONDITIONS

The subject site is located at 63-80 Woodland Street in Glastonbury, Connecticut, which is in Hartford County. The proposed tower and equipment compound will be located on top of a ridge. The area around the proposed tower compound has rolling grades and is densely wooded. A Satellite Photograph and a Topographic Map are presented on Figures 1 and 2 in this report.

FIELD EXPLORATION

Subsurface conditions were evaluated by obtaining one (1) test boring at the proposed tower location as shown on Figure 3. The test boring was initiated and completed on 04/03/2018. The drilling was performed with a track-mounted drill rig. A photograph of the drill rig and drill site is presented in Figure 4. The soil test boring was advanced using hollow stem auger drilling procedures to 15.2 ft below grade. The subsurface soils were generally sampled at 2.0 ft intervals for the first 12.0 ft and at 5.0 ft intervals thereafter. The boring was sampled by driving a 1 3/8 in. I.D. split spoon sampler in accordance with the standard penetration test procedures designated in ASTM D-1586. The sampler was first seated 6 in. to penetrate any loose cuttings and then driven an additional 12 in. with an automatic 140-pound hammer free falling 30 in. The number of hammer blows required to drive the sampler the final 12 in. is designated the standard penetration test N-value. Below 15.2 ft, rock coring procedures outlined in ASTM D-2113 were utilized to core into the underlying rock to a depth of 20.2 ft below grade. A boring log is attached in Appendix I.

LABORATORY CLASSIFICATION AND TESTING

The soil and rock samples were transported to our soil laboratory and examined by a geotechnical engineer. The soil samples were classified according to ASTM D-2487. Photographs of the soil and rock samples before classification are presented in Figure 5. Moisture content tests in accordance with ASTM D-2216 were conducted on all soil samples. Additionally, particle size analysis tests (ASTM D-422) and percent finer than No. 200 sieve tests (ASTM D-1140) were conducted on selected soil samples. Rock core samples were prepared and tested for unconfined compressive strength (ASTM D-2938). The laboratory test results are presented on the boring logs and in Appendix II. The soil and rock samples will be retained in our laboratory for a period of six months (180 days), after which, they will be discarded unless other instructions are received as to their disposition.

SITE GEOLOGY

The site is located within the New England physiographic province of the contiguous United States. The underlying bedrock is generally layered and complexly deformed. Bedrock types include sedimentary, igneous, and metamorphic rock consisting of conglomerate, sandstone, granite, and pyroclastic rocks. The present topography of this region is the result of preglacial, glacial, and postglacial erosion and deposition. The overburden soils include glacial till, glacial outwash sand and gravel, glaciolacustrine varved clay, glaciofluvial sand, eolian sand and silt, and marine clay. Glacial till is composed of boulders, gravel, sand, silt, and clay mixed in various proportions.

According to the geologic map of Connecticut, the site is underlain by Glastonbury Gneiss of Ordovician age. This formation consists of granitic gneiss and amphibolite.

FROST DEPTH

Based on the TIA Standard (TIA-222-G), dated August 2005, the recommended design frost penetration depth to be used for Hartford County, Connecticut is 40 inches (3.3 ft).

SUBSURFACE CONDITIONS

The boring encountered the general strata given in the following table.

Strata #	Approx. Depth (ft)	General Description
I	0.0 – 10.0	Loose to very dense silty fine to medium sand (SM)
II	10.0 – 15.2	Very dense fine sandy silt (ML)
III	15.2 – 20.5	Soft weathered granitic gneiss, moderately fractured

Additional details for each stratum are given on the attached boring log.

Groundwater was encountered in the soil boring B-1 at a depth of 10.0 ft at the time of drilling. Groundwater levels will fluctuate with seasonal and climatic changes and may be different at other times.

SOIL RESISTIVITY

Field soil resistivity tests were conducted according to procedures designated in ASTM G-57 and test results are presented in the following table. Soil resistivity values will vary with temperature and moisture content changes and may be different at other times.

Probe Spacing (ft)	Resistivity (Ohm-cm)	
	Line 1	Line 2
2	355,041	433,939
4	504,794	441,216
8	488,708	340,104
16	490,240	391,886

RECOMMENDATIONS

Foundations

The following recommendations are made based on our review of the test boring data and laboratory results and our past experience with similar projects and subsurface conditions. Ultimate soil and rock strength parameters are presented on Table 1 (attached). Based on the subsurface conditions and typical design foundation loads for similar monopoles, we recommend that either a caisson (drilled shaft) or pad/pier be used to support the new tower.

Caisson (Drilled Shaft)

Should a caisson (drilled shaft) foundation be used, the caisson (drilled shaft) will achieve compressive (downward) and tensile (uplift) resistance through skin friction along the side of the shaft. In addition to skin friction, bearing resistance at the caisson's tip will contribute to compressive capacity. We recommend the values given in the Table 2 (attached) be used for this project. Please note the tip bearing capacity and skin frictions are ultimate values. Appropriate factors of safety or resistance factors should be used. Lateral loads and overturning moment can be resisted by the lateral stiffness of the soil and rock. Parameters for analysis of the laterally loaded caisson are also given in Table 2.

Based on the subsurface soil conditions, excavation for the caisson (drilled shaft) should be possible using a large, truck-mounted, hydraulic-advanced drill rig. Rock coring equipment should be prepared if the bearing depth of the caisson is deeper than 15.0 ft below existing grade. All debris, loose or disturbed soil should be removed from the excavation prior to placing reinforced steel and/or concrete. Reinforcing steel and/or concrete should be placed immediately upon completion of the excavation.

The excavation may be susceptible to caving. Drilling fluid or temporary casing could be used to assist in keeping the drilled hole open. If temporary casing is used, we recommend it be removed from the excavation as concrete is being placed. Continuous vibration or other approved methods should be used during casing withdrawal to reduce the potential for void-space formation within the concrete. If water is present during concrete placement and/or drilling fluids are used to maintain hole stability, concrete should be pumped or otherwise discharged to the bottom of the hole via a hose or tremie pipe. The end of the hose or tremie pipe must remain below the top surface of any water, drilling fluid and the in-place concrete at all times. Additionally, concrete should be consolidated using vibration methods over the entire length and width of the caisson and the consolidation should be performed only after these fluids are removed and to the extent possible.

Pad & Pier

Should a pad & pier foundation be used, we recommend the pad & pier be reinforced with steel to resist and transfer lateral and axial loads, as well as prevent cracking and shrinkage due to temperature and moisture variations. Based on the subgrade conditions and frost penetration depth of the project site, we recommend the bottom of the pad foundation bears at a depth deeper than 4.0 ft. The tower's foundation capacity can be determined using the soil's bearing capacity, passive pressure resistance, and a sliding friction factor.

- **Net Ultimate Bearing Capacity and Ultimate Sliding Friction Factor:** Shown in Table 3 (attached). This table contains ultimate values and an appropriate factor of safety or resistance factor should be used.
- **Ultimate Passive Pressure vs. Depth:** Shown in Table 4 (attached). This table contains ultimate values and an appropriate factor of safety or resistance factor should be used. These values have been reduced for frost penetration to a depth of 3.3 ft.

The pad should bear on natural soils or on controlled structural fill placed on satisfactory, firm, and stable natural soils. The site should be stripped to suitable depths to remove any existing grass, topsoil, rootmat, or other deleterious material. Structural fill used to elevate the grade and/or backfill any excavations should consist of clean soils without deleterious inclusions and with maximum 3.0-inch particle size. On-site soils identified as silty sand are satisfactory for use as structural fill and backfill. Some of these soils may require aeration and drying prior to use as structural fill or backfill. The structural fill material should be placed in maximum of 8.0 inches loose lifts and compacted to a minimum of 98 percent of the maximum dry density as per ASTM D-698. The moisture content should be within -2 to +2 % of optimum moisture.

The pad & pier foundation system or mat foundation should be protected from freezing if built during the winter or subject to freezing temperatures during construction. Groundwater was encountered at 10.0 ft below grade at the time of drilling. Groundwater level should be closely monitored. If necessary, proper dewatering method should be used to lower down the water level to a depth at least 2.0 ft below the bearing depth of the pad foundation. Additionally, positive surface drainage should be provided to prevent rainwater water collection in foundation excavations or on subgrades of the construction area either during or after construction. Undercut or excavated areas should be sloped toward a corner to facilitate removal of any collected rainwater or surface runoff with a sump pump.

Construction Inspection

We recommend that the foundation excavation and fill placement process be monitored by a geotechnical engineer or representative thereof. Subsurface condition variances may occur at project site. Therefore, the excavations should be inspected to confirm that the bearing materials are similar to those encountered by the boring and that the subgrade has been properly prepared. The geotechnical engineer should be immediately notified should any subsurface conditions be discovered that will alter the conclusions and recommendations contained in this report. Further investigation and supplemental recommendations may be required if such a condition is encountered.

Samples of the proposed structural fill material should be obtained prior to fill placement operations for laboratory moisture/density testing (Proctor tests). The tests will then provide a basis for evaluating the in-place density requirements during compaction operations. A qualified soil technician should perform sufficient in-place density tests during the filling operations to verify that proper levels of compaction are being attained.

Prior to placement of concrete, the foundation excavation should be inspected to verify that the excavation is to the proper depth and reinforcing steel is placed as recommended. Concrete cylinders should be made for compressive strength testing at curing times of 7 days and 28 days, at a minimum.

LIMITATIONS

All opinions and conclusions are considered accurate to a reasonable degree of engineering certainty based upon the evidence available at the time of this report. All opinions and conclusions are subject to revision based upon receipt of new or additional/updated information. All services are provided exercising a level of care and diligence equivalent to the standard and care of our profession. No other warranty or guarantee, expressed or implied, is offered. Our services are confidential in nature and we will not release this report to any other party without the client's consent. The use of this engineering work is limited to the express purpose for which it was commissioned and it may not be reused, copied, or distributed for any other purpose without the written consent of FDH Velocitel.



Geotechnical Report
Site Name: Glastonbury
Site ID: CT-0007
04/10/2018

FDH Velocitel, 6521 Meridien Drive, Raleigh, NC 27616, Ph: 919.755.1012, Fax: 919.755.1031

TABLES

Table 1 – Ultimate Strength Parameters

Boring #	Depth (ft)	Unified Soil or Rock Classification	*Total Unit Weight (pcf)	Friction Angle (degrees)	Cohesion (psf)
B-1	0.0 – 2.0	SM	105	29	0
	2.0 – 5.0	SM	120	34	0
	5.0 – 15.0	SM/ML	130	38	0
	15.0 – 20.0	Bedrock	160	0	15,000

**Recommended groundwater depth for analysis is 10.0 ft. Utilize buoyant unit weight below this depth.*

Table 2 - Caisson (Drilled Shaft) Parameters

Depth (ft)	Net Ultimate Tip Bearing Capacity (ksf)	Ultimate Skin Friction (ksf)	Lateral Modulus (pci)	ϵ_{50} (in/in)
0.0 – 5.0	--	--	25	--
5.0 – 10.0	--	1.5	225	--
10.0 – 15.0	30.0	2.1	225	--
15.0 –	60.0	3.5	3000	0.0005

**We recommend the skin friction be ignored for the top 4.0 ft of the caisson.*

Table 3 - Gross Ultimate Bearing Capacity

Pad Bearing Depth (ft)	Gross Ultimate Bearing Capacity (psf)	Sliding Friction Factor
4.0 – 5.0	20,000	0.35
5.0 – 8.0	30,000	0.40

Table 4 - Ultimate Passive Pressure

Boring #	Depth (ft)	*Ultimate Passive Pressure (psf)
B-1	0.0 – 2.0	0 – 300
	2.0 – 3.3	370 – 650
	3.3 – 5.0	1295 – 2020
	5.0 – 8.0	2400 – 4040

**Ultimate passive pressure can be interpolated for foundation depths with the depth ranges given.*



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Site ID: CT-0007
04/10/2018

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FIGURES

FIGURE 1: Satellite Map



FIGURE 2: Topographic Map

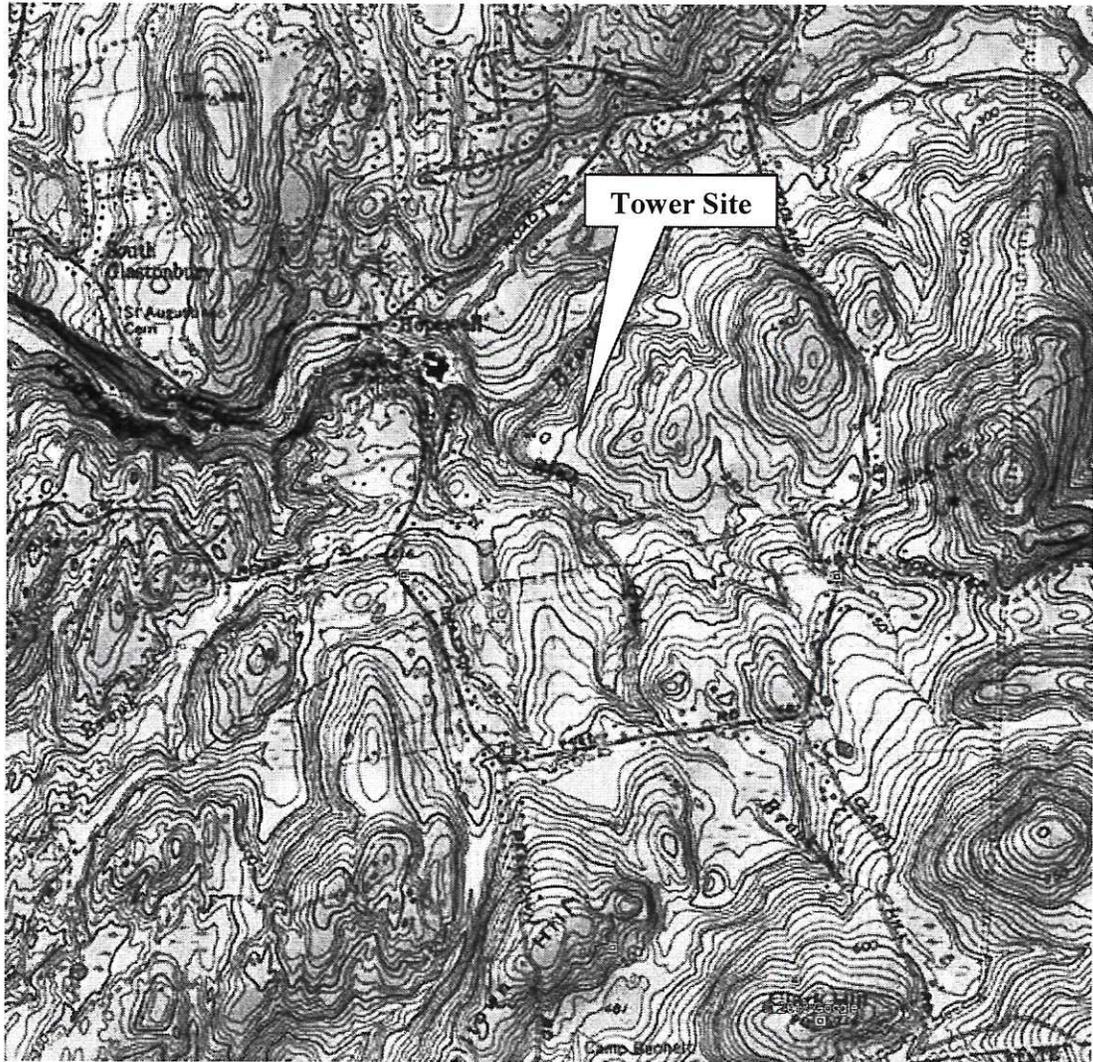


FIGURE 3: Boring Location Plan



LEGEND

-  — Approximate Boring Location
- Not to Scale

FIGURE 4: Photograph of Drill Rig and Tower Site



FIGURE 5: Photographs of Soil Samples





ENGINEERING INNOVATION

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Geotechnical Report
Site Name: Glastonbury
Site ID: CT-0007
04/10/2018

APPENDIX I – BORING LOG(S)



Geotechnical Report
Site Name: Glastonbury
Site ID: CT-0007
04/10/2018

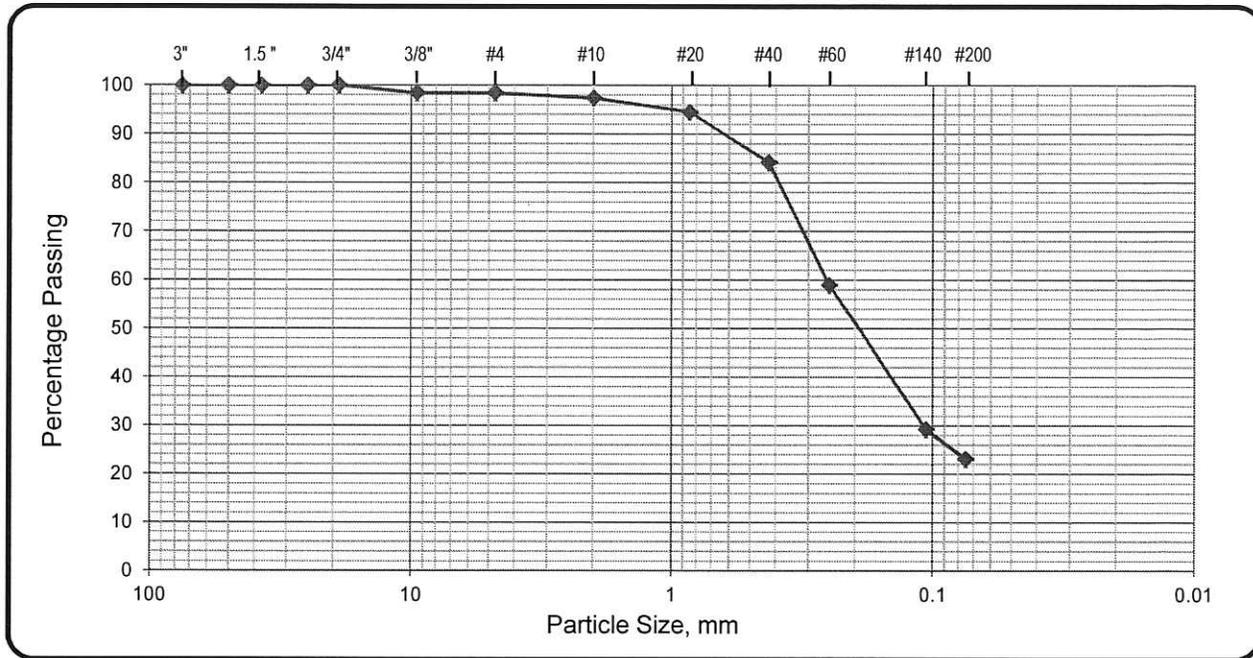
FDH Velocitel, 6521 Meridien Drive, Raleigh, NC 27616, Ph: 919.755.1012, Fax: 919.755.1031

APPENDIX II – LABORATORY TEST RESULTS



REPORT OF PARTICLE-SIZE ANALYSIS

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Fractional Component Percentages

<u>Plus 3"</u>	<u>Coarse Gravel</u>	<u>Fine Gravel</u>	<u>Coarse Sand</u>	<u>Medium Sand</u>	<u>Fine Sand</u>	<u>Fines</u>
0.0	0.0	1.6	1.0	13.3	61.0	23.1

Total Percentages:	Gravel: 1.6	Sand: 75.3	Fines: 23.1
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Sieve Designation	Amounts Retained (grams)	Amounts Retained (percent)	Percent Passing
3"	0.00	0.0	100.0
2"	0.00	0.0	100.0
1.5"	0.00	0.0	100.0
1"	0.00	0.0	100.0
0.75"	0.00	0.0	100.0
0.375"	2.03	1.6	98.4
#4	2.03	1.6	98.4
#10	3.28	2.6	97.4
#20	6.90	5.5	94.5
#40	19.94	15.9	84.1
#60	51.61	41.0	59.0
#140	88.97	70.7	29.3
#200	96.72	76.9	23.1

Project Name: Glastonbury
 Project Number: 18PGJC1600
 Site Identification: CT-0007
 Sample Identification: B1 SS3 (5ft - 7ft)
 Description: Silty sand

As-received water content, %: 8.6
 USCS Classification: SM
 Liquid Limit: ND
 Plastic Limit: ND
 Plasticity Index: ND

D₁₀ = ND
 D₃₀ = 0.11
 D₆₀ = 0.26

Total dry mass, g: 125.78

C_u: ND C_c: ND

Remarks: Entire sample was tested. ND=Not Determined.

Eco-Site

240 LEIGH FARM ROAD, SUITE 415
DURHAM, NC 27707

DEVELOPMENT & MANAGEMENT PLAN

DOCKET No.: 478

ECO-SITE: GLASTONBURY CT-0007A

T-MOBILE: CTHA026A

SITE ADDRESS:

63-80 WOODLAND STREET
GLASTONBURY, CT 06073
TOLLAND COUNTY

LATITUDE: 41° 39' 38.9" N
LONGITUDE: 72° 34' 26.8" W
ELEVATION: 319' AMSL

TAX/PIN #: G11/7800/W0002 PID: 1451
ZONING: RURAL RESIDENCE - AGRICULTURAL USE



TO OBTAIN LOCATION OF PARTICIPANTS UNDERGROUND FACILITIES BEFORE YOU DIG IN MASSACHUSETTS, CALL DIG SAFE SYSTEM, INC.
TOLL FREE: 1-888-344-7233 OR www.digsafe.com
MASSACHUSETTS STATUTE REQUIRES MIN OF 2 WORKING DAYS NOTICE BEFORE YOU EXCAVATE

Know what's below.
Call before you dig.



3	REVISED PER COMMENT	SKB	10/12/18
2	REVISED PER COMMENT	SKB	10/05/18
1	REVISED PER COMMENT	SKB	09/24/18
0	ISSUED FOR PERMIT	SKB	08/30/18

Drawn: SKB Date: 08/30/18
Designed: AD Date: 08/30/18
Checked: AD Date: 08/30/18

Project Number: 502-000

Project Title: GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For:



Drawing Title: TITLE SHEET

Drawing Scale: CD

Date: 09/24/18

UNAUTHORIZED ALTERATION OR ADDITION TO THIS DOCUMENT IS A VIOLATION OF APPLICABLE STATE AND/OR LOCAL LAWS

Drawing Number: T1



VICINITY MAP

1. HEAD NORTH ON I-91 N TOWARD CT-220E/ELM STREET.
2. TAKE EXIT 48 ON TO ELM STREET IN ENFIELD.
3. TURN RIGHT TO CONTINUE ON ELM STREET.
4. CONTINUE STRAIGHT ONTO MOODY ROAD.
5. MOODY ROAD BECOMES GEORGE WOOD ROAD.
6. TURN RIGHT ONTO TO BRACE ROAD
7. TURN RIGHT ONTO HALL HILL ROAD
8. SITE WILL BE ON YOUR LEFT

DRIVING DIRECTIONS

CONSTRUCTION OF TELECOMMUNICATION AND PUBLIC UTILITY FACILITY, CONSISTING OF A MONOPOLE TOWER, SPACE FOR CARRIER EQUIPMENT AND A UTILITY BACKBOARD WITHIN A FENCED COMPOUND. NO WATER OR SEWER IS REQUIRED.

PROJECT SUMMARY

DEVELOPER:
ECO-SITE
240 LEIGH FARM ROAD, SUITE 415
DURHAM, NC 27707
(919) 636-6810
ATTN:

POWER COMPANY:
EVERSOURCE
107 SELDON STREET
BERLIN MA, 06037
(800) 286-2000
ATTN: CUSTOMER SERVICE

TELEPHONE COMPANY:
TBD
(800) XXX-XXXX
ATTN: CUSTOMER SERVICE

PROPERTY OWNER:
DEBRA ROMANO
248 HALL HILL ROAD
SOMERS, CT 06071

PROJECT SUMMARY

DRWG. #	TITLE	REV.#	DATE
T1	TITLE SHEET	3	10/12/18
C1	GENERAL NOTES & LEGEND	3	10/12/18
C2	OVERALL SITE PLAN	3	10/12/18
C3	ENLARGED SITE LAYOUT	3	10/12/18
C4	TOWER ELEVATION - OPTION 1	3	10/12/18
C4A	TOWER ELEVATION - OPTION 2	3	10/12/18
C4B	ORIENTATION & ANTENNA MOUNTING DETAILS	3	10/12/18
C5	CIVIL DETAILS	3	10/12/18
C6	CIVIL DETAILS	3	10/12/18
C7	UTILITY RACK DETAIL	3	10/12/18
C8	T-MOBILE EQUIPMENT DETAILS	3	10/12/18
C9	T-MOBILE EQUIPMENT DETAILS	3	10/12/18
C10	ICE BRIDGE DETAILS	3	10/12/18
C11	GENERATOR BLOCK DIAGRAM	3	10/12/18
C12	GENERATOR DETAILS	3	10/12/18
C13	SITE SIGNAGE DETAILS	3	10/12/18
C14	RETAINING WALL DETAILS	3	10/12/18
C15	RETAINING WALL DETAILS	3	10/12/18
C16	RETAINING WALL DETAILS	3	10/12/18
EC1	GRADING & EROSION SEDIMENT CONTROL NOTES & DETAILS	3	10/12/18
EC2	GRADING PLAN	3	10/12/18
EC3	GRADING PLAN CONT'D	3	10/12/18
EC4	GRADING PLAN CONT'D	3	10/12/18
E1	OVERALL UTILITY PLAN	3	10/12/18
E2	OVERALL ROUTING DETAILS	3	10/12/18

DRAWING INDEX

TOWN OF SOMERS:
600 MAIN STREET, PO BOX 308
SOMERS, CT 06071
(860) 763-8201

PERMIT INFORMATION

GENERAL NOTES

- ALL DIMENSIONS TO, OF, AND ON EXISTING BUILDINGS, DRAINAGE STRUCTURES, AND SITE IMPROVEMENTS SHALL BE VERIFIED IN FIELD BY CONTRACTOR WITH ALL DISCREPANCIES REPORTED TO THE ENGINEER.
- DO NOT CHANGE SIZE NOR SPACING OF STRUCTURAL ELEMENTS.
- DETAILS SHOWN ARE TYPICAL; SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS UNLESS OTHERWISE NOTED.
- THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.
- BRACE STRUCTURES UNTIL ALL STRUCTURAL ELEMENTS NEEDED FOR STABILITY ARE INSTALLED. THESE ELEMENTS ARE AS FOLLOWS: LATERAL BRACING, ANCHOR BOLTS, ETC.
- DETERMINE EXACT LOCATION OF EXISTING UTILITIES, GROUNDS DRAINS, DRAIN PIPES, VENTS, ETC. BEFORE COMMENCING WORK.
- INCORRECTLY FABRICATED, DAMAGED, OR OTHERWISE MISFITTING OR NONCONFORMING MATERIALS OR CONDITIONS SHALL BE REPORTED TO THE OWNER PRIOR TO REMEDIAL OR CORRECTIVE ACTION. ANY SUCH ACTION SHALL REQUIRE APPROVAL.
- EACH CONTRACTOR SHALL COOPERATE WITH THE OWNER'S REPRESENTATIVE, AND COORDINATE HIS WORK WITH THE WORK OF OTHERS.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO EXAMINE ALL PLAN SHEETS AND SPECIFICATIONS AND COORDINATE HIS WORK WITH THE WORK OF ALL OTHER CONTRACTORS TO ENSURE THAT WORK PROGRESSION IS NOT INTERRUPTED.
- THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING A NEAT AND ORDERLY SITE, YARD AND GROUNDS. REMOVE AND DISPOSE OFF SITE ALL RUBBISH, WASTE MATERIALS, LITTER, AND ALL FOREIGN SUBSTANCES. REMOVE PETRO-CHEMICAL SPILLS, STAINS AND OTHER FOREIGN DEPOSITS. RAKE GROUNDS TO A SMOOTH EVEN-TEXTURED SURFACE.
- THE PLANS SHOW SOME KNOWN SUBSURFACE STRUCTURES, ABOVE-GROUND STRUCTURES AND/OR UTILITIES BELIEVED TO EXIST IN THE WORKING AREA, EXACT LOCATION OF WHICH MAY VARY FROM THE LOCATIONS INDICATED. IN PARTICULAR, THE CONTRACTOR IS WARNED THAT THE EXACT OR EVEN APPROXIMATE LOCATION OF SUCH PIPELINES, SUBSURFACE STRUCTURES AND/OR UTILITIES IN THE AREA MAY BE SHOWN OR MAY NOT BE SHOWN; AND IT SHALL BE HIS RESPONSIBILITY TO PROCEED WITH GREAT CARE IN EXECUTING ANY WORK. 48 HOURS BEFORE YOU DIG, DRILL OR BLAST, CALL 1-800-922-4455.
- THE OWNER OR OWNER'S REPRESENTATIVE SHALL BE NOTIFIED IN WRITING OF ANY CONDITIONS THAT VARY FROM THOSE SHOWN ON THE PLANS. THE CONTRACTOR'S WORK SHALL NOT VARY FROM THE PLANS WITHOUT THE EXPRESSED APPROVAL OF THE OWNER OR OWNER'S REPRESENTATIVE.
- THE CONTRACTOR IS INSTRUCTED TO COOPERATE WITH ANY AND ALL OTHER CONTRACTORS PERFORMING WORK ON THIS JOB SITE DURING THE PERFORMANCE OF THIS CONTRACT.
- THE CONTRACTOR SHALL RESTORE ALL PUBLIC OR PRIVATE PROPERTY DAMAGED OR REMOVED TO AT LEAST AS GOOD OF CONDITION AS BEFORE DISTURBED AS DETERMINED BY THE OWNER OR OWNER'S REPRESENTATIVE.
- THE CONTRACTOR SHALL COMPLY WITH ALL REQUIRED PERMITS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING, AND INCURRING THE COST OF ALL REQUIRED PERMITS, INCLUDING, BUT NOT LIMITED TO, THE BUILDING PERMIT, INSPECTIONS, CERTIFICATES, ETC.
- THE CONTRACTOR SHALL PROTECT EXISTING PROPERTY LINE MONUMENTATION. ANY MONUMENTATION DISTURBED OR DESTROYED, AS JUDGED BY THE OWNER OR OWNER'S REPRESENTATIVE SHALL BE REPLACED AT THE CONTRACTOR'S EXPENSE UNDER THE SUPERVISION OF A LICENSED LAND SURVEYOR.
- ALL TRENCH EXCAVATION AND ANY REQUIRED SHEETING AND SHORING SHALL BE DONE IN ACCORDANCE OSHA REGULATIONS FOR CONSTRUCTION.
- CONTRACTOR SHALL BE RESPONSIBLE FOR DEWATERING AND THE MAINTENANCE OF SURFACE DRAINAGE DURING THE COURSE OF WORK.
- ALL UTILITY WORK INVOLVING CONNECTIONS TO EXISTING SYSTEMS SHALL BE COORDINATED WITH THE OWNER OR OWNER'S REPRESENTATIVE AND THE UTILITY OWNER. NOTIFY THE OWNER OR OWNER'S REPRESENTATIVE AND THE UTILITY OWNER BEFORE EACH AND EVERY CONNECTION TO EXISTING SYSTEMS IS MADE.
- MAINTAIN FLOW FOR ALL EXISTING UTILITIES.
- ALL SITE FILL SHALL MEET SELECTED FILL STANDARDS AS DEFINED BY THE OWNER OR OWNER'S REPRESENTATIVE ON THE DRAWINGS.
- CONTRACTOR SHALL GRADE ALL AREAS ON THE SITE TO PROVIDE POSITIVE DRAINAGE AWAY FROM THE EQUIPMENT PAD AND THE TOWER.
- ALL IMPROVEMENTS TO CONFORM WITH LOCAL JURISDICTION CONSTRUCTION STANDARDS AND SPECIFICATIONS, LATEST EDITION.

STRUCTURAL STEEL NOTES

- STRUCTURAL STEEL SHALL CONFORM TO THE LATEST EDITION OF THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- ALL INTERIOR STRUCTURAL STEEL USED SHALL BE, WHEN DELIVERED, FINISHED WITH ONE COAT FABRICATOR'S NON-LEAD, RED OXIDE PRIMER. PRIMING SHALL BE PERFORMED AFTER SHOP FABRICATION TO THE GREATEST EXTENT POSSIBLE. ALL DINGS, SCRAPES, MARS, AND WELDS IN THE PRIMED AREAS SHALL BE REPAIRED BY FIELD TOUCH-UP PRIOR TO COMPLETION OF THE WORK.
- ALL EXTERIOR STEEL WORK SHALL BE GALVANIZED IN ACCORDANCE WITH SPECIFICATION ASTM A36 UNLESS OTHERWISE NOTED. GALVANIZING SHALL BE PERFORMED AFTER SHOP FABRICATION TO THE GREATEST EXTENT POSSIBLE. ALL DINGS, SCRAPES, MARS, AND WELDS IN THE GALVANIZED AREAS SHALL BE REPAIRED BY FIELD TOUCH-UP PRIOR TO COMPLETION OF THE WORK.
- DO NOT PLACE HOLES THROUGH STRUCTURAL STEEL MEMBERS EXCEPT AS SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.
- CONNECTIONS:
 - ALL WELDING SHALL BE DONE USING E70XX ELECTRODES AND WELDING SHALL CONFORM TO AISC AND AWS D1.1. WHERE FILLET WELD SIZES ARE NOT SHOWN, PROVIDE THE MINIMUM SIZE PER TABLE J2.4 IN THE AISC "MANUAL OF STEEL CONSTRUCTION", 9TH EDITION. AT THE COMPLETION OF WELDING, ALL DAMAGE TO GALVANIZED COATING SHALL BE REPAIRED.
 - BOLTED CONNECTIONS SHALL USE BEARING TYPE GALVANIZED ASTM A325 BOLTS (3/4" DIA) AND SHALL HAVE MINIMUM OF TWO BOLTS UNLESS NOTED OTHERWISE.
 - NON-STRUCTURAL CONNECTIONS FOR STEEL GRATING MAY USE 5/8" DIA. GALVANIZED ASTM A 307 BOLTS UNLESS NOTED OTHERWISE.
 - CONNECTION DESIGN BY FABRICATOR WILL BE SUBJECT TO REVIEW AND APPROVAL BY ENGINEER.

DESIGN DATA

- WIND LOADS: PER EIA/TIA G-222
ICE LOADS: 1/2" RADIAL ON ALL COMPONENTS & CABLE
SNOW LOAD: PER CT STATE BLDG. CODE.
SEISMIC LOADS: PER CT STATE BLDG CODE.

CONCRETE NOTES

- DESIGN AND CONSTRUCTION OF ALL CONCRETE ELEMENTS SHALL CONFORM TO THE LATEST EDITIONS OF THE FOLLOWING APPLICABLE CODES: ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS"; ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE";
- MIX DESIGN SHALL BE APPROVED BY OWNER'S REPRESENTATIVE PRIOR TO PLACING CONCRETE.
- CONCRETE SHALL BE NORMAL WEIGHT, 6% AIR ENTRAINED (±1.5%) WITH A MAXIMUM 4" SLUMP, AND HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3000 PSI UNLESS OTHERWISE NOTED.
- MAXIMUM AGGREGATE SIZE SHALL BE 1".
- THE FOLLOWING MATERIALS SHALL BE USED:

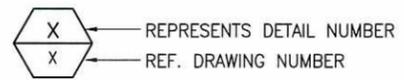
PORTLAND CEMENT:	ASTM C 150, TYPE I
REINFORCEMENT:	ASTM A 185
NORMAL WEIGHT AGGREGATE:	ASTM C 33
WATER:	DRINKABLE
ADMIXTURES:	NON-CHLORIDE CONTAINING
- REINFORCING DETAILS SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315.
- REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60, DEFORMED UNLESS NOTED OTHERWISE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WELDED STEEL WIRE FABRIC UNLESS NOTED OTHERWISE. SPLICES SHALL BE CLASS "B" AND ALL HOOKS SHALL BE STANDARD, UNO.
- THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:

CONCRETE CAST AGAINST EARTH.....	3 IN.
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 AND LARGER	2 IN.
#5 AND SMALLER & WWF	1 1/2 IN.
CONCRETE NOT EXPOSED TO EARTH OR WEATHER OR NOT CAST AGAINST THE GROUND:	
SLAB AND WALL	3/4 IN.
BEAMS AND COLUMNS	1 1/2 IN.

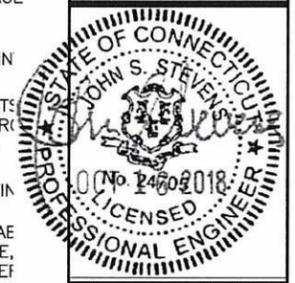
- A CHAMFER 3/4" SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNO, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.
- INSTALLATION OF CONCRETE EXPANSION/WEDGE ANCHOR, SHALL BE PER MANUFACTURER'S WRITTEN RECOMMENDED PROCEDURE. THE ANCHOR BOLT, DOWEL OR ROD SHALL CONFORM TO MANUFACTURER'S RECOMMENDATION FOR EMBEDMENT DEPTH OR AS SHOWN ON THE DRAWINGS. NO REBAR SHALL BE CUT WITHOUT PRIOR ENGINEERING APPROVAL WHEN DRILLING HOLES IN CONCRETE.
- CURING COMPOUNDS SHALL CONFORM TO ASTM C-309.
- ADMIXTURES SHALL CONFORM TO THE APPROPRIATE ASTM STANDARD AS REFERENCED IN ACI-301.
- DO NOT WELD OR TACKWELD REINFORCING STEEL.
- ALL DOWELS, ANCHOR BOLTS, EMBEDDED STEEL, ELECTRICAL CONDUITS, PIPE SLEEVES, GROUNDS AND ALL OTHER EMBEDDED ITEMS AND FORMED DETAILS SHALL BE IN PLACE BEFORE START OF CONCRETE PLACEMENT.
- LOCATE ADDITIONAL CONSTRUCTION JOINTS REQUIRED TO FACILITATE CONSTRUCTION AS ACCEPTABLE TO ENGINEER. PLACE REINFORCEMENT CONTINUOUSLY THROUGH JOINT REINFORCEMENT SHALL BE COLD BENT WHENEVER BENDING IS REQUIRED.
- PLACE CONCRETE IN A UNIFORM MANNER TO PREVENT THE FORMATION OF COLD JOINTS AND OTHER PLANES OF WEAKNESS. VIBRATE THE CONCRETE TO FULLY EMBED REINFORCEMENT. DO NOT USE VIBRATORS TO TRANSPORT CONCRETE THROUGH CHUTES OR FORMWORK.
- DO NOT PLACE CONCRETE IN WATER, ICE, OR ON FROZEN GROUND.
- DO NOT ALLOW CONCRETE SUBBASE TO FREEZE DURING CONCRETE CURING AND SETTING PERIOD, OR FOR A MINIMUM OF 14 DAYS AFTER PLACEMENT.
- FOR COLD-WEATHER AND HOT-WEATHER CONCRETE PLACEMENT, CONFORM TO APPLICABLE ACI CODES AND RECOMMENDATIONS. IN EITHER CASE, MATERIALS CONTAINING CHLORIDE, CALCIUM, SALTS, ETC. SHALL NOT BE USED. PROTECT FRESH CONCRETE FROM WEATHER FOR 7 DAYS MINIMUM.

CIVIL LEGEND

EXISTING		PROPOSED
UNDERGROUND ELECTRIC	FENCE	--- x x x ---
UNDERGROUND TELEPHONE	UNDERGROUND ELECTRIC	-----
OVERHEAD WIRES	UNDERGROUND TELEPHONE	-----
250	OVERHEAD TELEPHONE	-----
200	OVERHEAD ELECTRIC	-----
120.5 OR 120.5	5' OR 10' CONTOUR LINE	-----
	1' OR 2' CONTOUR LINE	-----
	SPOT ELEVATION	120.5 OR x 120.5
	PRIMARY PROPERTY OR R.O.W.	-----
	LEASE LINE	-----
	EASEMENT	-----
	UTILITY POLE	⊗
	TELEPHONE PEDESTAL	⊠
	CURB	=====
	ASPHALT PAVEMENT	=====
	BUILDING	▭
	TREES, SHRUBS, BUSHES	⊙ ⊗ ⊗



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0	ISSUED FOR PERMIT	SKB	08/30/18

Drawn: SKB Date: 08/30/18
 Designed: AD Date: 08/30/18
 Checked: AD Date: 08/30/18

Project Number: 502-000

Project Title: GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For:



Drawing Title:

GENERAL NOTES & LEGEND

Drawing Scale: CD
Date: 09/24/18

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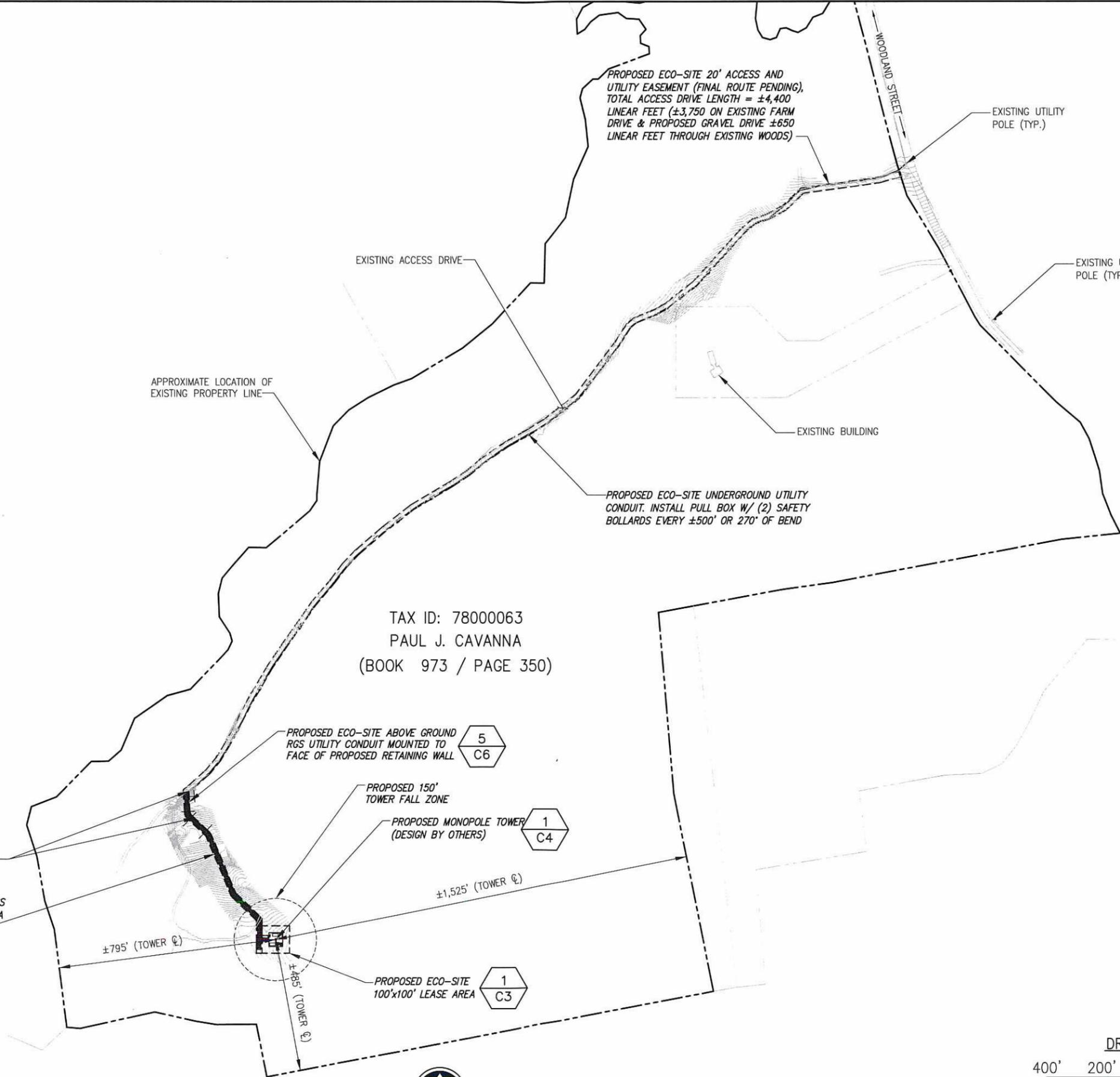
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C1

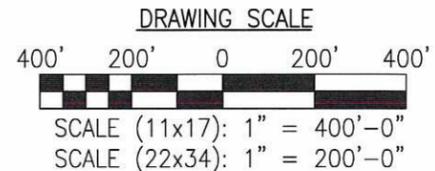
NOTE:
 • ROUTE OF UTILITY IS TO FOLLOW ACCESS EASEMENT UNLESS OTHERWISE DIRECTED BY UTILITY COMPANY.
 • FINAL INSTALLATION METHOD (OVERHEAD OR UNDERGROUND) TO BE DETERMINED.
 • HOURS OF CONSTRUCTION SHALL BE FROM 8:00AM - 6:00PM EST.

SHADED AREA REPRESENTS "INLAND WETLAND SOILS" FROM SOMERS CT GIS ONLINE DATABASE DELINEATION IN FIELD TO BE COMPLETED BY OTHERS. PER WETLAND INVESTIGATION BY OTHERS, NO WETLANDS EXIST ON THIS PARCEL.

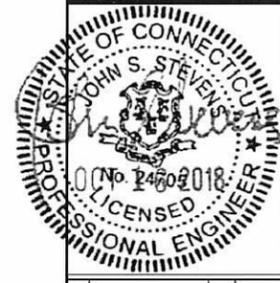
NOTE:
 • NO CONSTRUCTION IS TO BE COMPLETED BETWEEN THE DATE OF FEBRUARY 15TH & APRIL 15TH DUE TO VERNAL POOL.
 • HOURS OF CONSTRUCTION ARE FROM DAWN UNTIL DUSK



BASEMAPMING INFORMATION BASED ON INFORMATION OBTAINED FROM AERIAL PHOTOGRAPHY, INFORMATION PROVIDED BY ECO-SITE, A SITE WALK COMPLETED BY INFINIGY ENGINEERING ON 5/17/16 AND SURVEY COMPLETED BY CLIMAX DEVELOPMENT OF W.N.Y. TITLED "GLASTONBURY", DATED 6/14/16.



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Drawn: SKB Date: 08/30/18
 Designed: AJD Date: 08/30/18
 Checked: AJD Date: 08/30/18

Project Number:
 502-000

Project Title:
GLASTONBURY
 CT-0007A
 63-80 WOODLAND STREET
 GLASTONBURY, CT 06073

Prepared For:
Eco-Site

Drawing Title:
 OVERALL SITE PLAN

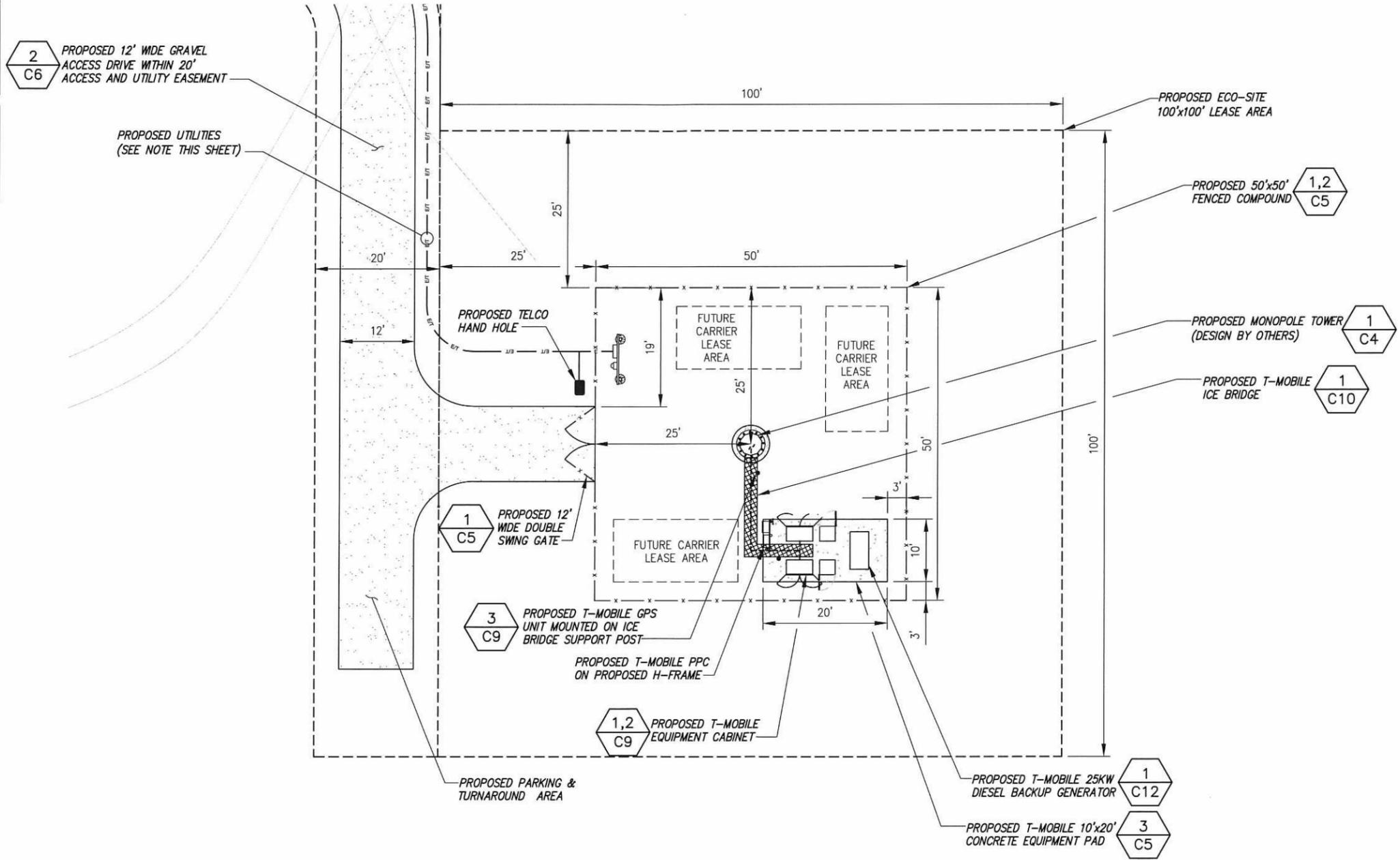
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 CD
 Date:
 09/24/18

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 Drawing Number:
C2

NOTE:
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 • FINAL INSTALLATION METHOD (OVERHEAD OR UNDERGROUND) TO BE DETERMINED.
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SHADED AREA REPRESENTS "INLAND WETLAND SOILS" FROM SOMERS CT GIS ONLINE DATABASE DELINEATION IN FIELD TO BE COMPLETED BY OTHERS. PER WETLAND INVESTIGATION BY OTHERS, NO WETLANDS EXIST ON THIS PARCEL.

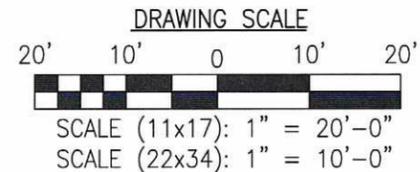
NOTE:
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 • HOURS OF CONSTRUCTION ARE FROM DAWN UNTIL DUSK



BASEMAPPING INFORMATION BASED ON INFORMATION OBTAINED FROM AERIAL PHOTOGRAPHY, INFORMATION PROVIDED BY ECO-SITE, A SITE WALK COMPLETED BY INFINIGY ENGINEERING ON 5/17/16 AND SURVEY COMPLETED BY CLIMAX DEVELOPMENT OF W.N.Y. TITLED "GLASTONBURY", DATED 6/14/16.



1 ENLARGED SITE LAYOUT
 SCALE: AS NOTED



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Drawn: SKB Date: 08/30/18
 Designed: AJD Date: 08/30/18
 Checked: AJD Date: 08/30/18

Project Number: 502-000

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 CT-0007A
 63-80 WOODLAND STREET
 GLASTONBURY, CT 06073

Prepared For: **Eco-Site**

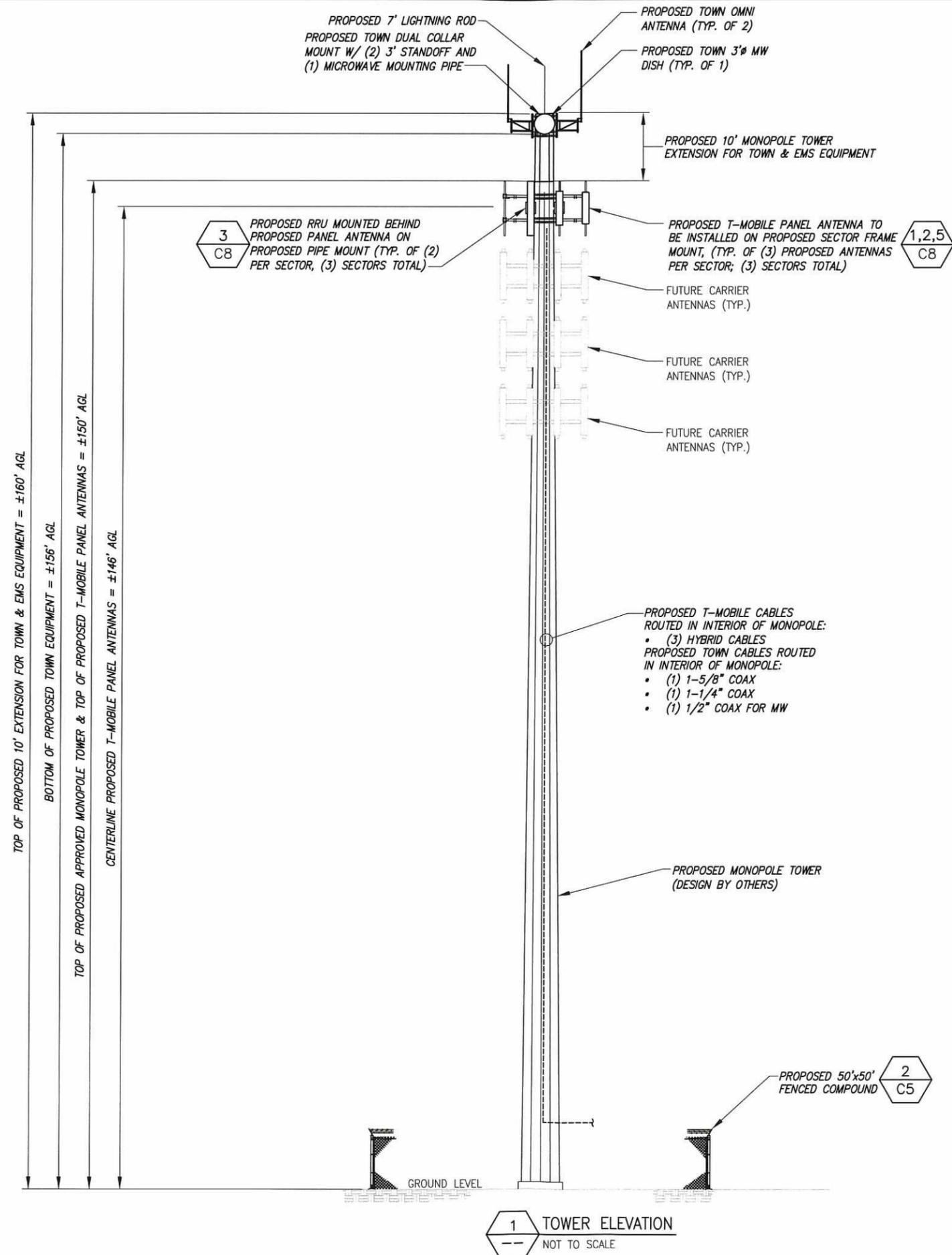
Drawing Title: ENLARGED SITE LAYOUT

Drawing Scale: CD
 Date: 09/24/18

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Drawing Number: **C3**

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Drawn: SKB Date: 08/30/18
 Designed: A.D. Date: 08/30/18
 Checked: A.D. Date: 08/30/18

Project Number: 502-000

Project Title: GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
 GLASTONBURY, CT 06073

Prepared For:



Drawing Title:

TOWER ELEVATION
 OPTION 1

Drawing Scale:

CD

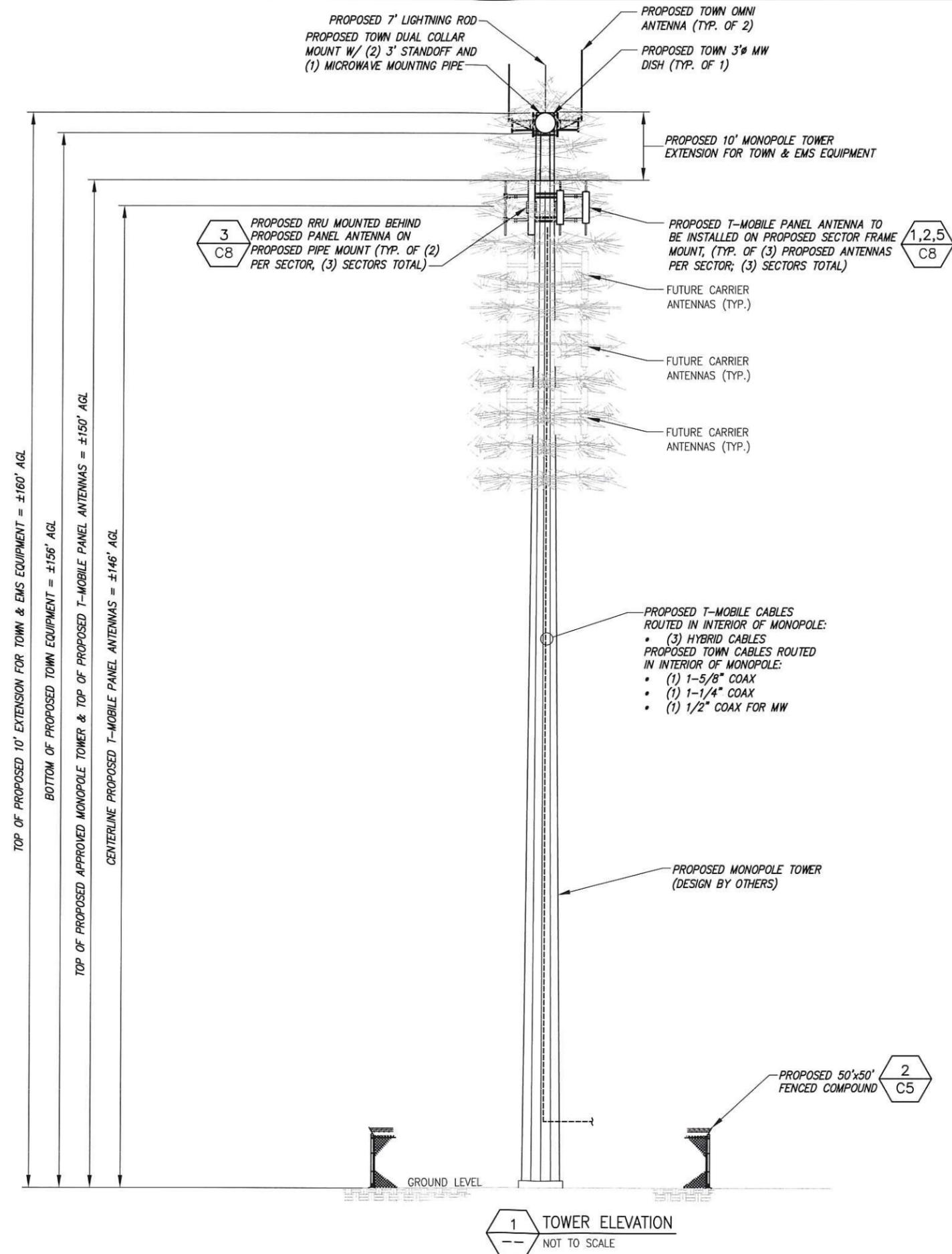
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Drawing Number:

C4

INFINIGY ENGINEERING ASSUMES NO LIABILITY FOR THE STRUCTURAL INTEGRITY OF THE PROPOSED TOWER INSTALLATION. A STRUCTURAL ANALYSIS MUST BE COMPLETED PRIOR TO START OF CONSTRUCTION.



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No.	Submittal / Revision	App'd	Date
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Designed:	AJD	Date:	08/30/18
Checked:	AJD	Date:	08/30/18

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Project Title: GLASTONBURY

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63-80 WOODLAND STREET
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Prepared For:



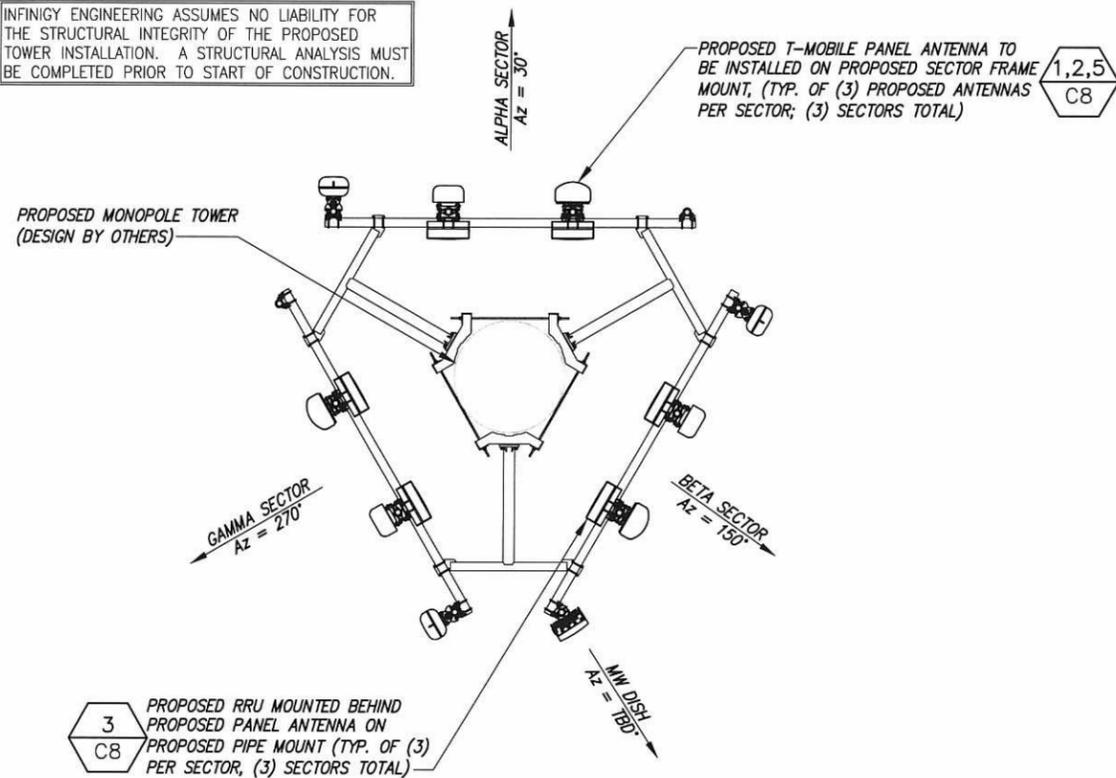
Drawing Title:
 TOWER ELEVATION
 OPTION 2

Drawing Scale:
 CD
 Date:
 09/24/18

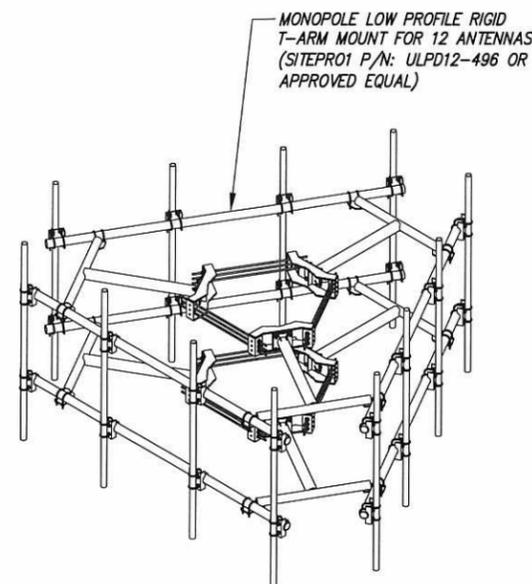
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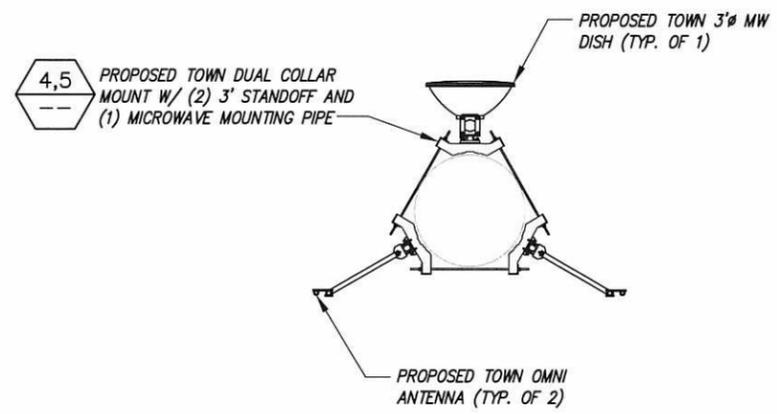
INFINIGY ENGINEERING ASSUMES NO LIABILITY FOR THE STRUCTURAL INTEGRITY OF THE PROPOSED TOWER INSTALLATION. A STRUCTURAL ANALYSIS MUST BE COMPLETED PRIOR TO START OF CONSTRUCTION.



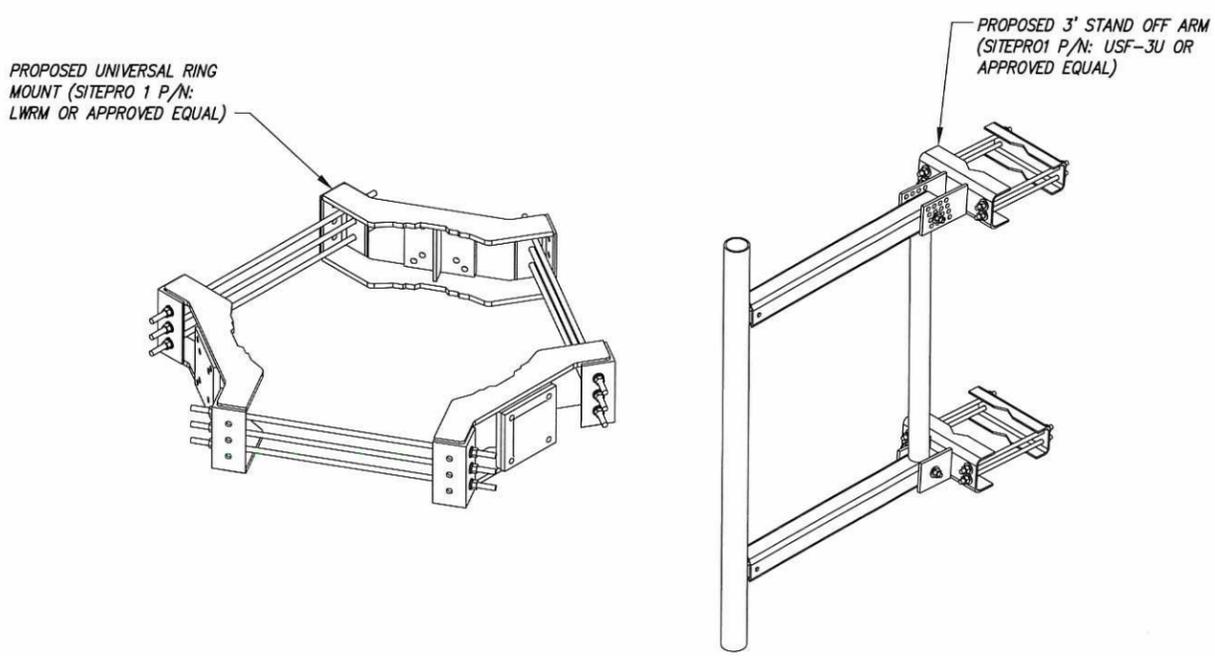
1 ANTENNA ORIENTATION PLAN (@ ±146' AGL)
--- NOT TO SCALE



2 ANTENNA FRAME DETAIL (@ ±146' AGL)
--- NOT TO SCALE



3 ANTENNA ORIENTATION PLAN (@ ±156' AGL)
--- NOT TO SCALE



4 COLLAR MOUNT DETAIL
--- NOT TO SCALE

5 STAND OFF ARM DETAIL
--- NOT TO SCALE

ANTENNA AND RRU SCHEDULE									
SECTOR	ANTENNA POSITION	ANTENNA MAKE	ANTENNA MODEL	RAD CTR. FT. AGL	AZIMUTH	RRU/ODU	E-TILT	M-TILT	CABLE
ALPHA	#1	ERICSSON	AIR32 KRD901146-1	150'-0"	30°	--	2' (L2100) 2' (L1900)	0°	(1) 6X12 HYBRID (SHARED BY ALPHA)
	#2	ANDREW	LNx-6515DS-A1M	150'-0"	30°	ERICSSON RRUS-11 B12	2' (L700)	0°	(1) 6X12 HYBRID (SHARED BY ALPHA)
	#3	RFS	APX16DWV-16DW-S-E-A20	150'-0"	30°	ERICSSON RRUS-11 B4	2' (U2100)	0°	(1) 6X12 HYBRID (SHARED BY ALPHA)
BETA	#1	ERICSSON	AIR32 KRD901146-1	150'-0"	150°	--	2' (L2100) 2' (L1900)	0°	(1) 6X12 HYBRID (SHARED BY BETA)
	#2	ANDREW	LNx-6515DS-A1M	150'-0"	150°	ERICSSON RRUS-11 B12	2' (L700)	0°	(1) 6X12 HYBRID (SHARED BY BETA)
	#3	RFS	APX16DWV-16DW-S-E-A20	150'-0"	150°	ERICSSON RRUS-11 B4	2' (U2100)	0°	(1) 6X12 HYBRID (SHARED BY BETA)
	#4	FASTBACK	IBR-1300	150'-0"	TBD	N/A	N/A	N/A	1/2" Ø COAX
GAMMA	#1	ERICSSON	AIR32 KRD901146-1	150'-0"	270°	--	2' (L2100) 2' (L1900)	0°	(1) 6X12 HYBRID (SHARED BY GAMMA)
	#2	ANDREW	LNx-6515DS-A1M	150'-0"	270°	ERICSSON RRUS-11 B12	2' (L700)	0°	(1) 6X12 HYBRID (SHARED BY GAMMA)
	#3	RFS	APX16DWV-16DW-S-E-A20	150'-0"	270°	ERICSSON RRUS-11 B4	2' (U2100)	0°	(1) 6X12 HYBRID (SHARED BY GAMMA)

6 RF LOADING CHART
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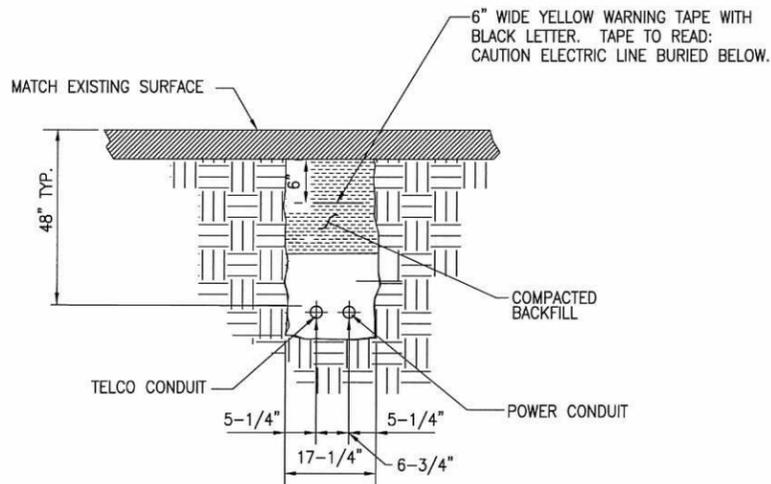
Prepared For: **Eco-Site**

Drawing Title: ORIENTATION & ANTENNA MOUNTING DETAILS

Drawing Scale: **CD**
Date: 09/24/18

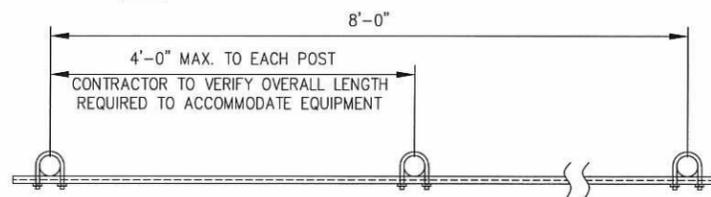
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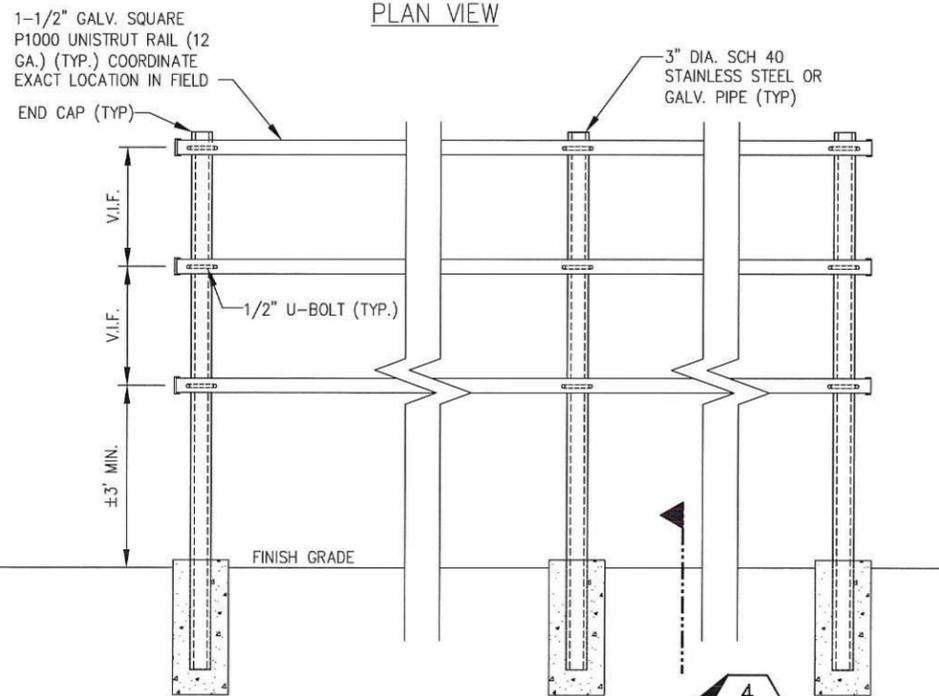


- NOTE:**
1. NUMBER AND SIZE OF CONDUITS MAY VARY. REFER TO CONSTRUCTION DRAWINGS FOR CONDUIT SIZE AND LOCATION. CONFIRM DIMENSIONS SHOWN WITH UTILITY COMPANY.
 2. CONTRACTOR TO VERIFY IN FIELD THE LOCATION, SIZE, TYPE, AND DEPTH OF ALL EXISTING UNDERGROUND UTILITIES PRIOR TO DIGGING THE SERVICE TRENCH. PROVIDE A MINIMUM OF 18" CLEARANCE BETWEEN PROPOSED UTILITIES AND EXISTING UTILITIES IN THE CASE OF UTILITY LINE CROSSINGS.

1 TYPICAL CONDUIT TRENCH DETAIL
--- NOT TO SCALE

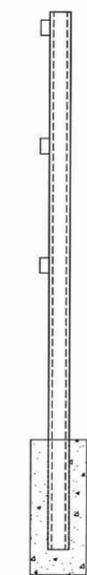


PLAN VIEW

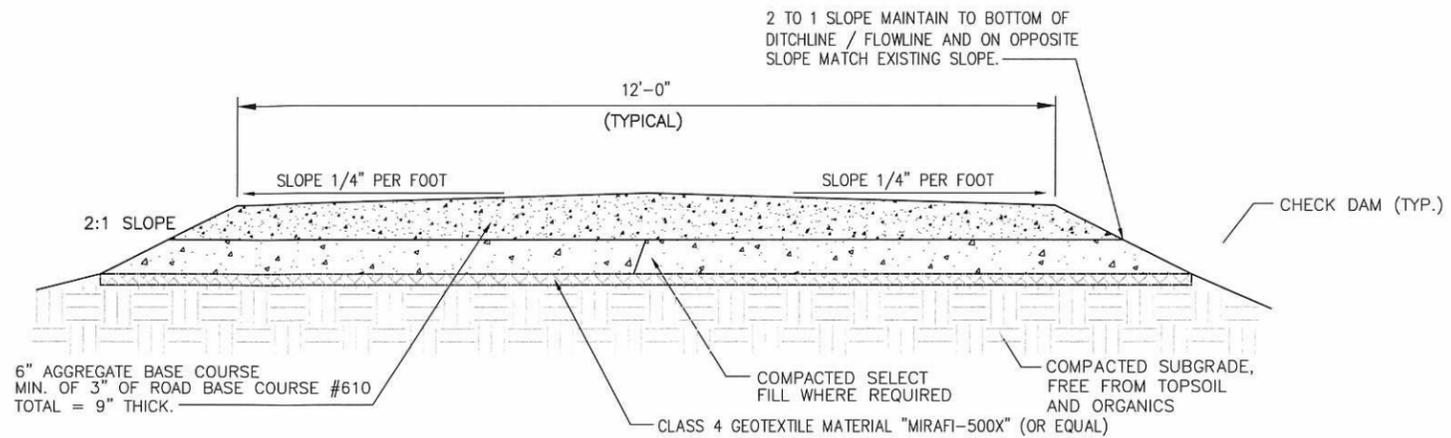


ELEVATION

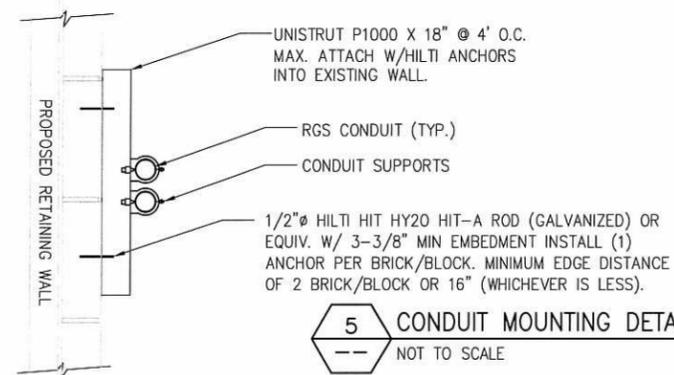
3 H-FRAME FABRICATION DETAIL
--- NOT TO SCALE



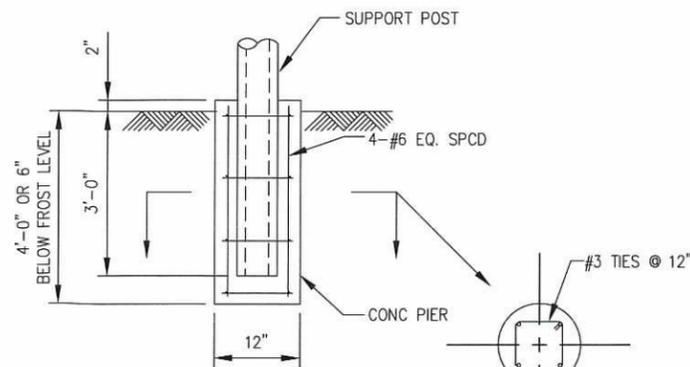
END VIEW



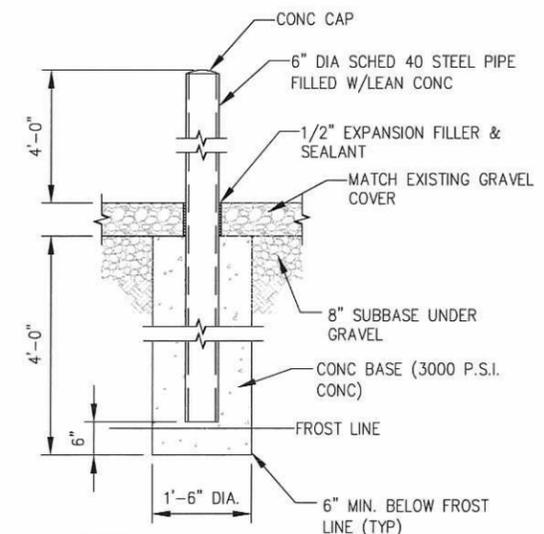
2 DRIVEWAY SECTION - CROWNED
--- NOT TO SCALE



5 CONDUIT MOUNTING DETAIL
--- NOT TO SCALE



4 SUPPORT POST FOOTING
--- NOT TO SCALE



6 BOLLARD DETAIL
--- NOT TO SCALE



3	REVISED PER COMMENT	SKB	10/12/18
2	REVISED PER COMMENT	SKB	10/05/18
1	REVISED PER COMMENT	SKB	09/24/18
0	ISSUED FOR PERMIT	SKB	08/30/18

Drawn: SKB Date: 08/30/18
Designed: AJD Date: 08/30/18
Checked: AJD Date: 08/30/18

Project Number: 502-000

Project Title: GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For:

Eco-Site

Drawing Title:

CIVIL DETAILS

Drawing Scale:

CD

Date: 09/24/18

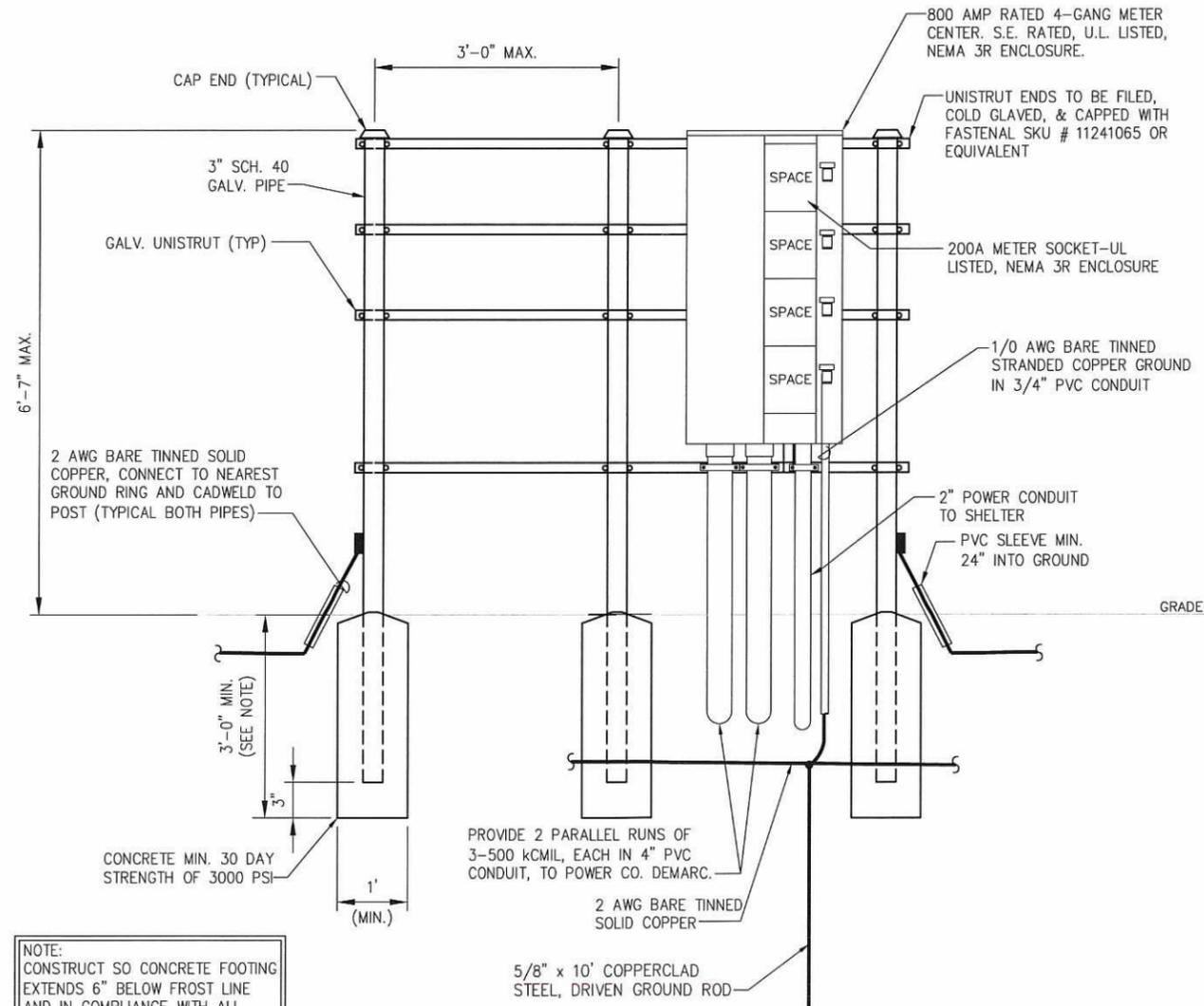
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Drawing Number:

C6

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1. ALL WORK SHALL CONFORM TO THE NATIONAL ELECTRICAL CODE, STATE BUILDING CODES AND THE LOCAL BUILDING CODES. ALL COMPONENTS SHALL BE U.L. LISTED.
2. REFER TO SITE LAYOUT PLAN FOR THE EXACT LOCATION OF H-FRAME.
3. CONTRACTOR TO COORDINATE WITH LOCAL UTILITY COMPANY FOR METER.
4. CONTRACTOR TO PROVIDE AND INSTALL METER SOCKET.
5. CONTRACTOR TO LOCATE METER RACK TO ENSURE WORKING SPACES REQUIRED BY THE NEC (ART. 110.26), STATE, OR LOCAL CODES ARE MAINTAINED BETWEEN FRONT OF ENCLOSURES AND THE CHAIN LINK FENCE.
6. SHOW LOCATION (INCLUDING DIMENSIONS) OF ALL CAPPED UNDERGROUND CONDUIT ON FINAL AS-BUILT DRAWINGS SUBMITTED TO OWNER.
7. COORDINATE EXACT LOCATION OF UNDERGROUND FEEDERS AND CIRCUITRY WITH THE OWNER.
8. CONTRACTOR SHALL COORDINATE WITH LOCAL ELECTRICAL AUTHORITY HAVING JURISDICTION (AHJ) AND OTHER TRADES TO DETERMINE "FROST" LINE, AND TYPES OF RACEWAYS REQUIRED FOR INSTALLATION. ALL CONDUITS ABOVE GROUND SHALL BE GALVANIZED CONDUIT.
9. CONTRACTOR TO CONTACT LOCAL UTILITY PRIOR TO PURCHASING METER CENTER TO VERIFY ANY PARTICULAR REQUIREMENTS, SUCH AS LEVER BYPASS, ETC.



NOTE:
CONSTRUCT SO CONCRETE FOOTING
EXTENDS 6" BELOW FROST LINE
AND IN COMPLIANCE WITH ALL
LOCAL AND STATE CODES.

NOTE:
EQUIPMENT AND CONDUITS ON BACK
NOT SHOWN FOR CLARITY PURPOSES.

1 H-FRAME DETAIL (FRONT)
C7 SCALE: AS NOTED



No.	Submital / Revision	App'd	Date
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CT-0007A
63-80 WOODLAND STREET
GLASTONBURY, CT 06073

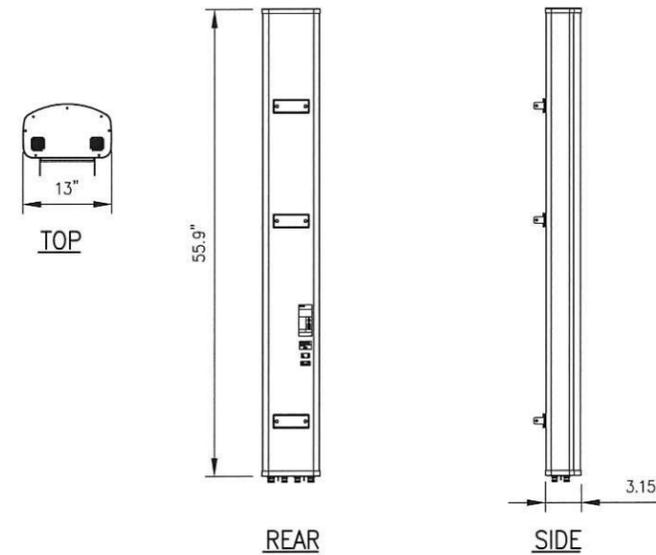
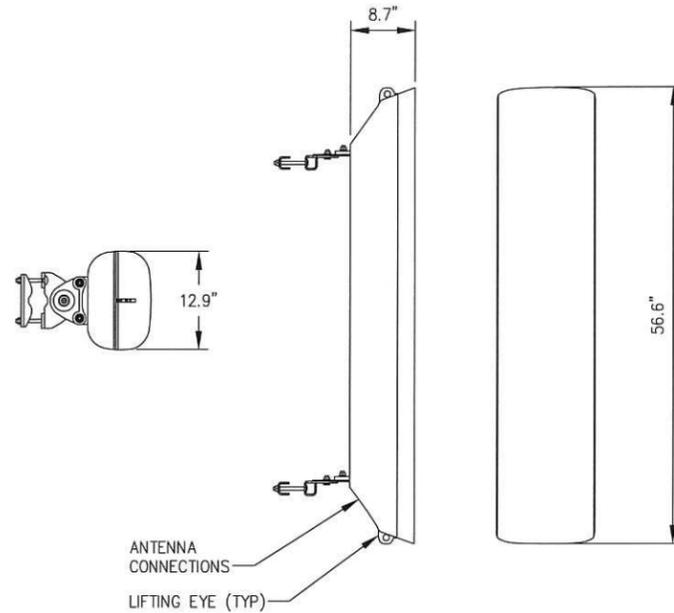
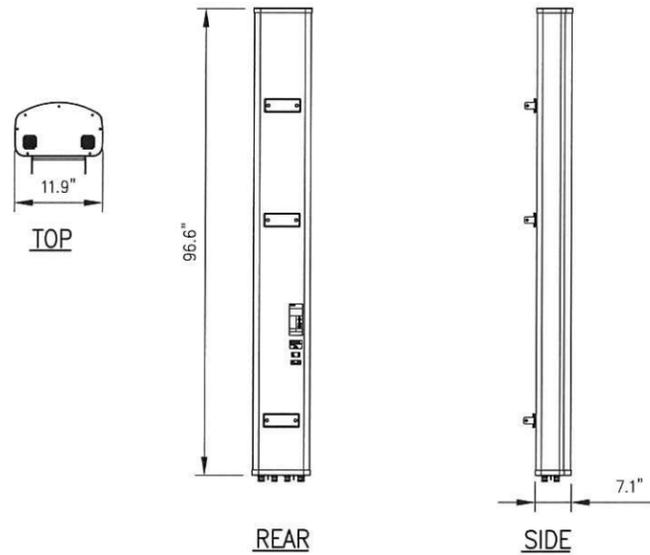
Prepared For:
Eco-Site

Drawing Title:
UTILITY RACK DETAIL

Drawing Scale:
Date:
09/24/18
CD

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Drawing Number:
C7



COMMSCOPE MODEL NO.:	LNx-6515DS-A1M
RADOME MATERIAL:	FIBERGLASS, UV RESISTANT
RADOME COLOR:	LIGHT GRAY
DIMENSIONS, HxWxD:	96.4"x11.9"x7.1" (2448 x 301 x 180.5 mm)
WEIGHT, W/ PRE-MOUNTED BRACKETS:	50.3 LBS
CONNECTOR:	7-16 DIN FEMALE

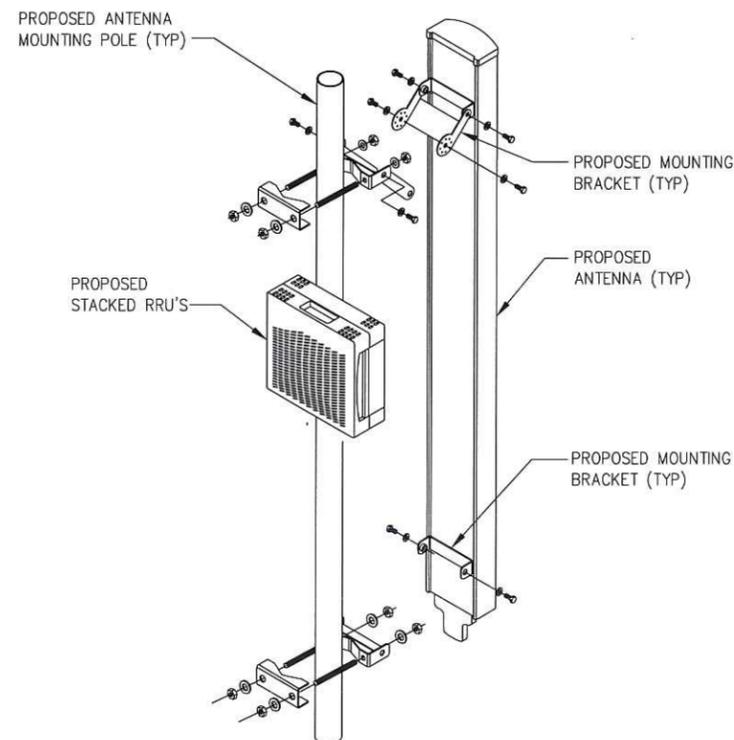
ERICSSON MODEL NO.:	AIR32 KRD901146-1
RADOME MATERIAL:	FIBERGLASS, UV RESISTANT
RADOME COLOR:	LIGHT GRAY
DIMENSIONS, HxWxD:	56.6"x12.9"x8.7"
WEIGHT, W/ PRE-MOUNTED BRACKETS:	132.2 LBS
CONNECTOR:	(2) 7-16 DIN FEMALE

RFS MODEL NO.:	APX16DWV-16DWV-S-E-A20
RADOME MATERIAL:	FIBERGLASS, UV RESISTANT
RADOME COLOR:	LIGHT GRAY
DIMENSIONS, HxWxD:	55.9"x13"x3.15" (1420 x 331 x 80 mm)
WEIGHT, W/ PRE-MOUNTED BRACKETS:	40.7 LBS
CONNECTOR:	7-16 DIN FEMALE

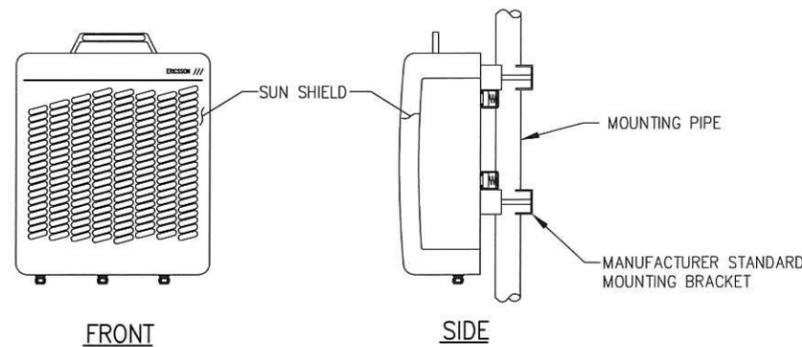
1 ANTENNA DETAIL
NOT TO SCALE

2 ANTENNA DETAIL
NOT TO SCALE

5 ANTENNA DETAIL
NOT TO SCALE

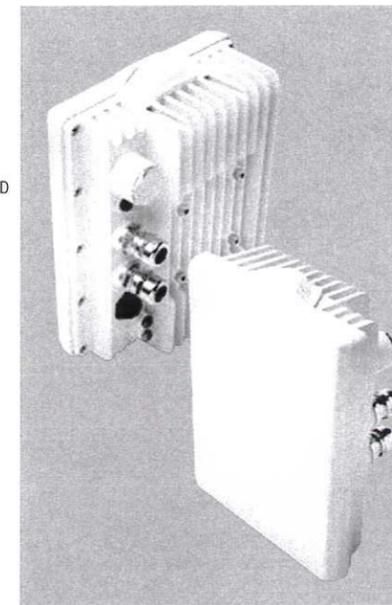


3 MOUNTING DETAIL
NOT TO SCALE



SPECIFICATIONS	
• HXWxD, (INCHES) :	24"x24"x9"
• WEIGHT (LBS) :	±60
• COLOR :	GRAY

4 ERICSSON RRU-11 DETAIL
NOT TO SCALE



Specifications	IBR
RADIO	
Speed and Range	Typical: Scalable up to 1.6 Gbps at 500m range (NLOS) and 2km range (LOS), 900 Mbps at 3km range (LOS), 300Mbps at 13km range (LOS)†
Latency	Typical: <400µsec
Frequency bands	FDD+ (no A or B side) operation across all 5 GHz UNII bands
Antenna Beamwidth	20 degrees, steerable over 40 degrees
EIRP	FCC: Up to +42 dBm
Adaptive Rate Modulation	Supported via proprietary adaptive algorithms
Interference Mitigation	Supported via proprietary avoidance and cancellation algorithms
Diversity	Supported via proprietary antenna array signal processing
Security	AES-256 OTA Encryption
SWITCH	
Carrier Ethernet Features	Y1731 and 802.1ag OAM, Q in Q, RFC 2544 reflection, QoS, Broadcast / Unknown / Multicast (BUM) filter, Configurable latency per queue
Interfaces	1 x GbE (Cu), 1 x GbE (SFP or Cu), 1 ALOS radio interface (see above)
QoS	802.1p and DSCP classification, strict priority scheduling, WDRR scheduling
Timing	1588v2 Transparent Clock
Management	HTTPS, ssh, Telnet, SNMP v2c & v3, IPv6, Dying Gasp
Dimensions (W x H x D)	200mm width, 260mm height, 90mm depth
Weight	4 kg
Power Input	IBR-1300: PoE IBR-1301: 90-240 VAC
Temperature	-40C to +60C operating -55C to +85C storage

6 MICROWAVE DETAIL
SCALE: NOT TO SCALE



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GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For:
Eco-Site

Drawing Title:
T-MOBILE EQUIPMENT DETAILS

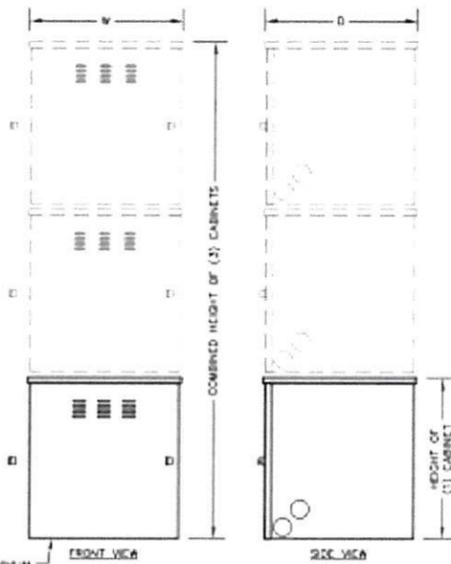
Drawing Scale:
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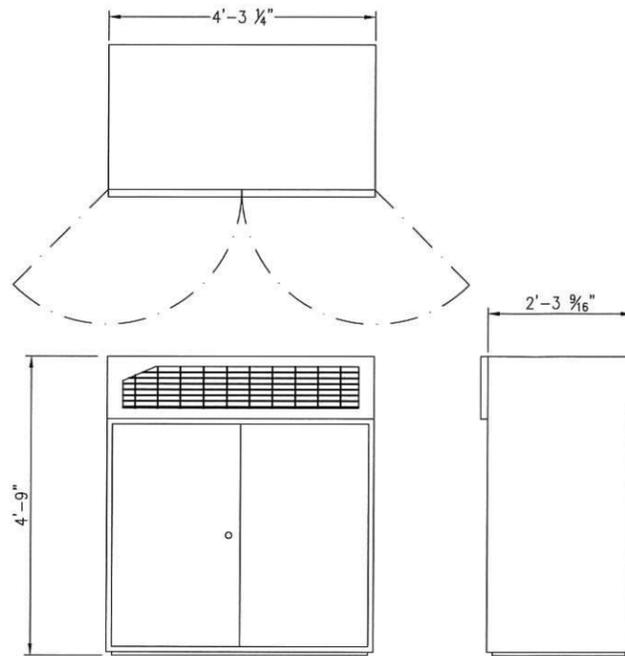
Drawing Number:
C8

BBU SPECIFICATIONS	
MANUF.	MFS DATA SERVICES
MODEL #	DUBEM-24LM
HEIGHT (1) CABINET	29.67"
HEIGHT (3) CABINETS	89.01"
WIDTH	28.54"
DEPTH	28.54"



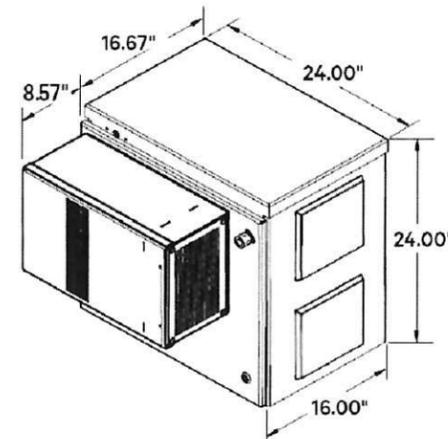
NOTE: MFS DATA SERVICES BBU CABINET, MODEL DUBEM-24LM (STACKABLE - 3 TOTAL PROPOSED)

1 ERICSSON BBU CABINET DETAIL
NOT TO SCALE



ERICSSON MODEL NO.: RBS 6102
DIMENSIONS, HxWxD: 4'-9"x4'-3 1/4"x2'-3 9/16"
WEIGHT: 772 LBS (W/O BATTERIES)

2 RBS 6102 DETAIL
SCALE: NOT TO SCALE

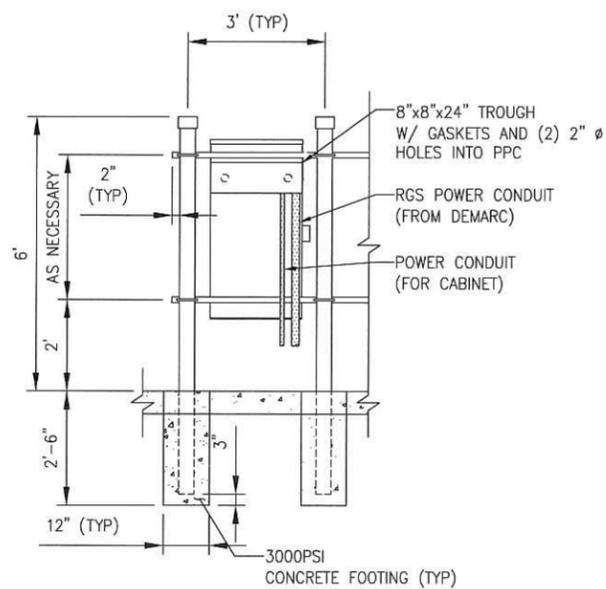


PHYSICAL CHARACTERISTICS

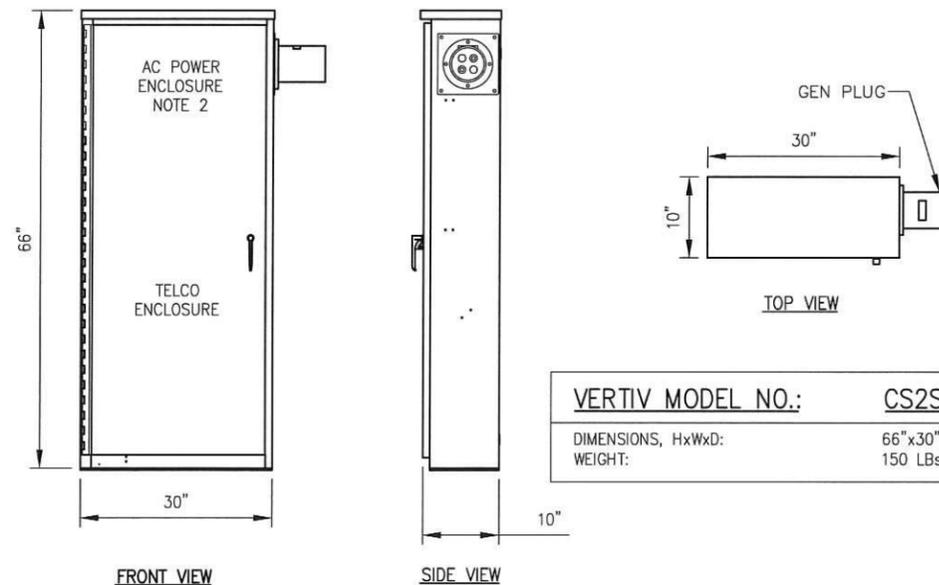
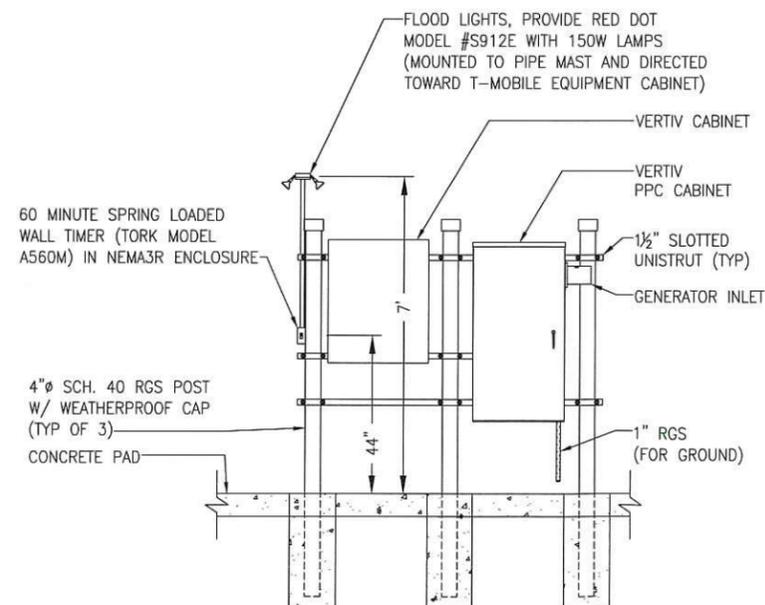
Framework Type	NetXtend™ Compact Enclosure
Available Space	Up to 14 RU, 19" W
Dimensions (H x W x D)	Enclosure: 24" x 24" x 16" Battery tray: 22" W x 13" D
Mounting	Wall or H-frame, pole mount (wall-mount kit included)
Weight, Equipped	Enclosure: 64 lb., w/out batteries Four (4) batteries: 36 lb. total
Access	Front

VERTIV MODEL NO.: NETXTEND COMPACT 2416
DIMENSIONS, HxWxD: 24"x24"x16"
WEIGHT: 64 LBS W/O BATTERIES

3 NETXTEND COMPACT BATTERY CABINET DETAIL
NOT TO SCALE



4 PPC BACKBOARD DETAIL
SCALE: NOT TO SCALE



VERTIV MODEL NO.: CS2S2-W736
DIMENSIONS, HxWxD: 66"x30"x10"
WEIGHT: 150 LBS

NOTES

- METER SOCKET BY THIS CONTRACT. METER TO BE SUPPLIED BY LOCAL UTILITY COMPANY.
- AC POWER ENCLOSURE. 200 AMP, 208/120V, 1Ø, 3W W/ GROUND. 200A/2P MAIN CIRCUIT BREAKER.
- ALL EQUIPMENT SHALL BE GROUNDED PER LATEST EDITION OF NEC AND AS INDICATED.
- ELECTRICAL EQUIPMENT SHALL BE MIN. 3'-0" FROM ANY STRUCTURE AND AS REQUIRED BY LOCAL UTILITY COMPANIES AND AHJ.
- CONTRACTOR MUST LABEL ALIKE BREAKERS IN DISTRIBUTION PANEL.
- REFER TO ACTUAL EQUIPMENT DRAWINGS.

5 PPC DETAIL
NOT TO SCALE



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0	ISSUED FOR PERMIT	SKB	08/30/18

Project Number: 502-000

Project Title: GLASTONBURY
CT-0007A
63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For: Eco-Site

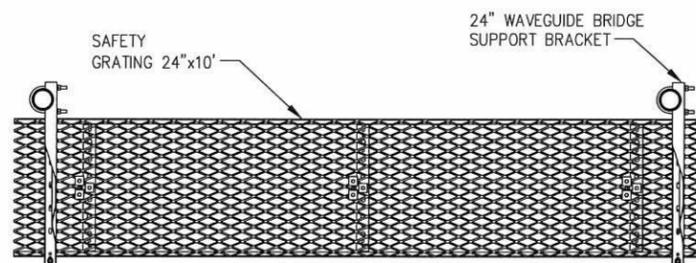
Drawing Title: T-MOBILE EQUIPMENT DETAILS

Drawing Scale: CD
Date: 09/24/18

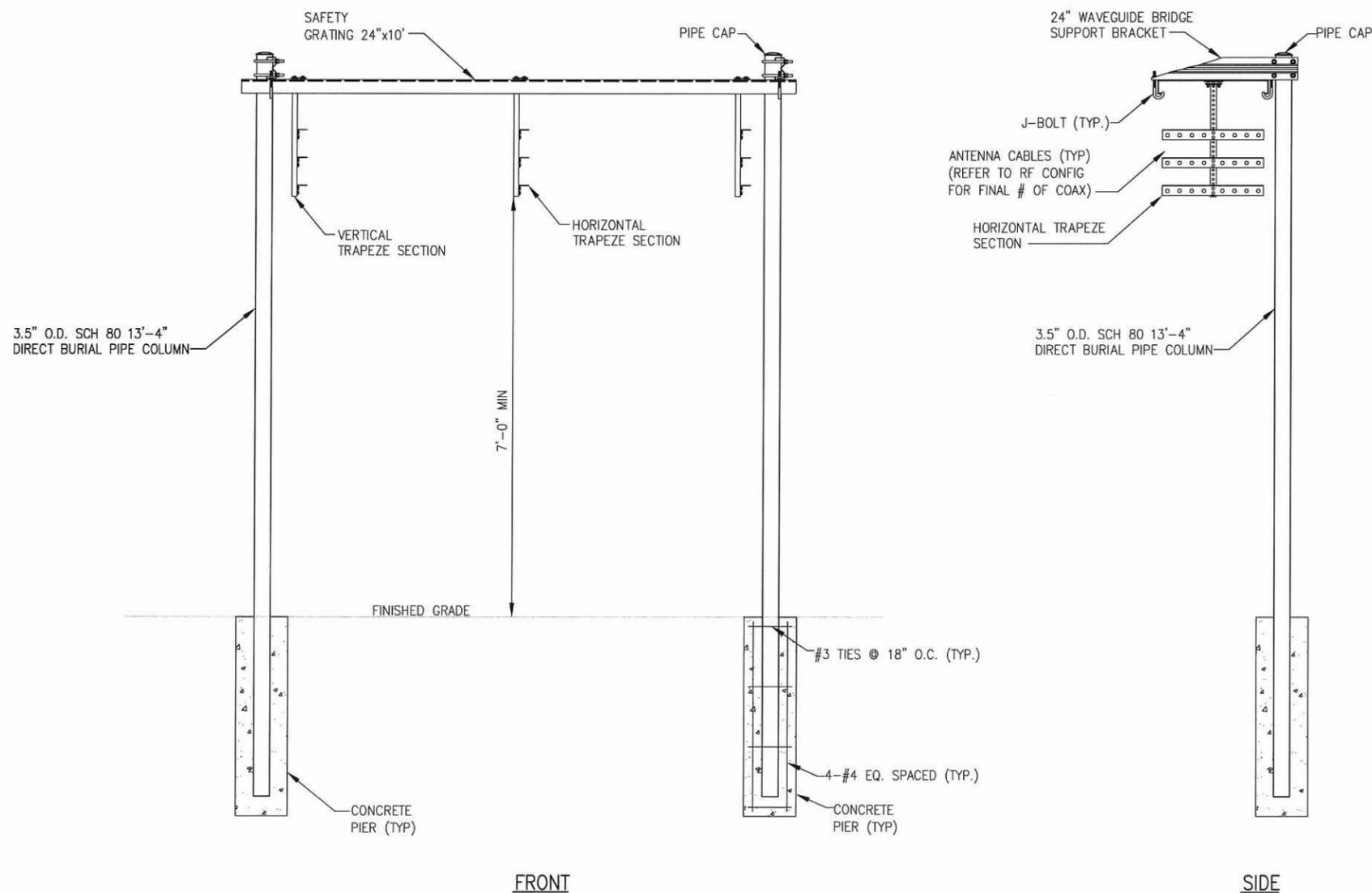
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Drawing Number: C9

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TOP



- NOTES:
1. USE SITE PRO 1 PARTS OR APPROVED EQUAL.
 2. SUPPORT POSTS SHALL BE LOCATED ON ALTERNATING SIDES OF ICE BRIDGE, SPACED NO MORE THAN 6'-0".
 3. ANY SPLICES OR CANTILEVERED SECTIONS OF THE ICE BRIDGE SHALL BE LOCATED WITHIN 2'-0" OF A SUPPORT POST.

1 WAVEGUIDE BRIDGE KIT (SITE PRO P/N: IB24D-T3)
-- NOT TO SCALE

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Project Title:
GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
GLASTONBURY, CT 06073

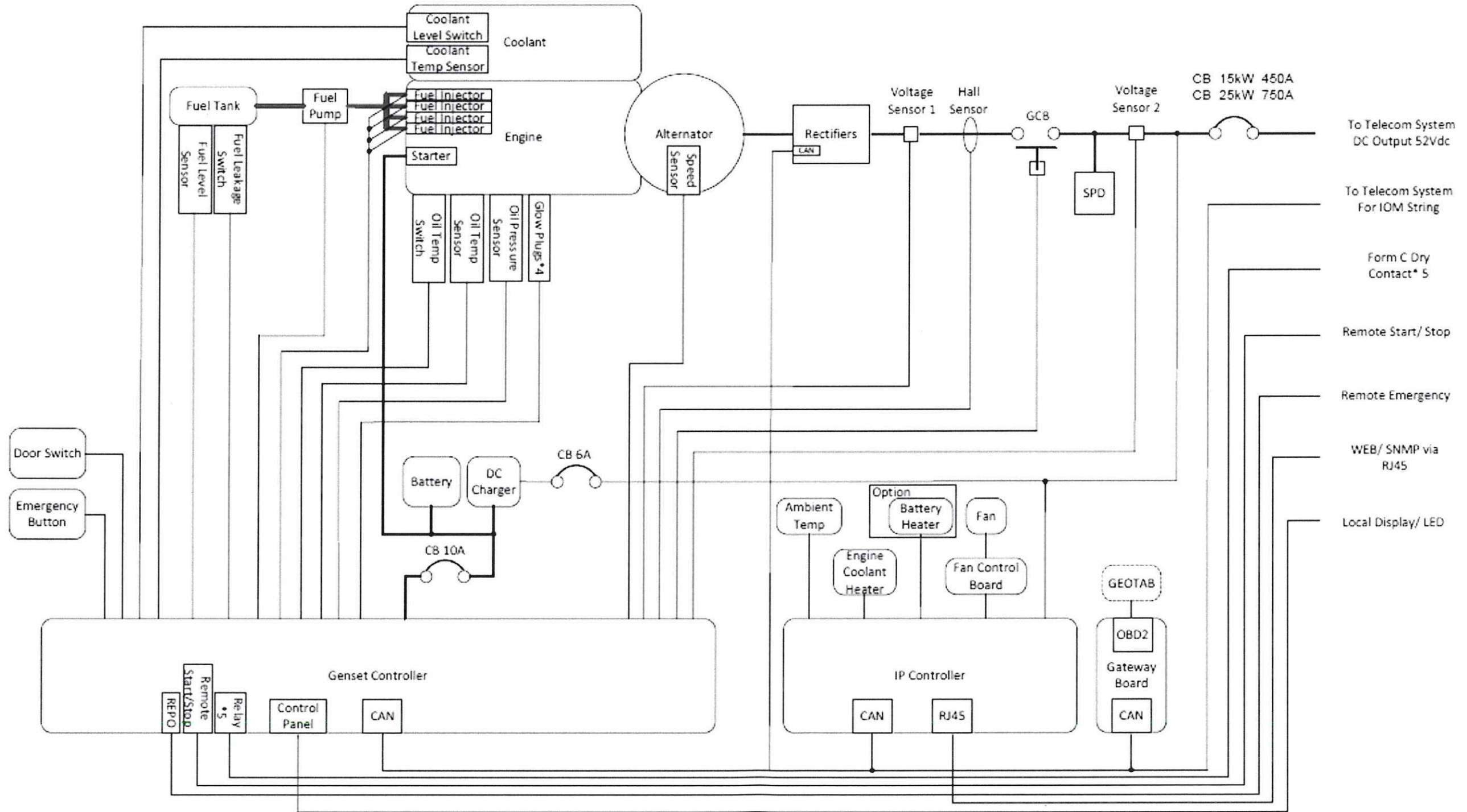
Prepared For:
Eco-Site

Drawing Title:
ICE BRIDGE DETAILS

Drawing Scale: **CD**
Date: 09/24/18

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C10



1 GENERATOR BLOCK DIAGRAM
NOT TO SCALE



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Prepared For: Eco-Site

Drawing Title: GENERATOR BLOCK DIAGRAM

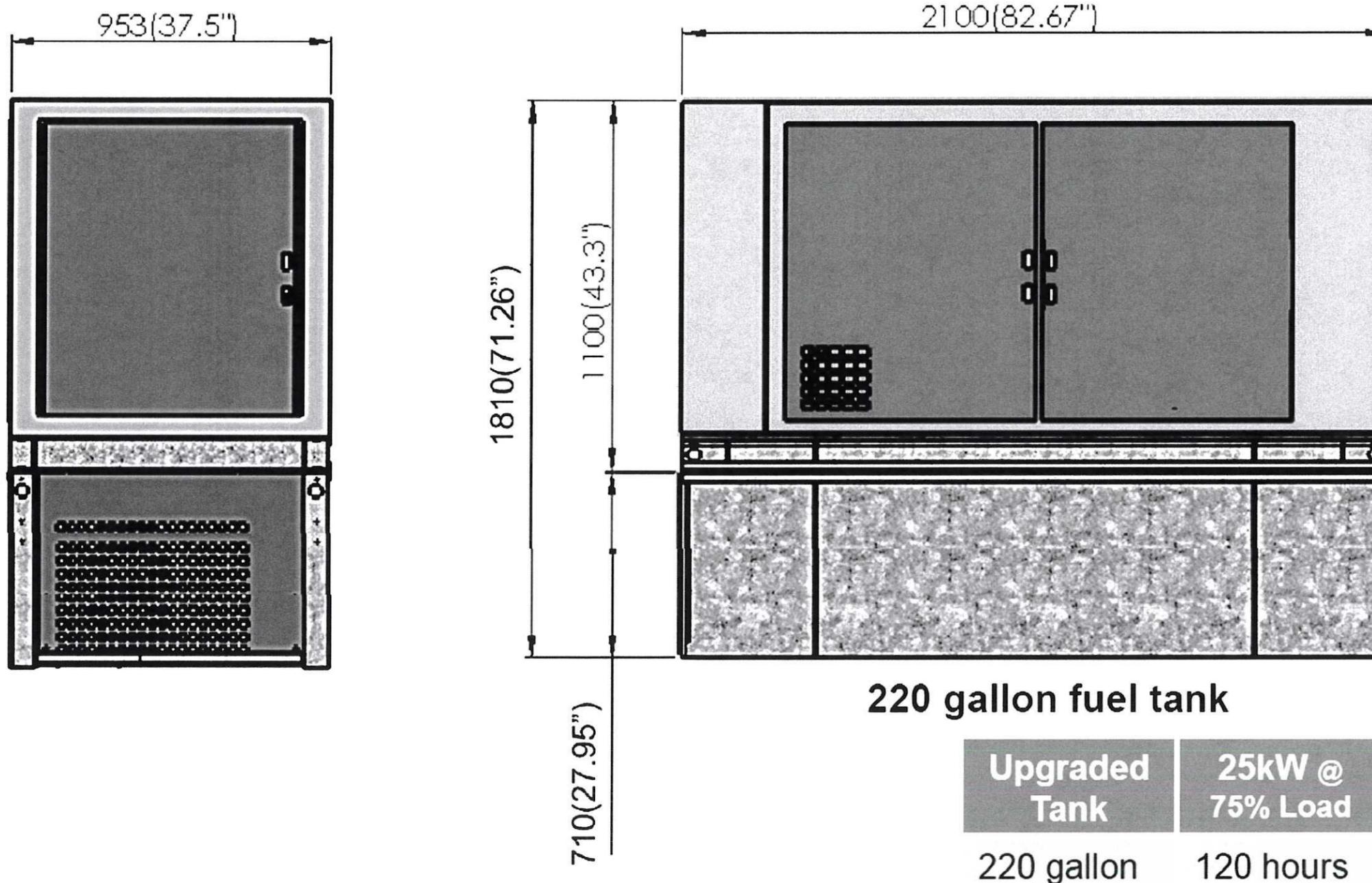
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220 Gallon Tank Dimensions 25kW DC Genset



Confidential

1 GENERATOR DETAIL
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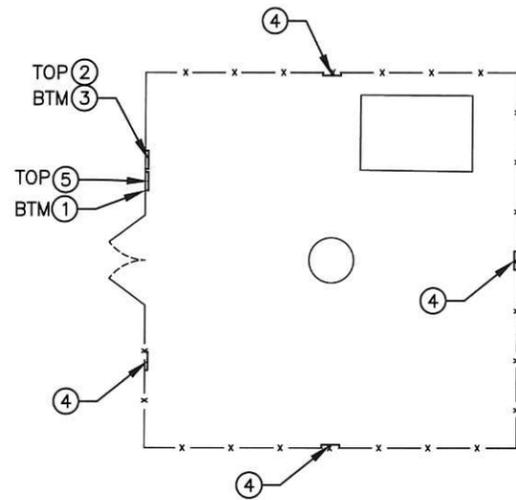
Prepared For:
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Drawing Title:
GENERATOR DETAILS

Drawing Scale:
Date: 09/24/18
CD

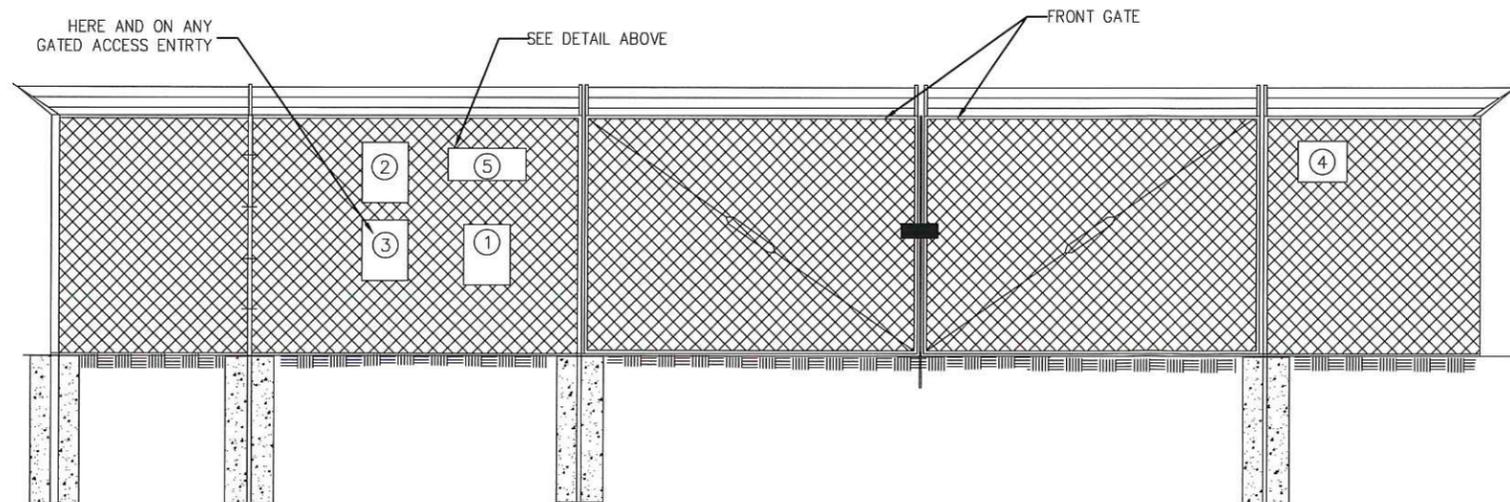
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Drawing Number:
C12



NOTE:
SEE TYPICAL SIGNS AND SPECIFICATIONS DETAIL
ON THIS SHEET FOR SIGN DESIGNATIONS.

1 OVERALL SIGN PLACEMENT PLAN VIEW
NOT TO SCALE



3 SITE SIGNAGE FRONT GATE VIEW
NOT TO SCALE

NOTICE



Beyond This Point you are entering an area where RF Emissions *may exceed* the FCC General Population Exposure Limits
Follow all posted signs and site guidelines for working in an RF environment

1 NOTICE - RF SIGN (BLUE)
12" x 18" DIGITAL PRINT MOUNTED TO 0.40 THICK ALUMINUM (OPERATIONS PROVIDED)



WARNING

ABOVE THIS POINT:
Radio frequency emissions may exceed controlled exposure limits.
For your safety, obey all posted signs, site guidelines and Federal Regulations for working in radio frequency environments.
In accordance with Federal Communications Commission rules on radio frequency emissions 47 CFR 1.1307(c)

2 WARNING - RF SIGN
12" x 18" DIGITAL PRINT MOUNTED TO 0.40 THICK ALUMINUM (OPERATIONS PROVIDED)

CAUTION



Beyond This Point you are entering a controlled area where RF Emissions *may exceed* the FCC Occupational Exposure Limits
Obey all posted signs and site guidelines for working in an RF environment

3 CAUTION - RF SIGN (YELLOW)
12" x 18" DIGITAL PRINT MOUNTED TO 0.40 THICK ALUMINUM (OPERATIONS PROVIDED)



4 NO-TRESSPASSING SIGN
12" x 18" DIGITAL PRINT MOUNTED TO 0.40 THICK ALUMINUM (OPERATIONS PROVIDED)

TOWER ID: _____
SITE NAME: _____
E911 ADDRESS: _____
FCC#: _____

Eco-Site®

FOR TOWER LEASING INFORMATION & EMERGENCY CONTACT
1-866-899-6191

5 ECO-SITE ID SIGN
18" HIGH X 24" WIDE (OPERATIONS PROVIDED)

2 TYPICAL SIGNS AND SPECIFICATIONS
NOT TO SCALE

SIGNAGE NOTES:
1. SIGNS SHALL BE FABRICATED FROM CORROSION RESISTANT PRESSED METAL, AND PAINTED WITH LONG LASTING UV RESISTANT COATINGS.
2. SIGNS (EXCEPT WHERE NOTED OTHERWISE) SHALL BE MOUNTED TO THE TOWER, GATE, AND FENCE USING A MINIMUM OF 9 GAUGE ALUMINUM WIRE, HOG RINGS (AS UTILIZED IN FENCE INSTALLATIONS) OR BRACKETS WHERE NECESSARY. BRACKETS SHALL BE OF SIMILAR METAL AS THE STRUCTURE TO AVOID GALVANIC CORROSION.

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Prepared For:
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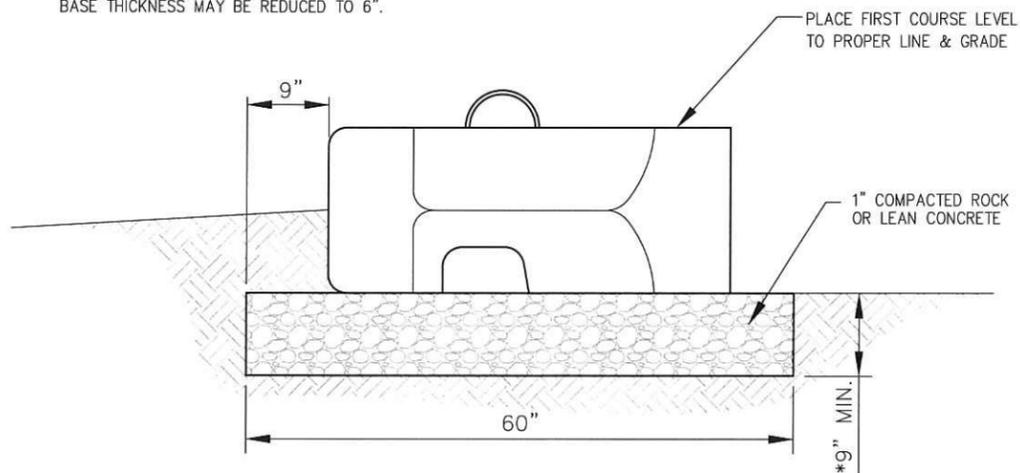
Drawing Title:
SITE SIGNAGE DETAILS

Drawing Scale: CD
Date: 09/24/18

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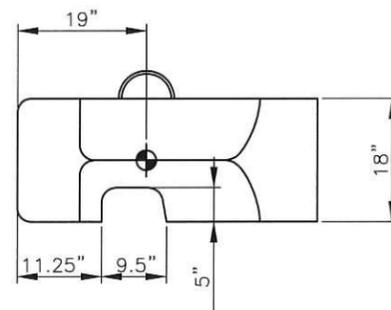
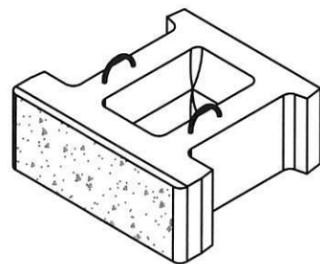
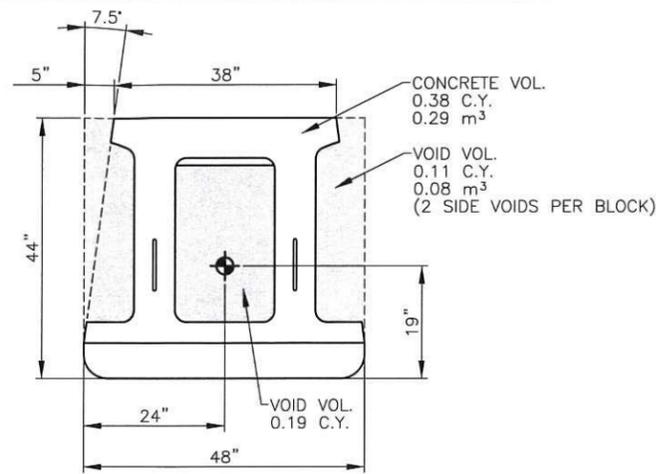
Drawing Number:
C13

*FOR WALL HEIGHTS OF 6' OR LESS,
BASE THICKNESS MAY BE REDUCED TO 6".



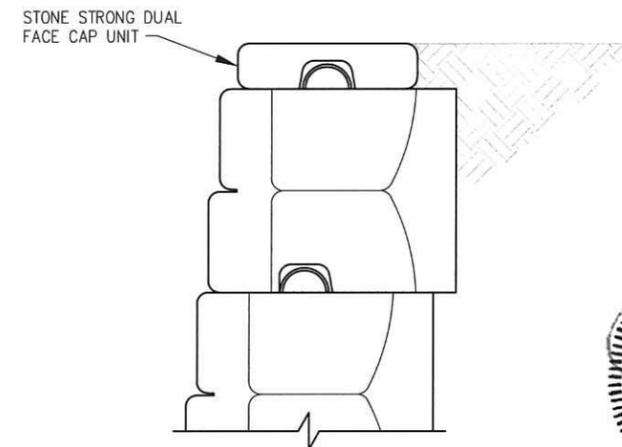
NOTE:
BEARING CONDITIONS SHALL BE OBSERVED BY
THE SITE GEOTECHNICAL ENGINEER. BASE
DIMENSIONS MAY BE INCREASED TO ADDRESS
DEFICIENT SOIL BEARING CONDITIONS.

1 6SF WALL BASE
--- NOT TO SCALE

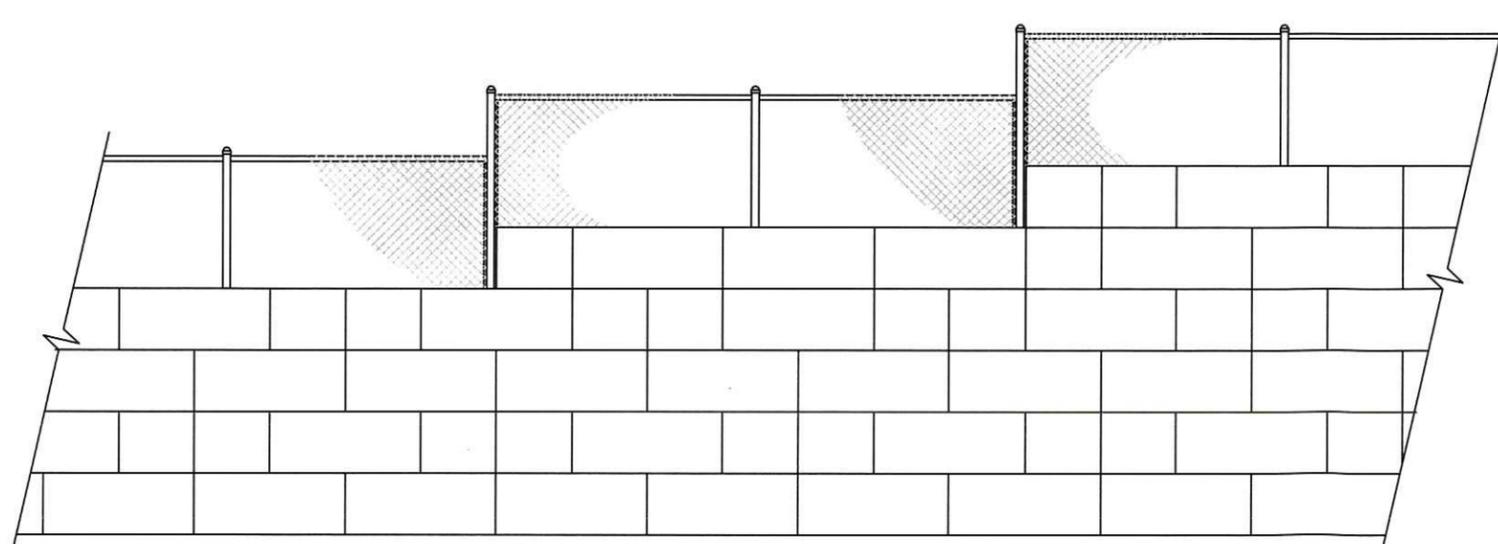


Block Wt.	1,500 lbs	680 kg
Form Wt.	1,800 lbs	815 kg
Concrete Volume	0.38 CY	0.29 m ³
Aggregate Infill	0.41 CY	0.31 m ³
(per face area)	0.1 tons/sf	1,000 kg/m ²

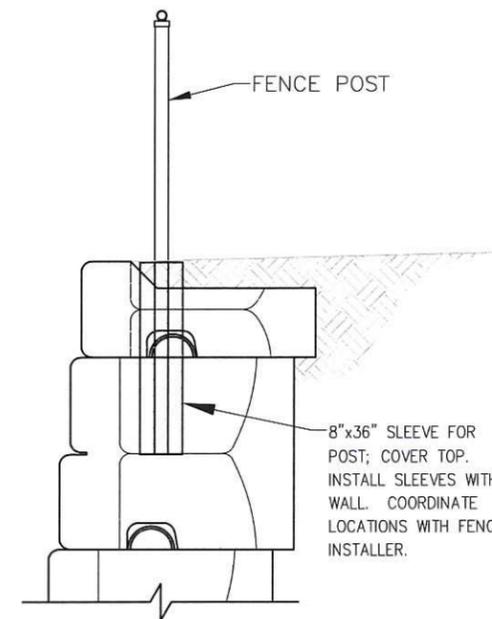
2 STONE STRONG 6sf UNIT
--- NOT TO SCALE



3 STONE STRONG CAP UNIT
--- NOT TO SCALE



4 TYPICAL FENCE CONFIGURATION
--- NOT TO SCALE



5 FENCE SLEEVE
--- NOT TO SCALE

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Prepared For:

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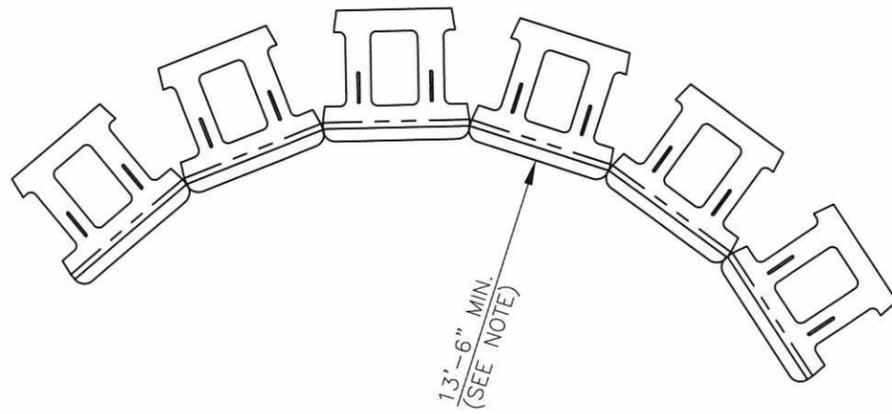
Drawing Title:
RETAINING WALL DETAILS

Drawing Scale:
CD
Date:
09/24/18

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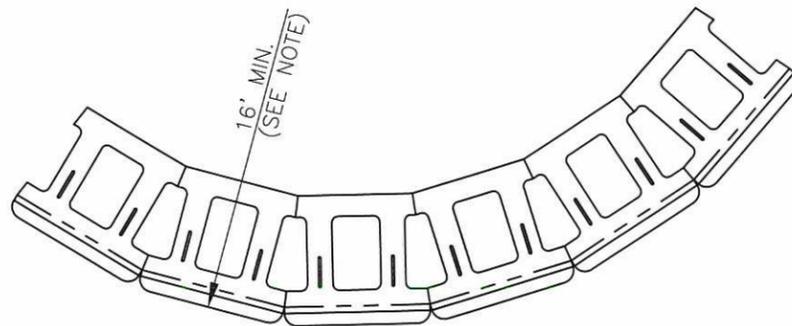
C14



NOTE:
MINIMUM RADIUS OCCURS AT LOWEST COURSE. RADIUS INCREASES 2" PER COURSE ABOVE, AS SHOWN ON TABLE.

Minimum Concave Radius		
Wall Height (ft)	Total # of Courses	Reqd. Radius at Top Course
3	2	13' 8"
4 1/2	3	13' 10"
6	4	14' 0"
7 1/2	5	14' 2"
9	6	14' 4"
10 1/2	7	14' 6"
12	8	14' 8"

1 MINIMUM CONCAVE RADIUS-6SF UNITS
NOT TO SCALE



NOTE:
MINIMUM RADIUS OCCURS AT TOP COURSE. REQUIRED RADIUS INCREASES 2" PER COURSE BELOW, AS SHOWN ON TABLE.

Minimum Convex Radius		
Wall Height (ft)	Total # of Courses	Reqd. Radius at First Course
3	2	16' 2"
4 1/2	3	16' 4"
6	4	16' 6"
7 1/2	5	16' 8"
9	6	16' 10"
10 1/2	7	17' 0"
12	8	17' 2"

2 MINIMUM CONVEX RADIUS-6SF UNITS
NOT TO SCALE

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1	REVISED PER COMMENT	SKB	09/24/18
0	ISSUED FOR PERMIT	SKB	08/30/18

Drawn: SKB Date: 08/30/18
Designed: AJD Date: 08/30/18
Checked: AJD Date: 08/30/18

Project Number: 502-000

Project Title: GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For:

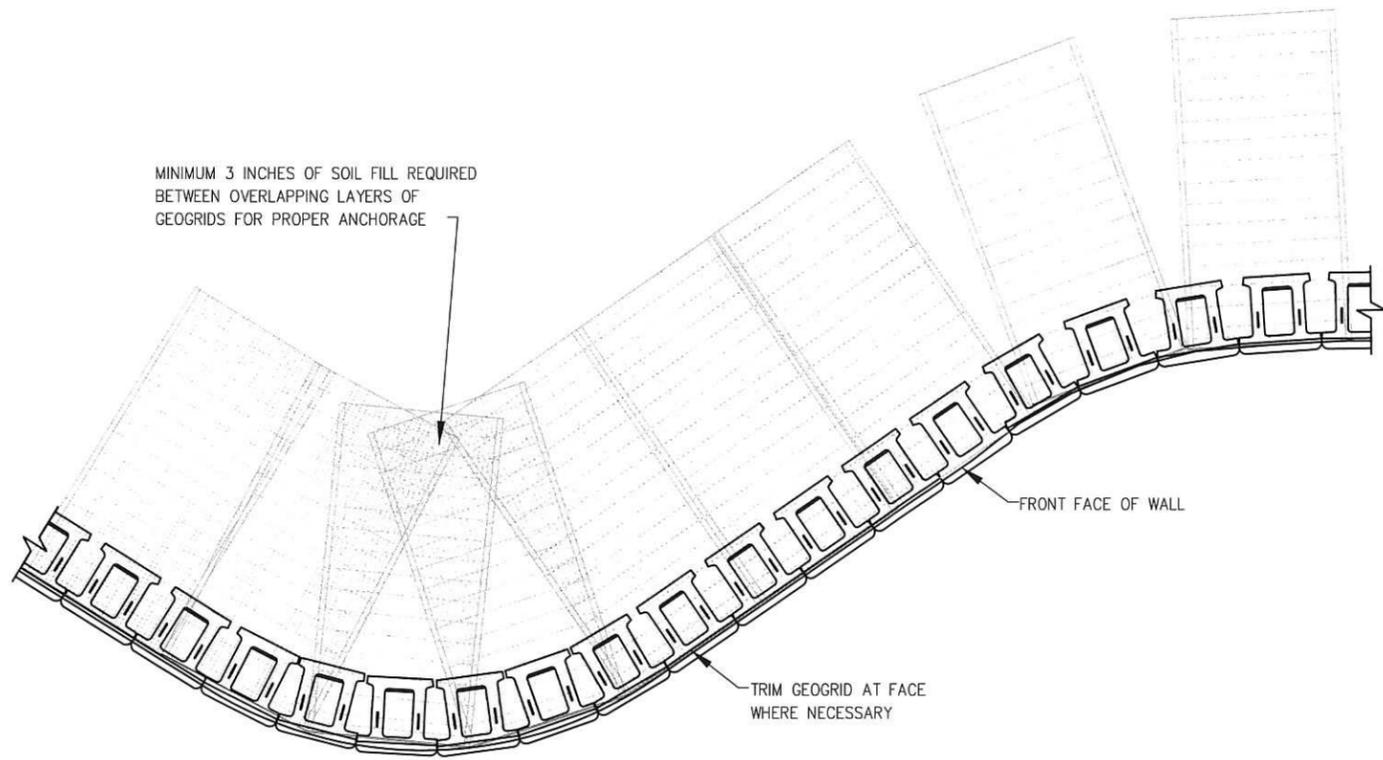
Eco-Site

Drawing Title: RETAINING WALL DETAILS

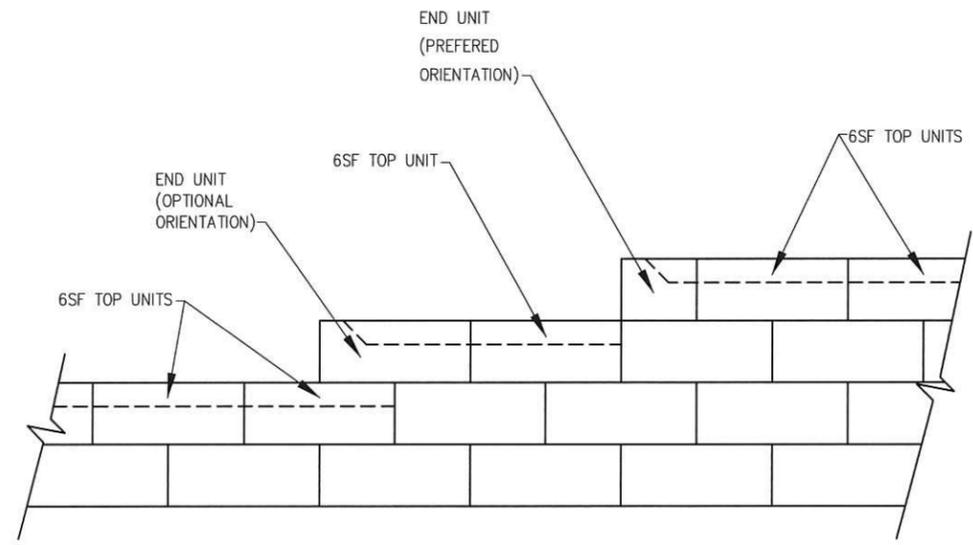
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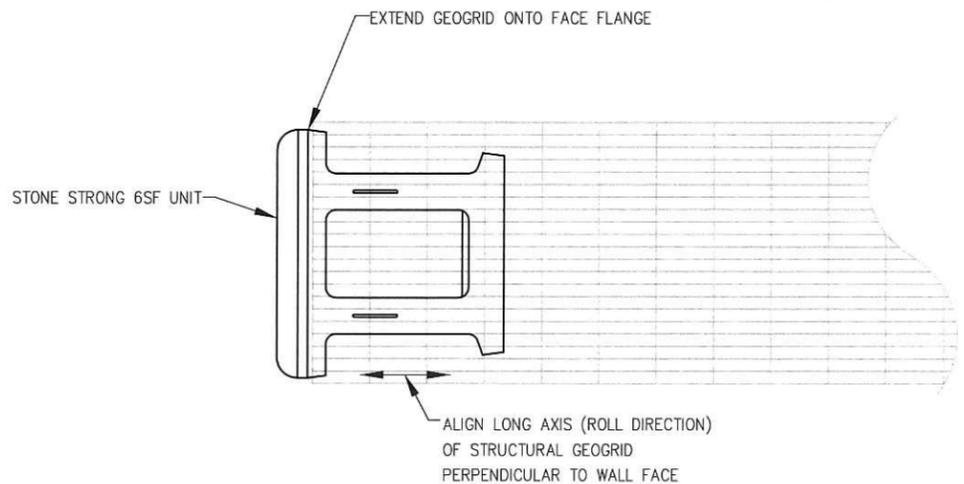
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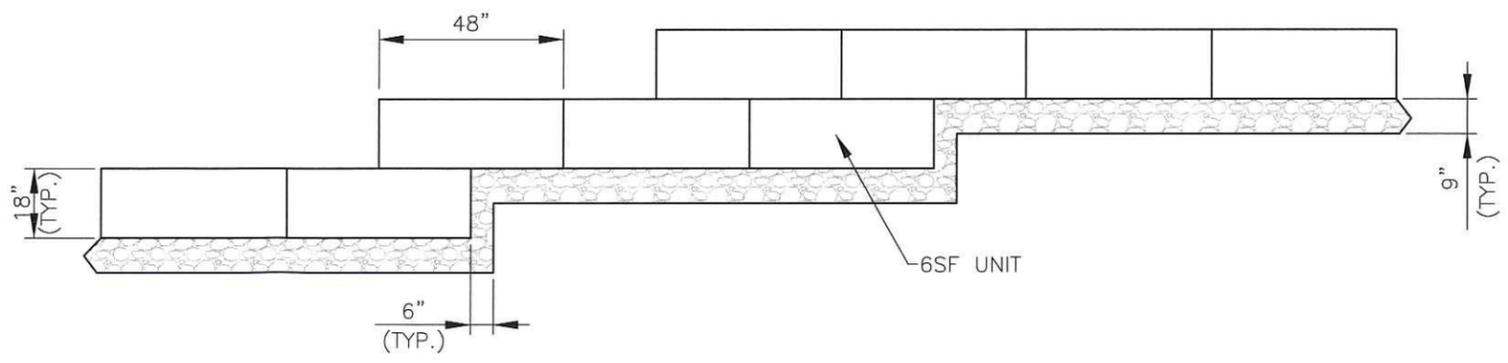
1 6SF GEOGRID PLACEMENT ON CURVES
NOT TO SCALE



2 TOP OF WALL STEPS
NOT TO SCALE



3 6SF GEOGRID ORIENTATION
NOT TO SCALE



4 6SF WALL BASE STEP
NOT TO SCALE

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GLASTONBURY
CT-0007A
63-80 WOODLAND STREET
GLASTONBURY, CT 06073

Prepared For:
Eco-Site

Drawing Title:
RETAINING WALL DETAILS

Drawing Scale: **CD**
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Drawing Number:
C16

GRADING & EXCAVATING NOTES:

- ALL EXCAVATIONS ON WHICH CONCRETE IS TO BE PLACED SHALL BE SUBSTANTIALLY HORIZONTAL ON UNDISTURBED AND UNFROZEN SOIL AND BE FREE FROM LOOSE MATERIAL AND EXCESS GROUNDWATER. DEWATERING FOR EXCESS GROUNDWATER SHALL BE PROVIDED IF REQUIRED.
- CONCRETE FOUNDATIONS SHALL NOT BE PLACED ON ORGANIC MATERIAL. IF SOUND SOIL IS NOT REACHED AT THE DESIGNATED EXCAVATION DEPTH, THE UNSATISFACTORY SOIL SHALL BE EXCAVATED TO ITS FULL DEPTH AND EITHER BE REPLACED WITH MECHANICALLY COMPACTED GRANULAR MATERIAL OR THE EXCAVATION BE FILLED WITH CONCRETE OF THE SAME QUALITY SPECIFIED FOR THE FOUNDATION.
- ANY EXCAVATION OVER THE REQUIRED DEPTH SHALL BE FILLED WITH EITHER MECHANICALLY COMPACTED GRANULAR MATERIAL OR CONCRETE OF THE SAME QUALITY SPECIFIED FOR THE FOUNDATION. CRUSHED STONE MAY BE USED TO STABILIZE THE BOTTOM OF THE EXCAVATION. STONE, IF USED, SHALL NOT BE USED AS COMPILING CONCRETE THICKNESS.
- AFTER COMPLETION OF THE FOUNDATION AND OTHER CONSTRUCTION BELOW GRADE, AND BEFORE BACKFILLING, ALL EXCAVATIONS SHALL BE CLEAN OF UNSUITABLE MATERIAL SUCH AS VEGETATION, TRASH, DEBRIS, AND SO FORTH.
- USE APPROVED MATERIALS CONSISTING OF EARTH, LOAM, SANDY CLAY, SAND
-BE FREE FROM CLODS OR STONES OVER 2-1/2" MAXIMUM DIMENSIONS
-BE PLACED IN 6" LAYERS AND COMPACTED TO 95% STANDARD PROCTOR EXCEPT IN GRASSED/LANDSCAPED AREAS, WHERE 90% STANDARD PROCTOR
- REMOVE ALL VEGETATION, TOPSOIL, DEBRIS, WET AND UNSATISFACTORY SOIL MATERIALS, OBSTRUCTIONS, AND DELETERIOUS MATERIALS FROM GROUND SURFACE PRIOR TO PLACING FILLS. PLOW, STRIP, OR BREAK UP SLOPED SURFACES STEEPER THAN THAN 1 VERTICAL TO 4 HORIZONTAL SO FILL MATERIAL WILL BOND WITH EXISTING SURFACE. WHEN SUBGRADE OR EXISTING GROUND SURFACE TO RECEIVE FILL HAS A DENSITY LESS THAN THAT REQUIRED FOR FILL, BREAK UP GROUND SURFACE TO DEPTH REQUIRED, PULVERIZE, MOISTURE-CONDITION OR AERATE SOIL AND RECOMPACT TO REQUIRED DENSITY.
- PROTECT EXISTING GRAVEL SURFACING AND SUBGRADE IN AREAS WHERE EQUIPMENT LOADS WILL OPERATE. USE PLANKING OR OTHER SUITABLE MATERIALS DESIGNED TO SPREAD EQUIPMENT LOADS. REPAIR DAMAGE TO EXISTING GRAVEL SURFACING OR SUBGRADE WHERE SUCH DAMAGE IS DUE TO THE CONTRACTOR'S OPERATIONS. DAMAGED GRAVEL SURFACING SHALL BE RESTORED TO MATCH THE ADJACENT UNDAMAGED GRAVEL SURFACING AND SHALL BE OF THE SAME THICKNESS.
- REPLACE EXISTING GRAVEL SURFACING ON AREAS FROM WHICH GRAVEL SURFACING IS REMOVED DURING CONSTRUCTION OPERATIONS. GRAVEL SURFACING SHALL BE REPLACED TO MATCH EXISTING ADJACENT GRAVEL SURFACING AND SHALL BE OF THE SAME THICKNESS. SURFACES OF GRAVEL SURFACING SHALL BE FREE FROM CORRUGATIONS AND WAVES. EXISTING GRAVEL SURFACING MAY BE EXCAVATED SEPARATELY AND REUSED IF INJURIOUS AMOUNTS OF EARTH, ORGANIC MATTER, OR OTHER DELETERIOUS MATERIALS ARE REMOVED PRIOR TO REUSE. FURNISH ALL ADDITIONAL GRAVEL RESURFACING MATERIAL AS REQUIRED. BEFORE GRAVEL SURFACING IS REPLACED, SUBGRADE SHALL BE GRADED TO CONFORM TO REQUIRED SUBGRADE ELEVATIONS, AND LOOSE OR DISTURBED MATERIALS SHALL BE THOROUGHLY COMPACTED. DEPRESSIONS IN THE SUBGRADE SHALL BE FILLED AND COMPACTED WITH APPROVED SELECTED MATERIAL. GRAVEL SURFACING MATERIAL MAY BE USED FOR FILLING DEPRESSIONS IN THE SUBGRADE, SUBJECT TO ENGINEER'S APPROVAL.
- DAMAGE TO EXISTING STRUCTURES AND UTILITIES RESULTING FROM CONTRACTOR'S NEGLIGENCE SHALL BE REPAIRED/REPLACED TO OWNER'S SATISFACTION AT CONTRACTOR'S EXPENSE.
- CONTRACTOR SHALL COORDINATE THE CONSTRUCTION SCHEDULE WITH PROPERTY OWNER SO AS TO AVOID INTERRUPTIONS TO PROPERTY OWNER'S OPERATIONS.
- ENSURE POSITIVE DRAINAGE DURING AND AFTER COMPLETION OF CONSTRUCTION.
- ALL CUT AND FILL SLOPES SHALL BE MAXIMUM 2 HORIZONTAL TO 1 VERTICAL.
- CONTRACTOR SHALL BE RESPONSIBLE FOR MONITORING SITE VEHICLE TRAFFIC AS TO NOT ALLOW VEHICLES LEAVING THE SITE TO TRACK MUD ONTO PUBLIC STREETS. THE CONTRACTOR IS RESPONSIBLE FOR CLEANING PUBLIC STREETS DUE TO MUDDY VEHICLES LEAVING THE SITE.

GENERAL EROSION & SEDIMENT CONTROL NOTES:

- THE SOIL EROSION AND SEDIMENT CONTROL MEASURES AND DETAILS AS SHOWN HEREIN AND STIPULATED WITHIN STATE STANDARDS SHALL BE FOLLOWED AND INSTALLED IN A MANNER SO AS TO MINIMIZE SEDIMENT LEAVING THE SITE.
- PRIOR TO COMMENCING LAND DISTURBANCE ACTIVITY, THE LIMITS OF LAND DISTURBANCE SHALL BE CLEARLY AND ACCURATELY DEMARCATED WITH STAKES, RIBBONS, OR OTHER APPROPRIATE MEANS.
- EROSION CONTROL DEVICES SHALL BE INSTALLED BEFORE GROUND DISTURBANCE OCCURS. THE LOCATION OF SOME OF THE EROSION CONTROL DEVICES MAY HAVE TO BE ALTERED FROM SHOWN ON THE APPROVED PLANS IF DRAINAGE PATTERNS DURING CONSTRUCTION ARE DIFFERENT FROM THE FINAL PROPOSED DRAINAGE PATTERNS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ACCOMPLISH EROSION CONTROL FOR ALL DRAINAGE PATTERNS CREATED AT VARIOUS STAGES DURING CONSTRUCTION. ANY DIFFICULTY IN CONTROLLING EROSION DURING ANY PHASE OF CONSTRUCTION SHALL BE REPORTED TO THE ENGINEER IMMEDIATELY.
- THE LOCATION OF SOME OF THE EROSION CONTROL DEVICES MAY HAVE TO BE ALTERED FROM THAT SHOWN ON THE PLANS IF DRAINAGE PATTERNS DURING CONSTRUCTION ARE DIFFERENT FROM THE FINAL PROPOSED DRAINAGE PATTERNS. ANY DIFFICULTY IN CONTROLLING EROSION DURING ANY PHASE OF CONSTRUCTION SHALL BE REPORTED TO THE ENGINEER IMMEDIATELY.

GENERAL EROSION & SEDIMENT CONTROL NOTES:

- CONTRACTOR SHALL MAINTAIN ALL EROSION CONTROL MEASURES UNTIL PERMANENT VEGETATION HAS BEEN ESTABLISHED. CONTRACTOR SHALL CLEAN OUT ALL SEDIMENT PONDS WHEN REQUIRED BY THE ENGINEER OR THE LOCAL JURISDICTION INSPECTOR. CONTRACTOR SHALL INSPECT EROSION CONTROL MEASURES AT THE END OF EACH WORKING DAY TO ENSURE MEASURES ARE FUNCTIONING PROPERLY.
- THE CONTRACTOR SHALL REMOVE ACCUMULATED SILT WHEN THE SILT IS WITHIN 12" OF THE TOP OF THE SILT FENCE.
- FAILURE TO INSTALL, OPERATE OR MAINTAIN ALL EROSION CONTROL MEASURES WILL RESULT IN ALL CONSTRUCTION BEING STOPPED ON THE JOB SITE UNTIL SUCH MEASURES ARE CORRECTED.
- SILT BARRIERS TO BE PLACED AT DOWNSTREAM TOE OF ALL CUT AND FILL SLOPES.
- ALL CUT AND FILL SLOPES MUST BE SURFACED ROUGHENED AND VEGETATED WITHIN SEVEN (7) DAYS OF THEIR CONSTRUCTION.
- CONTRACTOR SHALL REMOVE ALL EROSION & SEDIMENT CONTROL MEASURES AFTER COMPLETION OF CONSTRUCTION AND ESTABLISHMENT OF PERMANENT GROUND COVER.
- THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION CONTROL MEASURES AND PRACTICES PRIOR TO, OR CONCURRENT WITH, LAND-DISTURBING ACTIVITIES.

SEEDING GUIDELINES:

FINAL STABILIZATION OF ALL DISTURBED AREAS, UNLESS OTHERWISE NOTED, SHALL BE LOAMED AND SEEDED. LOAM SHALL BE PLACED AT A MINIMUM COMPACTED DEPTH OF 4". RECOMMENDED SEEDING DATES FOR PERMANENT VEGETATION SHALL BE BETWEEN JUNE 15 THROUGH AUGUST 1 AND SEPTEMBER 15 THROUGH OCTOBER 15. TEMPORARY VEGETATIVE MEASURES SHALL CONSIST OF AN ANNUAL OR PERENNIAL RYE GRASS WITH RECOMMENDED SEEDING DATES BEING FROM JUNE 1 THROUGH AUGUST 15 AND SEPTEMBER 30 THROUGH NOVEMBER 30.

EVALUATE PROPOSED COVER MATERIAL

BEFORE SPREADING COVER MATERIAL OVER THE DESIGNATED AREA, OBTAIN A REPRESENTATIVE SOIL SAMPLE AND SUBMIT TO A REPUTABLE SOIL TESTING LABORATORY FOR CHEMICAL AND PHYSICAL ANALYSIS. THE PRELIMINARY TEST IS NECESSARY TO DETERMINE THE REQUIRED INORGANIC AND/OR ORGANIC AMENDMENTS THAT ARE NEEDED TO ASSIST IN ESTABLISHING THE SEED MIXTURE IN AN ENVIRONMENTALLY AND ECONOMICALLY SOUND MANNER. THE RESULTS WILL GIVE THE COVER MATERIAL CHARACTERISTICS SUCH AS pH AND FERTILIZATION NEEDS. THESE RESULTS SHALL BE KEPT ON-SITE BY THE CONTRACTOR AND AVAILABLE FOR REVIEW BY THE COUNTY.

SEED BED PREPARATION

PROPOSED COVER MATERIAL SHOULD BE SPREAD EVENLY OVER THE SITE AREA IN A MINIMUM 4" LIFT VIA BULLDOZER/BUCKET LOADER. USING THE INFORMATION FROM THE SOIL ANALYSIS, CAREFULLY CALCULATE THE QUANTITIES OF LIMESTONE AND PRE-PLANT FERTILIZER NEEDED PRIOR TO APPLYING. PRE-PLANT AMENDMENTS CAN BE APPLIED WITH A BROADCAST AND/OR DROP SEEDER AND INCORPORATED WITH AN OFFSET DISK, YORK RAKE, AND/OR HAND RAKE. AFTER INCORPORATION THE PRE-PLANT SOIL AMENDMENTS, THE SEED BED SHOULD BE SMOOTH AND FIRM PRIOR TO SEEDING. THE FOLLOWING SEED MIXTURES SHALL BE USED AS NOTED:

SEED MIXTURE

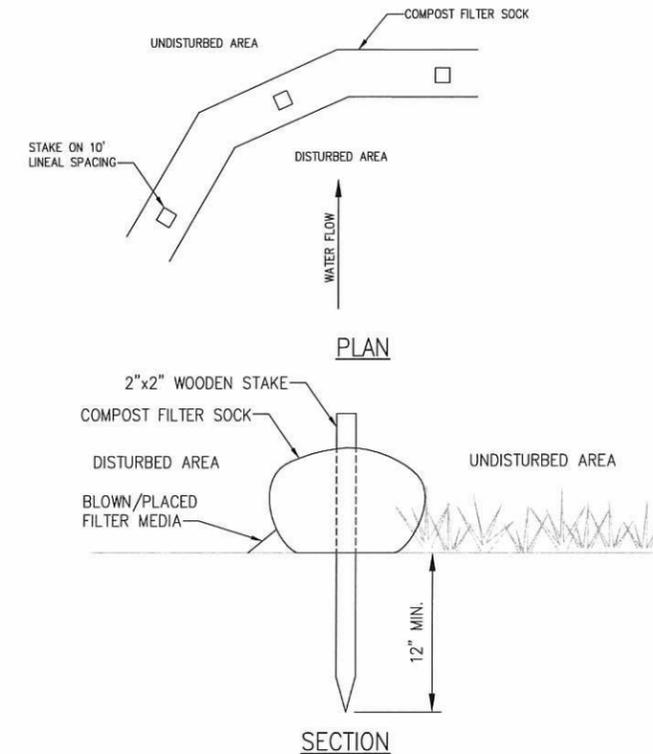
SPECIES/VARIETY	LBS/ACRE
CREeping RED FESCUE	20
KENTUCKY BLUEGRASS	20
PERENNIAL RYEGRASS	5

SEED TIME AND METHOD

THE PREFERRED TIME FOR SEEDING THE COOL SEASON MIXTURE IS LATE SUMMER. SOIL AND AIR TEMPERATURES ARE IDEAL FOR SEED GERMINATION AND SEEDING GROWTH. WEED COMPETITION IS REDUCED BECAUSE SEEDS OF MANY WEED SPECIES GERMINATE EARLIER IN THE GROWING SEASON. ADDITIONALLY, HERBICIDE USE IS GREATLY REDUCED. HOWEVER, SEEDING MAY BE DONE AT ANY OF THE ABOVE NOTED TIMES.

MULCHING

NEWLY SEEDED AREAS SHOULD BE MULCHED TO INSURE ADEQUATE MOISTURE FOR SUCCESSFUL TURF ESTABLISHMENT AND TO PROTECT AGAINST SURFACE MOVEMENT OF SEDIMENT-BOUND AGROCHEMICALS AND SOIL EROSION. IF MULCHING PROCEDURES ARE NOT SPECIFIED ON PLANS, APPLY GOOD QUALITY STRAW OR HAY AT A RATE OF 2 BALES/1000 SQ. FT. OTHER COMMERCIALY AVAILABLE MULCHES CAN BE USED.



- NOTES:**
- FILTER SOCK SHALL BE INSTALLED ON EXISTING LEVEL GRADE.
 - TRAFFIC SHALL NOT BE PERMITTED TO CROSS FILTER SOCKS.
 - ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT REACHES 1/2 THE ABOVE GROUND HEIGHT OF THE SOCK AND DISPOSED IN THE MANNER DESCRIBED ELSEWHERE IN THE PLAN.
 - SOCKS SHALL BE INSPECTED WEEKLY AND AFTER EACH RUNOFF EVENT. DAMAGED SOCKS SHALL BE REPAIRED ACCORDING TO MANUFACTURER'S SPECIFICATIONS OR REPLACED WITHIN 24 HOURS OF INSPECTION.
 - BIODEGRADABLE FILTER SOCK SHALL BE REPLACED AFTER 6 MONTHS; PHOTO DEGRADABLE SOCKS AFTER 1 YEAR. POLYPROPYLENE SOCKS SHALL BE REPLACED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. UPON STABILIZATION OF THE AREA TRIBUTARY TO THE SOCK, STAKES SHALL BE REMOVED. THE SOCK MAY BE LEFT IN PLACE AND VEGETATED OR REMOVED. IN THE LATTER CASE, THE MESH SHALL BE CUT OPEN AND THE MULCH SPREAD AS A SOIL SUPPLEMENT.



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Project Number: 502-000

Project Title: GLASTONBURY

CT-0007A

63-80 WOODLAND STREET
 GLASTONBURY, CT 06073

Prepared For:
Eco-Site

Drawing Title:
 GRADING & EROSION CONTROL NOTES & DETAILS

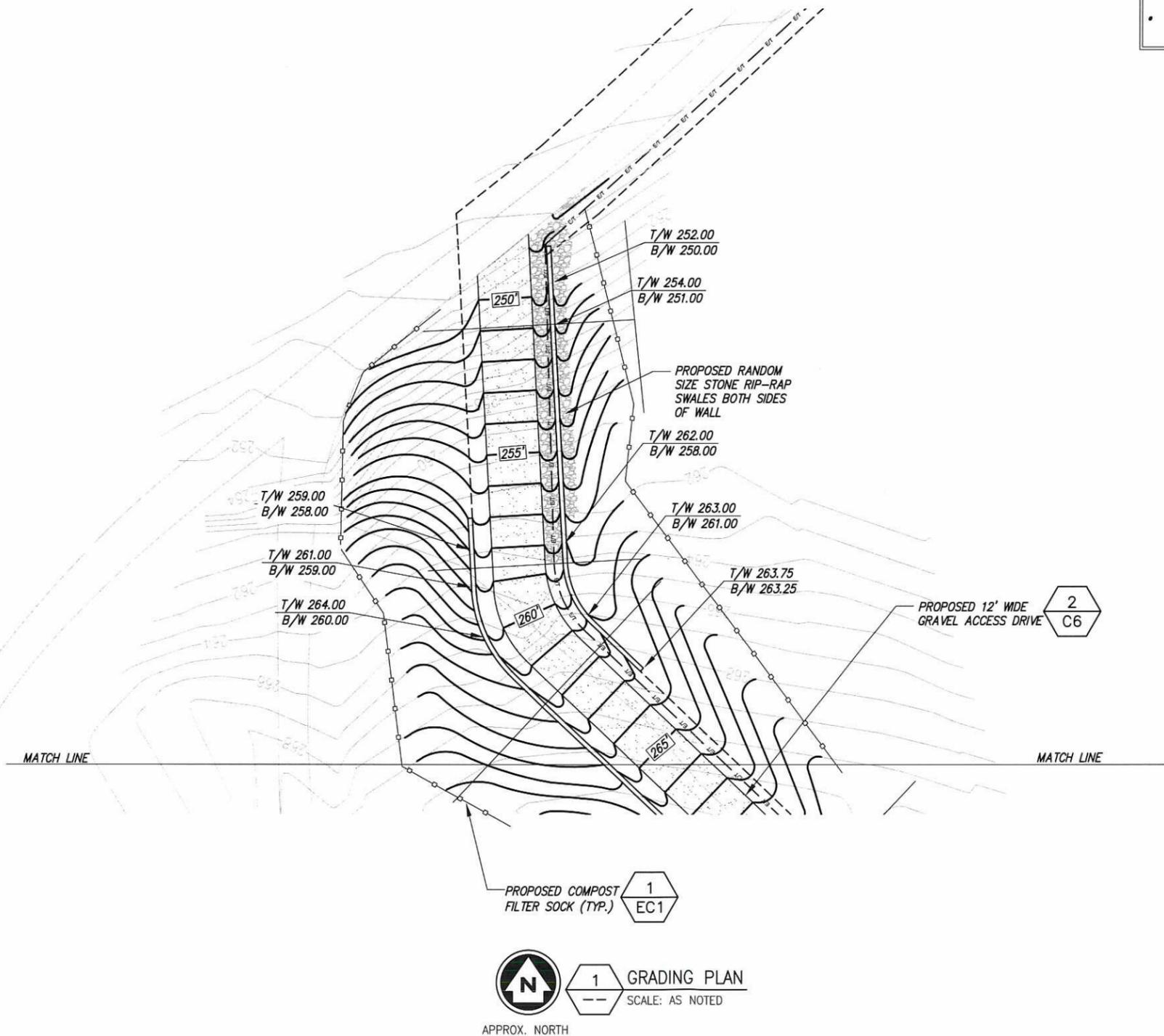
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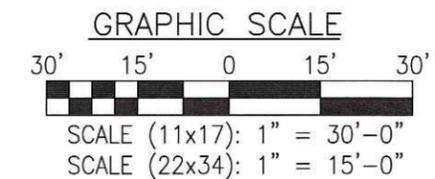
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TREE REMOVAL NOTE:

- IT IS ASSUMED THAT ONE TREE MEASURING 6" DBH OR GREATER EXISTS FOR EVERY 400 SQUARE FEET OF AREA TO BE IMPROVED.
- BASED ON A LEASE AREA OF 100'x100' AND A NEW ACCESS DRIVE CONSISTING OF 40,000 SQFT OF DISTURBANCE, THE TOTAL AREA TO BE CLEARED IS:
 $[(100)(100)] + [40,000] = 50,000$ SQUARE FEET
- ADDING AN APPROXIMATE 10% CONTINGENCY, THE TOTAL AREA TO BE CLEARED IS 55,000 SQUARE FEET.
- THEREFORE THE ESTIMATED NUMBER OF TREES TO BE CLEARED IS:
 $55,000/400 = 138$ TREES



BASEMAPPING INFORMATION BASED ON INFORMATION OBTAINED FROM AERIAL PHOTOGRAPHY, INFORMATION PROVIDED BY ECO-SITE, A SITE WALK COMPLETED BY INFINIGY ENGINEERING ON 5/17/16 AND SURVEY COMPLETED BY CLIMAX DEVELOPMENT, SIGNED AND STAMPED BY EARLE C. NEWMAN ON 11/18/16.



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 63-80 WOODLAND STREET
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Prepared For: Eco-Site

Drawing Title: GRADING PLAN

Drawing Scale: CD
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 CT-0007A
 63-80 WOODLAND STREET
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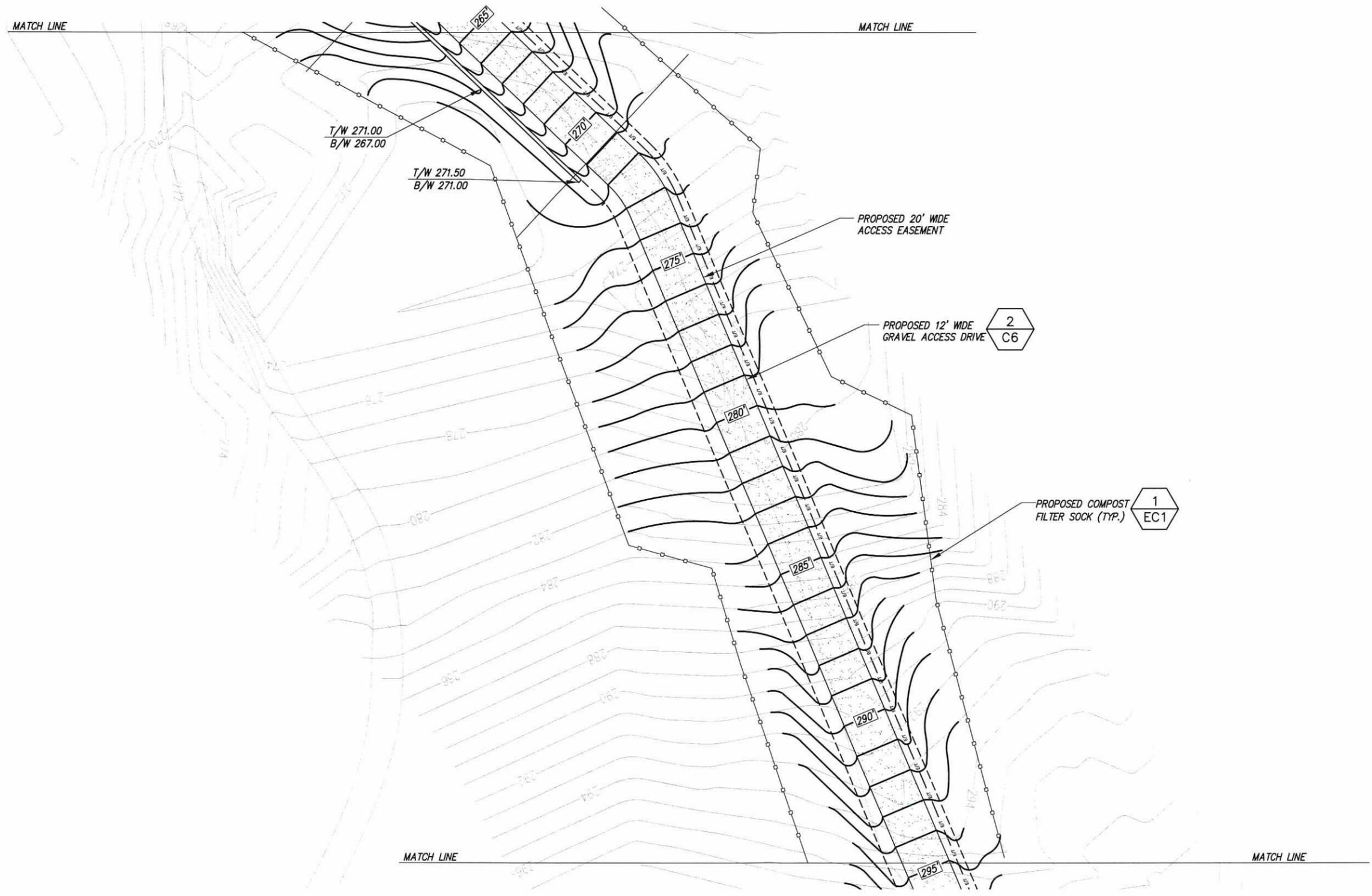
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Drawing Title:
**GRADING PLAN
 CONTINUED**

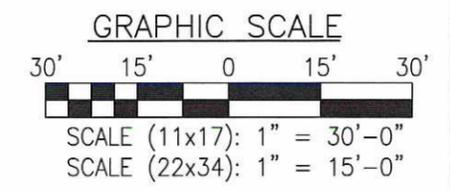
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EC3



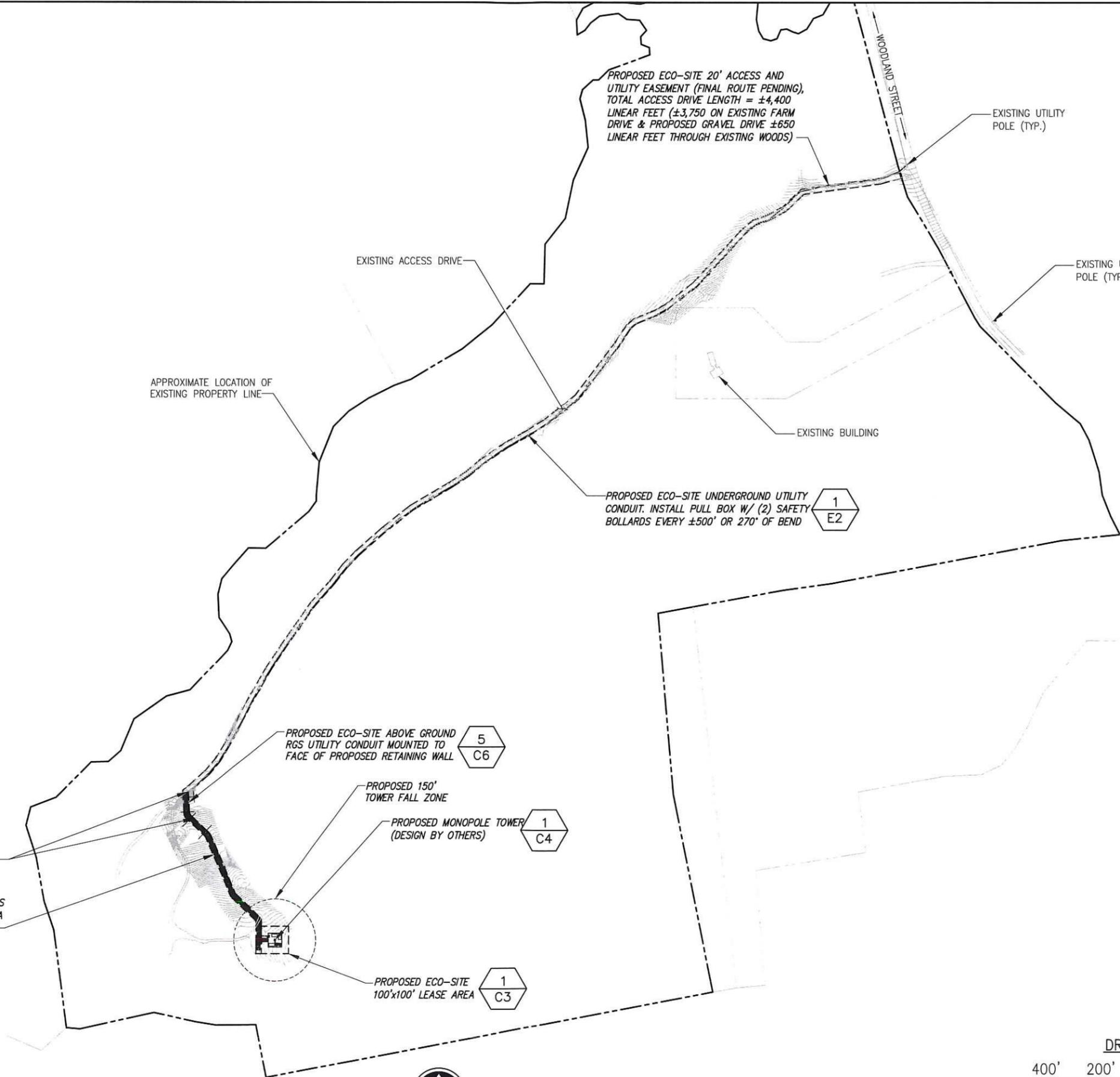
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 COMPLETED BY CLIMAX DEVELOPMENT, SIGNED AND
 STAMPED BY EARLE C. NEWMAN ON 11/18/16.



NOTE:
 • ROUTE OF UTILITY IS TO FOLLOW ACCESS EASEMENT UNLESS OTHERWISE DIRECTED BY UTILITY COMPANY.
 • FINAL INSTALLATION METHOD (OVERHEAD OR UNDERGROUND) TO BE DETERMINED.
 • HOURS OF CONSTRUCTION SHALL BE FROM 8:00AM - 6:00PM EST.

SHADED AREA REPRESENTS "INLAND WETLAND SOILS" FROM SOMERS CT GIS ONLINE DATABASE DELINEATION IN FIELD TO BE COMPLETED BY OTHERS. PER WETLAND INVESTIGATION BY OTHERS, NO WETLANDS EXIST ON THIS PARCEL.

NOTE:
 • NO CONSTRUCTION IS TO BE COMPLETED BETWEEN THE DATE OF FEBRUARY 15TH & APRIL 15TH DUE TO VERNAL POOL.
 • HOURS OF CONSTRUCTION ARE FROM DAWN UNTIL DUSK



3
E2

PROPOSED ECO-SITE UTILITY CONDUIT STUB UP LOCATION AT EDGE OF PROPOSED RETAINING WALL

2
C6

PROPOSED 12' WIDE GRAVEL ACCESS DRIVE (±650' LONG) WITHIN 20' ACCESS AND UTILITY EASEMENT (HATCHED AREA REPRESENTS NEW GRAVEL DRIVE)

5
C6

PROPOSED ECO-SITE ABOVE GROUND RGS UTILITY CONDUIT MOUNTED TO FACE OF PROPOSED RETAINING WALL

1
C4

PROPOSED 150' TOWER FALL ZONE

PROPOSED MONOPOLE TOWER (DESIGN BY OTHERS)

1
C3

PROPOSED ECO-SITE 100'x100' LEASE AREA

1
E2

PROPOSED ECO-SITE UNDERGROUND UTILITY CONDUIT. INSTALL PULL BOX W/ (2) SAFETY BOLLARDS EVERY ±500' OR 270' OF BEND

BASEMAPPING INFORMATION BASED ON INFORMATION OBTAINED FROM AERIAL PHOTOGRAPHY, INFORMATION PROVIDED BY ECO-SITE. A SITE WALK COMPLETED BY INFINIGY ENGINEERING ON 5/17/16 AND SURVEY COMPLETED BY CLIMAX DEVELOPMENT OF W.N.Y. TITLED "GLASTONBURY", DATED 6/14/16.



1 OVERALL UTILITY PLAN
 SCALE: AS NOTED

DRAWING SCALE
 400' 200' 0 200' 400'
 SCALE (11x17): 1" = 400'-0"
 SCALE (22x34): 1" = 200'-0"

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63-80 WOODLAND STREET
 GLASTONBURY, CT 06073

Prepared For:
Eco-Site

Drawing Title:
 OVERALL UTILITY PLAN

Drawing Scale: CD
 Date: 09/24/18

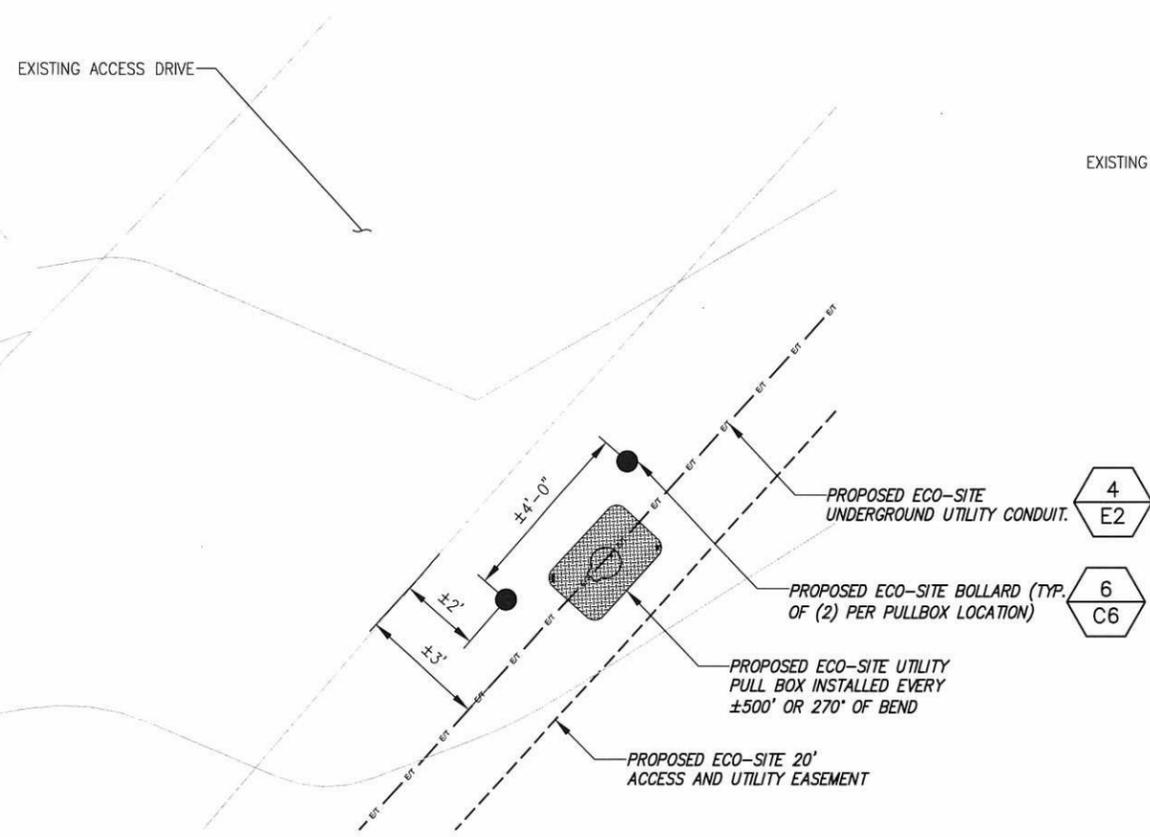
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E1

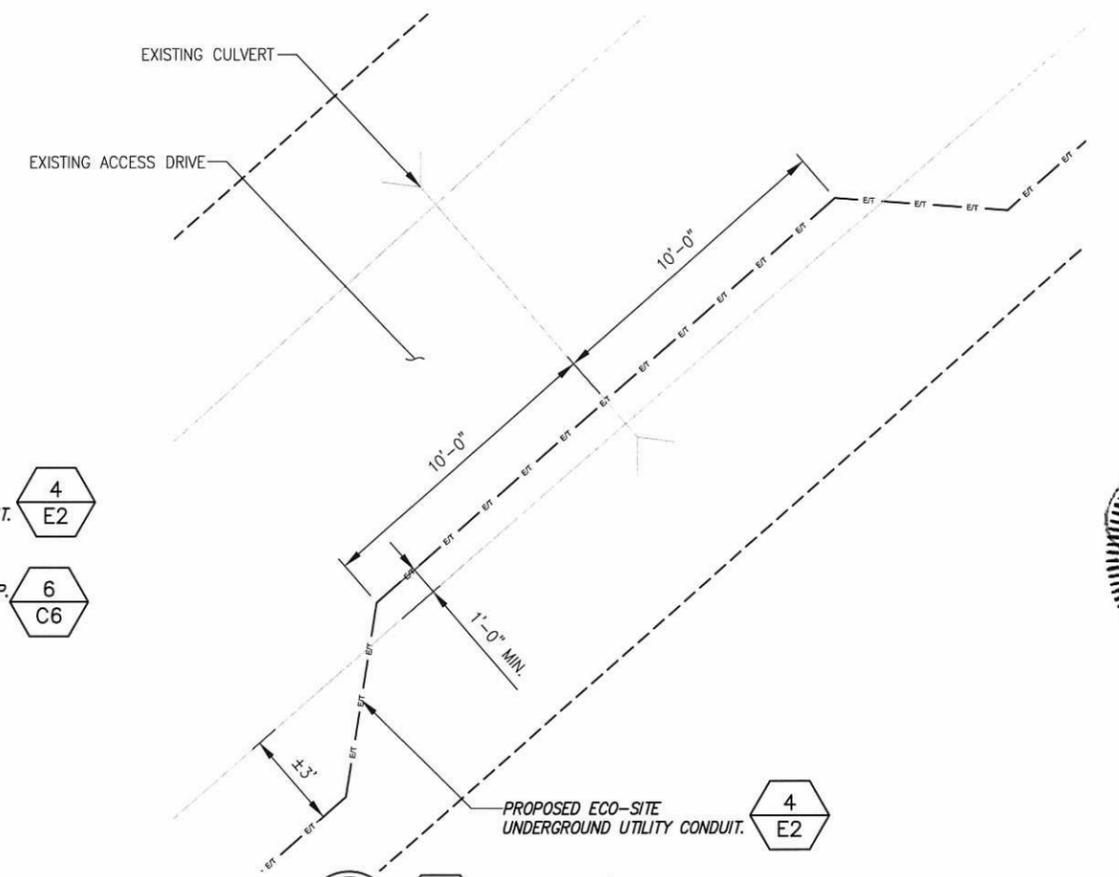
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 • FINAL INSTALLATION METHOD (OVERHEAD OR UNDERGROUND) TO BE DETERMINED.
 • HOURS OF CONSTRUCTION SHALL BE FROM 8:00AM - 6:00PM EST.

SHADED AREA REPRESENTS "INLAND WETLAND SOILS" FROM SOMERS CT GIS ONLINE DATABASE DELINEATION IN FIELD TO BE COMPLETED BY OTHERS. PER WETLAND INVESTIGATION BY OTHERS, NO WETLANDS EXIST ON THIS PARCEL.

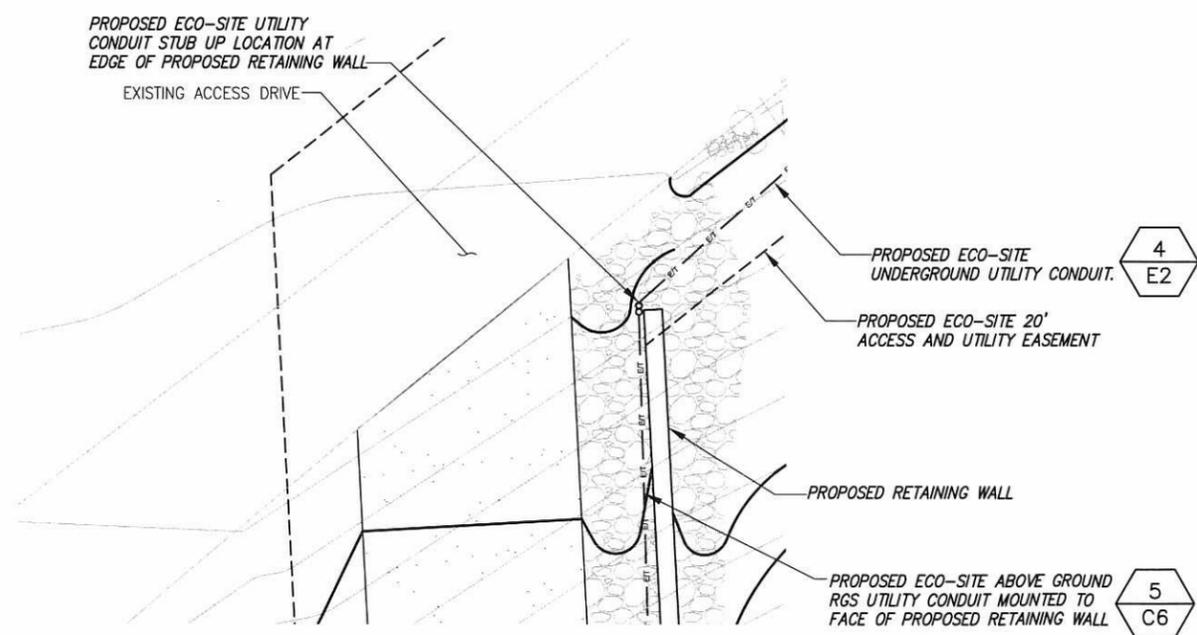
NOTE:
 • NO CONSTRUCTION IS TO BE COMPLETED BETWEEN THE DATE OF FEBRUARY 15TH & APRIL 15TH DUE TO VERNAL POOL.
 • HOURS OF CONSTRUCTION ARE FROM DAWN UNTIL DUSK



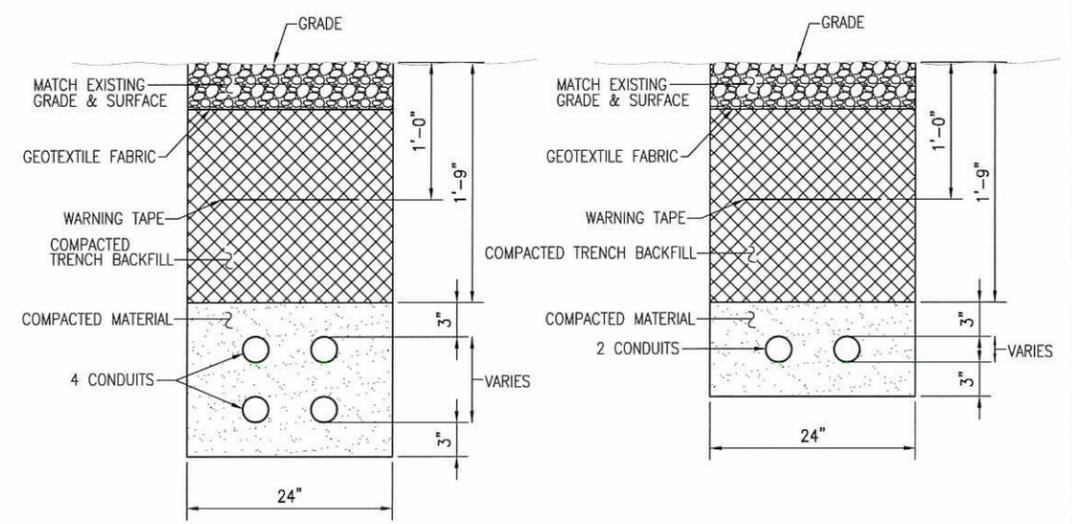
1 TYPICAL PULL BOX INSTALLATION DETAIL
 SCALE: AS NOTED
 APPROX. NORTH



2 TYPICAL U/G CONDUIT ROUTING @ CULVERT
 SCALE: AS NOTED
 APPROX. NORTH



3 ENLARGED CONDUIT ROUTING PLAN
 SCALE: AS NOTED
 APPROX. NORTH



4 TYPICAL ELECTRICAL COMMUNICATION TRENCH DETAILS
 NOT TO SCALE

BASEMAPPING INFORMATION BASED ON INFORMATION OBTAINED FROM AERIAL PHOTOGRAPHY, INFORMATION PROVIDED BY ECO-SITE, A SITE WALK COMPLETED BY INFINIGY ENGINEERING ON 5/17/16 AND SURVEY COMPLETED BY CLIMAX DEVELOPMENT OF W.N.Y. TITLED "GLASTONBURY", DATED 6/14/16.

INFINIGY
 1033 Watervliet Shaker Road | Albany, NY 12205
 Phone: 518-690-0790 | Fax: 518-690-0793
 www.infinigy.com



No.	Submital / Revision	App'd	Date
3	REVISED PER COMMENT	SKB	10/12/18
2	REVISED PER COMMENT	SKB	10/05/18
1	REVISED PER COMMENT	SKB	09/24/18
0	ISSUED FOR PERMIT	SKB	08/30/18

Drawn: SKB Date: 08/30/18
 Designed: AJD Date: 08/30/18
 Checked: AJD Date: 08/30/18

Project Number: 502-000
 Project Title: GLASTONBURY
 CT-0007A
 63-80 WOODLAND STREET
 GLASTONBURY, CT 06073

Prepared For:
Eco-Site

Drawing Title:
 UTILITY ROUTING DETAILS

Drawing Scale: CD
 Date: 09/24/18

UNAUTHORIZED ALTERATION OR ADDITION TO THIS DOCUMENT IS A VIOLATION OF APPLICABLE STATE AND/OR LOCAL LAWS

Drawing Number:
E2